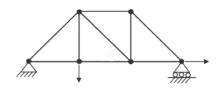
Exam 1 Solution

1a)
$$\begin{cases} f_{1x} \\ f_{2x} \end{cases} = \begin{bmatrix} k & -k \\ -k & k \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \end{Bmatrix} \quad U = \frac{1}{2} \{d\}^T [k] \{d\} = \frac{1}{2} \begin{bmatrix} u_1 & u_2 \end{bmatrix} \begin{bmatrix} k & -k \\ -k & k \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \end{Bmatrix}$$

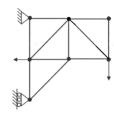
Version	f_{lx} (lb)	f_{Ix} (lb)	T/C?	U (lb-in)
1	500	-500	compression	125
2	-160	160	tension	16
3	-250	250	Tension	62.5

b) Version 1

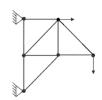


[k]
$$\{F\}$$
 [K] $\{D_{reduced}\}$ [$K_{reduced}\}$] 4×4 12×1 12×12 9×1 9×9

Version 2



Version 3



2. All versions

Theta (deg)	С	S	c^2	s ²	cs
135	$-\frac{\sqrt{2}}{2}$	$\frac{\sqrt{2}}{2}$	$\frac{1}{2}$	$\frac{1}{2}$	$-\frac{1}{2}$
180	-1	0	1	0	0

$$\begin{bmatrix} k^{(1)} \end{bmatrix} = \frac{E_1 A}{L} \begin{bmatrix} 1/2 & -1/2 & -1/2 & 1/2 \\ -1/2 & 1/2 & 1/2 & -1/2 \\ -1/2 & 1/2 & 1/2 & -1/2 \\ 1/2 & -1/2 & -1/2 & 1/2 \end{bmatrix} \quad \begin{bmatrix} k^{(2)} \end{bmatrix} = \frac{E_2 A}{L} \begin{bmatrix} 1 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 \\ -1 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 \end{bmatrix}$$

Global Equations (after applying loads and boundary conditions):

where
$$K_{11} = \frac{E_1 A}{2L} + \frac{E_2 A}{L}$$
, $K_{12} = -\frac{E_1 A}{2L}$ and $K_{22} = \frac{E_1 A}{2L}$

- a) Solve global equations for u_1 and v_1 .
- b) Stress in element 1

$$\sigma^{(1)} = \frac{E_1}{L} \begin{bmatrix} \frac{\sqrt{2}}{2} & -\frac{\sqrt{2}}{2} & -\frac{\sqrt{2}}{2} & \frac{\sqrt{2}}{2} \end{bmatrix} \begin{bmatrix} u_1 \\ v_1 \\ 0 \\ 0 \end{bmatrix}$$

c) Reaction forces

$$\begin{cases} F_{1x} \\ F_{1y} \\ F_{2x} \\ F_{2y} \\ F_{3x} \\ F_{3y} \end{cases} = \begin{cases} K_{11} & K_{12} \\ K_{12} & K_{22} \\ K_{13} & K_{23} \\ K_{14} & K_{24} \\ K_{15} & K_{25} \\ K_{16} & K_{26} \end{cases} \begin{cases} u_1 \\ v_1 \end{cases}$$

where
$$K_{11}=\frac{E_1A}{2L}+\frac{E_2A}{L},\; K_{12}=-\frac{E_1A}{2L},\; K_{22}=\frac{E_1A}{2L},\; K_{13}=-\frac{E_2A}{L},\; K_{23}=0\;,\; K_{14}=0\;,\; K_{24}=0\;,\; K_{15}=-\frac{E_1A}{2L}\;,\; K_{25}=\frac{E_1A}{2L}\;,\; K_{16}=\frac{E_1A}{2L}\;\; \text{and}\; K_{26}=-\frac{E_1A}{2L}$$

Version	u ₁ (in)	v ₁ (in)	$\sigma^{(1)}$ (psi)	Reaction at	F_x (lb)	F_{y} (lb)
			•	Node #		
1	004	0028	1,414 (T)	3	-2,000	2,000
2	015	025	1,768 (T)	2	2,500	0
3	009	0045	2,121 (T)	3	-3,000	3,000

3. a) Since
$$v_1 = \emptyset_1 = v_2 = 0$$
, equation 4 gives $M_2 = \frac{EI}{L^3} (4L^2) \, \emptyset_2$ and, from table D-1, $M_2 = \frac{wL^2}{30}$, we can solve for $\emptyset_2 = \frac{wL^3}{120 \, EI}$
$$[k] = \frac{EI}{L^3} \begin{bmatrix} 12 & 6L & -12 & 6L \\ 6L & 4L^2 & -6L & 2L^2 \\ -12 & -6L & 12 & -6L \\ 6L & 2L^2 & -6L & 4L^2 \end{bmatrix}$$

b) Reaction forces given by $\{F\} = [K]\{D\} - \{F_0\}$, or

which gives
$$F_{1y} = \left(\frac{EI}{L^3}\right)(6L)\emptyset_2 - \left(-\frac{7wL}{20}\right)$$
 and $M_1 = \left(\frac{EI}{L^3}\right)(2L^2)\emptyset_2 - \left(-\frac{wL^2}{20}\right)$.

Results:

Version	Ø ₂ (radians)	F_{1y} (N)	M_1 (N-m)
1	4.167e-4	12,000	20,000
2	4.167e-4	8,000	13,333
3	1.190e-3	4,000	6,667