## Created with

Company Name Project Title
Group/Team Name Subtitle
Designer Job Number

Date 19 /04 /2016 Method Limit State Design

**Design Conclusion** 

Endplate Pass

Endplate

**Connection Properties** 

Connection

Connection Title Flexible Endplate
Connection Type Shear Connection

**Connection Category** 

Connectivity Column flange-Beam web

Beam Connection Welded
Column Connection Bolted

Loading (Factored Load)

Shear Force (kN) 140

Components

Column Section ISSC 200

Material Fe 410

Beam Section ISMB 400

Material Fe 410

Hole STD

Plate Section 240X174X10

Thickness (mm) 10
Width (mm) 174
Depth (mm) 240
Hole STD

Weld

Type Double Fillet

Size (mm) 8

**Bolts** 

**HSFG** Type Grade 8.8 Diameter (mm) 20 **Bolt Numbers** 6 Columns (Vertical Lines) 2 3 Bolts Per Column Gauge (mm) 0 50 Pitch (mm) End Distance (mm) 70 Edge Distance (mm) 37

Assembly

Column-Beam Clearance (mm) 10

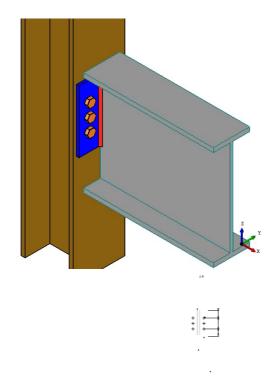
Created with

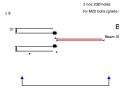
Company Name Project Title
Group/Team Name Subtitle
Designer Job Number

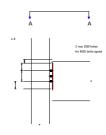
Date 19 /04 /2016 Method Limit State Design

| Check Bolt shear             | Required   | Provided  V <sub>dsb</sub> = (1000 000 010 010 010 010 010 010 010 0                    | Remark |
|------------------------------|--|---|--------|
| capacity (kN)                |  | $((800.0*0.6126*20*20)/(\sqrt{3}*1.25*1000)$<br>= 52.694<br>[cl. 10.3.3]<br>$V_{dsb}$ = | )      |
| Bolt bearing capacity (kN)   |  | (2.5*0.5*20*8.9*410)/(1.25*1000) = 82.0<br>[cl. 10.3.4]                                 |        |
| Bolt capacity (kN)           |  | Min (52.694, 82.0) = 52.694   | Pass   |
| No. of bolts                 | 140/52.694 = 2.7   | 6   | Pass   |
| No.of<br>column(s)           | ≤2   | 2   |        |
| No. of bolts per column      |  | 3   |        |
|                              | $\geq 2.5^* \ 20 = 50, \leq Min(32^*8.9, 300)$                             | •   |        |
| Bolt pitch (mm)              | cl. 10.2.2]  | 50  |        |
| Bolt gauge<br>(mm)           | $\geq$ 2.5*20 = 50, $\leq$ Min(32*8.9, 300 = 285 [cl. 10.2.2]              | 0   |        |
| End distance<br>(mm)         | ≥1.7*22.0 = 37.4, ≤12*8.9 = 106.8 [cl. 10.2.4]                             | 70  |        |
| Edge distance<br>(mm)        | ≥1.7*22.0 = 37.4, ≤12*8.9 = 106.8 [cl. 10.2.4]                             | 37  | Pass   |
| Block shear capacity (kN)    | 140  | V <sub>db</sub> = 203<br>[cl. 6.4.1]  |        |
| Plate thickness (mm)         | <sup>3</sup> ≥ 8   | 10  |        |
| ,                            | ≥0.6*400.0=240.0, ≤400.0-16.0-   |   |        |
| Plate height<br>(mm)         | 14.0-16.0-14.0- 10=330.0<br>[cl. 10.2.4, Insdag Detailing<br>Manual, 2002] | 240   | Pass   |
| Plate Width (mm)             | ≥174, ≤200.0=200.0   | 174   | Pass   |
| Effective weld length (mm)   |  | 240-2*8 = 224   |        |
| Weld strength (kN/mm)        | ≤ 0.313  | $f_{V=(0.7^*8^*410)/(\sqrt{3^*1.25^*1000})}$<br>= 1.06                                  | Pass   |
|                              |  | Created with  |        |
| Company Nam<br>Group/Team Na | ame Subtitle   |   |        |
| Designer<br>Date             | Job Number<br>19 /04 /2016 Method Li                                       | mit State Design  |        |

Views







## Created with

Company Name Project Title
Group/Team Name Subtitle
Designer Job Number

Date 19 /04 /2016 Metdod Limit State Design

Additional Comments