



# Periodic Material-Based Seismic Base Isolators for Small Modular Reactors

AMM Workshop  
US Department of Energy  
October 17 and 18, 2016

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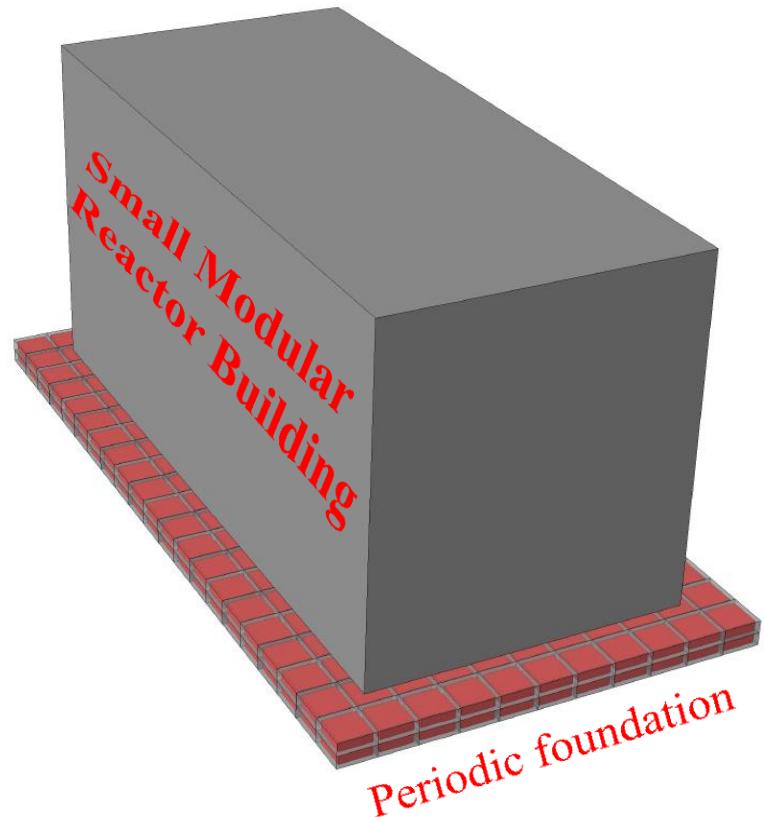
# Project overview

## Objective:

To develop a periodic foundation that can completely obstruct or change the energy pattern of the earthquake before it reaches the structure of small modular reactors (SMR).

## Scopes:

- (1) Perform comprehensive, analytical study on periodic foundations.
- (2) Design a SMR model with periodic foundations.
- (3) Verify the effectiveness of periodic foundations through shake table tests.
- (4) Perform finite element simulation of SMR supported by periodic foundations.



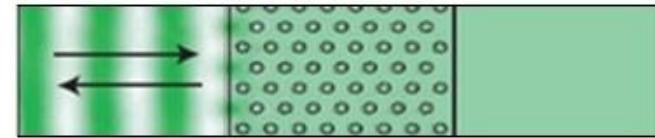
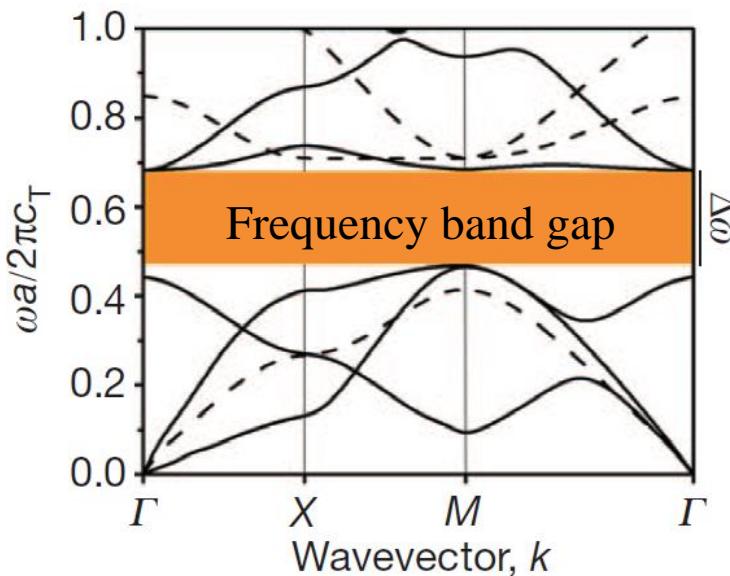
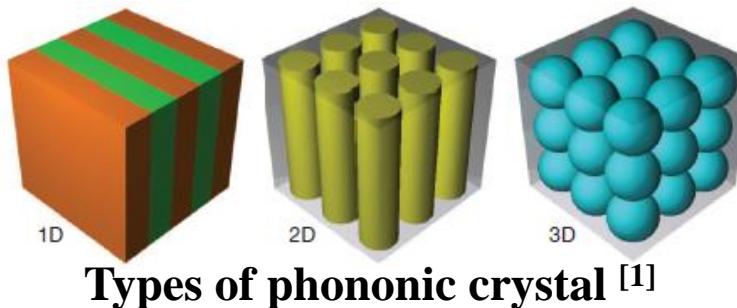
# Project schedule

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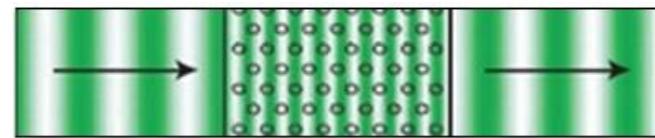
Task	2014	2015				2016				2017		
	Oct-Dec	Jan-Mar	Apr-Jun	Jul-Sep	Oct-Dec	Jan-Mar	Apr-Jun	Jul-Sep	Oct-Dec	Jan-Mar	Apr-Jun	Jul-Sep
1	Review of previous work and literature											
2		Theoretical study on periodic foundations										
3			Design of periodic foundations									
4					Experimental study of periodic foundations							
5							Experimental data analysis and numerical simulation of periodic foundations					
6												Preparation of final report

# Basic concept

Phononic crystal is a novel composite developed in solid-state-physics



Wave propagation with frequency  
within the frequency band gap



Wave propagation with frequency  
outside of the frequency band gap

Wave Propagation [2]

[1] Maldovan, M. (2013). Sound and heat revolutions in phononics. *Nature*, 503(7475), 209-217.

[2] Thomas, E. L., Gorishnyy, T., & Maldovan, M. (2006). Phononics: Colloidal crystals go hypersonic. *Nature materials*, 5(10), 773-774.

# Calculating dispersion curve

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The governing equation of motion for a continuum body with isotropic elastic material

$$\rho(\mathbf{r}) \frac{\partial^2 \mathbf{u}}{\partial t^2} = \nabla \left\{ [\lambda(\mathbf{r}) + 2\mu(\mathbf{r})] (\nabla \cdot \mathbf{u}) \right\} - \nabla \times [\mu(\mathbf{r}) \nabla \times \mathbf{u}] \quad \text{Eq. 1}$$

Where:  $\mathbf{r}$  is coordinate vector;  $\rho(\mathbf{r})$  is the density  
 $\mathbf{u}(\mathbf{r})$  is displacement vector;  $\lambda(\mathbf{r})$  and  $\mu(\mathbf{r})$  are the Lamé constant

The periodic boundary condition equation:

$$\mathbf{u}(\mathbf{r} + \mathbf{a}, t) = e^{i\mathbf{K} \cdot \mathbf{a}} \mathbf{u}(\mathbf{r}, t) \quad \text{Eq. 2}$$

Where:  $\mathbf{K}$  is the wave vector  
 $\mathbf{a}$  is unit cell size

Substituting Eq. 2 to the governing equation Eq.1, the wave equation can be transferred into a Eigen value problem as follow:

$$(\mathbf{\Omega}(\mathbf{K}) - \omega^2 \mathbf{M}) \cdot \mathbf{u} = 0 \quad \text{Eq. 3}$$

# Calculating dispersion curve

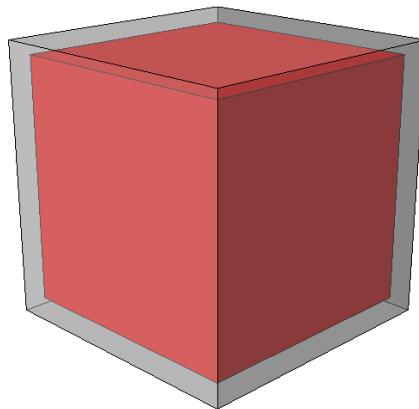


**Eigen value problem:**

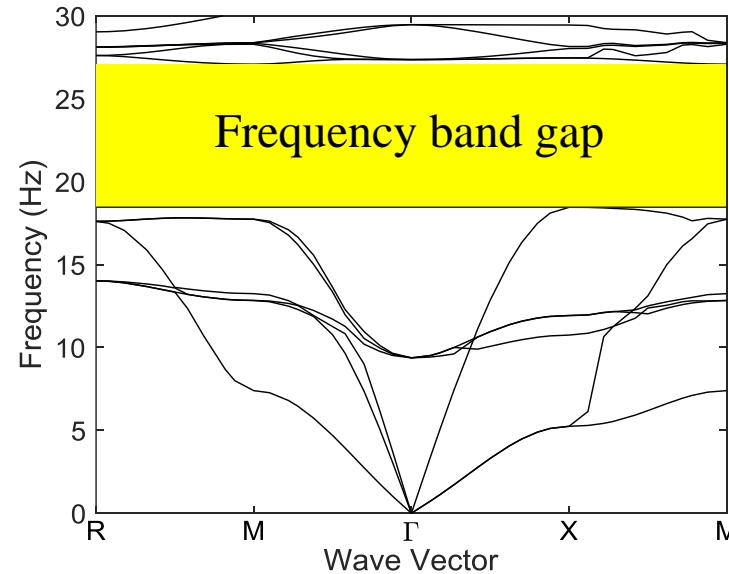
$$(\Omega(\mathbf{K}) - \omega^2 \mathbf{M}) \cdot \mathbf{u} = 0$$

Where:  $\Omega$  is the stiffness matrix  
 $\mathbf{M}$  is the mass matrix

For each wave vector ( $\mathbf{K}$ ) a series of corresponding frequencies ( $\omega$ ) can be obtained.



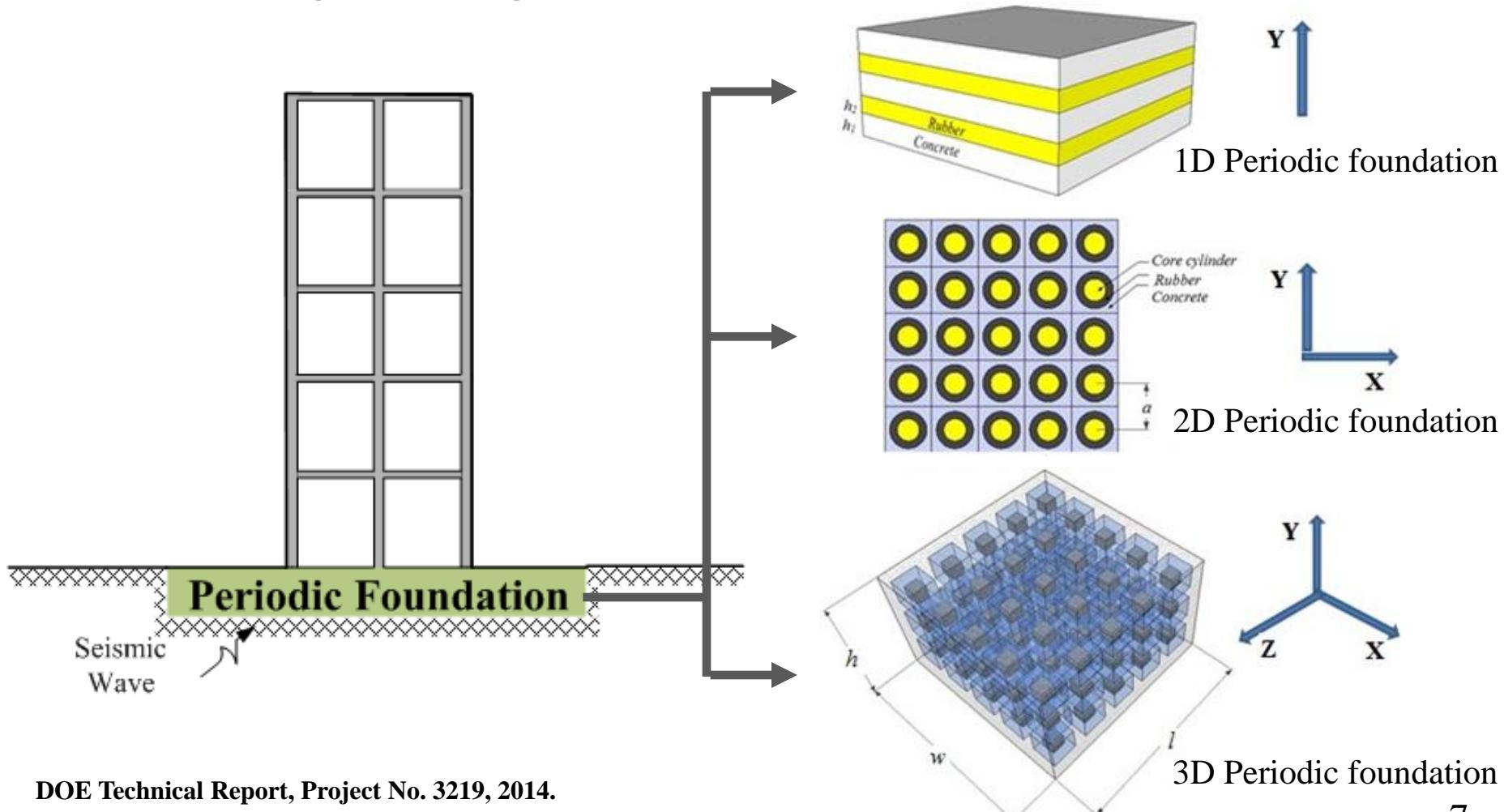
**Typical unit cell of 3D two-component periodic foundation**



**Typical dispersion curve for 3D two-component unit cell**

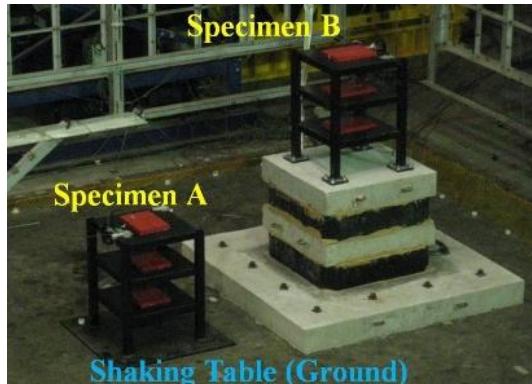
# Application of phononic crystals

In civil engineering field

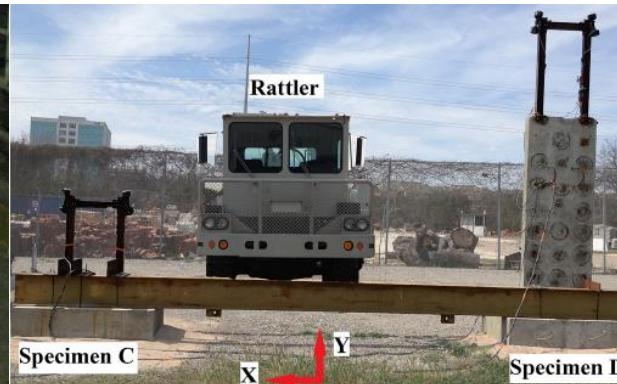


# Experimental study on periodic foundations

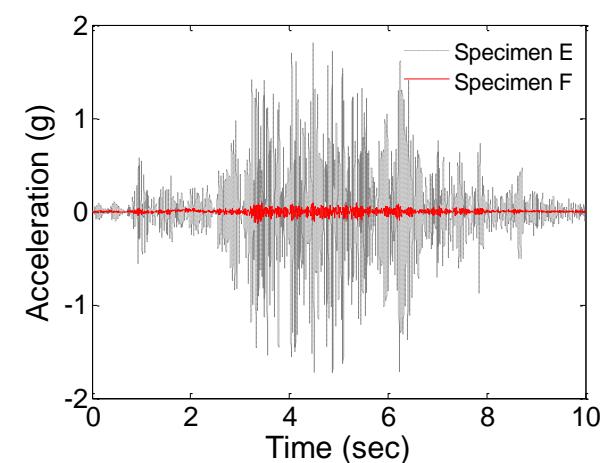
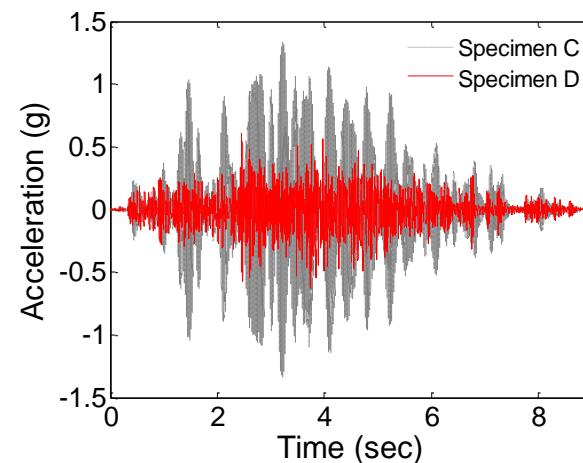
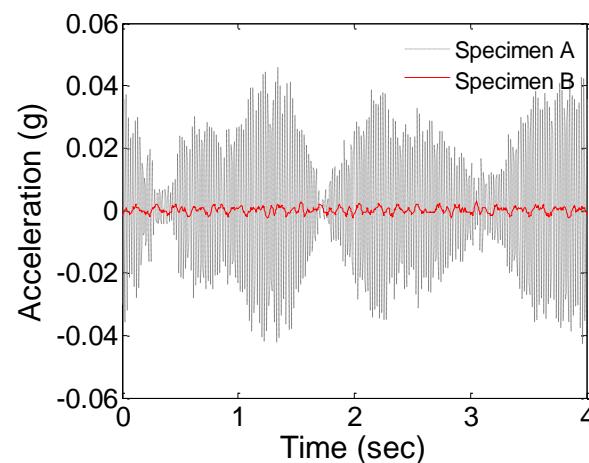
**1D Periodic foundation<sup>[1]</sup>**



**2D Periodic foundation<sup>[2]</sup>**



**3D Periodic foundation<sup>[3]</sup>**



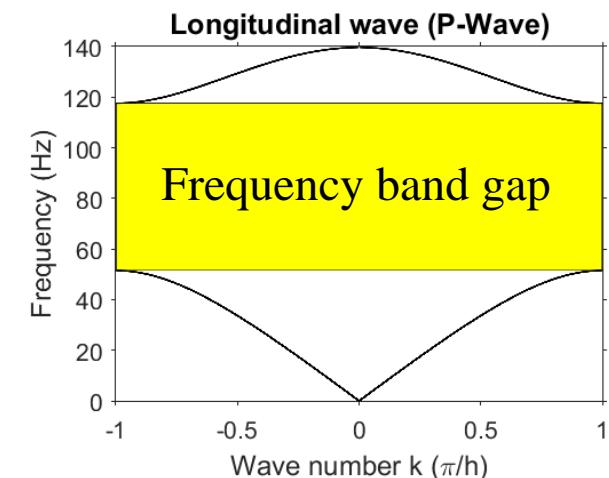
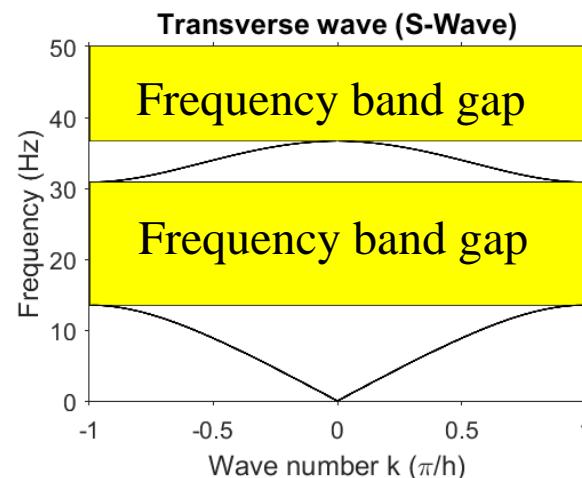
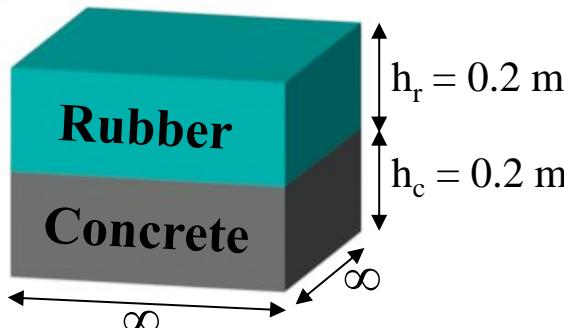
[1] Xiang et al. (2012). Periodic materials-based vibration attenuation in layered foundations: experimental validation. *Smart materials and structures*, 21, 1-10.

[2] Yan et al. (2014). Seismic isolation of two dimensional periodic foundations. *Journal of Applied Physics*, 116(4).

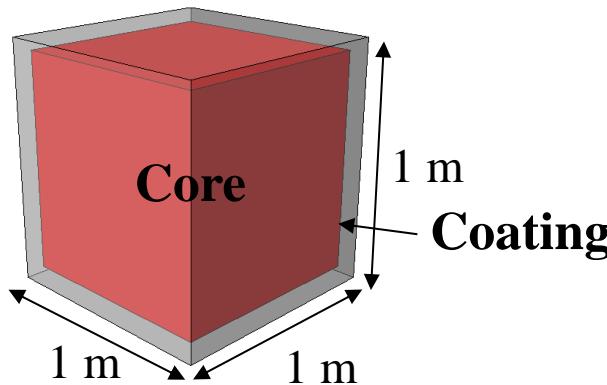
[3] Yan et al. (2015). Three dimensional periodic foundations for base seismic isolation. *Smart Materials and Structures*, 24(7), 075006.

# Theoretical study on periodic foundations

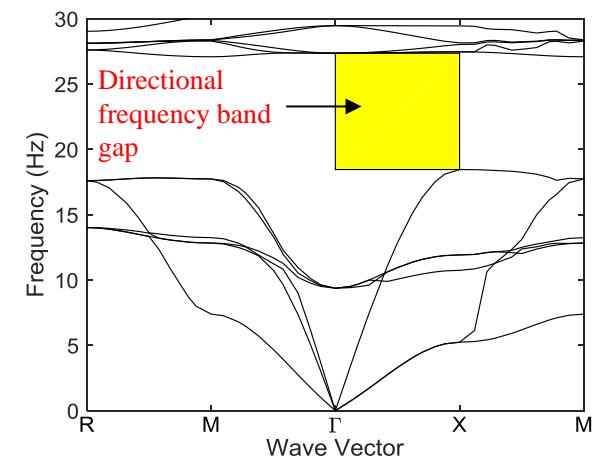
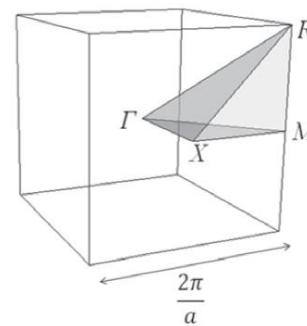
## 1D periodic foundation



## 3D periodic foundation

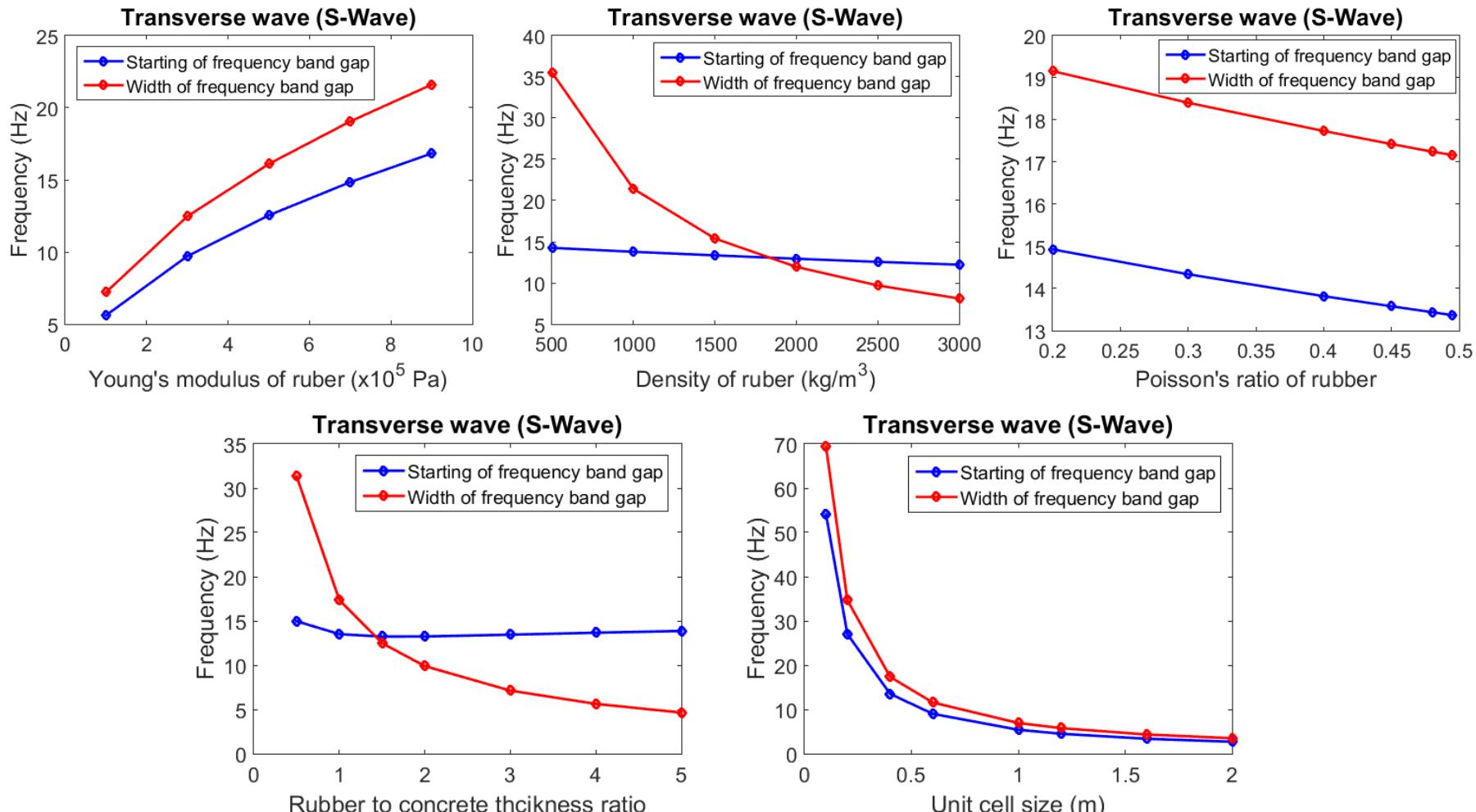


First irreducible Brillouin zone



# Parametric study of 1D periodic foundation

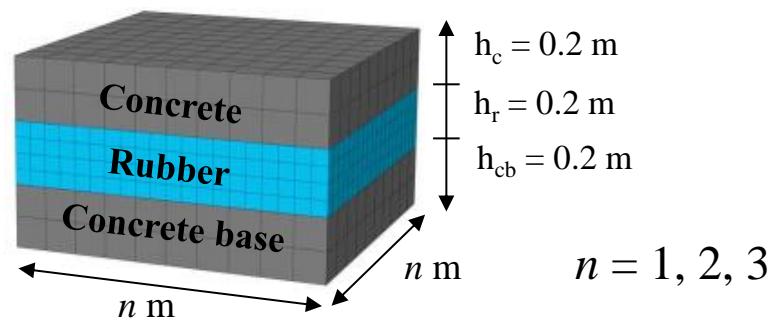
## Effect of material and geometric properties on the first frequency band gap



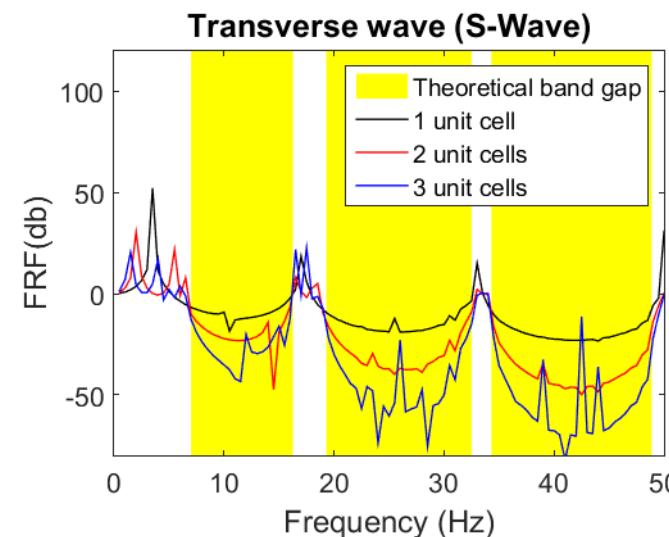
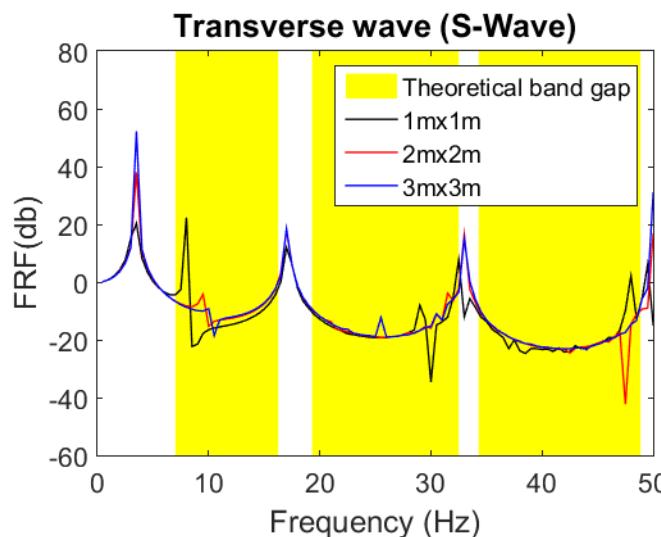
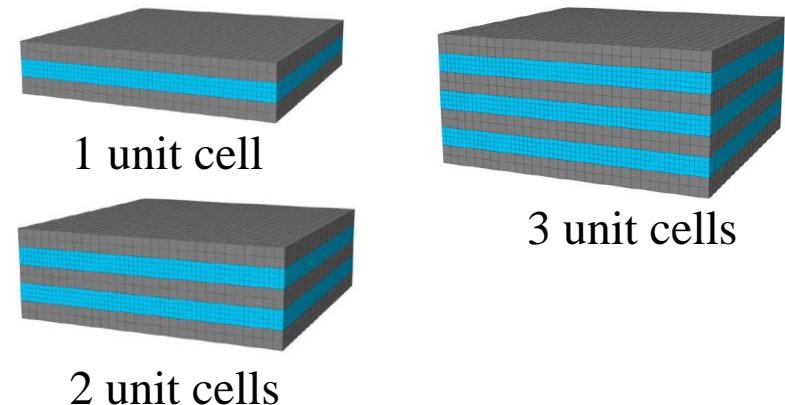
# Parametric study of 1D periodic foundation

## Effect of cross section size

Consider 1 unit cell on concrete base



## Effect of number of unit cells



Frequency response:

$$\text{FRF} = 20 \log(\delta_{\text{out}} / \delta_{\text{inp}})$$

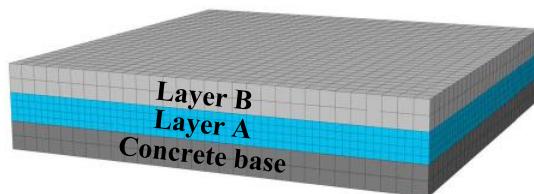
where:

$\delta_{\text{out}}$  = amplitude of output disp  
 $\delta_{\text{inp}}$  = amplitude of input disp

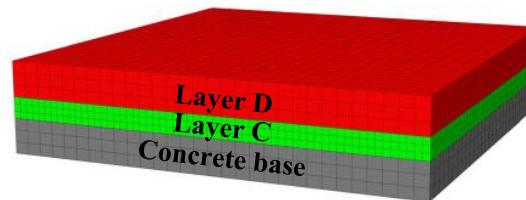
# Parametric study of 1D periodic foundation

## Effect of combined unit cells

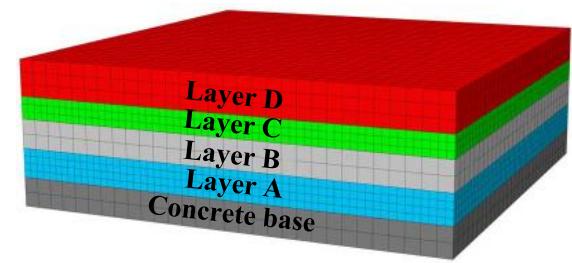
Unit cell-1



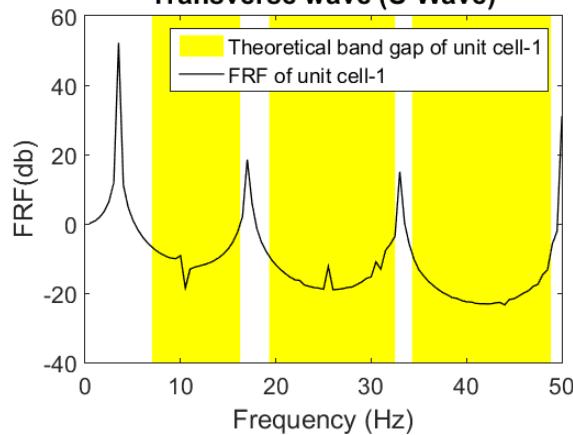
Unit cell-2



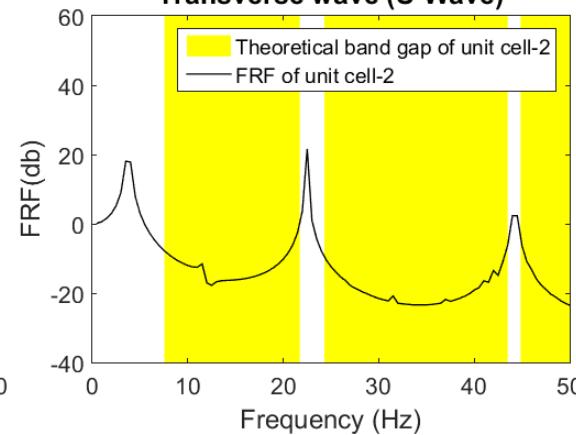
Unit cell-3



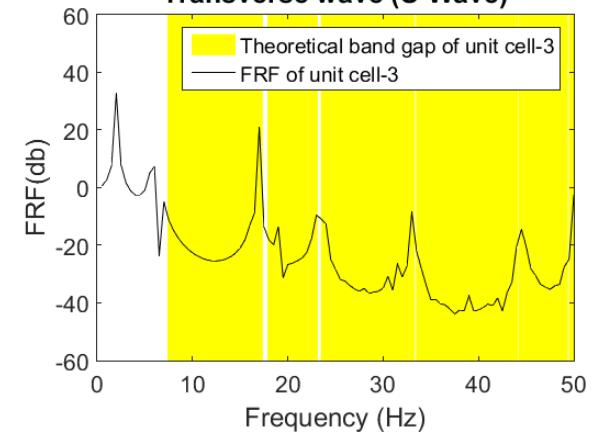
Transverse wave (S-Wave)



Transverse wave (S-Wave)

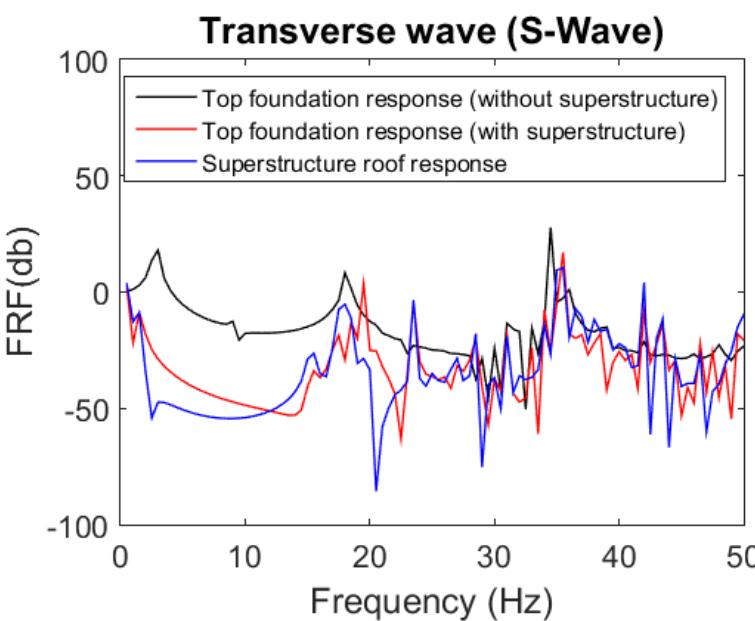
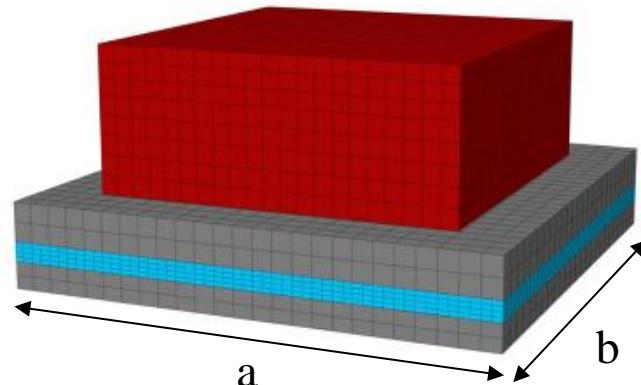


Transverse wave (S-Wave)

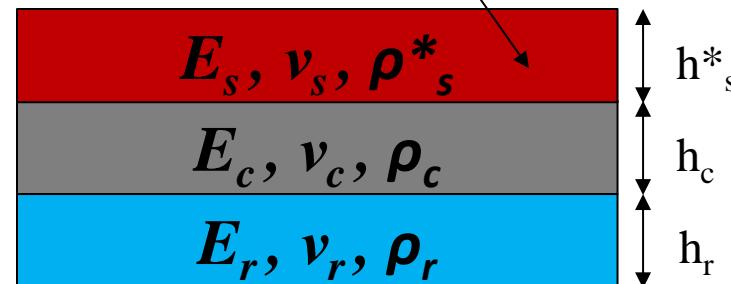


# Parametric study of 1D periodic foundation

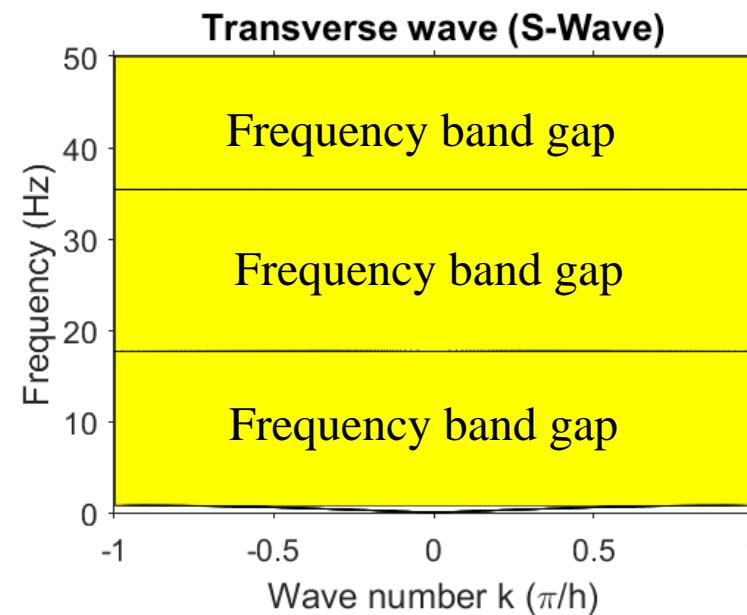
## Effect of superstructure



## Equivalent layer

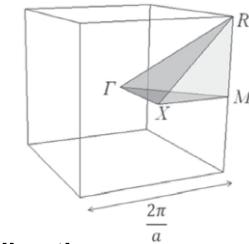
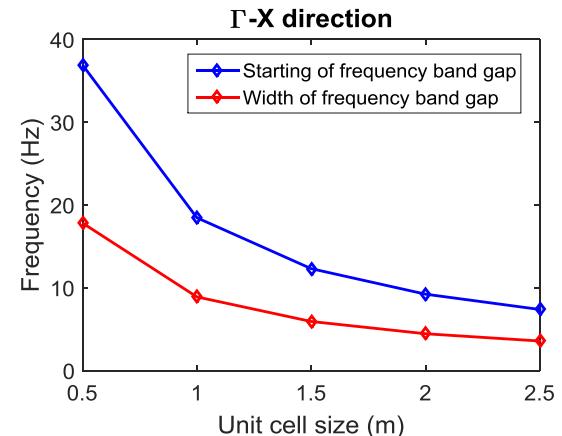
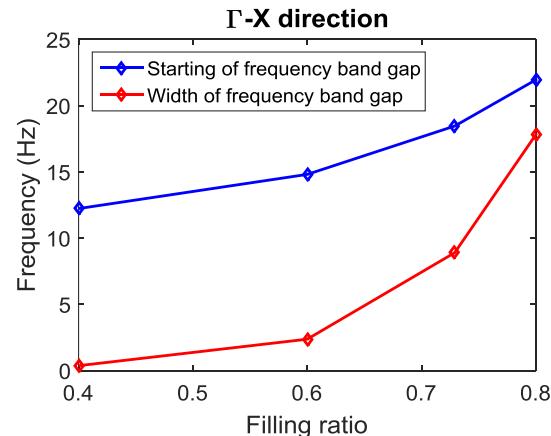
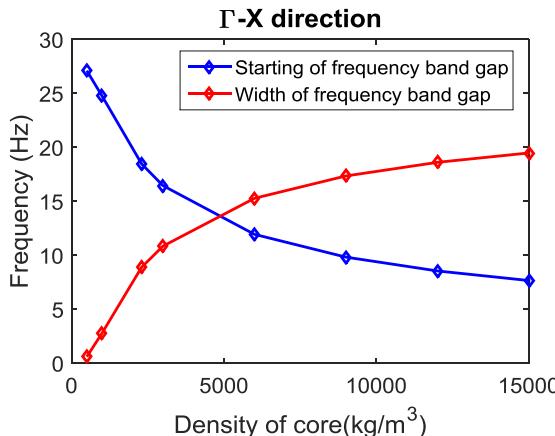
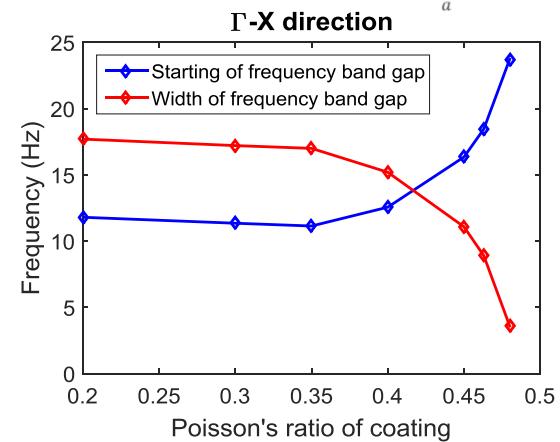
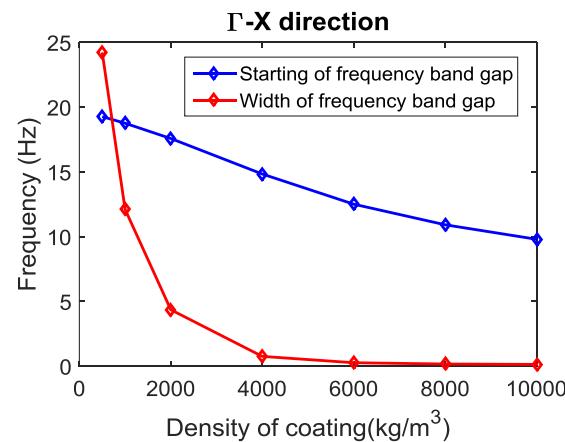
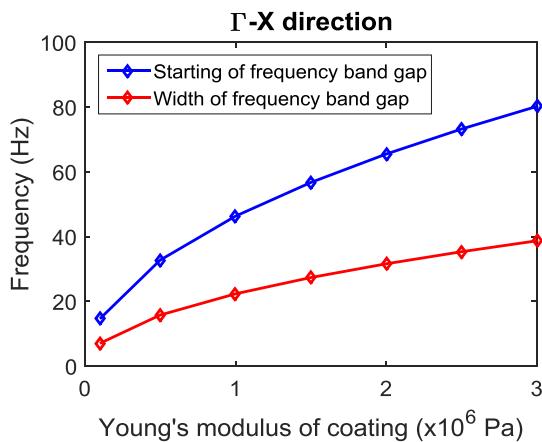


$$\text{Where: } \rho_s^* = \frac{W_{\text{superstructure}}}{a \times b \times h_s^*}$$



# Parametric study of 3D periodic foundation

**Effect of material and geometric properties on the first directional frequency band gap**



# Design guidelines of 1D and 3D periodic foundations



## 1D periodic foundation

Starting of frequency band gap (S-Wave) =  $13.51F_1(E_r)F_2(\rho_r)F_3(v_r)F_4(\rho_c)F_5(S)F_6(r)$

Width of frequency band gap (S-Wave) =  $17.36G_1(E_r)G_2(\rho_r)G_3(v_r)G_4(\rho_c)G_5(S)G_6(r)$

Starting of frequency band gap (P-Wave) =  $51.5H_1(E_r)H_2(\rho_r)H_3(v_r)H_4(\rho_c)H_5(S)H_6(r)$

Width of frequency band gap (P-Wave) =  $66.1I_1(E_r)I_2(\rho_r)I_3(v_r)I_4(\rho_c)I_5(S)I_6(r)$

## 3D periodic foundation

Starting of directional frequency band gap =  $18.46J_1(E_r)J_2(\rho_r)J_3(v_r)J_4(\rho_c)J_5(S)J_6(f_r)$

Width of directional frequency band gap =  $8.9K_1(E_r)K_2(\rho_r)K_3(v_r)K_4(\rho_c)K_5(S)K_6(f_r)$

The equations are function of:

$E_r$  : Young's modulus of rubber

$\rho_r$  : Density of rubber

$v_r$  : Poisson's ratio of rubber

$\rho_c$  : Density of concrete

$S$  : Unit cell size (S)

$f_r$  : Filling ratio

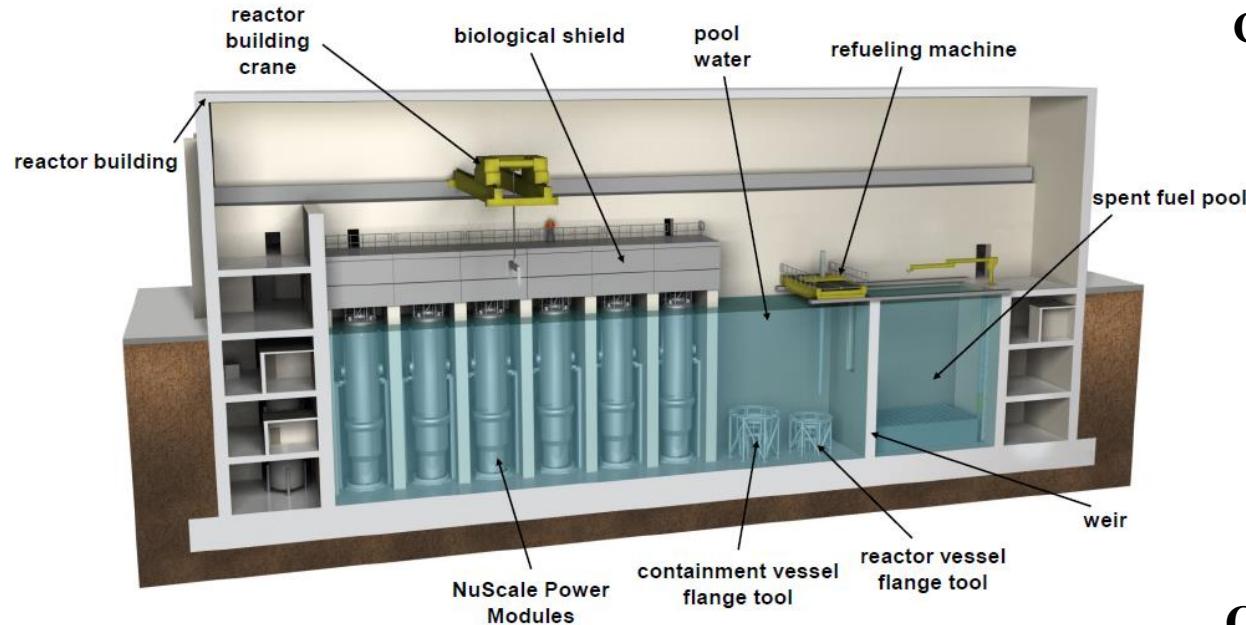
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# Task 3: Design of periodic foundations

# Small modular reactor (SMR) building

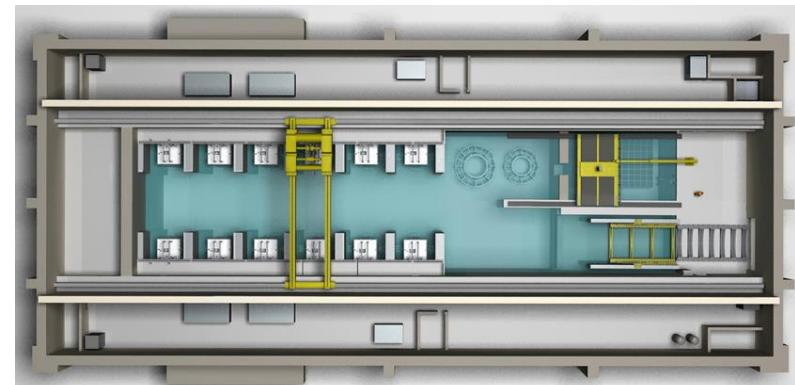


Cutaway elevation view



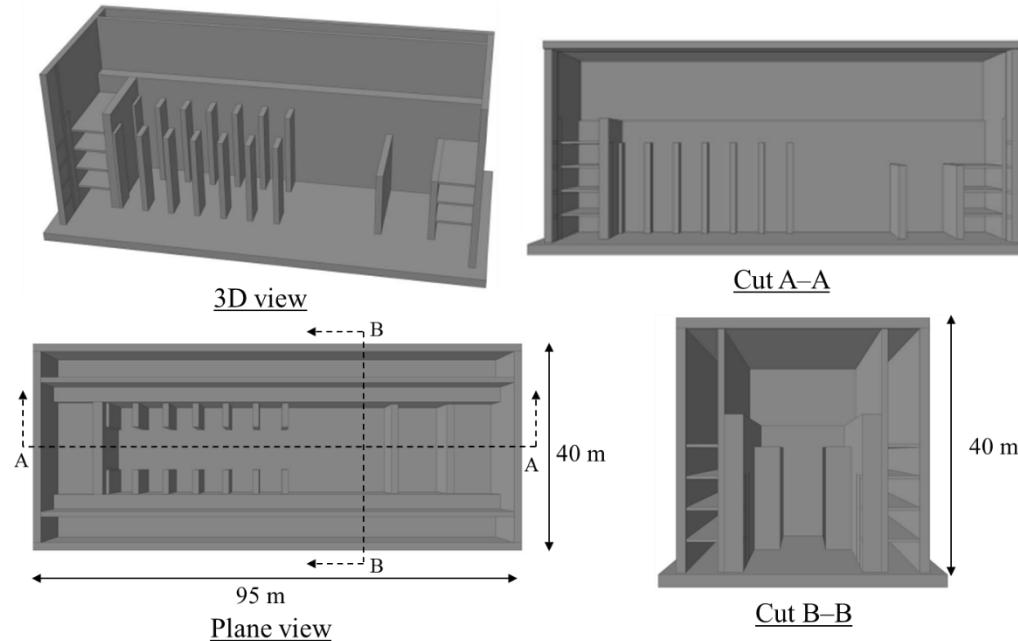
Courtesy of NuScale Power

Overhead plan view



# SMR building

Finite element model of representative of NuScale's SMR Building



## Material Properties:

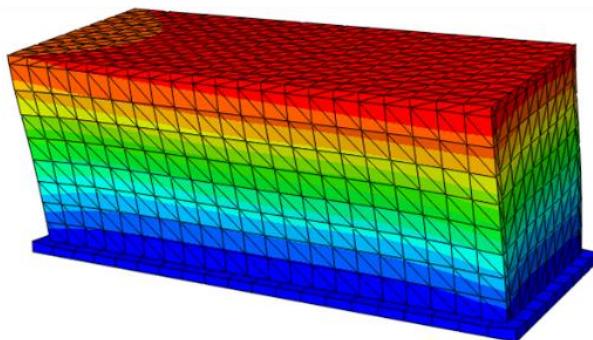
- Reinforced concrete ( $E_s = 31400 \text{ MPa}$ ,  $\rho = 2300 \text{ kg/m}^3$ ,  $\nu = 0.2$ )

## Mass of non-structural components:

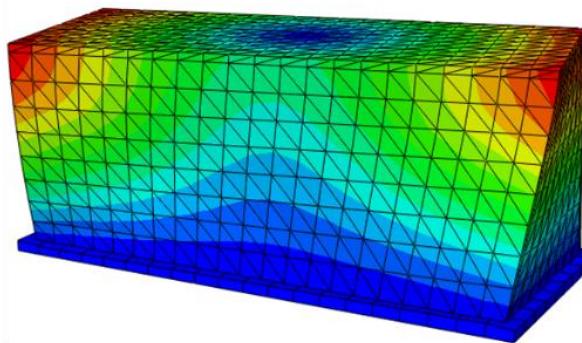
- Water in reactor pool = 5.09 million gallon =  $19.28 \times 10^6 \text{ kg}$
- Small modular reactors (12 units) =  $12 \times (8 \times 10^5) = 9.6 \times 10^6 \text{ kg}$
- Crane and utilities =  $8 \times 10^5 \text{ kg}$

# SMR building

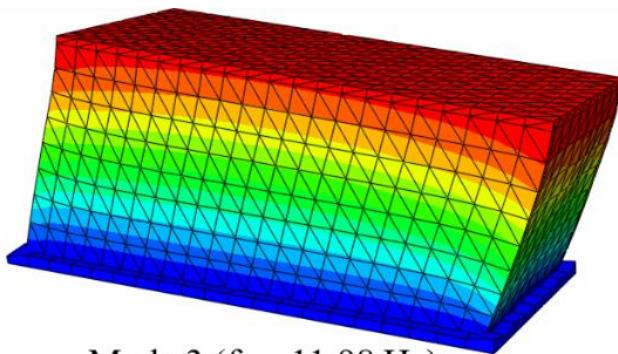
## Mode shapes and natural frequencies of SMR building



Mode 1 ( $f_n = 6.77 \text{ Hz}$ )



Mode 2 ( $f_n = 11.14 \text{ Hz}$ )

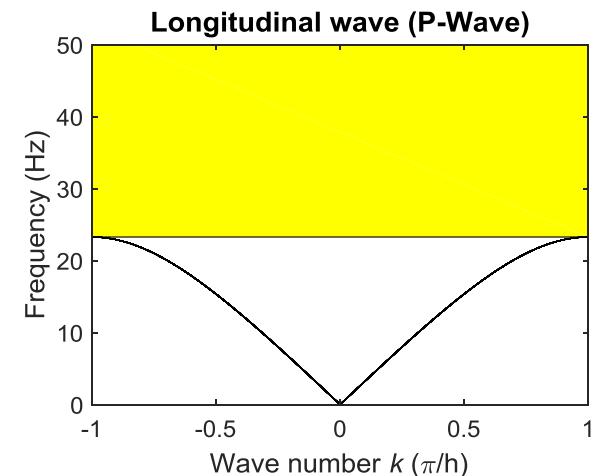
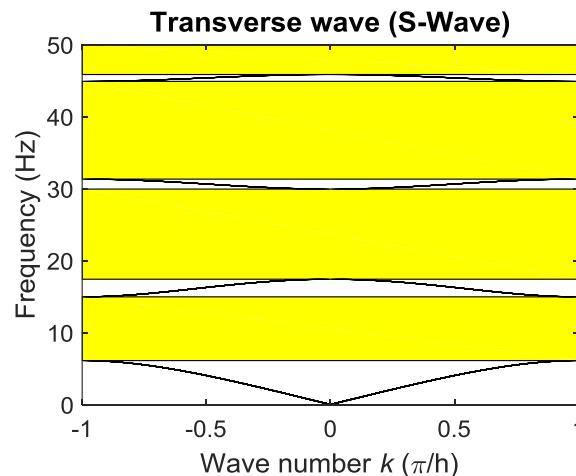
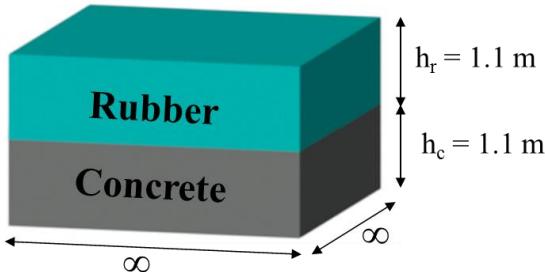


Mode 3 ( $f_n = 11.98 \text{ Hz}$ )

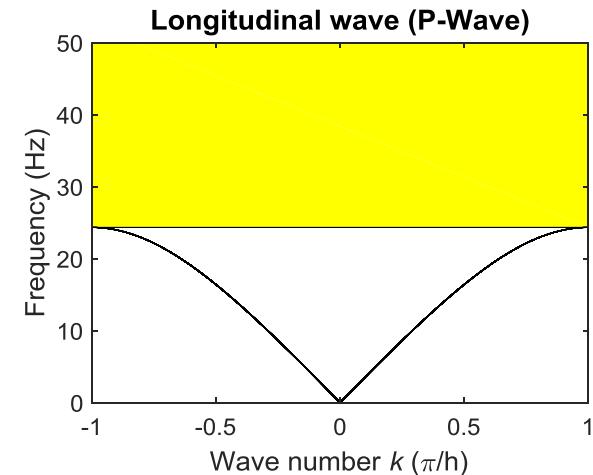
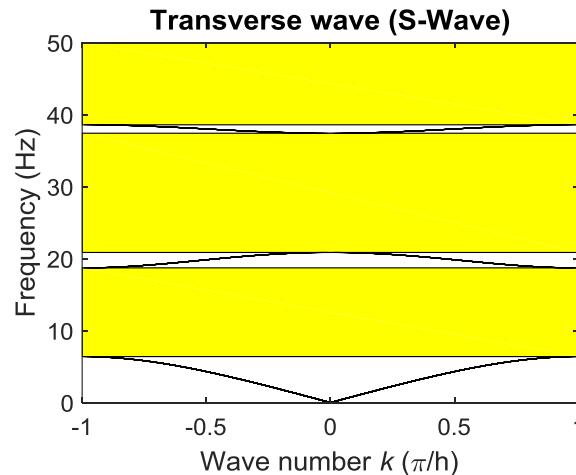
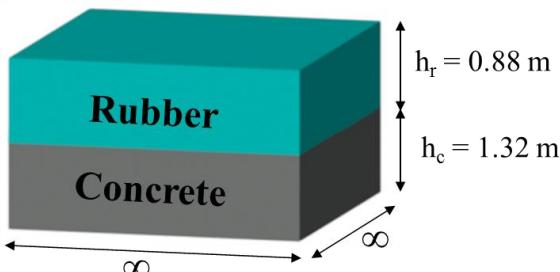
# Design of 1D periodic foundation



Unit cell 1



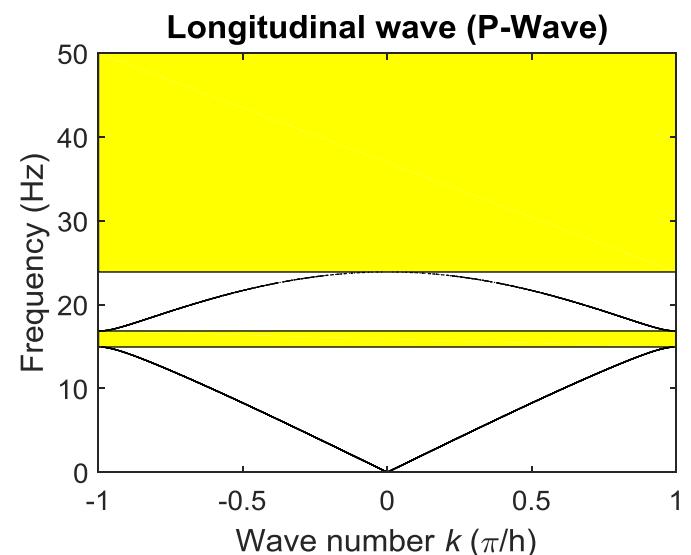
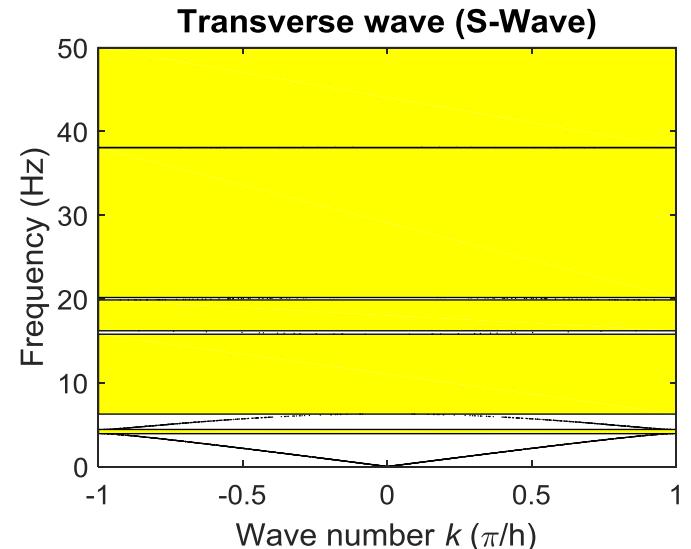
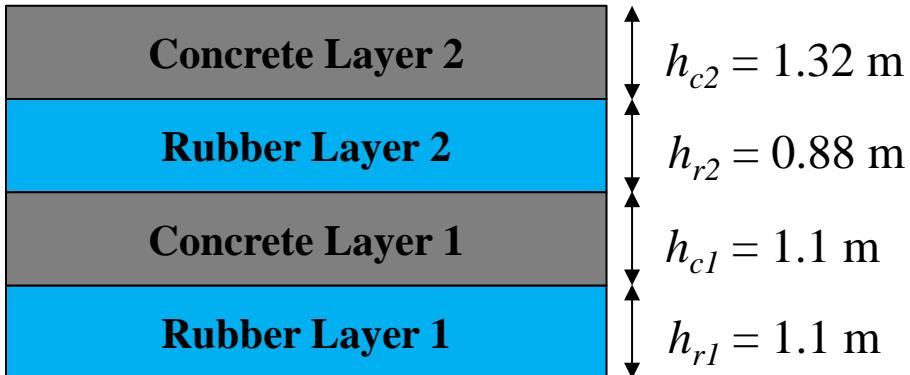
Unit cell 2



# Design of 1D periodic foundation



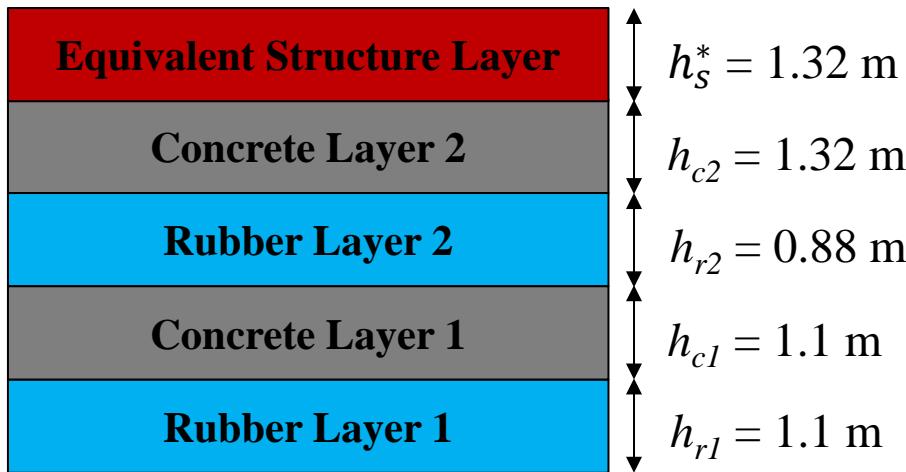
## Combined unit cell



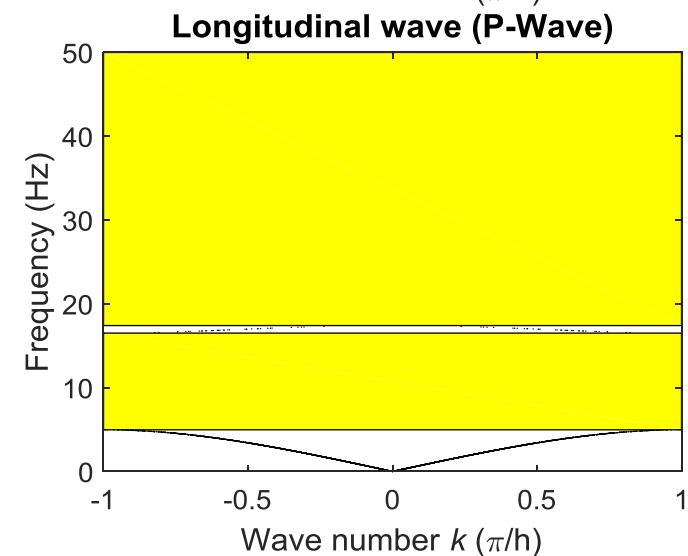
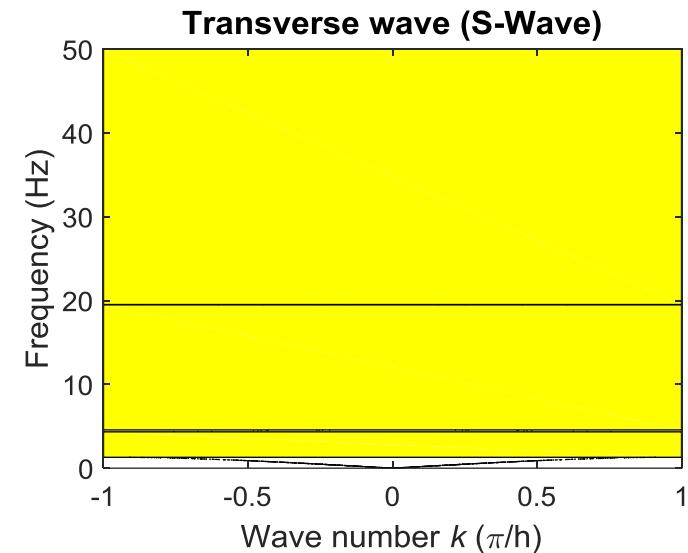
Material	Young's Modulus (MPa)	Density (kg/m <sup>3</sup> )	Poisson's Ratio
Concrete	31400	2300	0.2
Rubber	3.49	1100	0.463

# Design of 1D periodic foundation

## (Combined unit cell with equivalent structure layer)

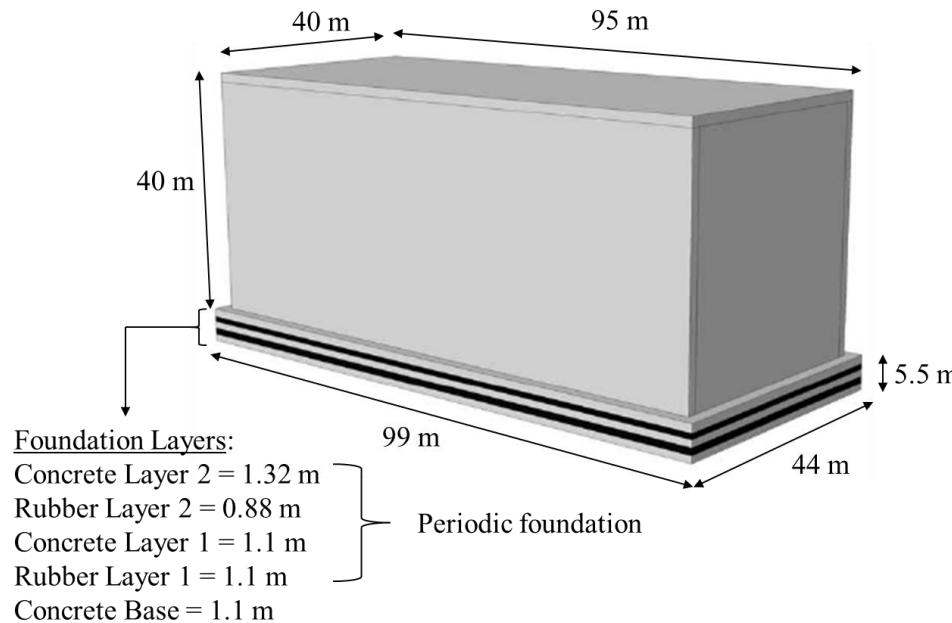


Material	Young's Modulus (Pa)	Density (kg/m <sup>3</sup> )	Poisson's Ratio
Concrete	$3.14 \times 10^{10}$	2300	0.2
Rubber	$3.49 \times 10^6$	1100	0.463
Equivalent Super Strc	$3.14 \times 10^{10}$	24247.2	0.2

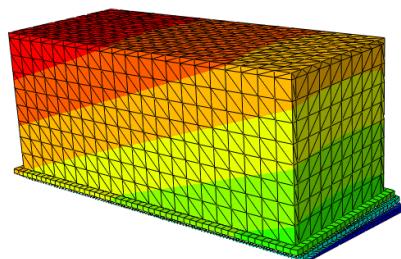


# Design of 1D periodic foundation

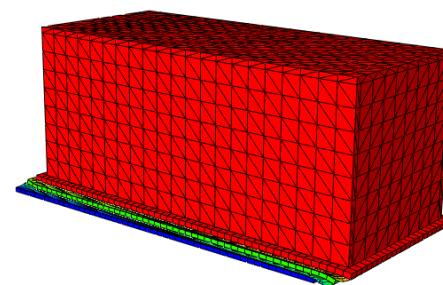
Designed 1D periodic foundation supporting SMR building



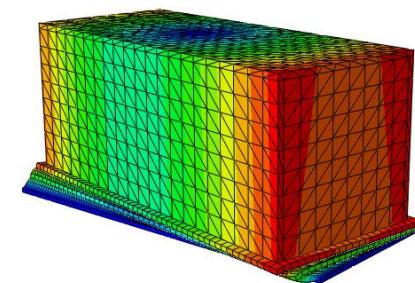
Natural frequencies of structure system



Mode 1 ( $f_n = 0.59$  Hz)



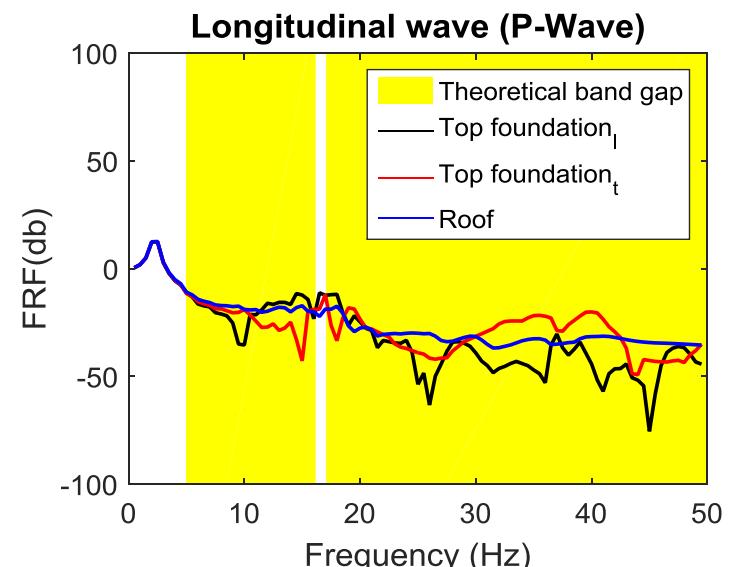
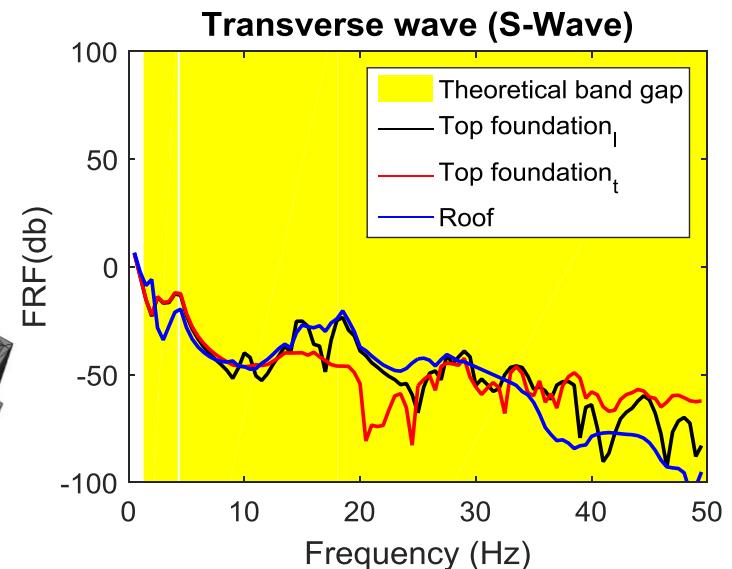
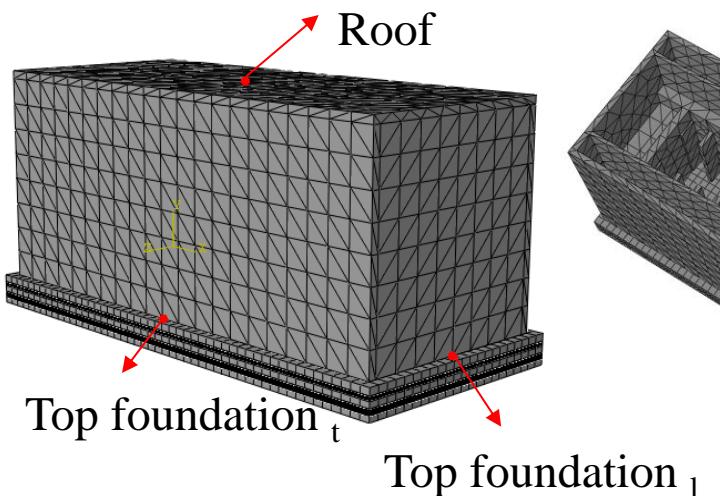
Mode 2 ( $f_n = 0.63$  Hz)



Mode 3 ( $f_n = 0.64$  Hz)

# Design of 1D periodic foundation

## Frequency response function



Damping ratio:

$$\xi_{\text{concrete}} = 4 \%$$

$$\xi_{\text{rubber}} = 10 \%$$

Frequency response:

$$\text{FRF} = 20 \log(\delta_{\text{out}} / \delta_{\text{inp}})$$

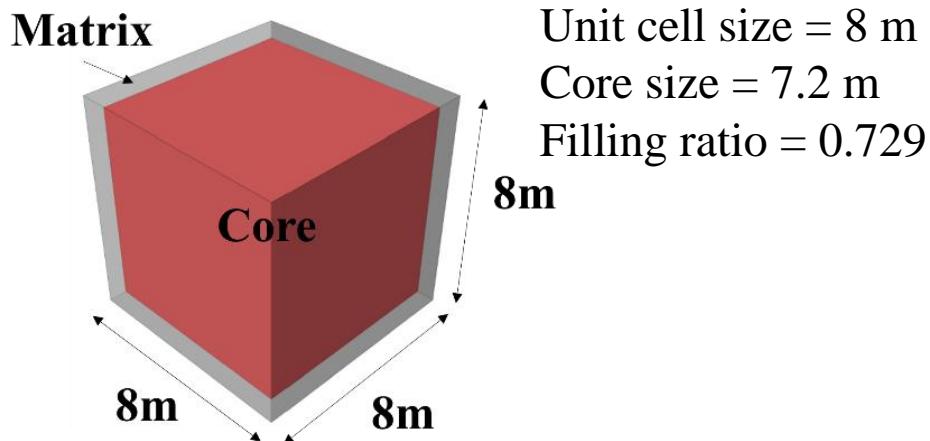
where:

$\delta_{\text{out}}$  = amplitude of output disp

$\delta_{\text{inp}}$  = amplitude of input disp

# Design of 3D periodic foundation

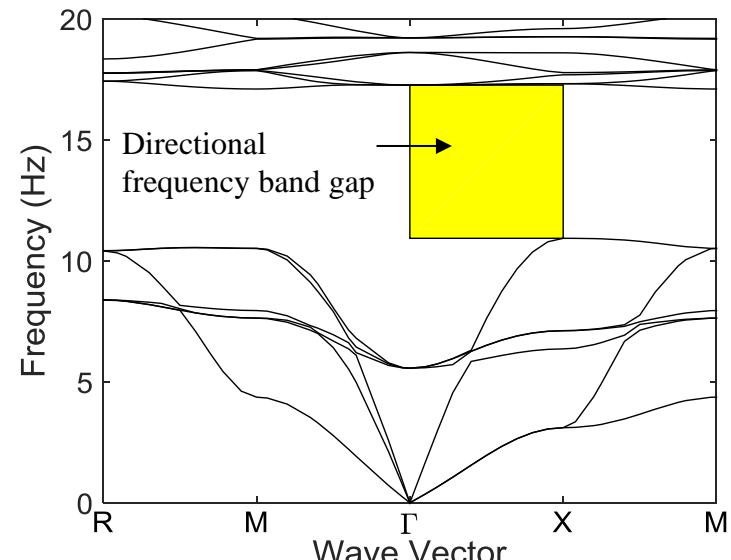
## One unit cell of 3D periodic foundation



## Material properties

Material	Young's Modulus (MPa)	Density (kg/m <sup>3</sup> )	Poisson's Ratio
Reinforced Concrete	31400	2300	0.2
Rubber	3.49	1100	0.463

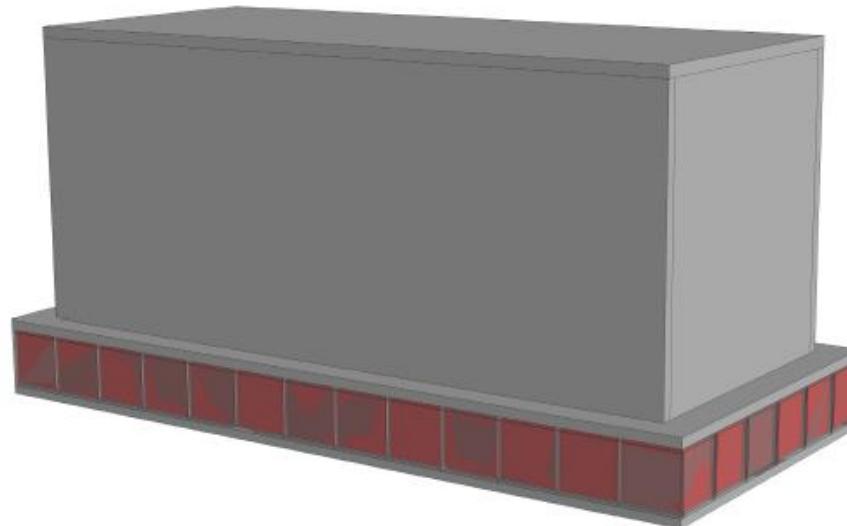
## Dispersion curve for infinite number of unit cells



Starting of 1<sup>st</sup> frequency band gap = 11.18 Hz  
Width of 1<sup>st</sup> frequency band gap = 5.98 Hz

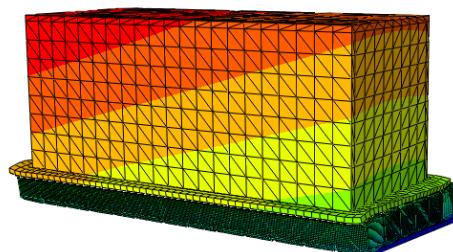
# Design of 3D periodic foundation

Designed 3D periodic foundation supporting SMR building

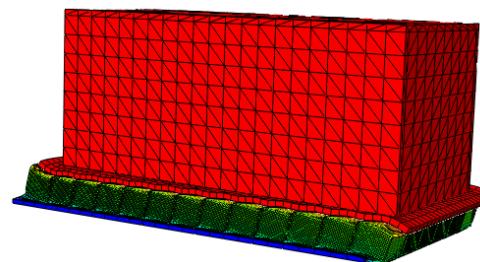


Unit cells on periodic foundation: 13 X 6 X 1

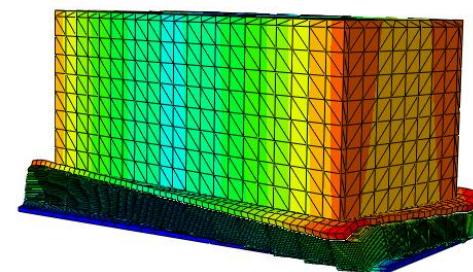
Natural frequencies of structure system



Mode 1 ( $f_n = 0.86$  Hz)



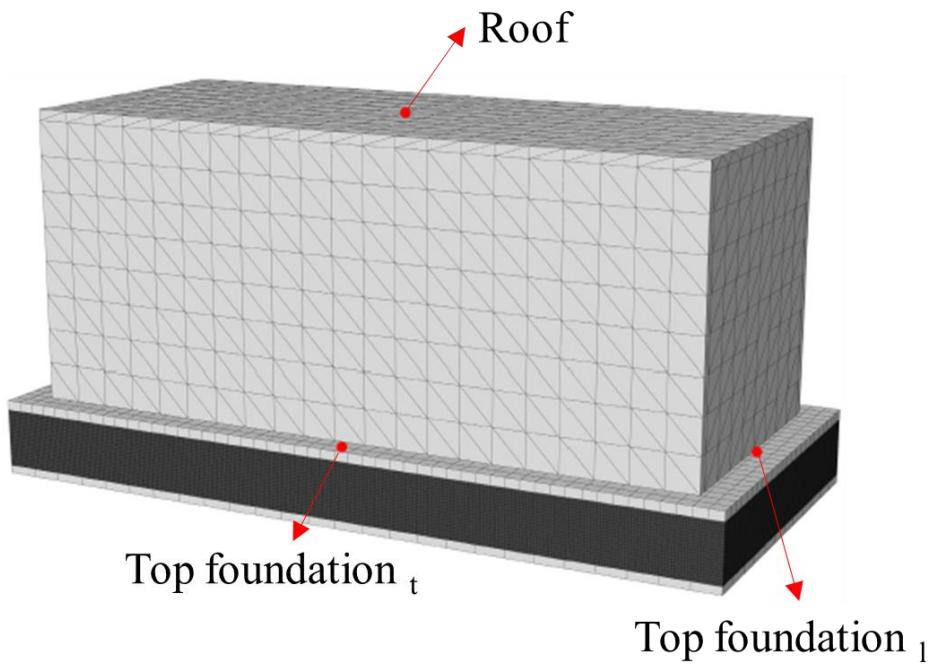
Mode 2 ( $f_n = 0.91$  Hz)



Mode 3 ( $f_n = 0.95$  Hz)

# Design of 3D periodic foundation

## Frequency response function



Damping ratio:

$$\xi_{\text{concrete}} = 4 \%$$

$$\xi_{\text{rubber}} = 10 \%$$

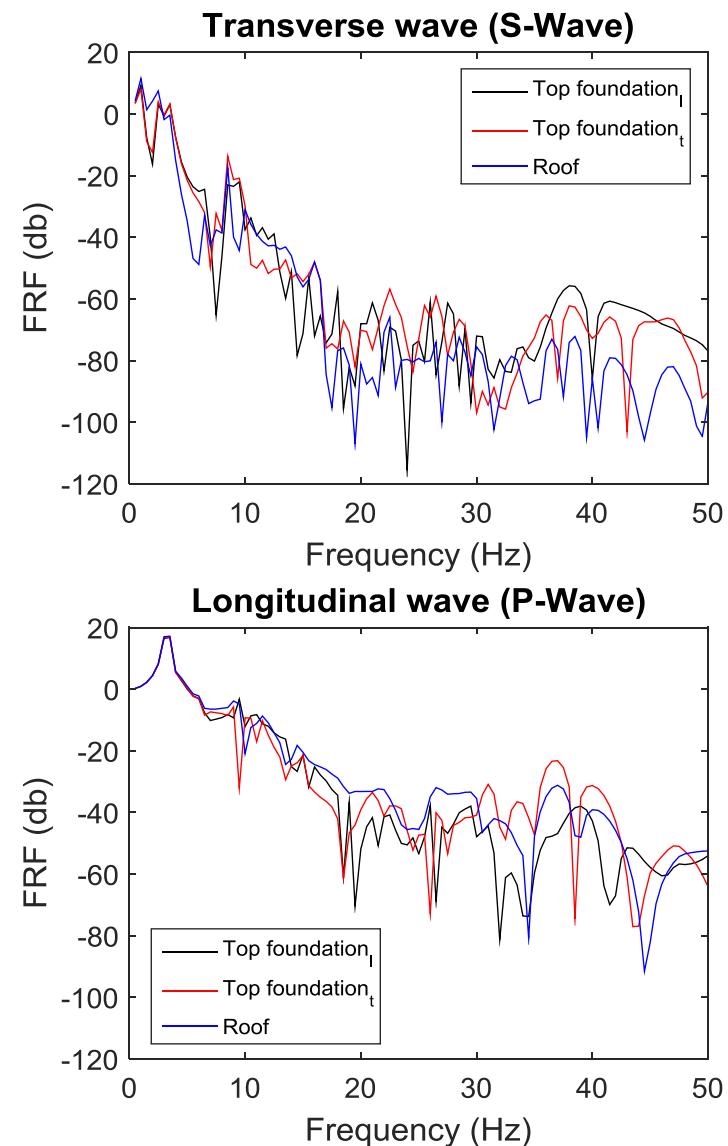
Frequency response:

$$\text{FRF} = 20 \log(\delta_{\text{out}} / \delta_{\text{inp}})$$

where:

$\delta_{\text{out}}$  = amplitude of output disp

$\delta_{\text{inp}}$  = amplitude of input disp



# Similitude requirements for dynamic models

Model type		True ultimate strength model	Models with artificial mass	Gravity forces neglected	Models with strain distortion
Scaling Parameters		(1)	(2)	(3)	(4)
Length	$l_r$	$l_r$	$l_r$	$l_r$	$l_r$
Time	$t_r$	$l_r^{\frac{1}{2}}$	$l_r^{\frac{1}{2}}$	$l_r(E/\rho)_r^{-\frac{1}{2}}$	$(\varepsilon_r l_r)^{\frac{1}{2}}$
Frequency	$\omega_r$	$l_r^{-\frac{1}{2}}$	$l_r^{-\frac{1}{2}}$	$l_r^{-1}(E/\rho)_r^{\frac{1}{2}}$	$(\varepsilon_r l_r)^{-\frac{1}{2}}$
Velocity	$v_r$	$l_r^{\frac{1}{2}}$	$l_r^{\frac{1}{2}}$	$(E/\rho)_r^{\frac{1}{2}}$	$(\varepsilon_r l_r)^{-\frac{1}{2}}$
Gravitational acceleration	$g_r$	1	1	neglected	1
Acceleration	$a_r$	1	1	$l_r^{-1}(E/\rho)_r$	1
Mass density	$\rho_r$	$E_r/l_r$	**	$\rho_r$	$\varepsilon_r E_r l_r^{-1}$
Strain	$\varepsilon_r$	1	1	1	$\varepsilon_r$
Stress	$\sigma_r$	$E_r$	$E_r$	$E_r$	$E_r \varepsilon_r$
Modulus of elasticity	$E_r$	$E_r$	$E_r$	$E_r$	$E_r$
Specific stiffness	$(E/\rho)_r$	$l_r$	**	$(E/\rho)_r$	$l_r \varepsilon_r^{-\frac{1}{2}}$
Displacement	$\delta_r$	$l_r$	$l_r$	$l_r$	$l_r \varepsilon_r$
Force	$F_r$	$E_r l_r^2$	$E_r l_r^2$	$E_r l_r^2$	$E_r l_r^2 \varepsilon_r$
Energy	$(EN)_r$	$E_r l_r^3$	$E_r l_r^3$	$E_r l_r^3$	$E_r l_r^3 \varepsilon_r^2$

To fit the shake table, the length scale is decided to be  $l_r = \frac{l_m}{l_p} = \frac{1}{22}$

m = model  
p = prototype

# Similitude requirements for dynamic models

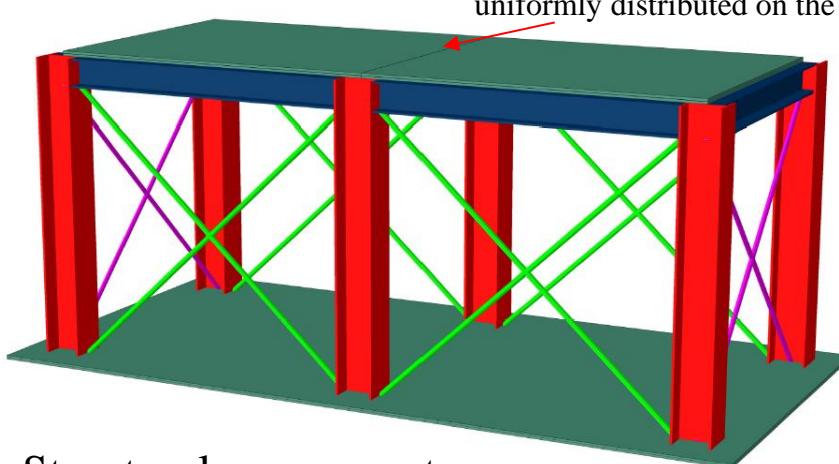
## Essential scaled parameters:

- Frequency band gaps  $\frac{1}{\sqrt{l_r}} = \sqrt{22}$
- Natural frequency of periodic foundation structure system  $\frac{1}{\sqrt{l_r}} = \sqrt{22}$
- Natural frequency of superstructure only  $\frac{1}{\sqrt{l_r}} = \sqrt{22}$
- Duration of earthquake record  $\sqrt{l_r} = \frac{1}{\sqrt{22}}$

Parameter	Prototype	Model	Required scale
Natural frequency of superstructure (Hz)	6.77	31.1	$\sqrt{22} = 4.69$
Natural frequency of 1D periodic foundation structure system (Hz)	0.59	3	$\sqrt{22} = 4.69$
First theoretical S-Wave frequency band gap of 1D periodic foundation with equivalent superstructure layer (Hz)	1.3 – 4.34	6.12 – 20.34	$\sqrt{22} = 4.69$
Natural frequency of 3D periodic foundation structure system (Hz)	0.86	4.06	$\sqrt{22} = 4.69$
First theoretical directional frequency band gap of 3D periodic foundation (Hz)	11.18 – 17.16	50.9 – 75.03	$\sqrt{22} = 4.69$
Sampling time of earthquake record (sec)	0.005	0.001066	$1/\sqrt{22} = 0.213$

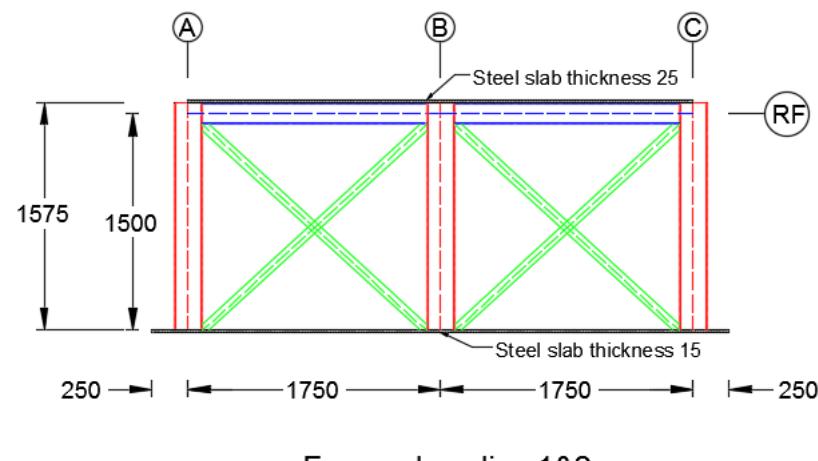
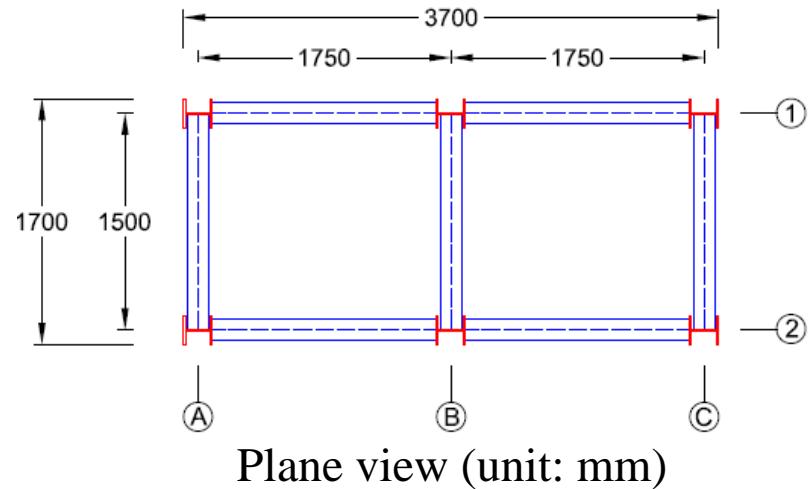
# Scaled SMR Building

Designed small scale SMR building



Structural components:

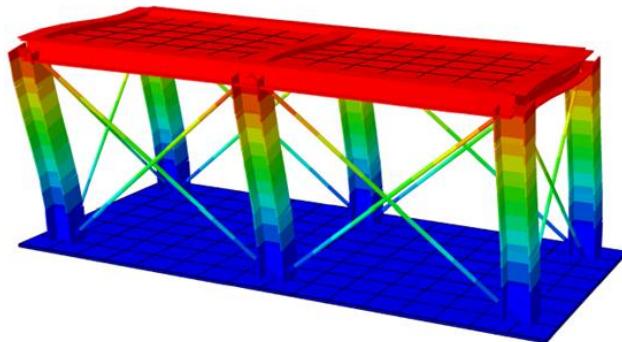
Structural component	Cross-section
Column	200X200X8X12
Beam	150X150X7X10
Longitudinal brace	65X65X6
Transverse brace	50X50X5



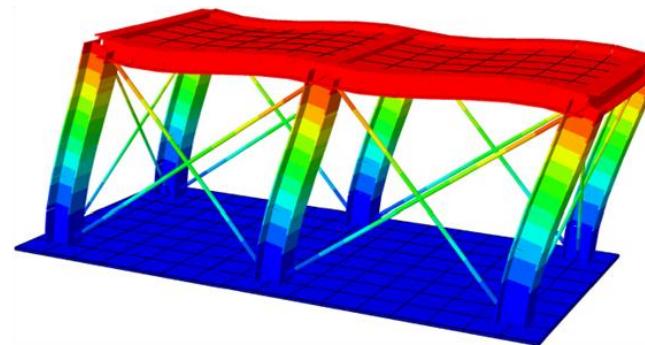
Elevation view (unit: mm)

# Scaled SMR Building

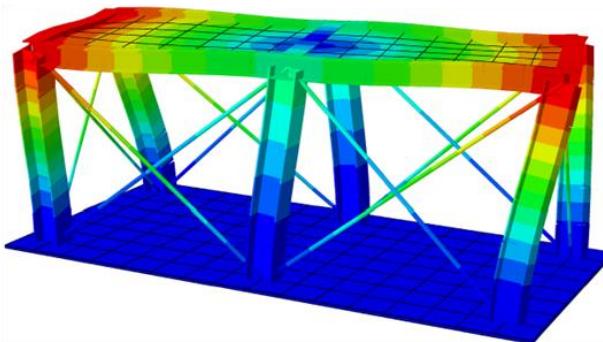
Mode shapes and natural frequencies of structure system



Mode 1 ( $f_n = 31.1 \text{ Hz}$ )



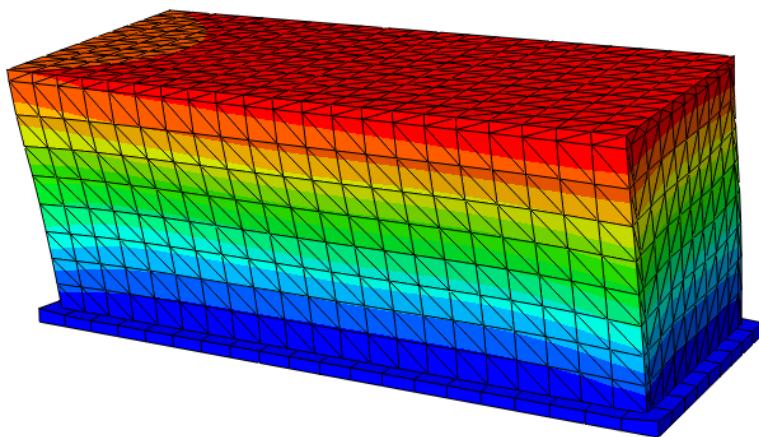
Mode 2 ( $f_n = 53.55 \text{ Hz}$ )



Mode 3 ( $f_n = 56.73 \text{ Hz}$ )

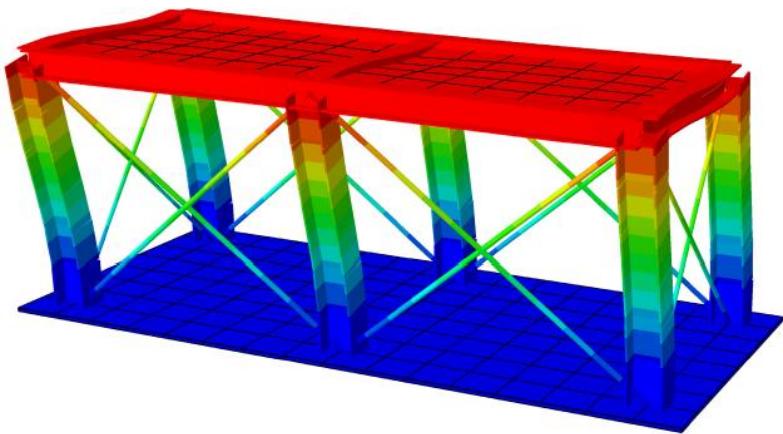
# SMR building

Full-scale SMR building



Mode 1 ( $f_{np} = 6.77 \text{ Hz}$ )

Scaled model of SMR building



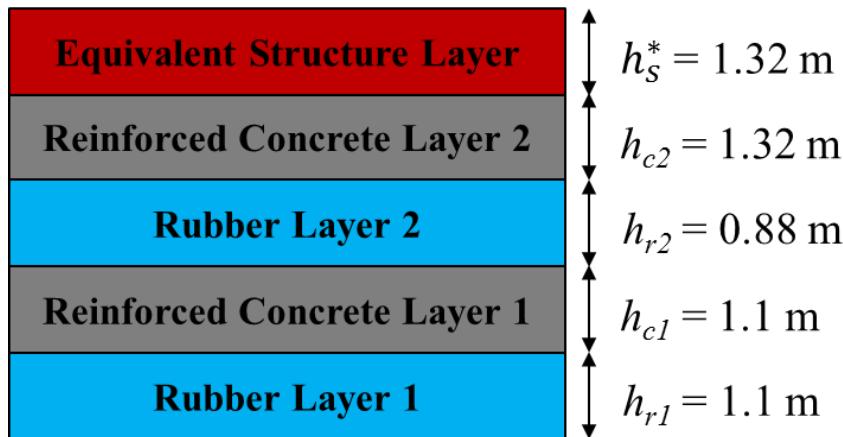
Mode 1 ( $f_{nm} = 31.1 \text{ Hz}$ )

$$\begin{aligned} f_{nm} &= f_{np} \sqrt{22} \\ &= 6.77 \times 4.69 = 31.75 \text{ Hz} \approx 31.1 \text{ Hz} \end{aligned}$$

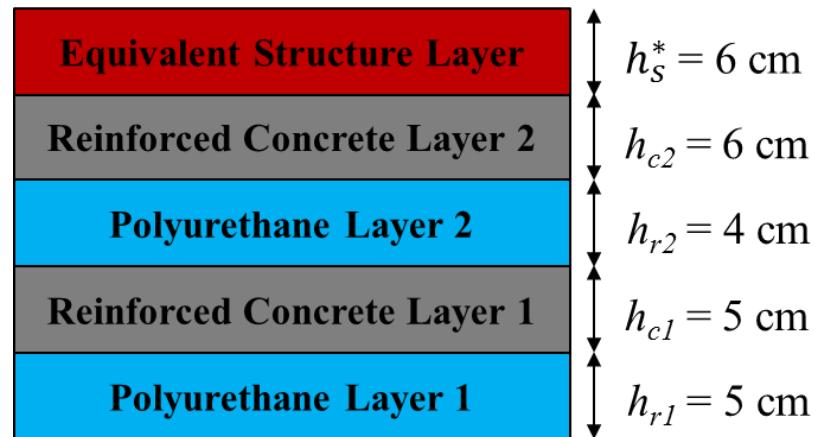
# Scaled 1D periodic foundation



## Full-scale 1D periodic foundation unit cell



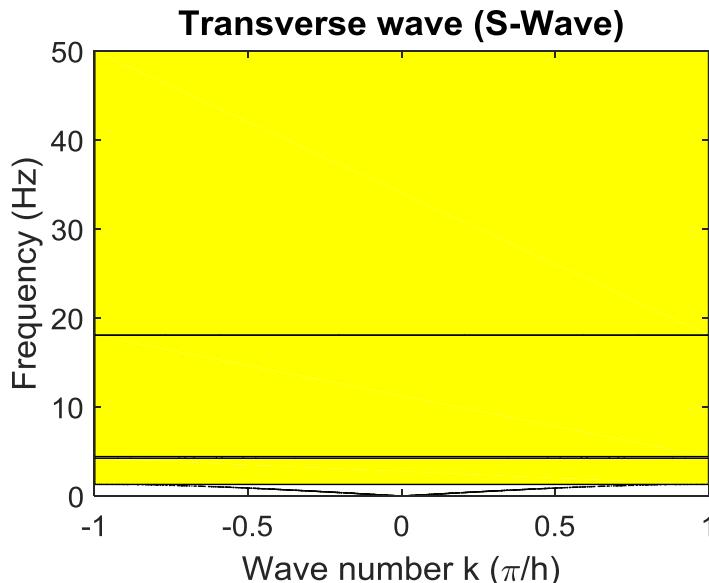
## Scaled model of 1D periodic foundation unit cell



Material	Material for Prototype			Material for Model		
	Young's Modulus (MPa)	Density (kg/m <sup>3</sup> )	Poisson's Ratio	Young's Modulus (MPa)	Density (kg/m <sup>3</sup> )	Poisson's Ratio
Reinforced Concrete	31400	2300	0.2	31400	2300	0.2
Rubber	3.49	1100	0.463	0.1586	1100	0.463
Equivalent superstructure layer	31400	24247.2	0.2	31400	24247.2	0.2

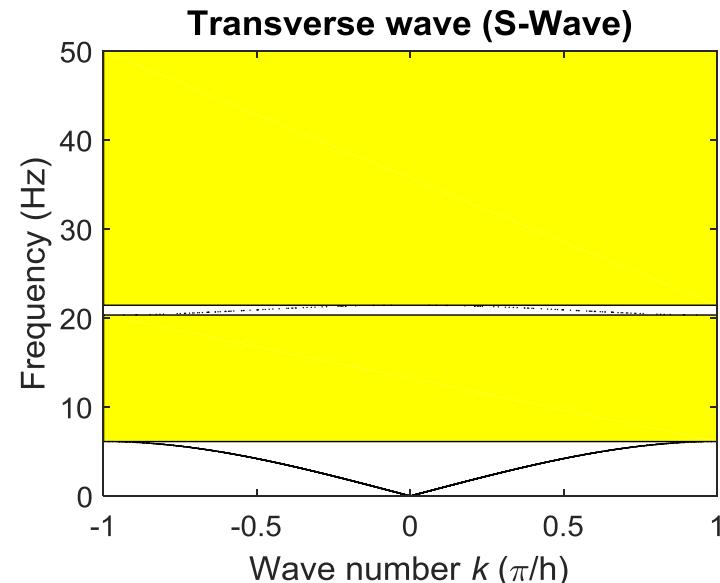
# Scaled 1D periodic foundation

Theoretical frequency band gap of full-scale unit cell



$1^{\text{st}}$  Frequency band gap = 1.3–4.34 Hz

Theoretical frequency band gap of scaled model unit cell



$1^{\text{st}}$  Frequency band gap = 6.12–20.34 Hz

**Frequency scale:**

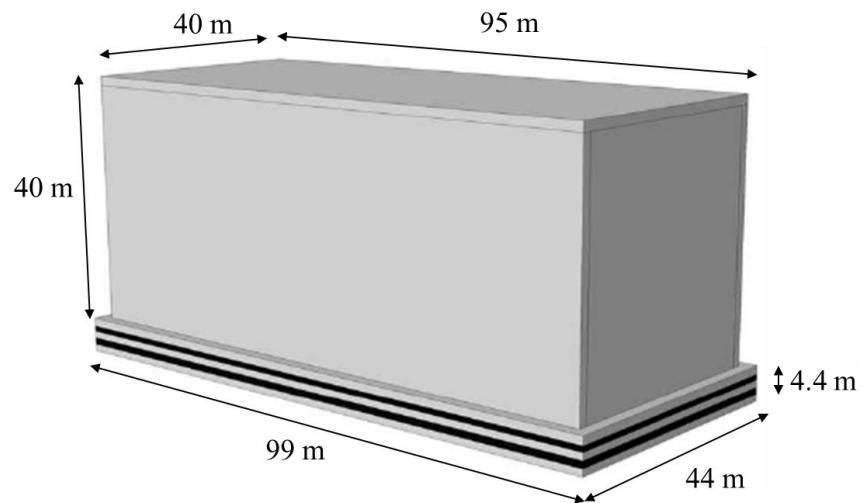
$$\text{Starting of band gap scale: } \frac{f_m}{f_p} = \frac{6.12}{1.3} = 4.7 \quad \text{close to } \frac{1}{\sqrt{l_r}} = \sqrt{22} = 4.69$$

$$\text{End of band gap scale: } \frac{f_m}{f_p} = \frac{20.34}{4.34} = 4.69 \quad \text{same as } \frac{1}{\sqrt{l_r}} = \sqrt{22} = 4.69$$

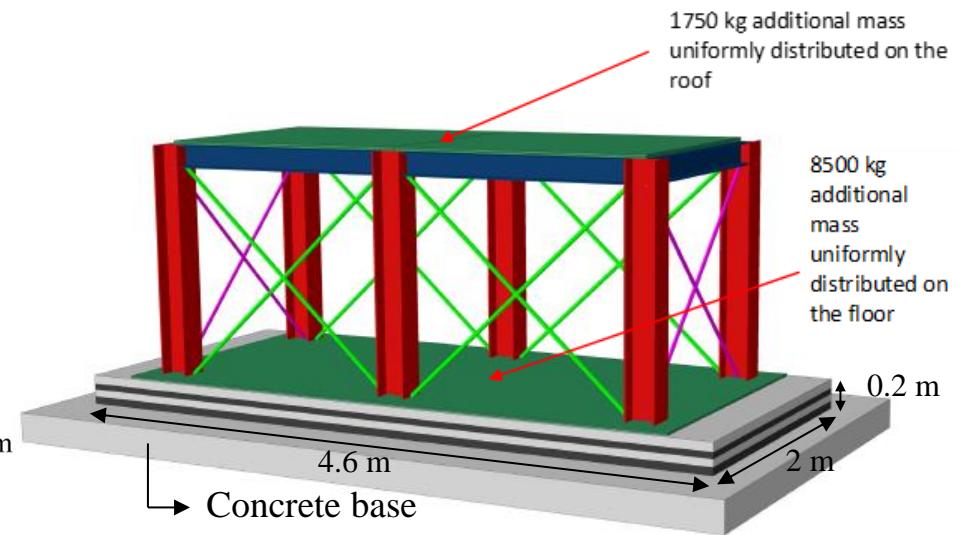
# Scaled 1D periodic foundation



Prototype design



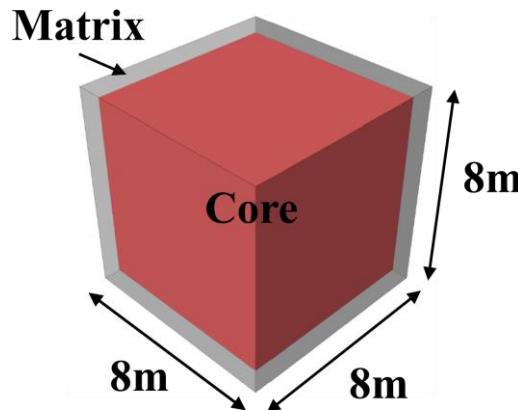
Model design



# Scaled 3D periodic foundation

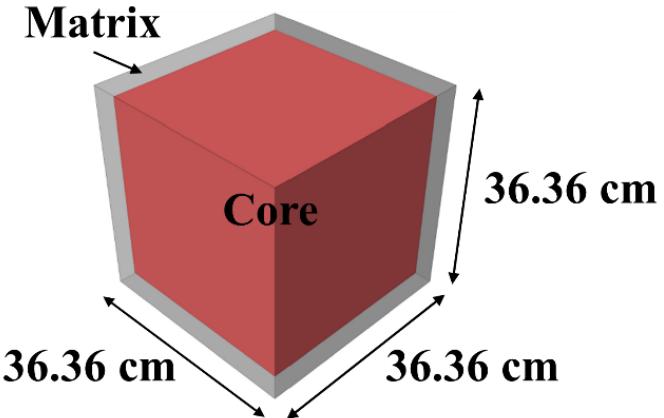


Full-scale 3D periodic foundation unit cell



Unit cell size = 8 m; Core size = 7.2 m;

Scaled model of 3D periodic foundation unit cell



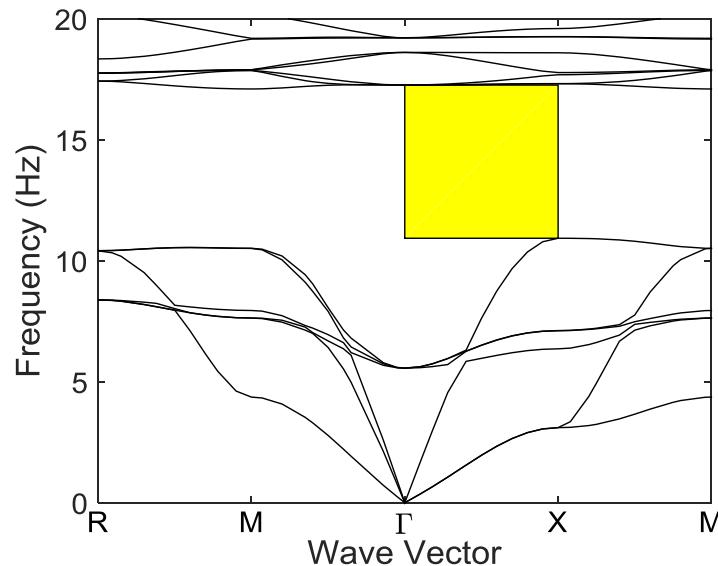
Unit cell size = 36.36 cm; Core size = 32.5 cm;

Material	Material for Prototype			Material for Model		
	Young's Modulus (MPa)	Density (kg/m <sup>3</sup> )	Poisson's Ratio	Young's Modulus (Pa)	Density (kg/m <sup>3</sup> )	Poisson's Ratio
Concrete	31400	2300	0.2	31400	2300	0.2
Rubber	3.49	1100	0.463	0.1586	1100	0.463

# Scaled 3D periodic foundation

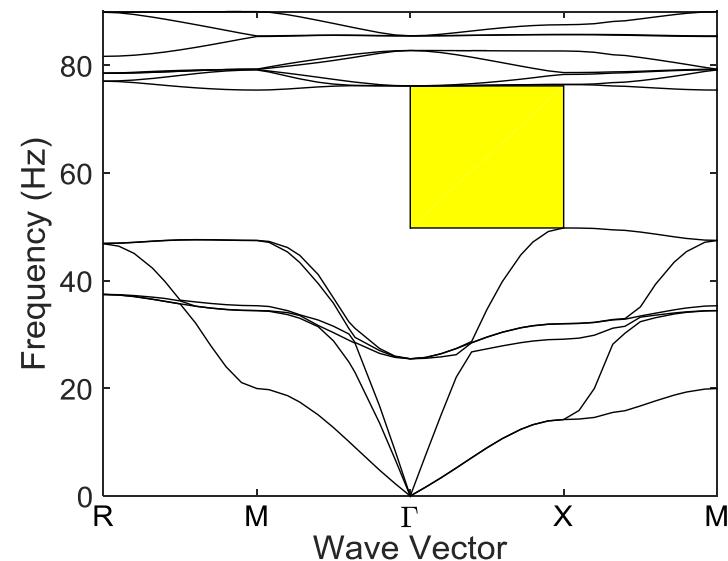


Theoretical frequency band gap of full-scale unit cell



Frequency band gap = 11.18–17.16 Hz

Theoretical frequency band gap of scaled model unit cell



Frequency band gap = 50.9–75.03 Hz

**Frequency scale:**

$$\text{Starting of band gap scale: } \frac{f_m}{f_p} = \frac{50.9}{11.18} = 4.55 \quad \text{Close to } \sqrt{22} = 4.69$$

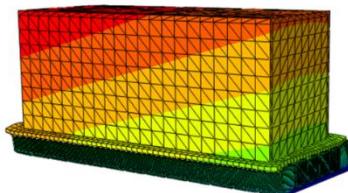
$$\text{End of band gap scale: } \frac{f_m}{f_p} = \frac{75.03}{17.16} = 4.37 \quad \text{Close to } \sqrt{22} = 4.69$$

# Modal Comparison

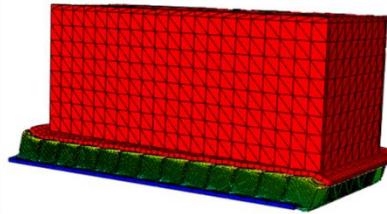


## Mode shapes and natural frequencies of structure system

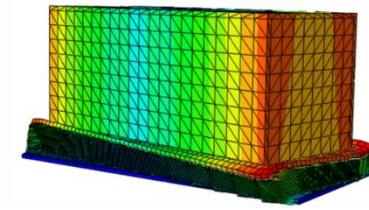
Full-scale structure system



Mode 1 ( $f_n = 0.86 \text{ Hz}$ )

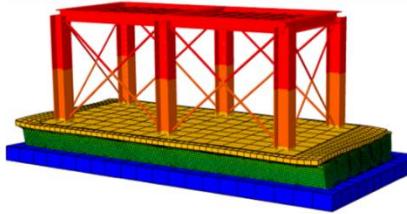


Mode 2 ( $f_n = 0.91 \text{ Hz}$ )

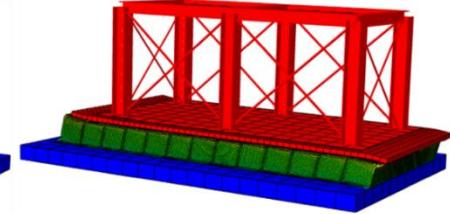


Mode 3 ( $f_n = 0.95 \text{ Hz}$ )

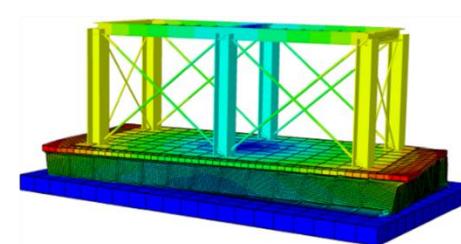
Scaled model of structure system



Mode 1 ( $f_n = 4.06 \text{ Hz}$ )



Mode 2 ( $f_n = 4.17 \text{ Hz}$ )



Mode 3 ( $f_n = 4.76 \text{ Hz}$ )

### Frequency scale:

$$\text{Mode 1 scale} \quad \frac{4.06}{0.86} = 4.72$$

$$\text{Mode 2 scale} \quad \frac{4.17}{0.91} = 4.58$$

$$\text{Mode 3 scale} \quad \frac{4.76}{0.95} = 5.01$$

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# Task 4: Experimental Study of Periodic Foundations

# Fabrication of test specimen

Construction of superstructure



Casting of concrete layers



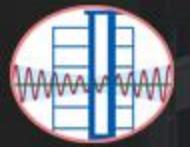
Resin solution and polyurethane glue



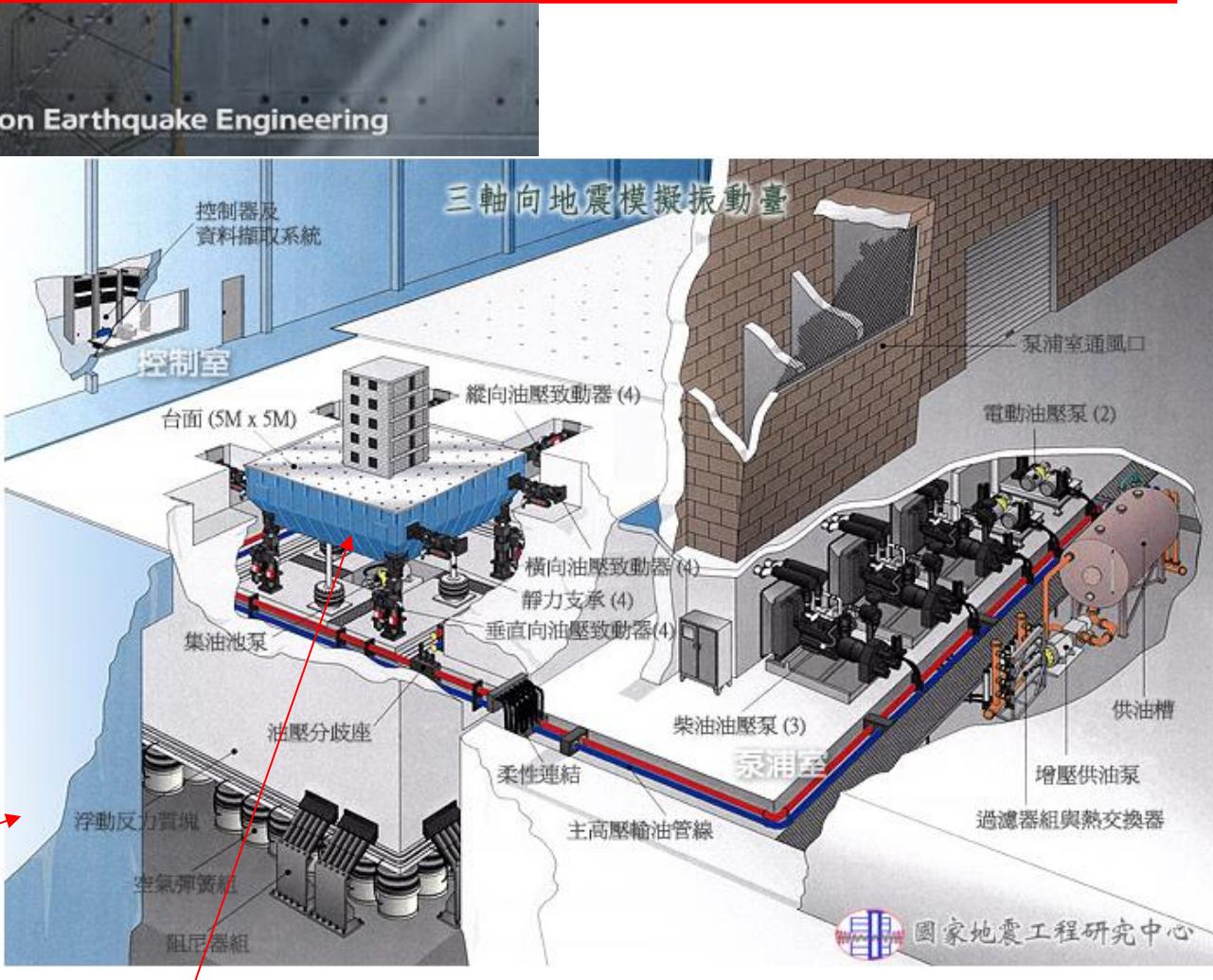
Construction of 1D periodic foundation



# Test Facility



**NARLabs 國家實驗研究院  
國家地震工程研究中心**  
National Center for Research on Earthquake Engineering



12 actuators to simulate motions in 6 degrees of freedom

# Experimental test of 1D periodic foundation

Case 1



Case 2



Case 3



Case 4

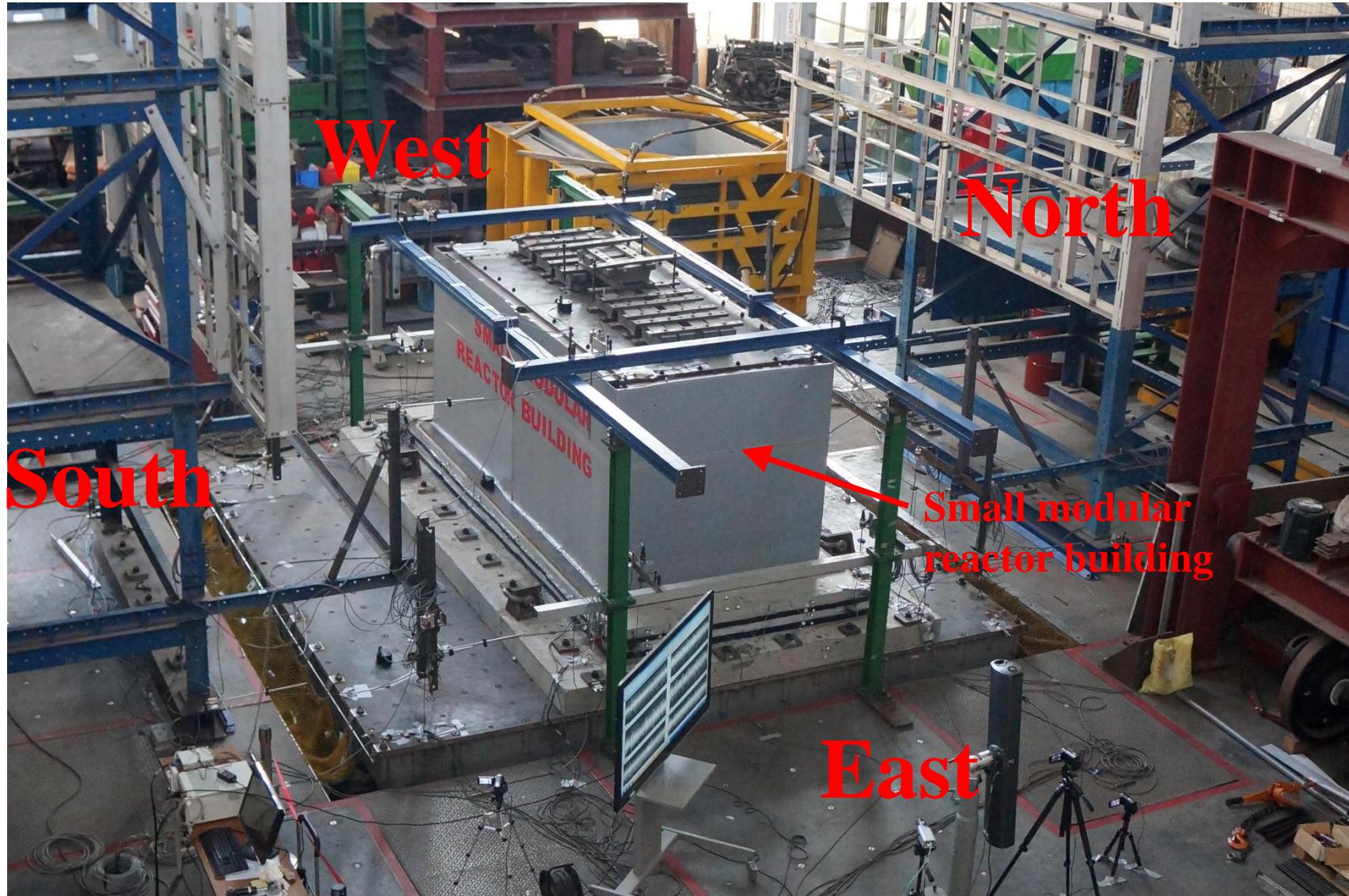


# Experimental test of 1D periodic foundation

## Test items:

- For Cases 1 and 3 (foundations tests)
  - Scanning frequency tests in 3 directions (horizontal, vertical, and torsional)
- For Cases 2 and 4 (structure systems tests)
  - White noise tests in 3 directions (horizontal, vertical, and torsional)
  - Scanning frequency tests in 3 directions (horizontal, vertical, and torsional)
  - Seismic tests in 3 directions (horizontal, vertical, and torsional)
  - Harmonic tests in 3 directions (horizontal, vertical, and torsional)

# Test setup

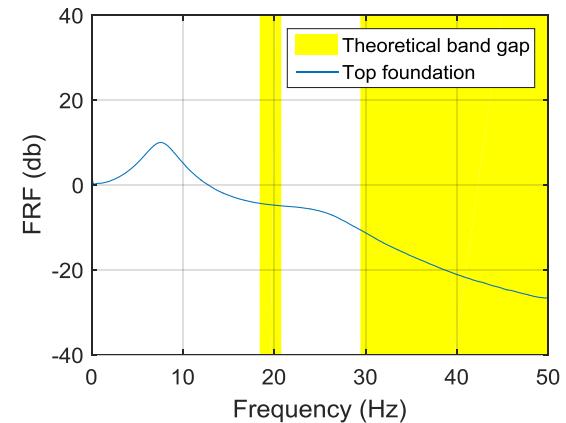
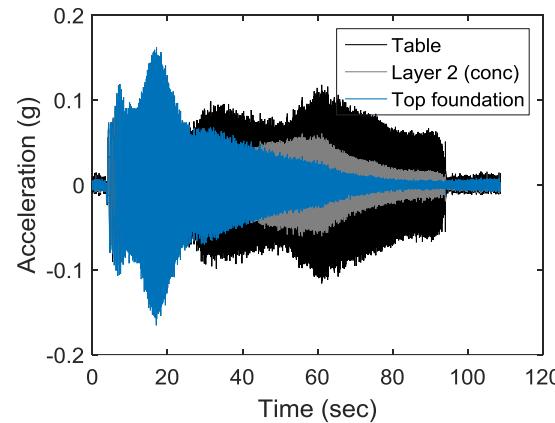


# Scanning frequency tests for foundations (Cases 1 and 3)

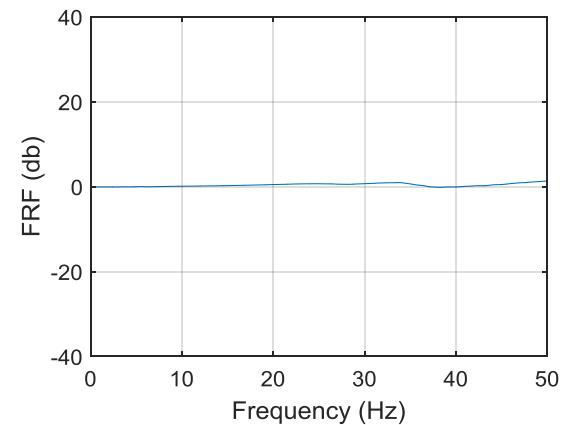
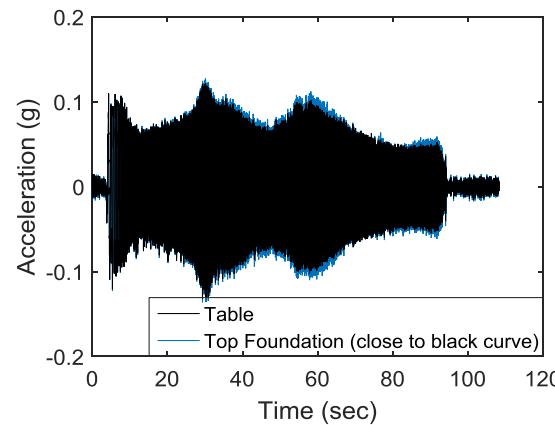


## Horizontal direction

Case 1



Case 3



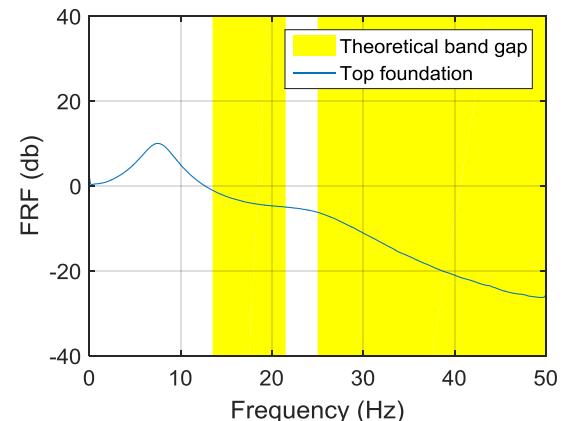
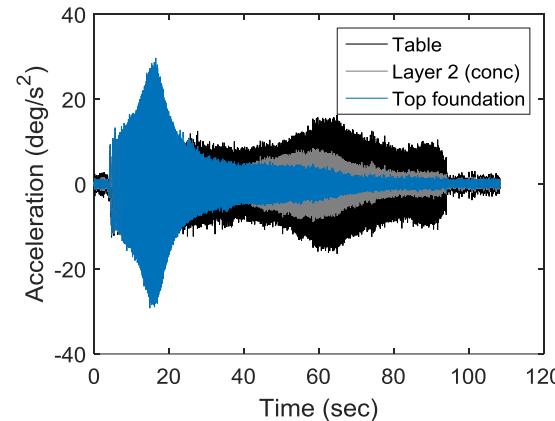
- Attenuation zone in horizontal direction for Case 1 is found at 12.74 – 50 Hz, which is close to theoretical frequency band gaps.
- In Case 3, the response at the top of concrete foundation is the same as input, which implies the foundation is rigidly attached to the shake table.

# Scanning frequency tests for foundations (Cases 1 and 3)

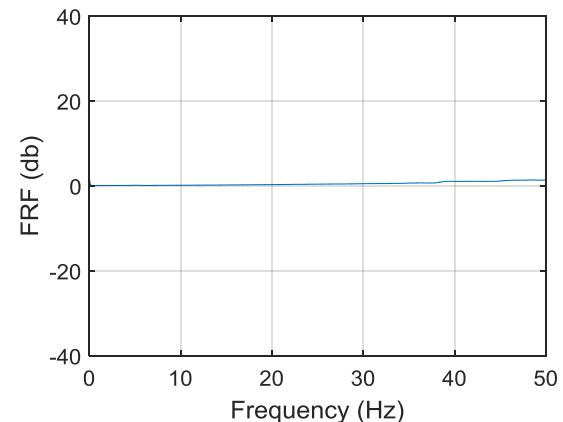
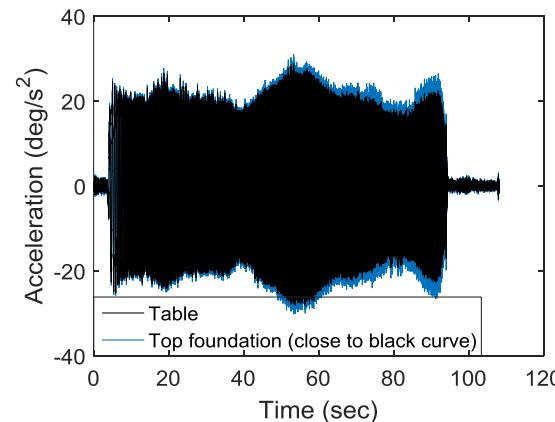


## Torsional mode

### Case 1



### Case 3

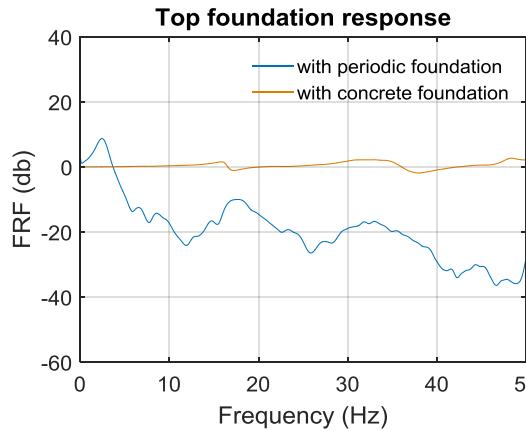


- Attenuation zone in torsional mode for Case 1 is found at 12.7 – 50 Hz, which is close to theoretical frequency band gaps.
- In Case 3, the response at the top of concrete foundation is the same as input, which implies the foundation is rigidly attached to the shake table.

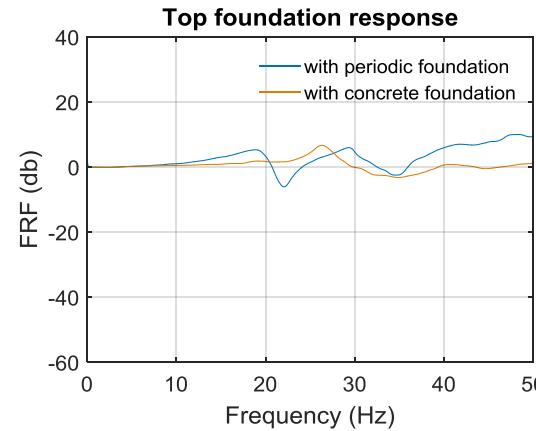
# Scanning frequency tests for structure systems (Cases 2 and 4)



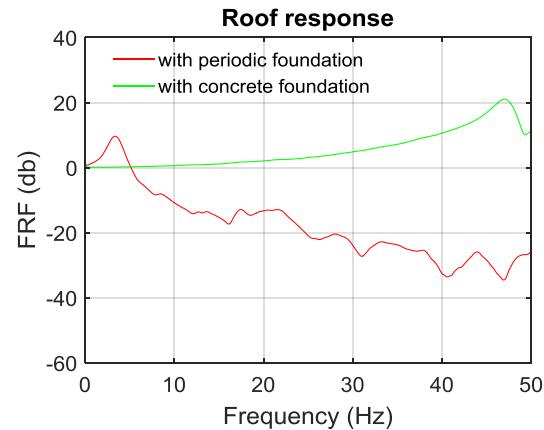
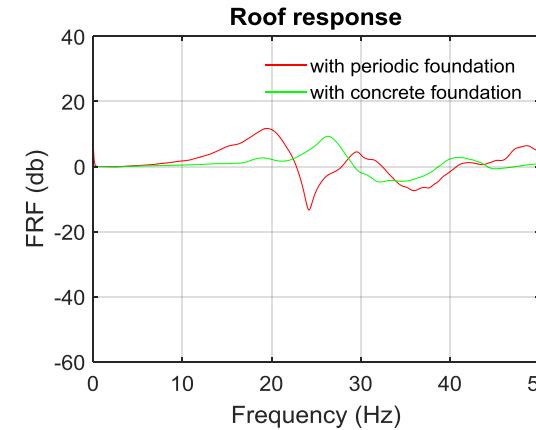
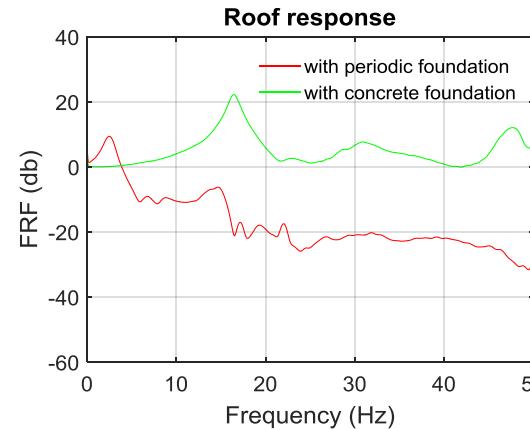
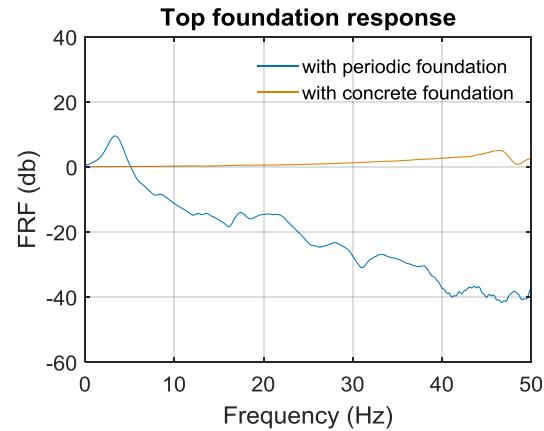
## Horizontal direction



## Vertical direction



## Torsional mode

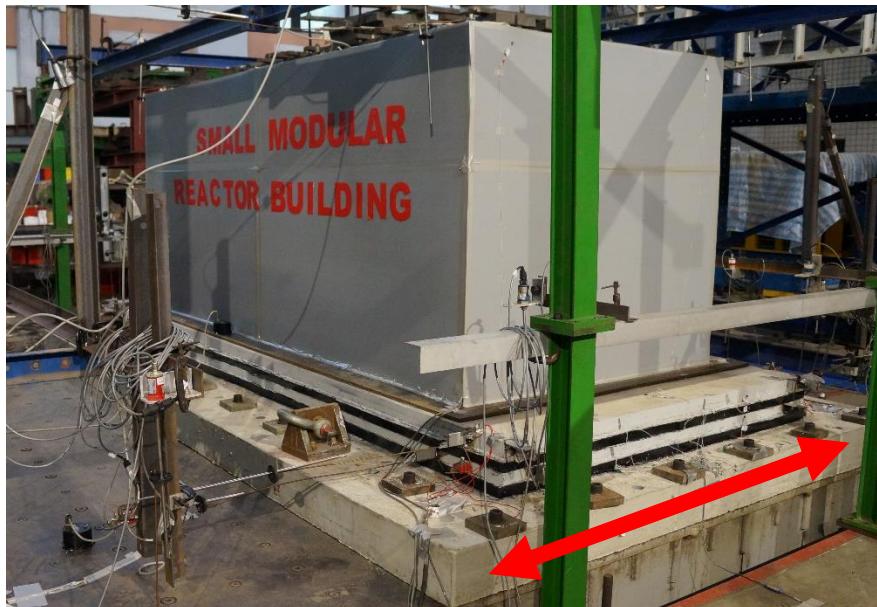


- In Case 2, large response reductions are observed in all three directions. Where FRF of -10 and -20 respectively correspond to 68.38% and 90% response reduction.
- In Case 4, the responses at the roof are mostly amplified in all three directions.

# White noise tests for structure systems (Cases 2 and 4)

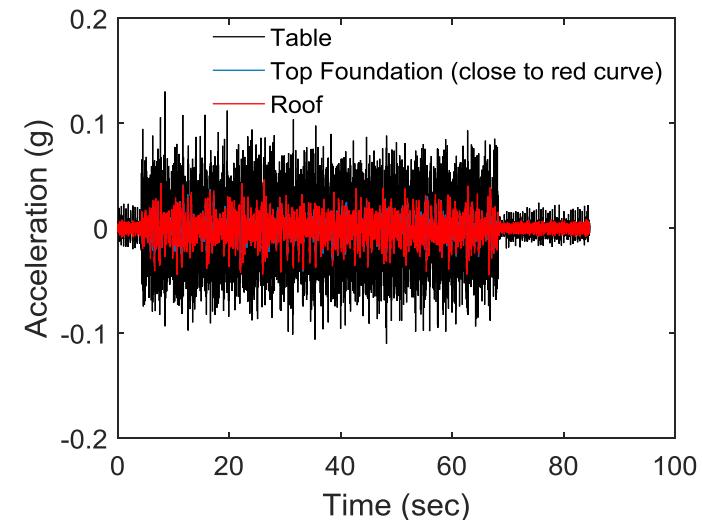


Case 2 white noise test in **horizontal** direction

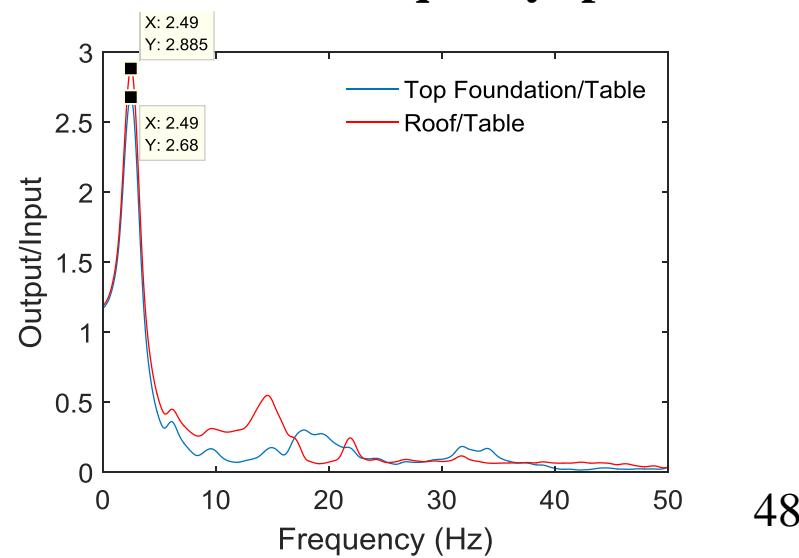


Natural frequency of structure system  
is 2.5 Hz (close to analytical result)

**Acceleration response**



**Normalized Frequency spectra**



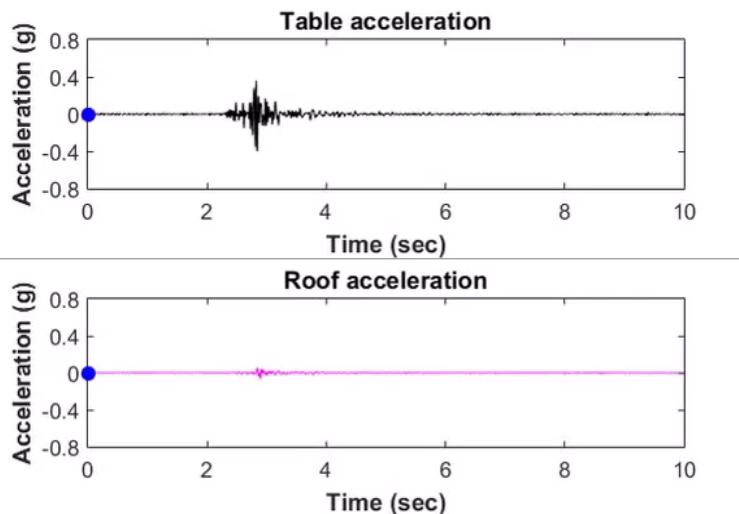
# Seismic tests for structure systems in horizontal direction (Cases 2 and 4)

Earthquake event: **Gilroy** (05/14/2002)

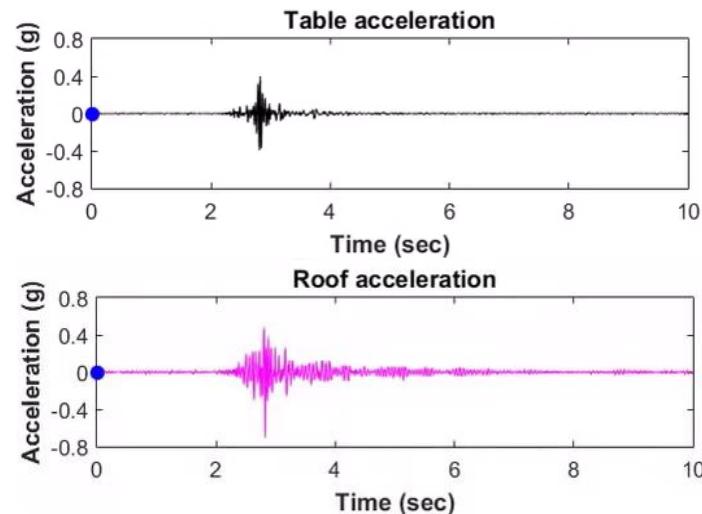
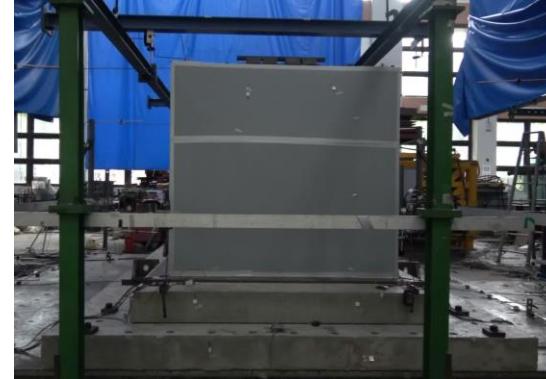
Station: Gilroy array #3

Orientation:  $58^{\circ}$

**Case 2 (with periodic foundation)**



**Case 4 (with concrete foundation)**



# Seismic tests for structure systems in horizontal direction (Cases 2 and 4)

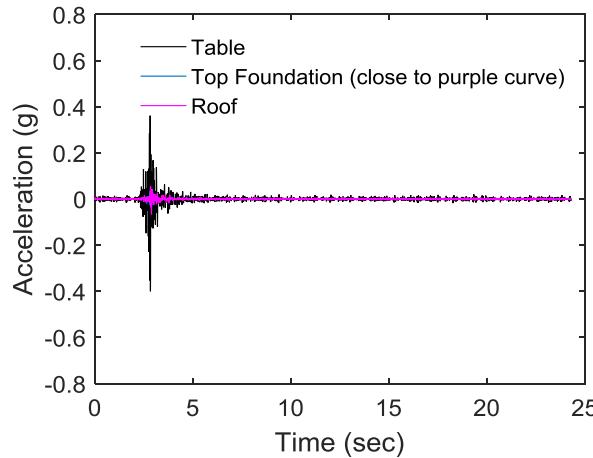


Main frequency content of earthquake is **inside** frequency band gaps of periodic foundation

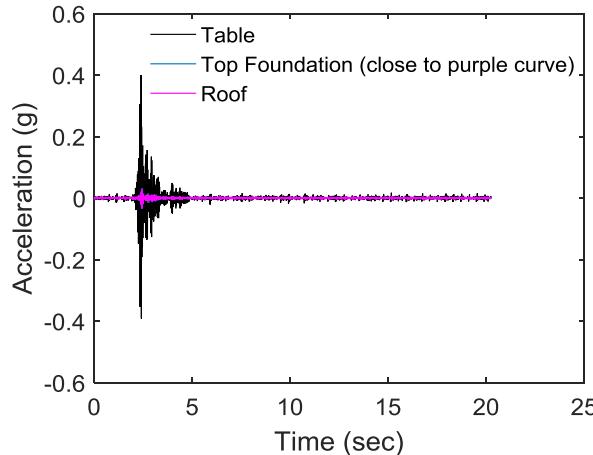
Earthquake

Gilroy

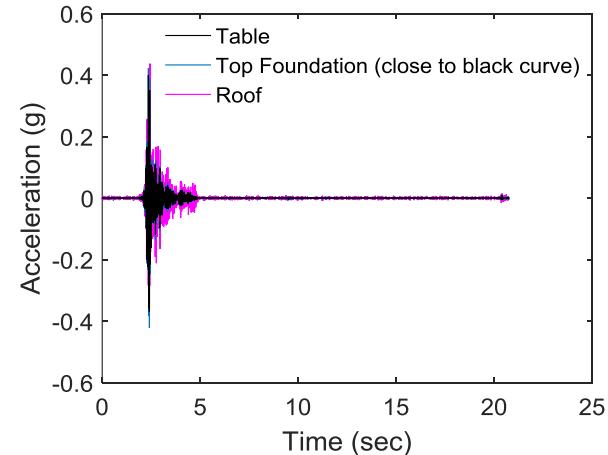
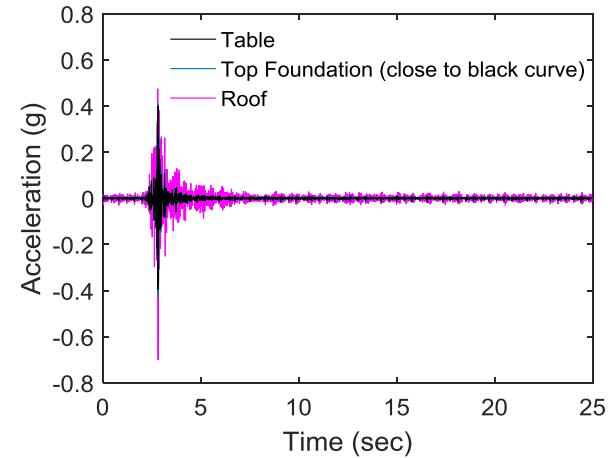
With periodic foundation



Oroville



With concrete foundation



# Seismic tests for structure systems in horizontal direction (Cases 2 and 4)

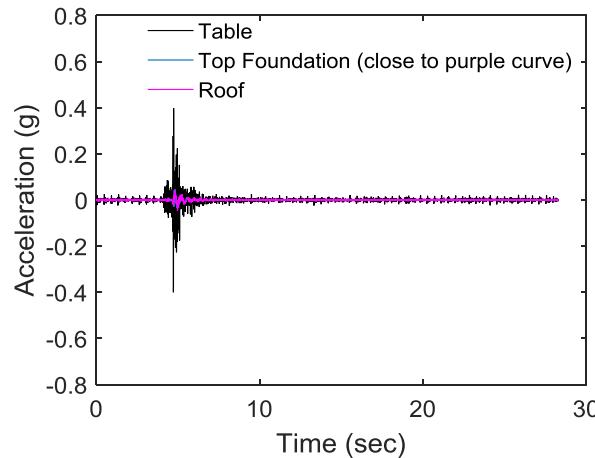


Main frequency content of earthquake is **inside** frequency band gaps of periodic foundation

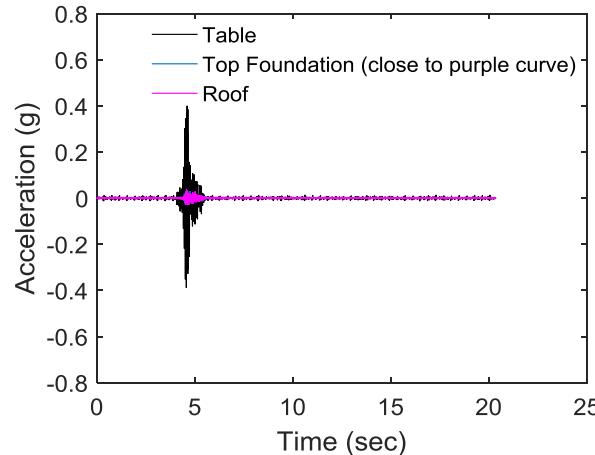
Earthquake

Anza

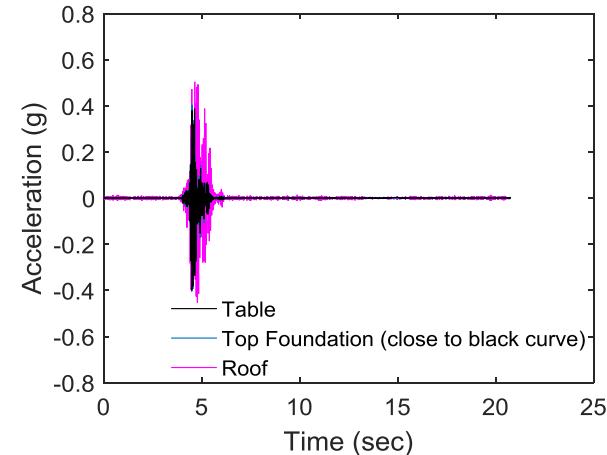
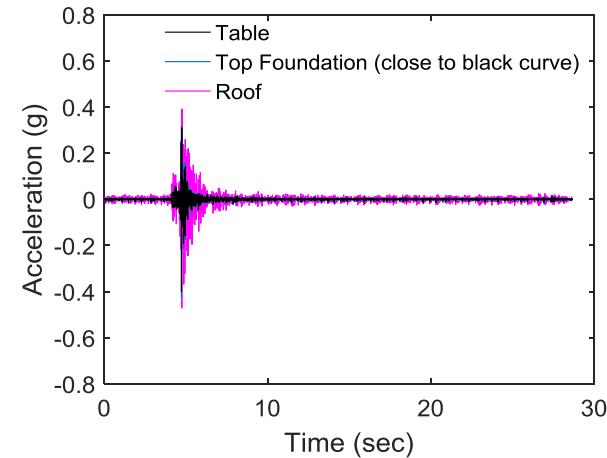
With periodic foundation



Bishop



With concrete foundation



# Seismic tests for structure systems in horizontal direction (Cases 2 and 4)

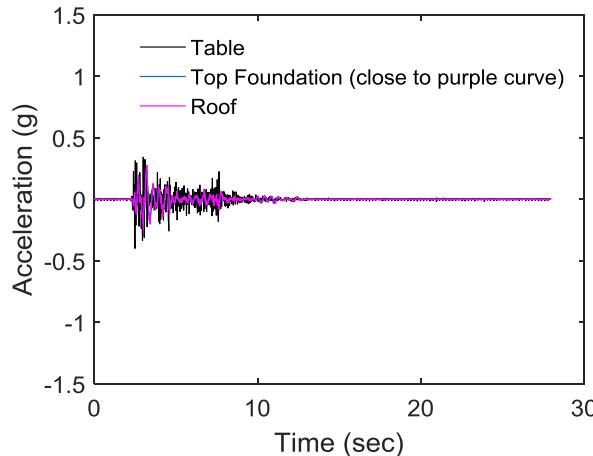


Main frequency content of earthquake is widely distributed

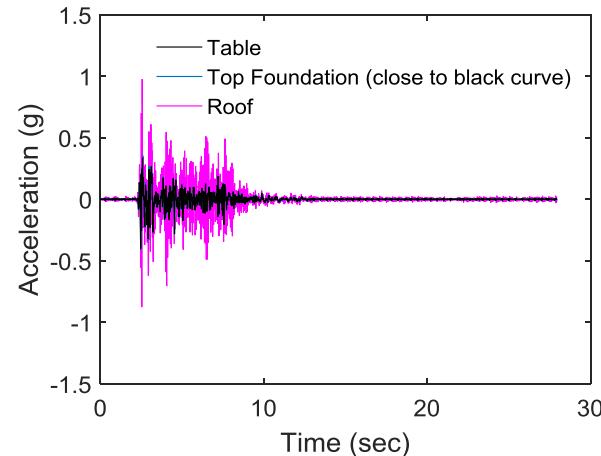
Earthquake

El Centro

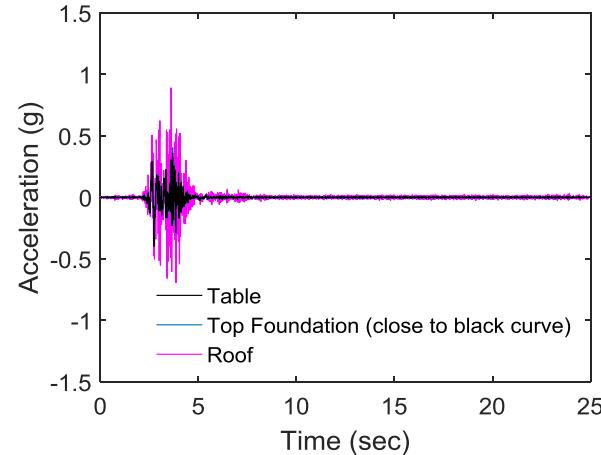
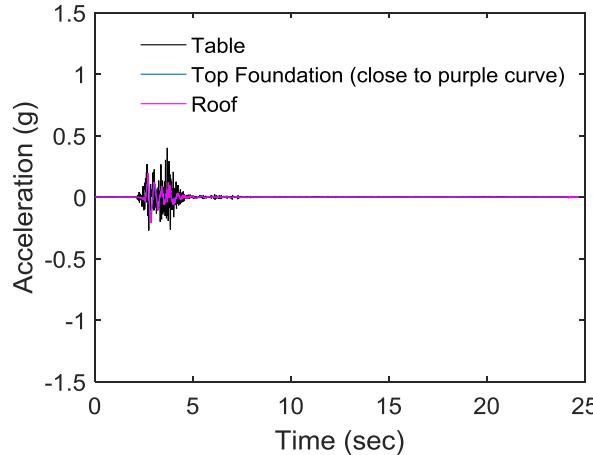
With periodic foundation



With concrete foundation



San Fernando



# Seismic tests for structure systems in horizontal direction (Cases 2 and 4)

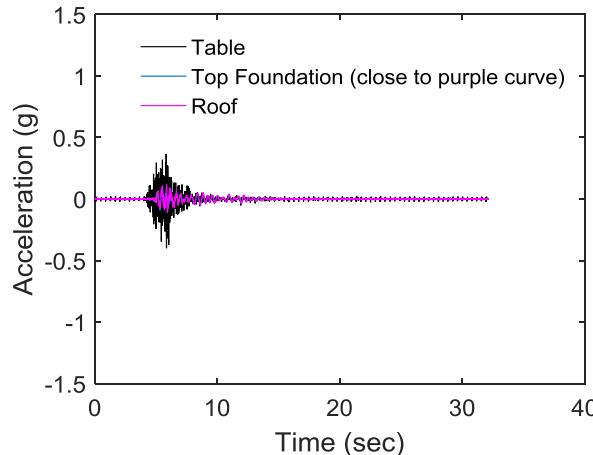


Main frequency content of earthquake is widely distributed

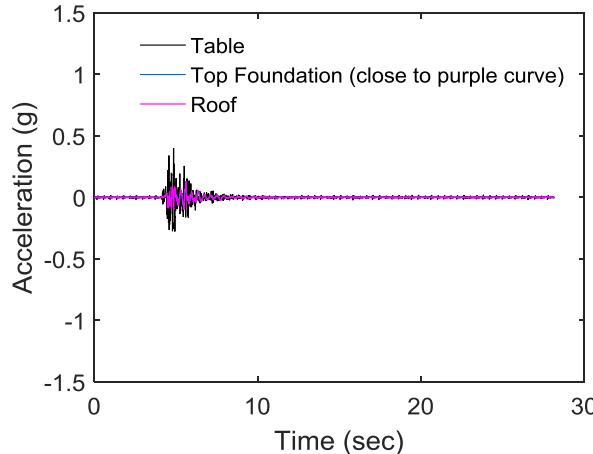
Earthquake

Northridge

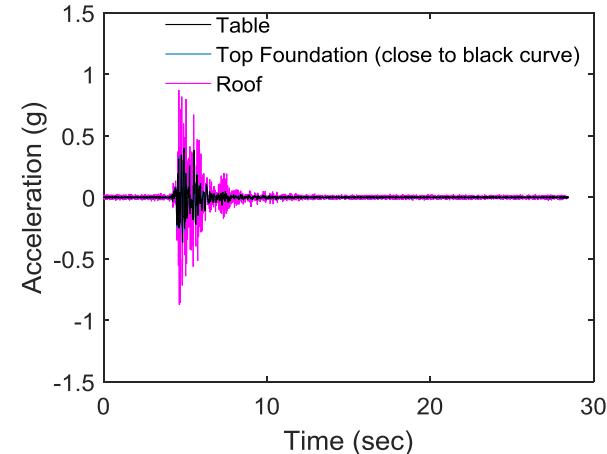
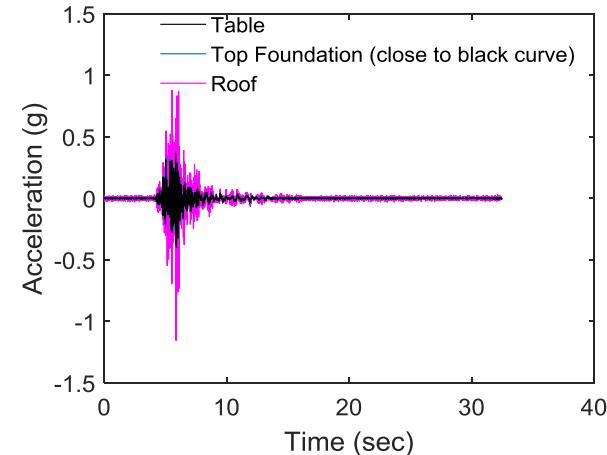
With periodic foundation



Loma Prieta



With concrete foundation



# Harmonic tests for structure systems in vertical direction (Cases 2 and 4)

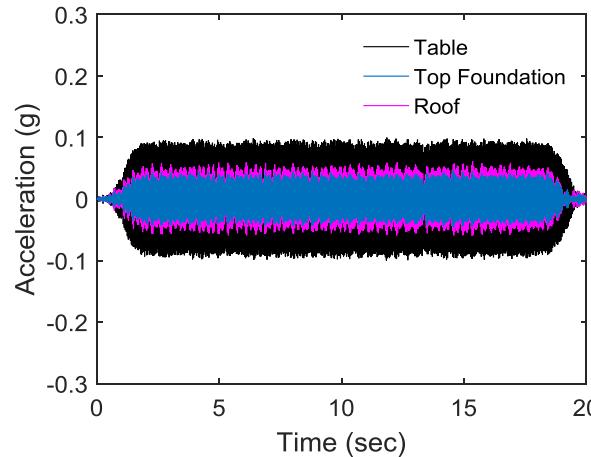


Frequency is inside frequency band gaps of periodic foundation

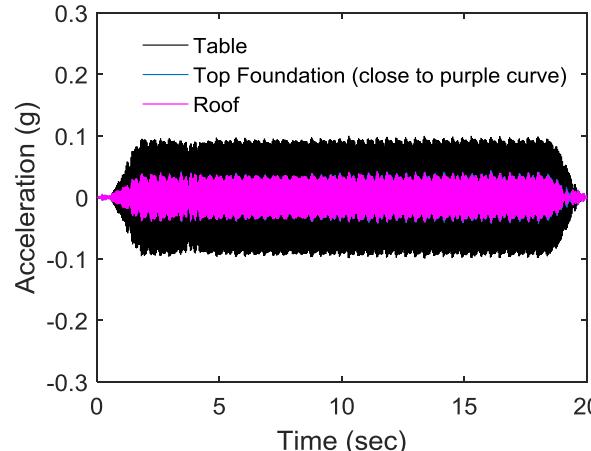
Wave

Sine 24 Hz

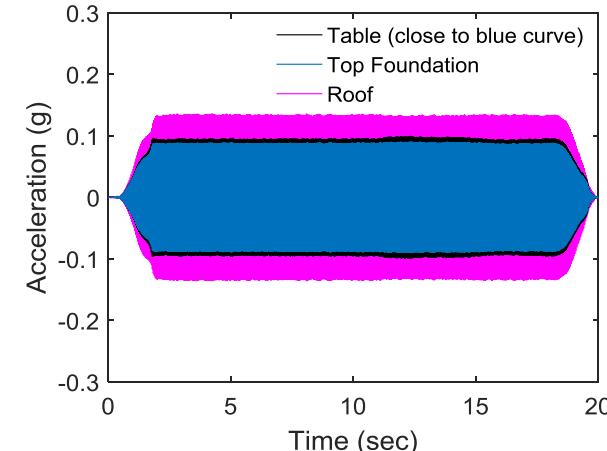
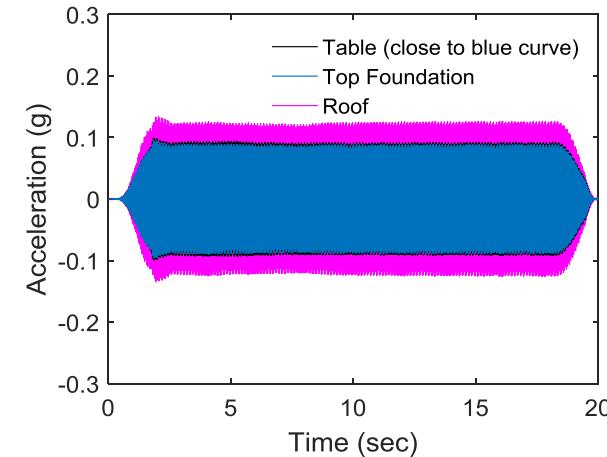
With periodic foundation



Sine 25 Hz



With concrete foundation



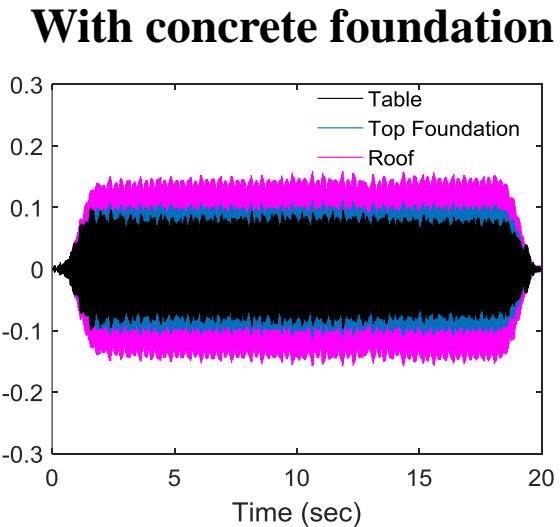
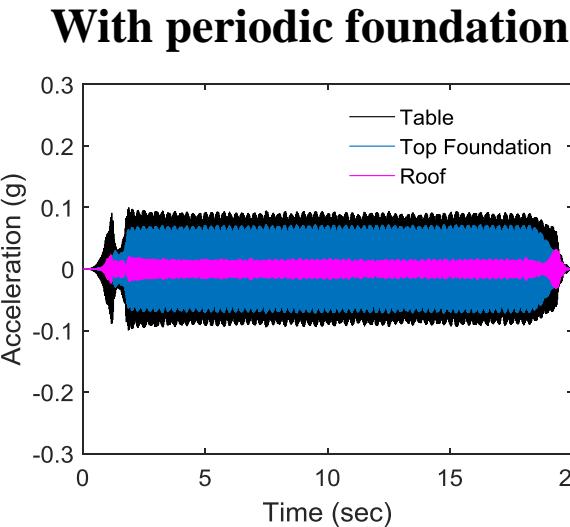
# Harmonic tests for structure systems in vertical direction (Cases 2 and 4)



Frequency is inside frequency band gaps of periodic foundation

Wave

Sine 34 Hz



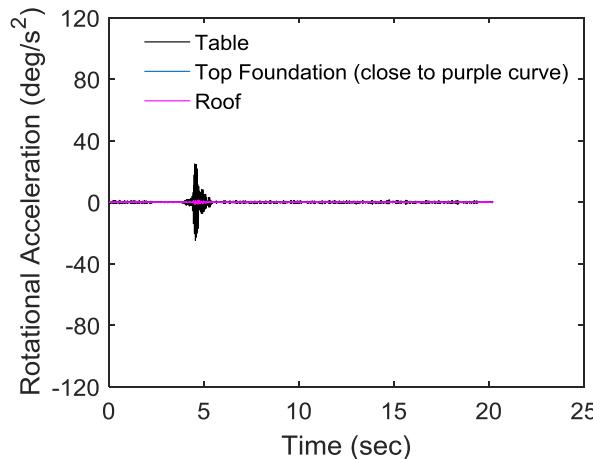
# Seismic tests for structure systems in torsional mode (Cases 2 and 4)



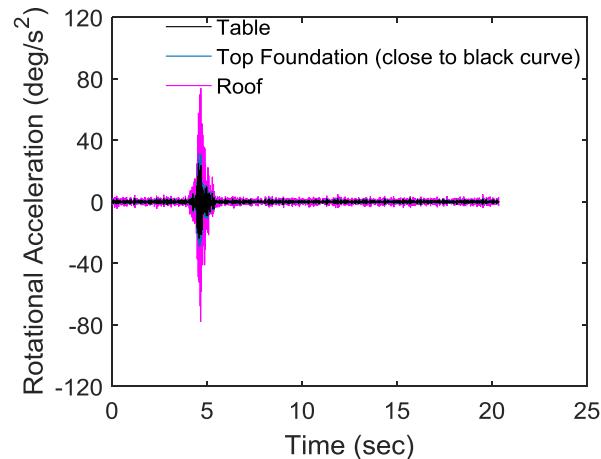
## Earthquake Bishop

(Main frequency content of earthquake is inside frequency band gaps of periodic foundation)

### With periodic foundation

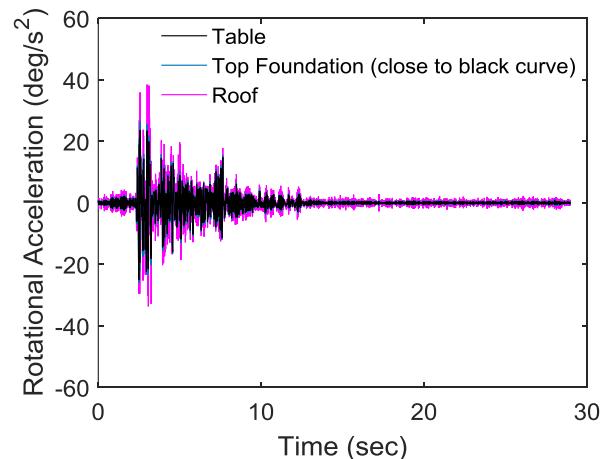
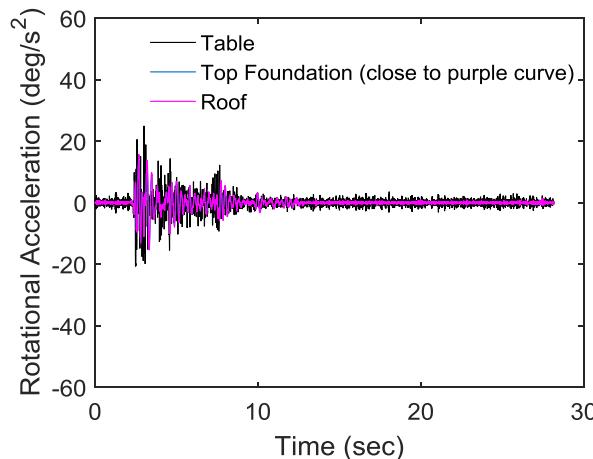


### With concrete foundation



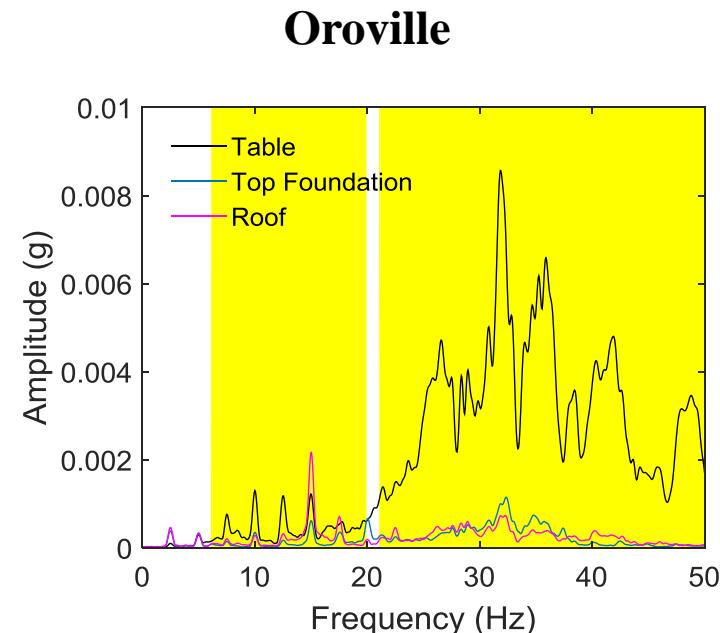
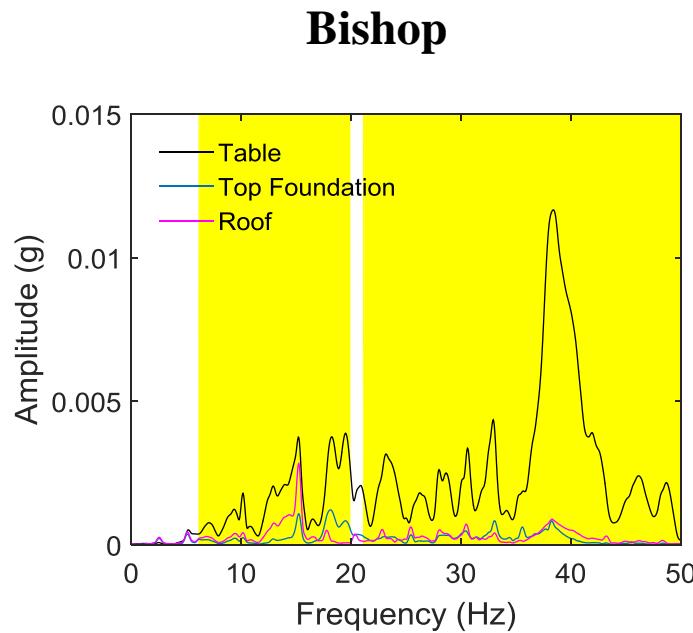
## El Centro

(Main frequency content of earthquake is widely distributed)



# Discussions

Effect of frequency band gap on acceleration response



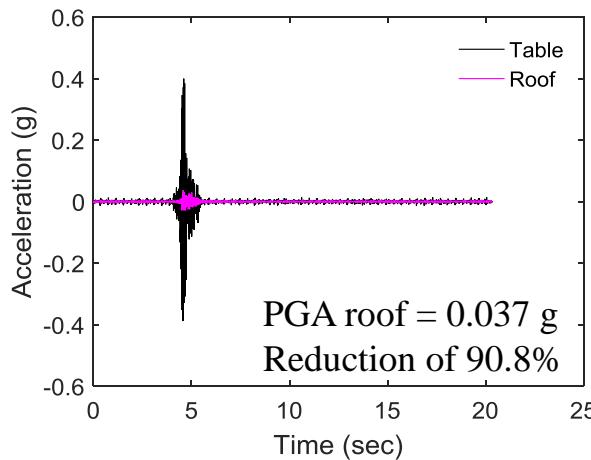
# Discussions

## Effect of frequency band gap on acceleration response

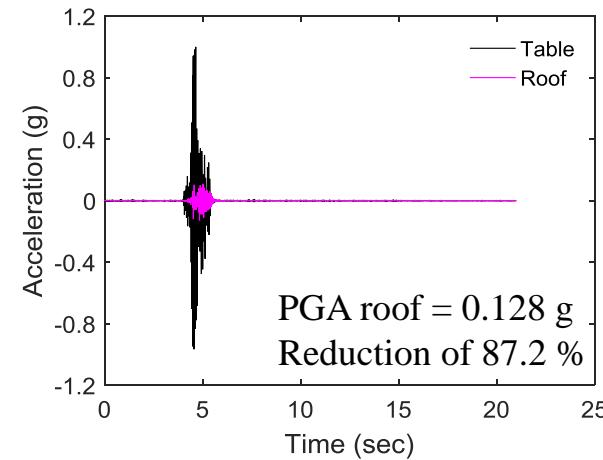
Earthquake

Bishop

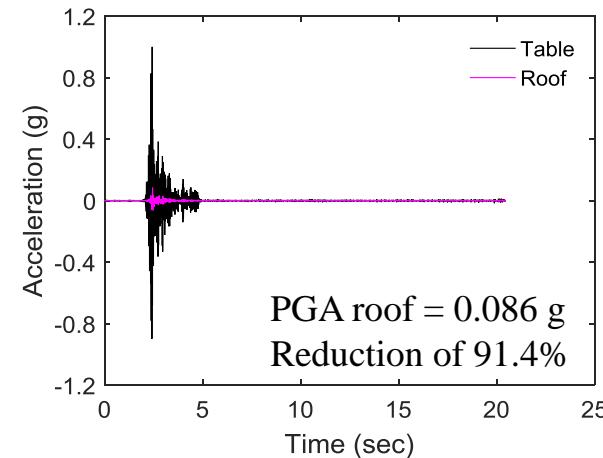
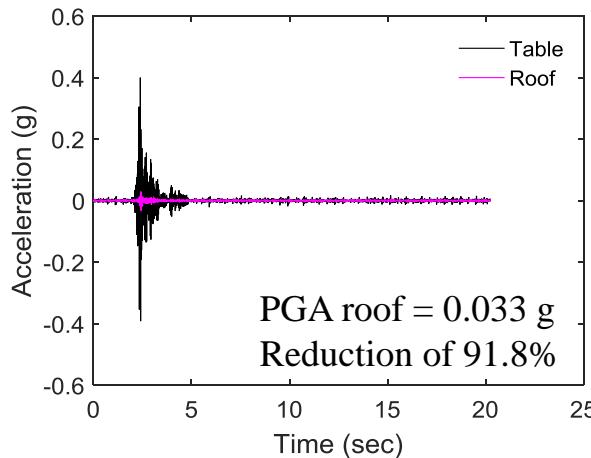
**PGA at table 0.4 g**



**PGA at table 1 g**



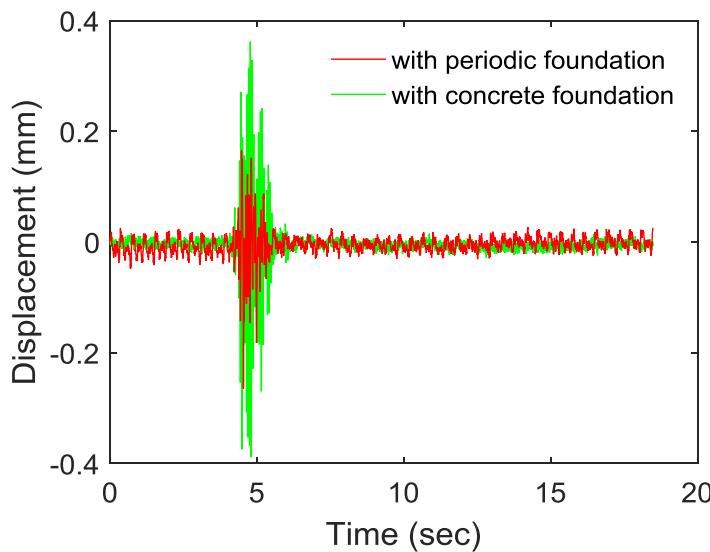
Oroville



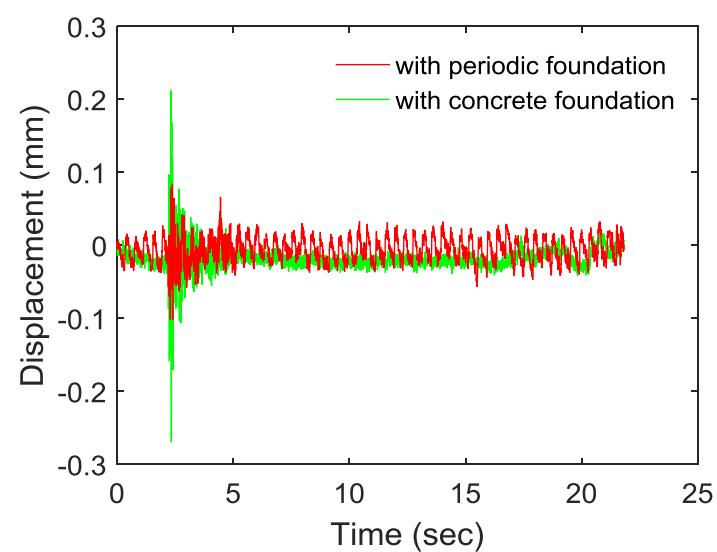
# Discussions

Roof to table relative displacement response

Bishop



Oroville



# Conclusions

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- The basic concept on how to find the frequency band gaps was reviewed.
- A set of design guidelines for periodic foundations was proposed.
- The test specimen of 1D periodic foundation has been constructed, and some tests have been performed.
- The test results show that periodic foundations can effectively reduce the vibration response when the frequency contents of the incoming waves are within the frequency band gap of the periodic foundation supported SMR building.

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**Thank you for your attention.**