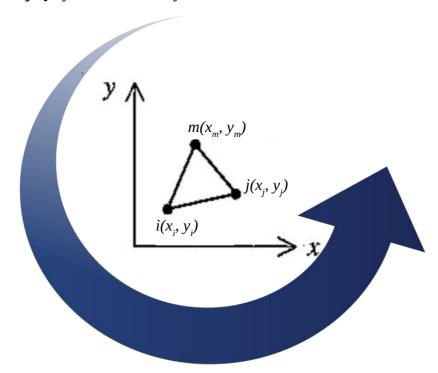
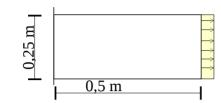
Trójkątny element liniowy

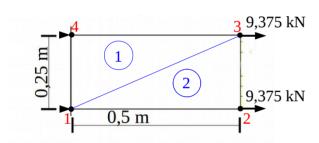




E = 210 GPa

$$v = 0.3$$
 (NU)
 $t = 0.025 \text{ m}$
 $w = 3000 \text{ kN} / \text{m}^3$

1. Dyskretyzacja



Elementy	i	j	m
1	1	3	4
2	1	2	3

2. Utworzenie lokalnych macierzy sztywności (k₁, k₂) dla każdego elementu wymiary lokalnych macierzy sztywności?

3. Zebranie informacji z lokalnych macierzy sztywności do globalnej macierzy sztywności K wymiary globalnej macierzy sztywności?

Zasadniczy układ równań liniowych (zawsze niereprezentatywny):

K – globalna macierz sztywności

U-wektor przemieszczeń węzłów układu

F – wektor sił (reakcje i obciążenia zewnętrzne) w węzłach układu

co to oznacza, że układ równań jest niereprezentatywny?

4. Uwzględnienie warunków brzegowych (informacje z warunków zadania dotyczące przemieszczeń węzłów układu i sił w węzłach układu)

po co uwzględniamy warunki brzegowe?

k*u=f

mały układ równań liniowych (reprezentatywny)

5. Rozwiązanie układu równań (reprezentatywnego)

6. Postprocessing

Wykorzystywane funkcje Matlaba (M-pliki)

1

```
LinearTriangleElementArea.m
```

```
function y = LinearTriangleElementArea(xi,yi,xj,yj,xm,ym)
                             This function returns the area of the
%LinearTriangleElementArea
                              linear triangular element whose first
응
                             node has coordinates (xi,yi), second
용
                             node has coordinates (xj,yj), and
                              third node has coordinates (xm,ym).
y = (xi*(yj-ym) + xj*(ym-yi) + xm*(yi-yj))/2;
2
LinearTriangleElementStiffness.m
function y = LinearTriangleElementStiffness(E,NU,t,xi,yi,xj,yj,xm,ym,p)
%LinearTriangleElementStiffness
                                   This function returns the element
                                   stiffness matrix for a linear
                                   triangular element with modulus of
용
용
                                   elasticity E, Poisson's ratio NU,
                                   thickness t, coordinates of the
용
                                   first node (xi,yi), coordinates of
용
                                   the second node (xj,yj), and
용
                                   coordinates of the third node
용
                                   (xm,ym). Use p = 1 for cases of
용
응
                                   plane stress, and p = 2 for cases
용
                                   of plane strain.
                                   The size of the element stiffness
응
                                   matrix is 6 x 6.
A = (xi*(yj-ym) + xj*(ym-yi) + xm*(yi-yj))/2;
betai = yj-ym;
betaj = ym-yi;
betam = yi-yj;
gammai = xm-xj;
gammaj = xi-xm;
gammam = xj-xi;
B = [betai 0 betaj 0 betam 0 ;
   0 gammai 0 gammaj 0 gammam ;
   gammai betai gammaj betaj gammam betam]/(2*A);
if p == 1
   D = (E/(1-NU*NU))*[1 NU 0 ; NU 1 0 ; 0 0 (1-NU)/2];
elseif p == 2
```

D = (E/(1+NU)/(1-2*NU))*[1-NU NU 0 ; NU 1-NU 0 ; 0 0 (1-2*NU)/2];

3

y = t*A*B'*D*B;

K(2*m-1,2*j-1) = K(2*m-1,2*j-1) + k(5,3);

K(2*m-1,2*j) = K(2*m-1,2*j) + k(5,4);

```
LinearTriangleAssemble.m
function y = LinearTriangleAssemble(K,k,i,j,m)
%LinearTriangleAssemble
                          This function assembles the element
용
                          stiffness matrix k of the linear
응
                          triangular element with nodes i, j,
                          and m into the global stiffness matrix K.
응
                          This function returns the global stiffness
용
                          matrix K after the element stiffness matrix
                          k is assembled.
K(2*i-1,2*i-1) = K(2*i-1,2*i-1) + k(1,1);
K(2*i-1,2*i) = K(2*i-1,2*i) + k(1,2);
K(2*i-1,2*j-1) = K(2*i-1,2*j-1) + k(1,3);
K(2*i-1,2*j) = K(2*i-1,2*j) + k(1,4);
K(2*i-1,2*m-1) = K(2*i-1,2*m-1) + k(1,5);
K(2*i-1,2*m) = K(2*i-1,2*m) + k(1,6);
K(2*i,2*i-1) = K(2*i,2*i-1) + k(2,1);
K(2*i,2*i) = K(2*i,2*i) + k(2,2);
K(2*i,2*j-1) = K(2*i,2*j-1) + k(2,3);
K(2*i,2*j) = K(2*i,2*j) + k(2,4);
K(2*i,2*m-1) = K(2*i,2*m-1) + k(2,5);
K(2*i,2*m) = K(2*i,2*m) + k(2,6);
K(2*j-1,2*i-1) = K(2*j-1,2*i-1) + k(3,1);
K(2*j-1,2*i) = K(2*j-1,2*i) + k(3,2);
K(2*j-1,2*j-1) = K(2*j-1,2*j-1) + k(3,3);
K(2*j-1,2*j) = K(2*j-1,2*j) + k(3,4);
K(2*j-1,2*m-1) = K(2*j-1,2*m-1) + k(3,5);
K(2*j-1,2*m) = K(2*j-1,2*m) + k(3,6);
K(2*j,2*i-1) = K(2*j,2*i-1) + k(4,1);
K(2*j,2*i) = K(2*j,2*i) + k(4,2);
K(2*j,2*j-1) = K(2*j,2*j-1) + k(4,3);
K(2*j,2*j) = K(2*j,2*j) + k(4,4);
K(2*j,2*m-1) = K(2*j,2*m-1) + k(4,5);
K(2*j,2*m) = K(2*j,2*m) + k(4,6);
K(2*m-1,2*i-1) = K(2*m-1,2*i-1) + k(5,1);
K(2*m-1,2*i) = K(2*m-1,2*i) + k(5,2);
```

```
K(2*m-1,2*m-1) = K(2*m-1,2*m-1) + k(5,5);
K(2*m-1,2*m) = K(2*m-1,2*m) + k(5,6);
K(2*m, 2*i-1) = K(2*m, 2*i-1) + k(6,1);
K(2*m, 2*i) = K(2*m, 2*i) + k(6, 2);
K(2*m,2*j-1) = K(2*m,2*j-1) + k(6,3);
K(2*m,2*j) = K(2*m,2*j) + k(6,4);
K(2*m, 2*m-1) = K(2*m, 2*m-1) + k(6,5);
K(2*m,2*m) = K(2*m,2*m) + k(6,6);
y = K;
4
LinearTriangleElementStresses.m
function y = LinearTriangleElementStresses(E,NU,t,xi,yi,xj,yj,xm,ym,p,u)
%LinearTriangleElementStresses
                                  This function returns the element
                                  stress vector for a linear
                                  triangular element with modulus of
응
                                  elasticity E, Poisson's ratio NU,
용
용
                                  thickness t, coordinates of the
                                  first node (xi,yi), coordinates of
응
용
                                  the second node (xj,yj),
                                  coordinates of the third node
응
응
                                  (xm,ym), and element displacement
용
                                  vector u. Use p = 1 for cases of
                                  plane stress, and p = 2 for cases
응
용
                                  of plane strain.
                                  The size of the element stress
용
                                  vector is 3 x 1.
A = (xi*(yj-ym) + xj*(ym-yi) + xm*(yi-yj))/2;
betai = yj-ym;
betaj = ym-yi;
betam = yi-yj;
gammai = xm-xj;
gammaj = xi-xm;
gammam = xj-xi;
B = [betai 0 betaj 0 betam 0 ;
   0 gammai 0 gammaj 0 gammam ;
   gammai betai gammaj betaj gammam betam]/(2*A);
if p == 1
   D = (E/(1-NU*NU))*[1 NU 0 ; NU 1 0 ; 0 0 (1-NU)/2];
elseif p == 2
   D = (E/(1+NU)/(1-2*NU))*[1-NU NU 0 ; NU 1-NU 0 ; 0 0 (1-2*NU)/2];
y = D*B*u;
5
LinearTriangleElementPStresses.m
function y = LinearTriangleElementPStresses(sigma)
%LinearTriangleElementPStresses
                                  This function returns the element
                                   principal stresses and their
용
                                   angle given the element
                                   stress vector.
R = (sigma(1) + sigma(2))/2;
Q = ((sigma(1) - sigma(2))/2)^2 + sigma(3)*sigma(3);
M = 2*sigma(3)/(sigma(1) - sigma(2));
```

```
s1 = R + sqrt(Q);
s2 = R - sqrt(Q);
theta = (atan(M)/2)*180/pi;
y = [s1 ; s2 ; theta];
```
