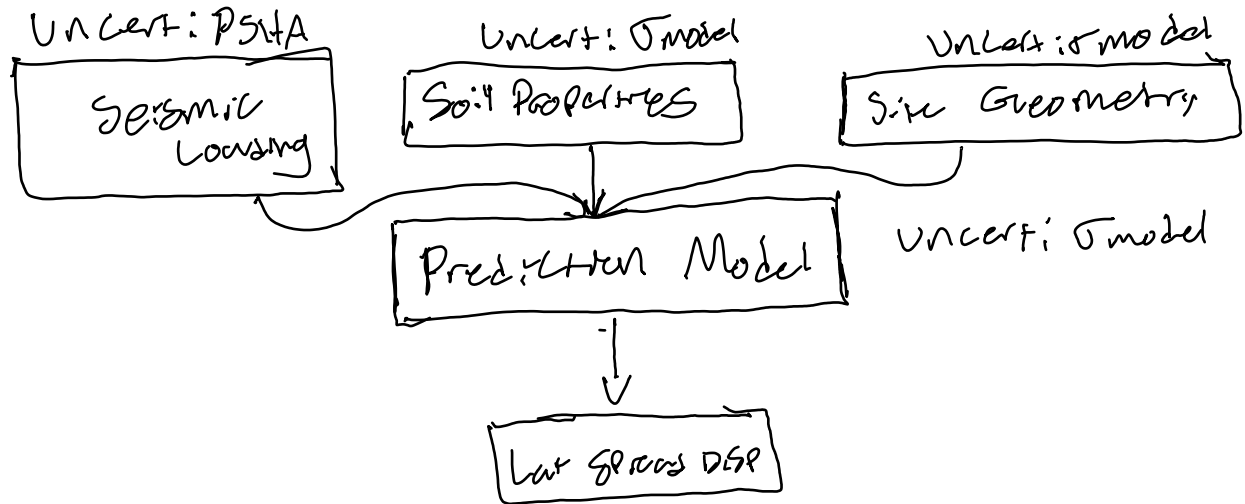


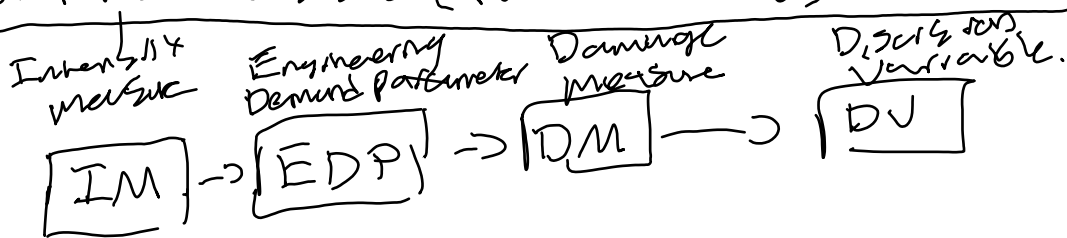
Performance based Liquefaction Analysis

Evaluate Risk & Hazard consistent & efficient

- 1) Different ways to handle seismic loading & uncertainty
- 2) introduce PIER Performance Based Earthquake Engineering (PBEE) framework.



1. Deterministic \longrightarrow Scenario based (Single Scenario)
2. Pseudo-Probabilistic \longrightarrow ONLY seismic uncertainty accounted at a single return period.
3. Performance based (Full Probabilistic)



IM; magnitude, Distance, PGA, CAV

DV; cost, life lost, safety

DM; cosmetic damage, Displacement, Collapse

EDP; lateral spread displacements, settlement of building

λ = Hazard Curve stuff $\frac{1}{T_r}$

$$\lambda_{DV} = \int_{I_M} \int_{EOP} \int_{DM} G(DV|DM) G(DM|EOP) G(EOP|IM) d\lambda_{IM}$$

$G(\dots) \rightarrow P[DM > dm^* | EOP]$
 Probability of DM exceeds dm^* given EOP

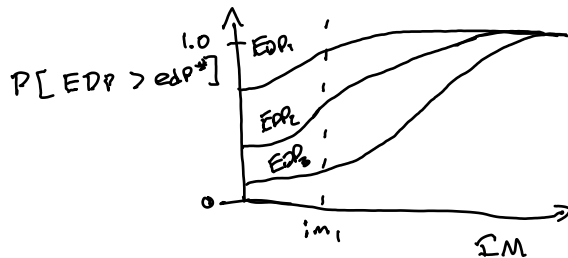
fragility function

Hazard Curve

$$\lambda_{EOP} = \int_{I_M} G(EOP|IM) d\lambda_{IM}$$

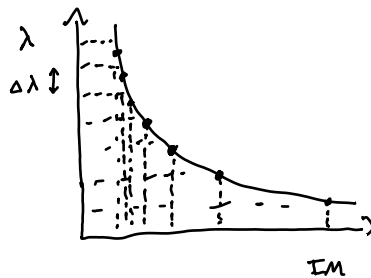
$$\lambda_{EOP^*} = \sum_{i=1}^{N_{IM}} P[EOP > EOP^* | IM_i] \Delta \lambda_{IM}$$

$$EOP_1 < EOP_2 < EOP_3$$



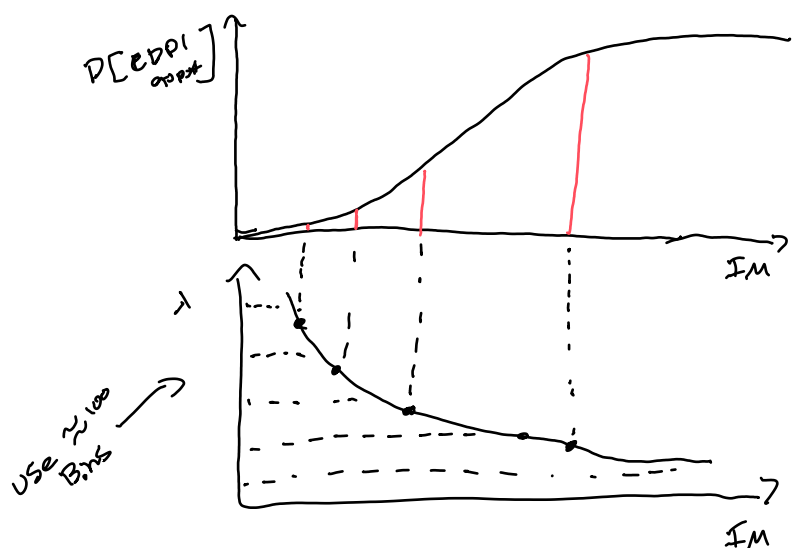
fragility curve

a function of probability

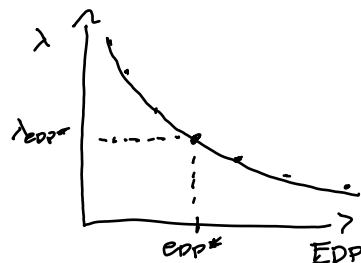


Hazard Curve

Consider 1 fragility curve



$$(\dots + 1 + 1 + 1 + \dots) * \Delta \lambda_{IM} = \lambda_{EDP*}$$



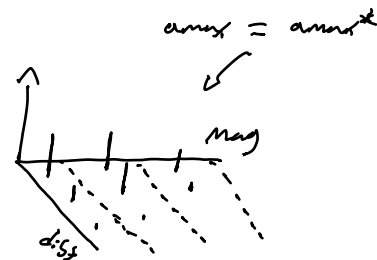
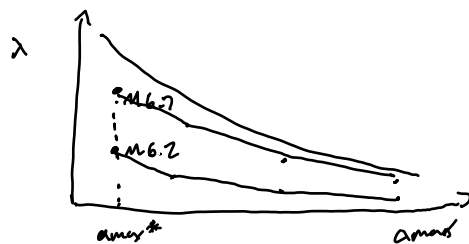
Kramer & Maxwell (2007)

Liquefaction triggering

Looking at
1 sublayer of Soil

$$F_{SL}^* = \sum_{i=1}^{N_{max}} \sum_{j=1}^{N_m} P[F_{SL} < F_{SL}^* | a_{max,i}, M_j] \Delta \lambda_{a_{max}, M}$$

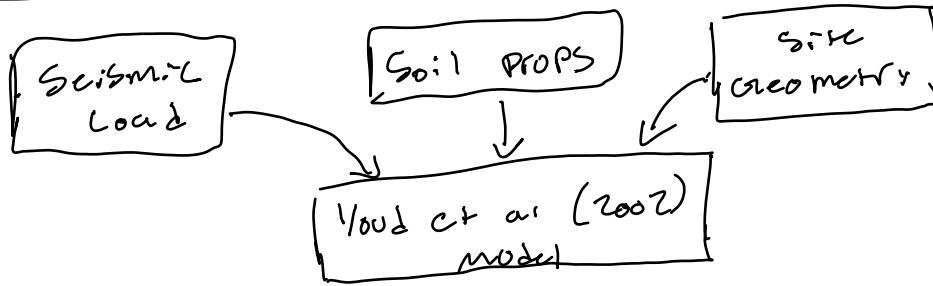
Upper tail
lambda
mean Annual
rate of
non exceedance



Each point on a_{max} hazard curve has disaggregated the disaggregation has the Percent contribution of each mag \leq the % cont of a_{max} and multiply by original λ value this gives a hazard curve of a_{max} for each magnitude

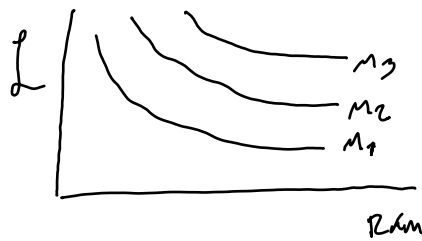
* Repeat for every Sublayer.

Lateral Spread



$$\log(D_H) = \underbrace{\phi_M M + \phi_R R}_{IM} \dots \underbrace{\phi_{T15} + \phi_{S15} \dots}_{\text{Soil Props}} \underbrace{\phi_S S + \phi_W W(\gamma)}_{\text{Site Geometry}}$$

$$L = \phi_M M + \phi_R R \dots \phi_R R^*$$



* Graphs Attenuates just like PGA etc.

L = Loading Parameter

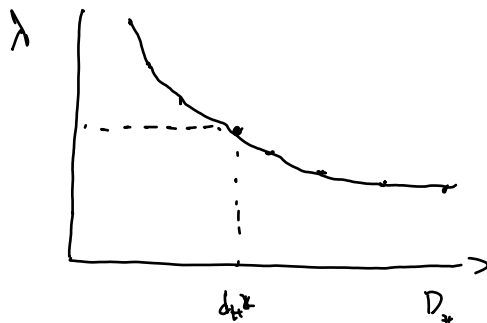
S = Site Parameter

$$S = (\phi_{G15} \dots \phi_W W(\gamma))^{(-1)}$$

$$\log(D_H) = L - S$$

$$\lambda_{EDP^*} = \sum P[EDP > edp^* | IM_i] \Delta \lambda_{IM}$$

$$\lambda_{dk^*} = \sum_{i=1}^{N_f} P[D_H > d_k^* | L_i, S] \Delta \lambda_L$$

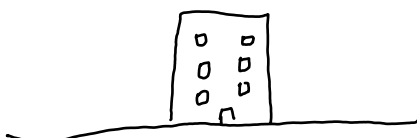


Look up \rightarrow Bozorgnia CAV
model
Kramer model CAV

Cambell and Bozorgnia 2010 GMPE cav equation is long and more complicated than the



Volumetric Strain



Under Structure

Free Field

* Assumes Clean sand

Soil Properties

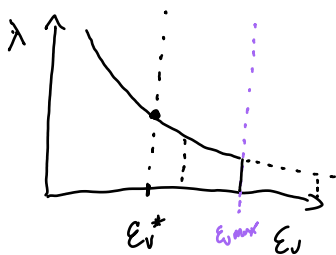
Seismic Loading

Predict E_v

$\Delta \lambda$ Probability of non exceedance.

$$\lambda_{ev*} = \sum_{i=1}^{N_{FS_i}} \sum_{j=1}^{N_{amax}} \sum_{k=1}^{N_M} P[E_v > E_{v*} | q_v, FS_{i,j}] \cdot P[FS_{i,j} < FS_{i,j} | a_{max,j}, M_k] \Delta \lambda_{amax,M}$$

$\Delta \lambda_{FS_i}$



CAV

Crustal $\ln \bar{Y} = a_0 + a_1 M_w + a_2 M_w^2 + [(b_1 + b_2 M_w) \ln(R)] + b_3 R + f_1 F_R + f_2 F_N$

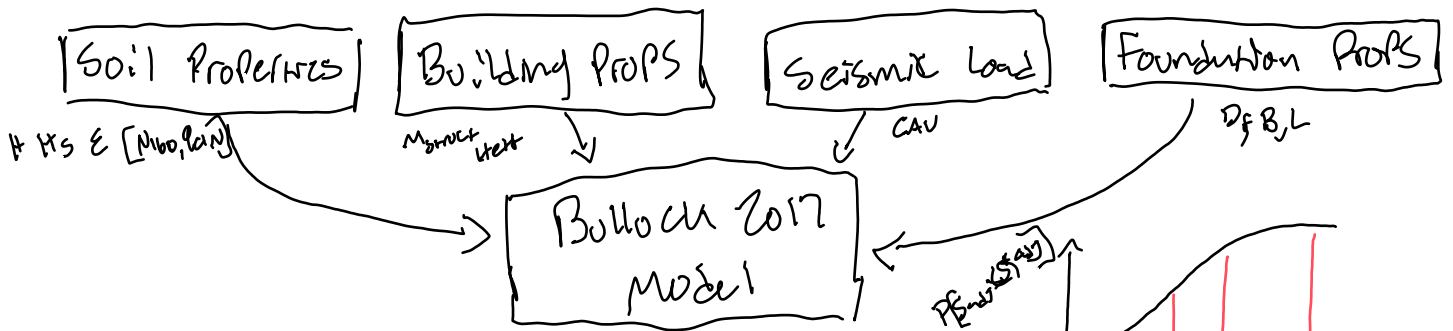
* What is R (Distance to Rupture)

* Assume $R = R_{rup}$ until further knowledge

Use σ_e for uncertainty.



Bullock Settlements for Shallow founded structures

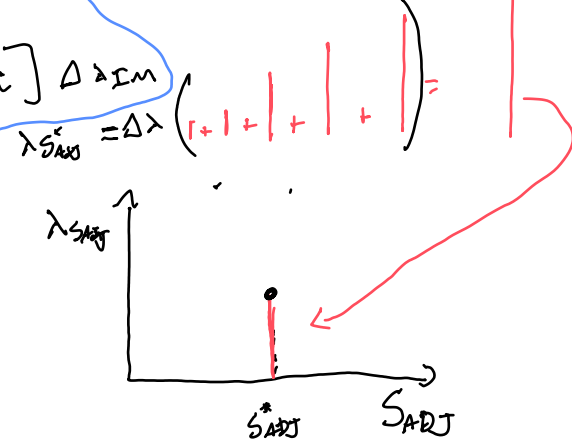
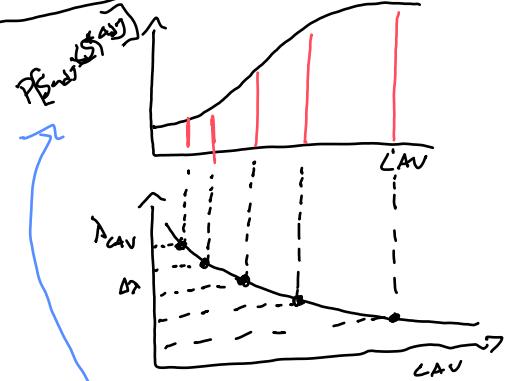


$\ln(S)_{Adj}$

$$\lambda_{EDP}^* = \sum_{i=1}^{N_{EDP}} P[EDP > c_{EDP}^* | IM_i] \Delta \lambda_{IM}$$

$$1 - \Phi\left[\frac{\ln(S)_{Adj} - \ln(S)_{Adj}^*}{\sigma}\right]$$

Auto calculate f_{SL}



Depth	Width	St N	Description	Soil type gamma / N
→				
→				
→				

Depth	q _u	f _s	u

... Calculate Settlement

