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#<https://github.com/derrickyebs>

```
import numpy as np L = 12
```

```
#length of beam in meters w =
```

```
10 #intensity of load in KN/m
```

```
#A
```

```
#Bending moment(M) and shear force(V) at the first end, x=0 x
```

```
= 0
```

```
M = (w*(-6*x**2 + 6*L*x-L**2))/12 V =
```

```
w*(L/2 - x) m= 'The bending moment at
```

```
x=0 is ' n= 'the shear force at x=0 is ' print()
```

```
print('(a.1)' + m + str(M) + ' and ', n + str(V))
```

```
#Bending moment(M) and shear force(V) at the first end, x=L=10
```

```
x = L
```

```
M = (w*(-6*x**2 + 6*L*x-L**2))/12
```

```
V = w*(L/2 - x) a= 'The bending
```

```
moment at x=L is ' b= 'the shear
```

```
force at x=L is '
```

```
print() print('(a.2)' + m + str(M) + ' and ', n +
```

```
str(V))
```

#B

When the bending moment is zero, we get an expression $x^2 - Lx + L^2/6 = 0$

#from the expression

a = 1

b = -L

c = $L^2/6$

#Using the Almighty formula the two roots are;

discriminant = $b^2 - 4ac$

root_1b = $(-b + \sqrt{\text{discriminant}})/2a$

root_2b = $(-b - \sqrt{\text{discriminant}})/2a$

print()

print('(b) The points of contra-flexure are {0} and {1}'.format(root_1b, root_2b))

#C

When the shear force is zero, $x = L/2$

""" $x = L/2$

print()

print('(c) The point at which V=0 is {}'.format(x))

#D

p = 0

s = 0.01

```
q = L + s
```

```
x = np.arange(p,q,s)
```

```
M = (w*(-6*x**2 + 6*L*x-L**2))/12
```

```
print()
```

```
print('(d) Using the initialized variable,the bending moment at each step in the array is {0}'.format(M))
```

#E

```
V = w*(L/2 - x)
```

```
print()
```

```
print('(e) The shear force for each step along the span is {}'.format(V))
```

#F

Let the absolute value of the bending moment array be AM

Also let the minimum AM be m_AM

```
AM = abs(M)
```

```
m_AM = min(AM)
```

When the bending moment is m_AM, we get an expression $x^2 - Lx + (L^2/6) + (2*m_AM)/w = 0$

#from the above expression

```
a = 1
```

```
b = -L
```

```
c = (L**2/6)+(2*m_AM)/w
```

```
#Using the Almighty formula the two roots are;
```

```
discriminant = b**2 - 4*a*c
```

```
root_1f = (-b + np.sqrt(discriminant))/2*a
```

```
root_2f = (-b - np.sqrt(discriminant))/2*a
```

```
print()
```

```
print('(f) The points along L at which the absolute values of the bending moment array is minimum are {0} and {1}'.format(root_1f,root_2f))
```

```
#G
```

```
"""
```

```
Let the relative errors be r_e
```

```
"""
```

```
r_e1 = ((root_1b - root_1f)/root_1b*100)
```

```
r_e2 = ((root_2f - root_2b)/root_2f*100)
```

```
print()
```

```
print('(g) The relative errors between estimated points of contra-flexure are {0}% and {1}%'.format(r_e1,r_e2))
```

```
#H
```

```
"""
```

```
Let the maximum bending moment be max_M and the minimum bending moment be min_M
```

```
"""
```

```
#for the maximum
```

```
max_M = max(M)
```

```
"""
```

When the bending moment is max_M, we get an expression $x^2 - Lx + (L^2/6) + (2 \cdot \text{max_M})/w = 0$

"""

a= 1

b = -L

c = (L**2/6)+(2*max_M)/w

#Using the Almighty formula the two roots are;

discriminant = b**2 - 4*a*c

root_1 = (-b + np.sqrt(discriminant))/2*a

root_2 = (-b - np.sqrt(discriminant))/2*a print()

print('(h.1) The points at which the maximum bending moment occur are {0} and {1}'.format(root_1,root_2))

#for the minimum

min_M = min(M)

"""

When the bending moment is min_M, we get an expression $x^2 - Lx + (L^2/6) + (2 \cdot \text{min_M})/w = 0$

"""

a = 1

b = -L

c = (L**2/6)+(2*min_M)/w

#Using the Almighty formula the two roots are;

discriminant = b**2 - 4*a*c

root_1 = (-b - np.sqrt(discriminant))/2*a

root_2 = (-b + np.sqrt(discriminant))/2*a print()

print('(h.2) The points at which the minimum bending moment occur are {0} and

```
{1}'.format(root_1,root_2))
```