

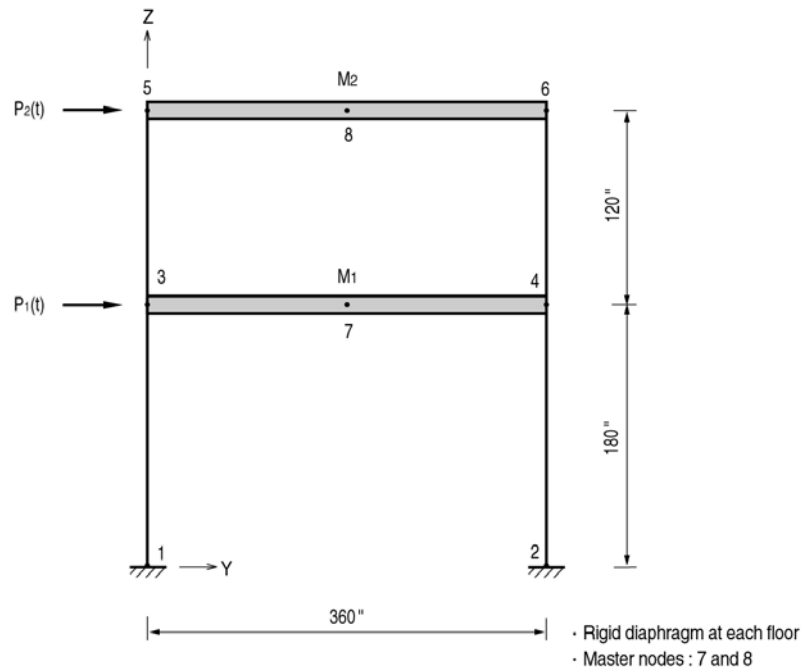
# TH-4

## Title

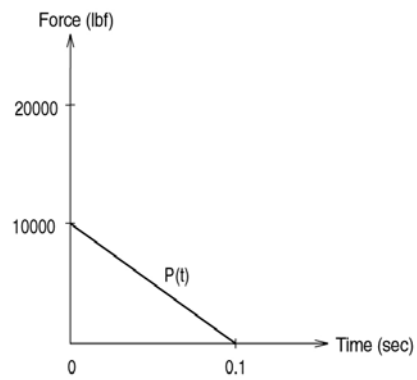
Dynamic modal response for 2-D rigid frame

## Description

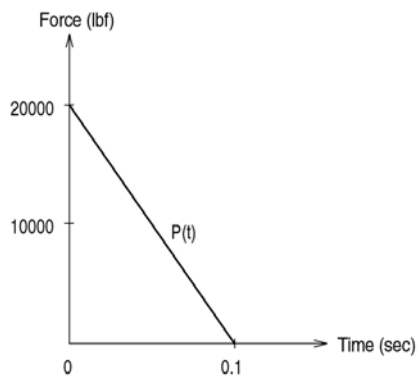
Perform a time history analysis of a structure under lateral dynamic loads.  
Calculate the natural frequencies, the maximum displacement and the corresponding time.



(a) 2-D rigid frame under lateral loads



(b) Impulse at the 1<sup>st</sup> floor



(c) Impulse at the 2<sup>nd</sup> floor

*Structural geometry and analysis model*

## Model

### *Analysis Type*

2-D time history analysis

### *Unit System*

in, lbf

### *Dimension*

Length	L	= 360 in
Level height	H <sub>1</sub> (1 <sup>st</sup> )	= 180 in
	H <sub>2</sub> (2 <sup>nd</sup> )	= 120 in
Mass	M <sub>1</sub> (1 <sup>st</sup> )	= 136 lbf · sec <sup>2</sup> /in (in the Y direction)
	M <sub>2</sub> (2 <sup>nd</sup> )	= 66 lbf · sec <sup>2</sup> /in (in the Y direction)
Analysis time	t	= 0.2 sec
Time step	Δt	= 0.001 sec

### *Element*

Beam Element

### *Material*

Modulus of elasticity E =  $30 \times 10^6$  psi

### *Section Property*

Columns (1 <sup>st</sup> floor)	Moment of inertia	I <sub>yy</sub> = 248.6 in <sup>4</sup>
Columns (2 <sup>nd</sup> floor)	Moment of inertia	I <sub>yy</sub> = 106.3 in <sup>4</sup>
Beams	Moment of inertia	I <sub>yy</sub> = $1.0 \times 10^{15}$ in <sup>4</sup> (Rigid)

### *Boundary Condition*

- Nodes 1 and 2 ; Constrain all DOFs.
- Nodes 3 ~ 6 ; Constrain Dz and Rx. (Only Dy allowed)
- Nodes 7 and 8 ; Constrain Dy of all nodes at each floor to these nodes.  
(Master nodes)

**Analysis Case**

Impulse loads are applied in the Y direction.

1<sup>st</sup> floor ;  $P_1(t) = 10000(1-t/td)$  lbf (td = 0.1)

2<sup>nd</sup> floor ;  $P_2(t) = 20000(1-t/td)$  lbf (td = 0.1)

Number of natural frequencies to be computed = 2

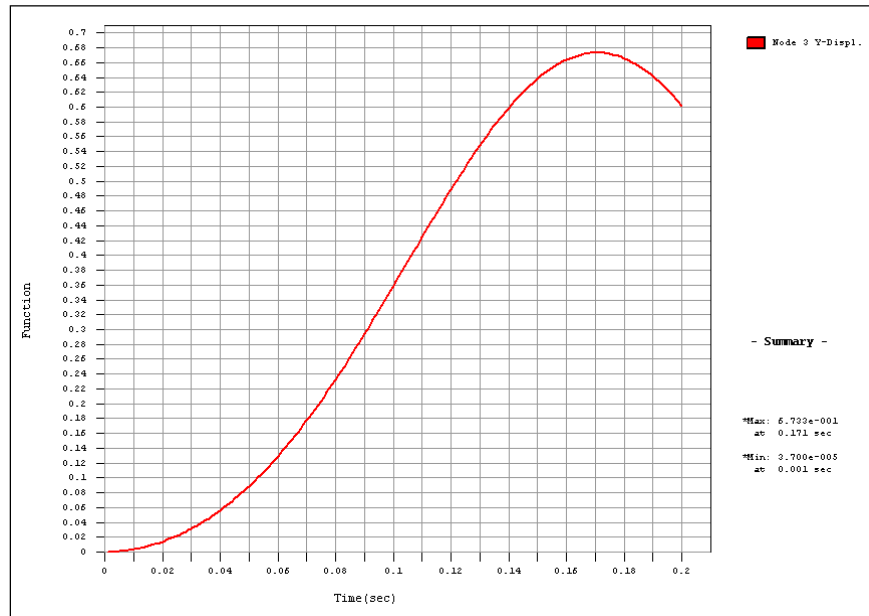
**Results****Eigenvalue Analysis Results**

EIGENVALUE ANALYSIS													
	Mode No	Frequency		Period		Tolerance							
		(rad/sec)	(cycle/sec)	(sec)									
	1	11.827886	1.882466	0.531218	2.0316e-016								
	2	32.901865	5.236495	0.190967	2.6255e-014								
MODAL PARTICIPATION MASSES(%) PRINTOUT													
	Mode No	TRAN-X		TRAN-Y		TRAN-Z		ROTN-X		ROTN-Y		ROTN-Z	
		MASS	SUM	MASS	SUM	MASS	SUM	MASS	SUM	MASS	SUM	MASS	SUM
	1	0.00	0.00	98.72	98.72	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2	0.00	0.00	1.28	100.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

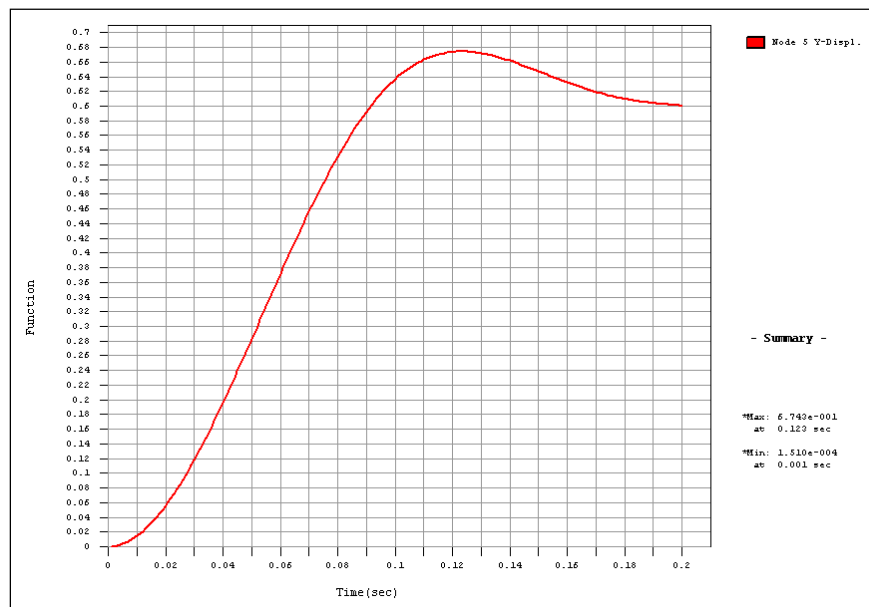
**Displacements**

	Node	Load	DX (in)	DY (in)	DZ (in)	RX ([rad])	RY ([rad])	RZ ([rad])
▶	1	LCOMB1(max)	0.000000	0.000000	0.000000	0.000000	0.000000	0.000000
	2	LCOMB1(max)	0.000000	0.000000	0.000000	0.000000	0.000000	0.000000
	3	LCOMB1(max)	0.000000	0.673300	0.000000	0.000000	0.000000	0.000000
	4	LCOMB1(max)	0.000000	0.673300	0.000000	0.000000	0.000000	0.000000
	5	LCOMB1(max)	0.000000	0.674290	0.000000	0.000000	0.000000	0.000000
	6	LCOMB1(max)	0.000000	0.674290	0.000000	0.000000	0.000000	0.000000
	7	LCOMB1(max)	0.000000	0.673300	0.000000	0.000000	0.000000	0.000000
	8	LCOMB1(max)	0.000000	0.674290	0.000000	0.000000	0.000000	0.000000
	1	LCOMB1(min)	0.000000	0.000000	0.000000	0.000000	0.000000	0.000000
	2	LCOMB1(min)	0.000000	0.000000	0.000000	0.000000	0.000000	0.000000
	3	LCOMB1(min)	0.000000	0.000037	0.000000	0.000000	0.000000	0.000000
	4	LCOMB1(min)	0.000000	0.000037	0.000000	0.000000	0.000000	0.000000
	5	LCOMB1(min)	0.000000	0.000151	0.000000	0.000000	0.000000	0.000000
	6	LCOMB1(min)	0.000000	0.000151	0.000000	0.000000	0.000000	0.000000
	7	LCOMB1(min)	0.000000	0.000037	0.000000	0.000000	0.000000	0.000000
	8	LCOMB1(min)	0.000000	0.000151	0.000000	0.000000	0.000000	0.000000
	1	LCOMB1(all)	0.000000	0.000000	0.000000	0.000000	0.000000	0.000000
	2	LCOMB1(all)	0.000000	0.000000	0.000000	0.000000	0.000000	0.000000
	3	LCOMB1(all)	0.000000	0.673300	0.000000	0.000000	0.000000	0.000000
	4	LCOMB1(all)	0.000000	0.673300	0.000000	0.000000	0.000000	0.000000
	5	LCOMB1(all)	0.000000	0.674290	0.000000	0.000000	0.000000	0.000000
	6	LCOMB1(all)	0.000000	0.674290	0.000000	0.000000	0.000000	0.000000
	7	LCOMB1(all)	0.000000	0.673300	0.000000	0.000000	0.000000	0.000000
	8	LCOMB1(all)	0.000000	0.674290	0.000000	0.000000	0.000000	0.000000

### Displacements



Y-displacements at the node 3



Y-displacements at the node 5

## Comparison of Results

### *Natural Frequencies*

		Unit : Hz		
Result		Ref. 1	SAP2000	MIDAS/Civil
Natural frequency	1 <sup>st</sup> mode	11.8	11.8	11.8
	2 <sup>nd</sup> mode	32.9	32.9	32.9

### *Maximum Displacements*

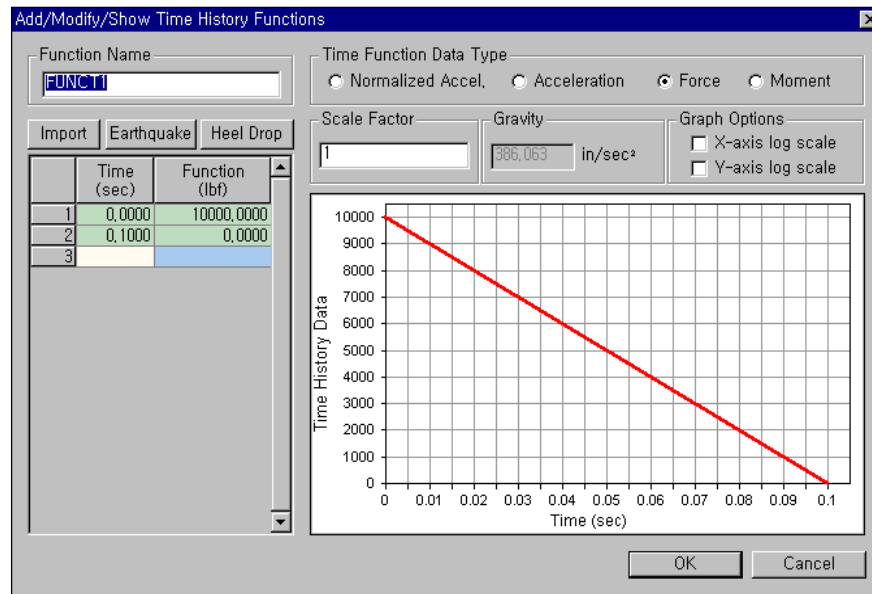
		Unit : sec, in		
Node	Time at which the maximum displacement occurs (t)	Maximum displacement ( $\delta_{Y,max}$ )		
	SAP2000	MIDAS/Civil	SAP2000	MIDAS/Civil
3	0.171	0.171	0.673	0.673
5	0.123	0.123	0.674	0.674

## References

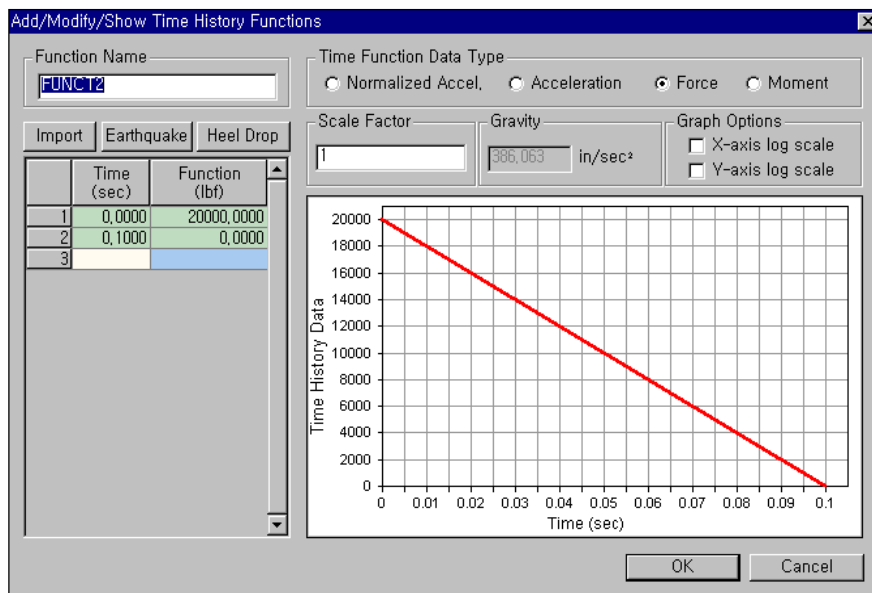
Paz, Mario, “*Structural Dynamics ; Theory and Computation*”, 3rd Edition, Van Nostrand Reinhold, New York, 1991, Example 11-1.

“*SAP90, A Series of Computer Programs for the Finite Element Analysis of Structures, Structural Analysis Verification Manual*”, Computer and Structures, Inc., 1992.

### Time History Loading Data



(a) Time history loads applied at the 1<sup>st</sup> floor



(b) Time history loads applied at the 2<sup>nd</sup> floor