# **Buckling-2**

### **Title**

3-Member Frame

# **Description**

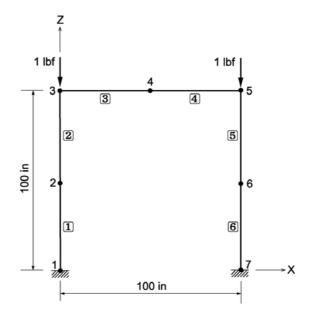
A plane frame structure is subjected to two vertical point forces.

The buckling load is determined for each case in which different number of elements per member has been considered.

Case 1: 2 Elements per member

Case 2: 4 Elements per member

Case 3: 8 Elements per member



Structural geometry and analysis model

# **MODEL**

#### Analysis Type

2-D buckling analysis

#### Unit System

in, lbf

#### Dimension

Length 100 in Height 100 in

#### Element

Beam elements

#### Material

Modulus of elasticity  $E = 1.0 \times 10^6 \text{ psi}$ 

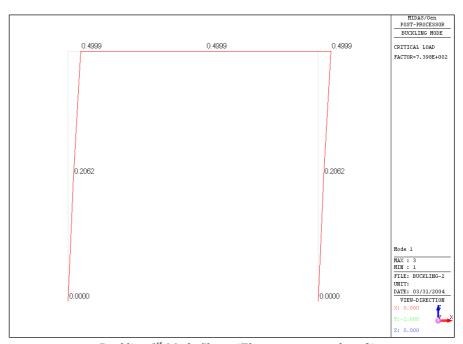
#### Section

 $\label{eq:Area} Area \qquad \qquad A = 1.0 \ in^2$   $\label{eq:Area} Moment \ of \ inertia \qquad I_{yy} = 1.0 \ in^4$ 

### **Boundary Condition**

Node 1, 7: Constrain all DOFs

# **Results**



Buckling 1st Mode Shape (Elements per member: 2)

## **Buckling load factor**

Node	Mode	ux	UY		
BUCKLING ANALYSIS					
	Mode	Eigenvalue	Tolerance		
	1	739,8	5,8092e-014		
	2	2577,1	3,5889e-007		

Elements per member: 2

Node	Mode	ux	UY			
	BUCKLING ANALYSIS					
	Mode	Eigenvalue	Tolerance			
	1	737,6	1,4407e-012			
	2	2524,7	9,9644e-007			

Elements per member: 4

Node	Mode	UX	UY		
BUCKLING ANALYSIS					
	Mode	Eigenvalue	Tolerance		
	1	737,5	1,6957e-015		
	2	2518,6	4,0617e=007		

Elements per member: 8

# **Comparison of Results**

Case	Buckling Load Factor			
	Theoretical	MIDAS	Ratio	
Case 1		739.8	1.003	
Case 2	737.9	737.6	0.999	
Case 3		737.5	0.999	

# Reference

S. P. Timoshenko and J. M. Gere, "Theory of Elastic Stability", McGraw-Hill, N.Y., 1961.