

<b>Company Name</b>		<b>Project Title</b>	
<b>Group/Team Name</b>		<b>Subtitle</b>	
<b>Designer</b>		<b>Job Number</b>	
<b>Date</b>	<b>19 /04 /2016</b>	<b>Method</b>	<b>Limit State Design</b>

## Design Conclusion

**Finplate** **Pass**

## Finplate

## Connection Properties

### Connection

Connection Title	Single Finplate
Connection Type	Shear Connection

### Connection Category

Connectivity	Beam-Beam
Beam Connection	Bolted
Column Connection	Welded

### Loading (Factored Load)

Shear Force (kN)	100
------------------	-----

### Components

<b>Column Section</b>	ISMB 500
-----------------------	----------

Material	Fe 410
----------	--------

<b>Beam Section</b>	ISMB 300
---------------------	----------

Material	Fe 410
----------	--------

Hole	STD
------	-----

<b>Plate Section</b>	200X100X10
----------------------	------------

Thickness (mm)	10
----------------	----

Width (mm)	100
------------	-----

Depth (mm)	200
------------	-----

Hole	STD
------	-----

### Weld

Type	Double Fillet
------	---------------

Size (mm)	8
<b>Bolts</b>	
Type	HSFG
Grade	8.8
Diameter (mm)	20
Bolt Numbers	3
Columns (Vertical Lines)	1
Bolts Per Column	3
Gauge (mm)	0
Pitch (mm)	60
End Distance (mm)	40
Edge Distance (mm)	40
<b>Assembly</b>	
Column-Beam Clearance (mm)	20

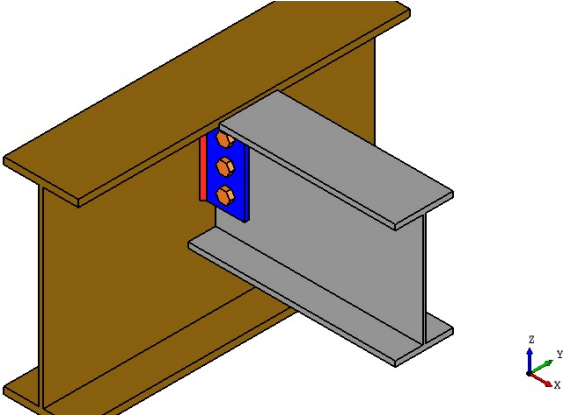



Company Name		Project Title	
Group/Team Name		Subtitle	
Designer		Job Number	
Date	19 /04 /2016	Method	Limit State Design

Design Check			
Check	Required	Provided	Remark
Bolt shear capacity (kN)		$V_{dsb} = (800 \times 0.6126 \times 20 \times 20) / (\sqrt{3} \times 1.25 \times 1000)$ $= 90.529$ [cl. 10.3.3]	
Bolt bearing capacity (kN)		$V_{dsb} = (2.5 \times 0.508 \times 20 \times 7.7 \times 410) / (1.25 \times 1000)$ $= 64.15$ [cl. 10.3.4]	
Bolt capacity (kN)		Min (90.529, 64.15) = 64.15	Pass
No. of bolts	100/64.15 = 1.6	3	Pass
No. of column(s)	$\leq 2$	1	
No. of bolts per column		3	
Bolt pitch (mm)	$\geq 2.5 \times 20 = 50,$ $\leq \text{Min}(32 \times 7.7, 300) = 247$ [cl. 10.2.2]	60	
Bolt gauge (mm)	$\geq 2.5 \times 20 = 50,$ $\leq \text{Min}(32 \times 7.7, 300) = 247$ [cl. 10.2.2]	0	
End distance	$\geq 1.7 \times 22 = 37.4, \leq 12 \times 7.7$ $= 92.4$	40	

(mm)	[cl. 10.2.4]		
<b>Edge distance (mm)</b>	$\geq 1.7 \cdot 22 = 37.4, \leq 12 \cdot 7.7 = 92.4$ [cl. 10.2.4]	40	<b>Pass</b>
<b>Block shear capacity (kN)</b>	100	$V_{db} = 269$	
<b>Plate thickness (mm)</b>	$(5 \cdot 100 \cdot 1000) / (200 \cdot 250) = 10.0$ [Owens and Cheal, 1989]	10	
<b>Plate height (mm)</b>	$\geq 0.6 \cdot 300 = 180.0, \leq 300 - 13 - 14 - 17 - 17 - 5 = 234.0$ [cl. 10.2.4, Insdag Detailing Manual, 2002]	200	<b>Pass</b>
<b>Plate width (mm)</b>		100	
<b>Plate moment capacity (kNm)</b>	$(2 \cdot 90.529 \cdot 60^2) / (60 \cdot 1000) = 9.053$	$M_d = (1.2 \cdot 250 \cdot Z) / (1000 \cdot 1.1) = 18.18$ [cl. 8.2.1.2]	<b>Pass</b>
<b>Effective weld length (mm)</b>		$200 - 2 \cdot 8 = 184$	
<b>Weld strength (kN/mm)</b>	$\sqrt{[(9053 \cdot 6) / (2 \cdot 184^2)]^2 + [100 / (2 \cdot 184)]^2} = 0.847$	$f_v = (0.7 \cdot 8 \cdot 410) / (\sqrt{3} \cdot 1.25) = 1.06$ [cl. 10.5.7]	<b>Pass</b>
<b>Weld thickness (mm)</b>	$\text{Max}((0.847 \cdot 1000 \cdot \sqrt{3} \cdot 1.25) / (0.7 \cdot 410), 10 \cdot 0.8) = 8.0$ [cl. 10.5.7, Insdag Detailing Manual, 2002]	8	<b>Pass</b>



<b>Company Name</b>		<b>Project Title</b>	
<b>Group/Team Name</b>		<b>Subtitle</b>	
<b>Designer</b>		<b>Job Number</b>	
<b>Date</b>	<b>19 /04 /2016</b>	<b>Method</b>	<b>Limit State Design</b>

<b>Views</b>			
			
			

<b>Company Name</b>		<b>Project Title</b>	
<b>Group/Team Name</b>		<b>Subtitle</b>	
<b>Designer</b>		<b>Job Number</b>	
<b>Date</b>	<b>19 /04 /2016</b>	<b>Method</b>	<b>Limit State Design</b>

<b>Additional Comments</b>	
----------------------------	--