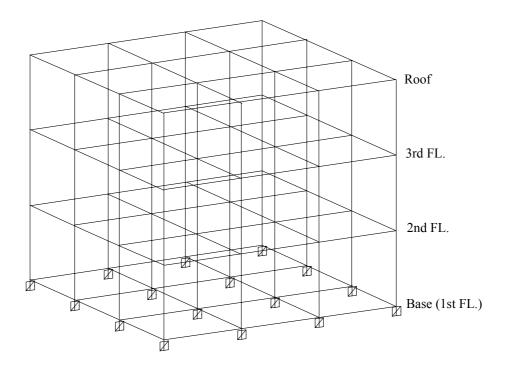
Title

Boundary nonlinear time history analysis

Description

Perform a boundary nonlinear time history analysis of a structure subject to lateral dynamic loads.

Determine the shear deformation, shear force and force-deformation curve for the nonlinear link No.1.



Structural analysis model

Model

Analysis Type

3-D boundary nonlinear time history analysis

Unit System

m, kN

Dimension

Length 12.0 m Width 12.0 m Height 10.5 m

 $\begin{array}{ll} \text{Damping ratio} & \quad \xi = 0.05 \\ \text{Analysis Time} & \quad t = 39.98 \text{ sec} \\ \text{Time step} & \quad \Delta \ t = 0.02 \text{ sec} \\ \end{array}$

Element

Beam element

Material

Modulus of elasticity $E = 2.9863 \times 10^7 \text{ kN/m}^2$ Poisson's ratio v = 0.25

Section Property

Columns 97~111, 118~123, 130~144 $B \times H = 0.3 \text{ m} \times 0.3 \text{ m}$ Columns 112~117, 124~129 $B \times H = 0.3 \text{ m} \times 0.4 \text{ m}$ Beams 1~96 $B \times H = 0.3 \text{ m} \times 0.4 \text{ m}$

Boundary Condition

Nodes 1~16 ; Constrain all DOFs. Nodes 1~16, 17~32 ; Nonlinear Link

Nonlinear Link Properties

Type; Lead Rubber Bearing Isolator

Spring Properties

| <u> r </u> | |
|---|----------------------------|
| DOF | Effective Stiffness (kN/m) |
| Dx | 80000 |
| Dy | 523 |
| Dz | 523 |

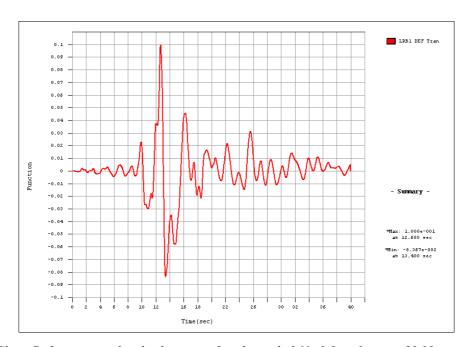
Nodes 81~84 ; Constrain Dx, Dy & Rz of all nodes at each floor to these nodes (Master nodes)

Load Case

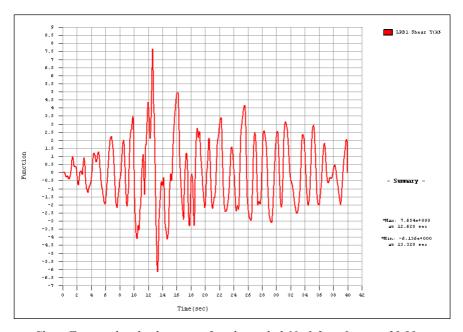
Loma Prieta Earthquake in 270 Deg and 0 Deg (Oakland Outer Wharf, 1989) are applied in the X & Y directions respectively.

Results

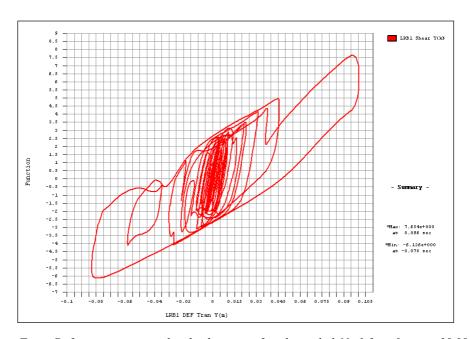
Time History Analysis Results



Shear Deformation in local y direction of nonlinear link No.1 from 0 sec. to 39.98 sec.



Shear Force in local y direction of nonlinear link No.1 from 0 sec. to 39.98 sec.



Force-Deformation curve in local y direction of nonlinear link No.1 from 0 sec. to 39.98 sec.

Comparison of Results

Maximum Inter-story Drifts

Unit: m

| Result | D_X | | D_{Y} | |
|----------------|----------------|-------------|---------------------------|-------------|
| | 3-D Basis-Tabs | MIDAS/Civil | 3-D Basis-Tabs | MIDAS/Civil |
| Roof | 0.0007 | 0.0008 | 0.0008 | 0.0007 |
| 3rd FL. | 0.0012 | 0.0012 | 0.0013 | 0.0011 |
| 2nd FL. | 0.0012 | 0.0012 | 0.0013 | 0.0013 |
| Base (1st FL.) | 0.1192 | 0.1190 | 0.1007 | 0.1000 |

Maximum Top Floor Acceleration

Unit: m/sec²

| Result | A_{X} | | A_{Y} | |
|--------|----------------|-------------|----------------|-------------|
| | 3-D Basis-Tabs | MIDAS/Civil | 3-D Basis-Tabs | MIDAS/Civil |
| Roof | 0.5116 | 0.5847 | 0.4928 | 0.4815 |

Reference

Reinhorn, A. M., Nagarajaiah, S., et al., "3-D Basis-Tabs: Version 2.0, Computer Program for Nonlinaer Dynamic Analysis of Three Dimensional Base Isolated Structures", Technical Report NCEER-94-0018, Nat. Ctr. for Earthquake Engrg. Res., State University of New York, Buffalo, N.Y., 1994.