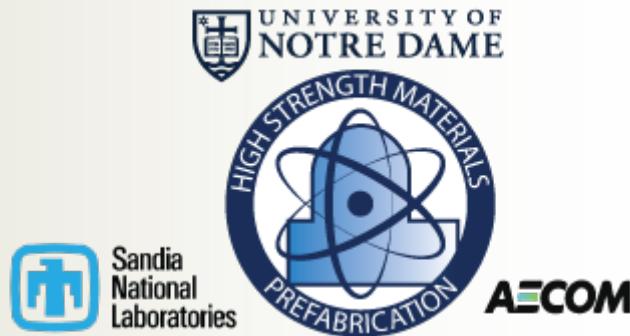
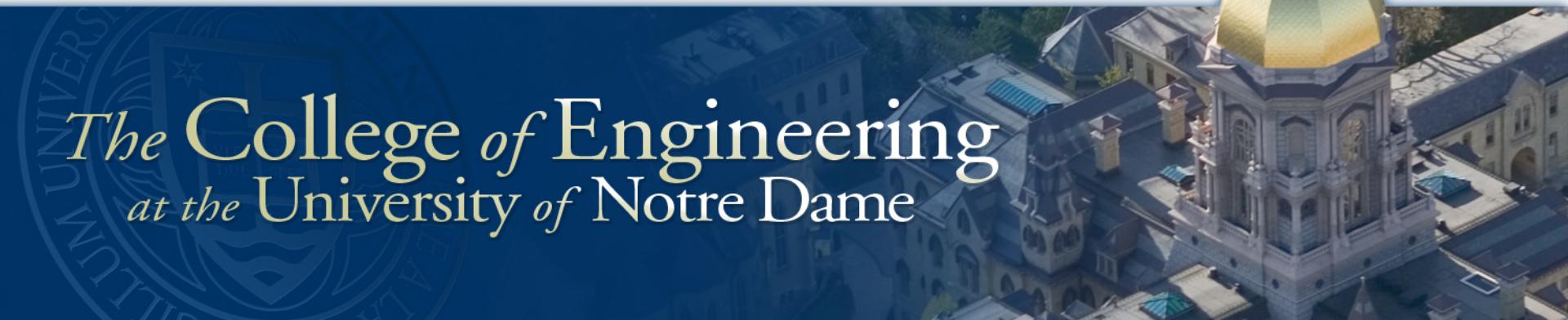


# Prefabricated High-Strength Rebar Systems with High-Performance Concrete for Accelerated Construction of Nuclear Concrete Structures



*The College of Engineering  
at the University of Notre Dame*



# Primary Objective

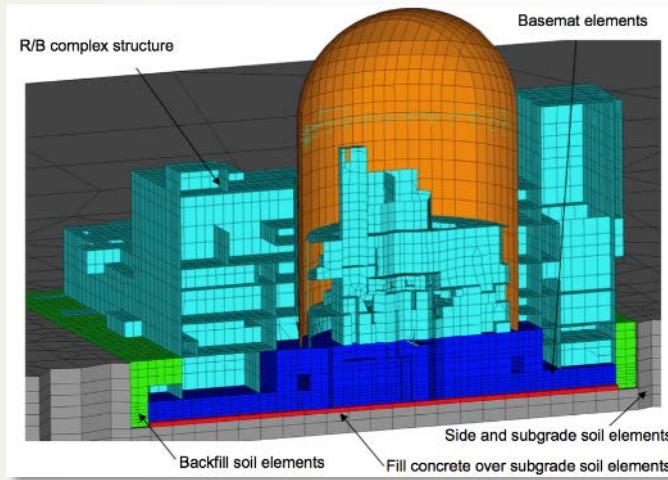
Reduce field construction times and fabrication costs of reinforced concrete nuclear structures through:

- 1) High-strength reinforcing steel bars (rebar)
- 2) Prefabricated rebar assemblies, including headed anchorages
- 3) High-strength concrete



# Scope

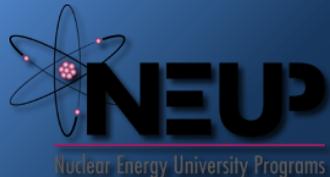
- Explore effectiveness, code conformity, and viability of existing high-strength materials
- Focus on shear walls – most common lateral load resisting members in nuclear structures (pressure vessels not in scope)
- Aim to reduce complexities in rebar to improve construction quality and ease of inspection



US-APWR Design Control Doc.



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# Collaboration



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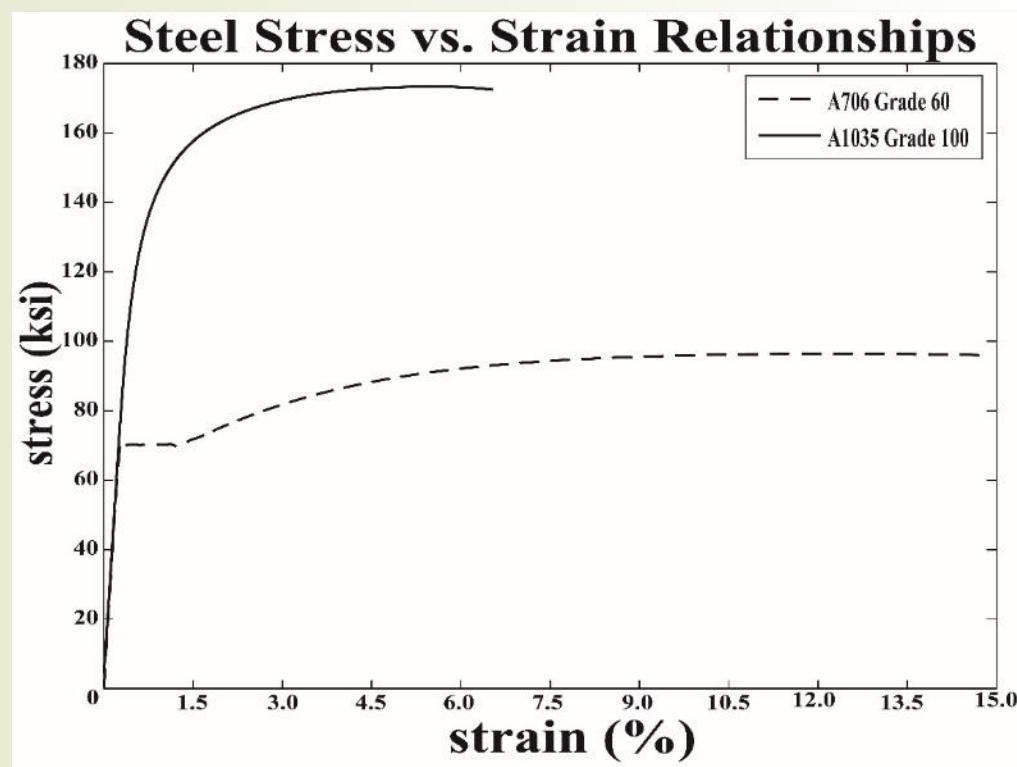


# Project Tasks

1. Evaluation of High-Strength Materials
  - Limit-benefit Analysis
  - Cost-benefit Analysis
2. Evaluation of Prefabricated Rebar Cages
3. Optimization, Modeling, and Design
  - Pre-test Analyses
4. Experimental Testing
  - Deep Beam (Wall Slice) Specimens
  - Shear Wall Specimens
5. Design/Modeling/Construction Recommendations

# 1. High-Strength Materials: Scope

- High-strength rebar (up to Grade 120) with high-strength concrete (up to 15 ksi compressive strength)
- Concrete strength of 5 ksi typical in current practice
- ACI 349 limits headed bars and shear reinforcement to Grade 60



# 1. High-Strength Materials

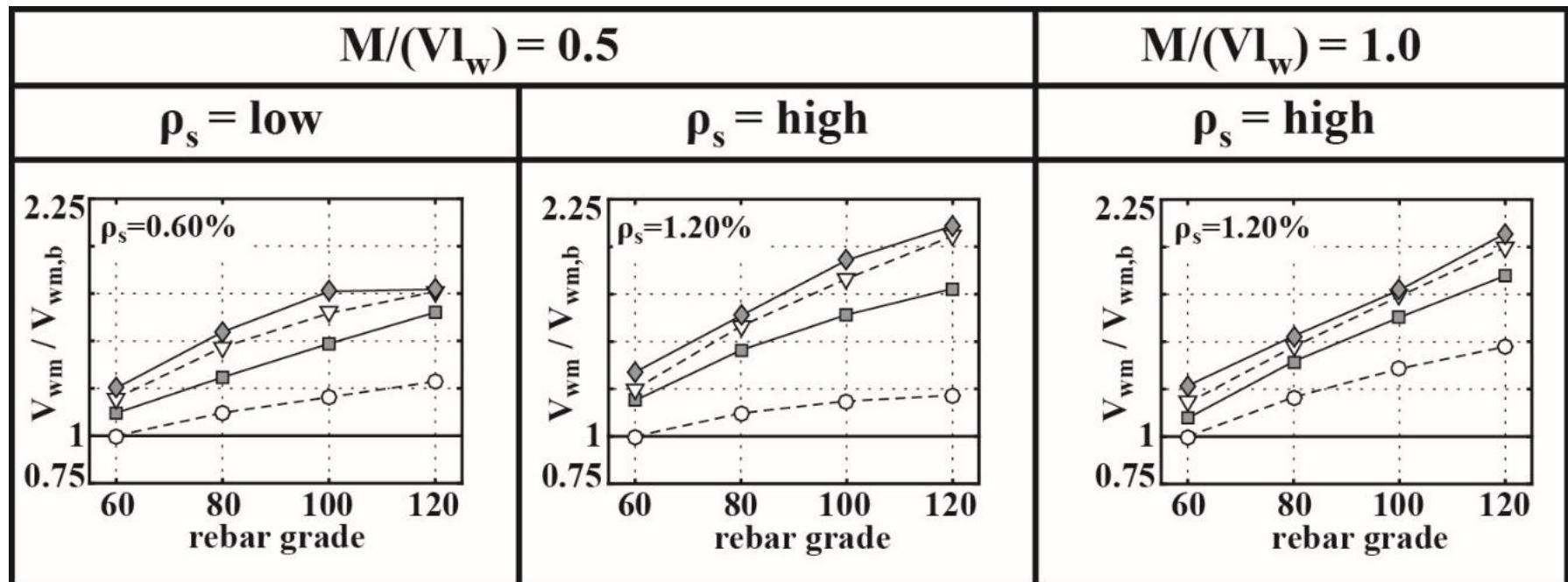
Numerical limit-benefit study to establish effects of high-strength materials on peak lateral strength of low-aspect-ratio shear walls:

- Parametric numerical investigation of 192 walls
- Peak strength predicted via finite element model

Parameter	Wall 1	Wall 2	Wall 3
length, $l_w$ (ft)	20	60	120
height, $h_w$ (ft)	40	120	120
thickness, $t_w$ (in.)	15	45	45
moment to shear ratio, $M/(Vl_w)$	<b>0.5, 1.0</b>	<b>0.5, 1.0</b>	<b>0.5, 1.0</b>
concrete strength, $f'_c$ (ksi)	<b>5, 10, 15, 20</b>	<b>5, 10, 15, 20</b>	<b>5, 10, 15, 20</b>
rebar strength, $f_y$ (ksi)	<b>60, 80, 100, 120</b>	<b>60, 80, 100, 120</b>	<b>60, 80, 100, 120</b>
reinforcement ratio, $\rho_s$ (%)	<b>0.25, 0.50</b>	<b>0.60, 1.20</b>	<b>0.60, 1.20</b>

# 1. High-Strength Materials

Results for Wall 2 (60 ft x 120 ft x 45 in.):



-○-  $f'_c = 5.00 \text{ ksi}$

-■-  $f'_c = 10.0 \text{ ksi}$

-▽-  $f'_c = 15.0 \text{ ksi}$

-◆-  $f'_c = 20.0 \text{ ksi}$

$V_{wm}$  = Predicted peak lateral strength

$V_{wm,b}$  = Predicted peak lateral strength of “benchmark” with normal strength materials

# 1. High-Strength Materials

## Summary of results of limit-benefit analysis

- Combination of high-strength rebar with high-strength concrete resulted in a higher-performing structure than with either high-strength material on its own
- Significant benefits by using concrete strength of  $f'_c = 10$  ksi, with diminishing returns for higher strengths
- Greatest benefits of high-strength materials for walls with large rebar ratios,  $\rho_s$

# 1. High-Strength Materials

Numerical cost-benefit study of economic effectiveness of high-strength materials for low-aspect-ratio shear walls:

- Parametric numerical investigation of 2304 walls
- Construction cost metric ( $\Gamma$ ) includes rebar material cost, rebar labor cost, and concrete material cost ( $C_w$ ), normalized by peak strength ( $V_{wm}$ ):  $\Gamma = \frac{C_w}{V_{wm}}$

Parameter	Wall 1	Wall 2	Wall 3
length, $l_w$ (ft)	20	60	120
height, $h_w$ (ft)	40	120	120
thickness, $t_w$ (in.)	10, <b>15</b> , 20	30, <b>45</b> , 60	30, <b>45</b> , 60
moment to shear ratio, $M/(Vl_w)$	<b>0.5</b> , 1.0	<b>0.5</b> , 1.0	<b>0.5</b> , 1.0
concrete strength, $f'_c$ (ksi)	5, 10, 15, 20	5, 10, 15, 20	5, 10, 15, 20
rebar strength, $f_y$ (ksi)	<b>60</b> , 80, 100, 120	<b>60</b> , 80, 100, 120	<b>60</b> , 80, 100, 120
reinforcement ratio, $p_s$ (%)	low to high	low to high	low to high

# 1. High-Strength Materials

Summary of results of cost-benefit analysis:

- Combination of high-strength rebar with high-strength concrete resulted in greatest economic benefits for walls with lower  $M/(Vl_w)$  ratios and large reinforcement ratios,  $\rho_s$
- A concrete strength of  $f'_c = 10$  ksi showed the largest incremental reduction in construction cost
- Rebar grades greater than 100 can lead to decreased economic benefits due to the increased unit cost

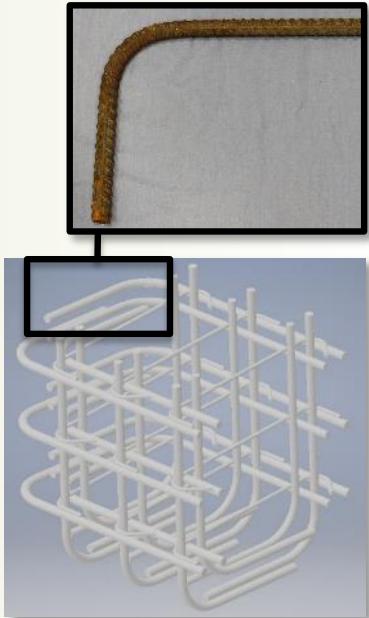
## 2. Prefabricated Rebar

- Evaluating prefab rebar cages for:
  - transportability
  - liftability
  - modularity
- Using mini-scale rapid prototyping



**Most Congested  
(current)**

*Multiple layers  
of hooked  
Grade 60 bars*



*Fewer layers  
of headed high-  
strength bars*

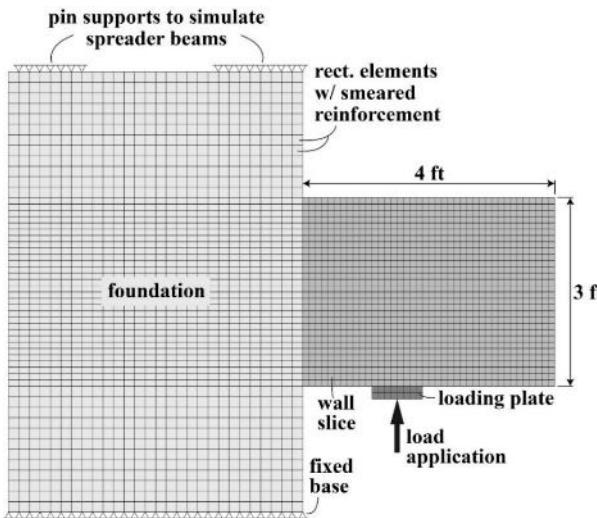


**Least Congested  
(envisioned)**

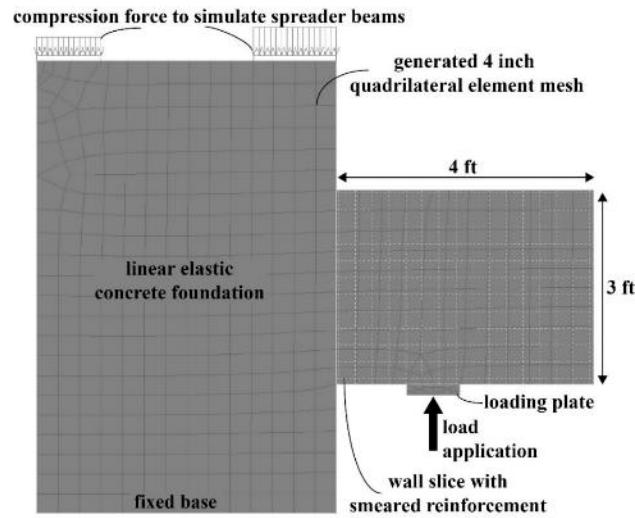


# 3. Optimization, Modeling and Design

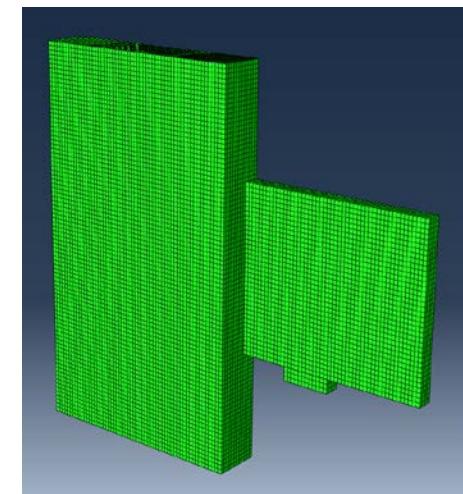
Pre-test Analyses of Deep Beam (Wall Slice) and Shear Wall Specimens in Vector2, ATENA, and ABAQUS



Vector2



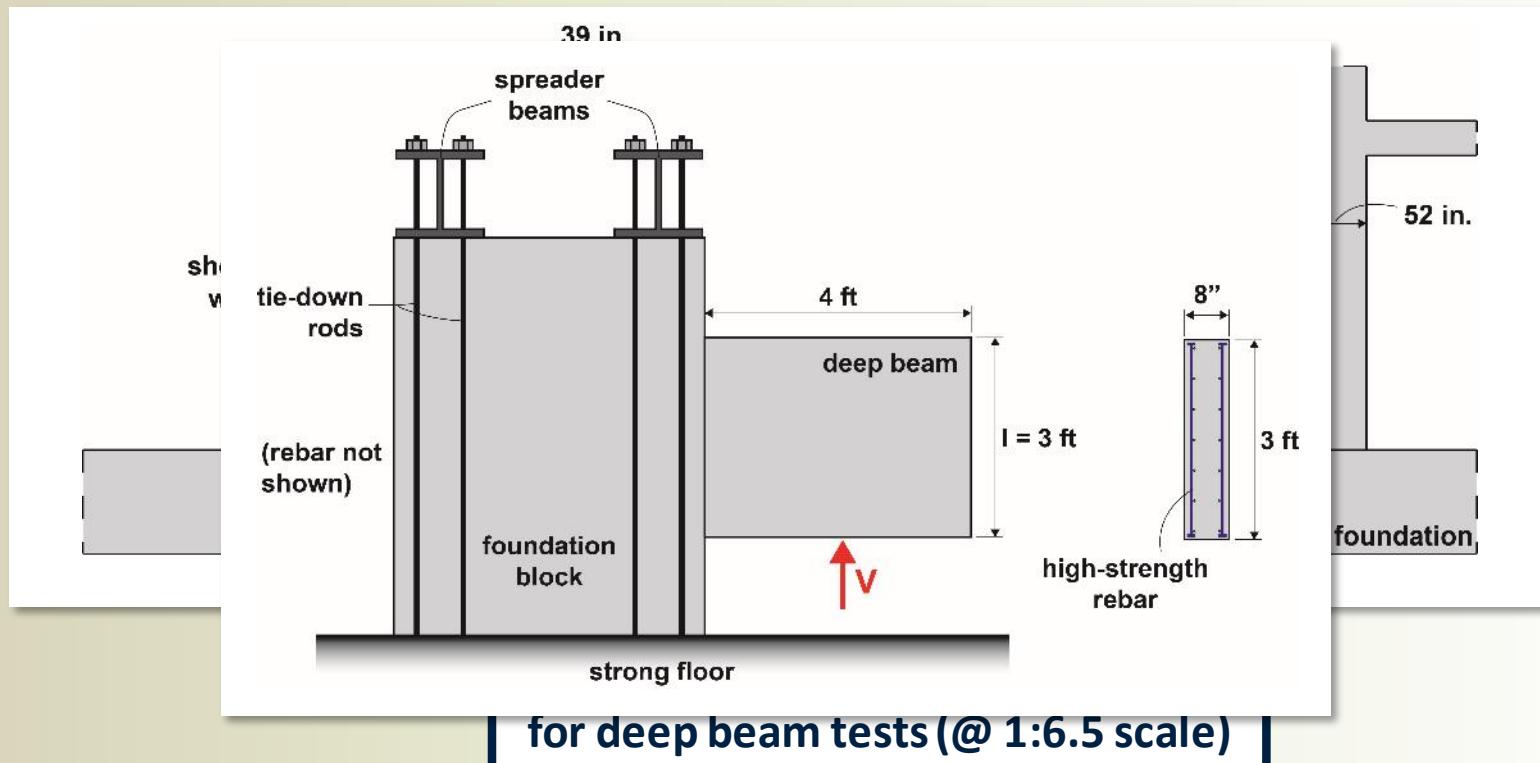
ATENA



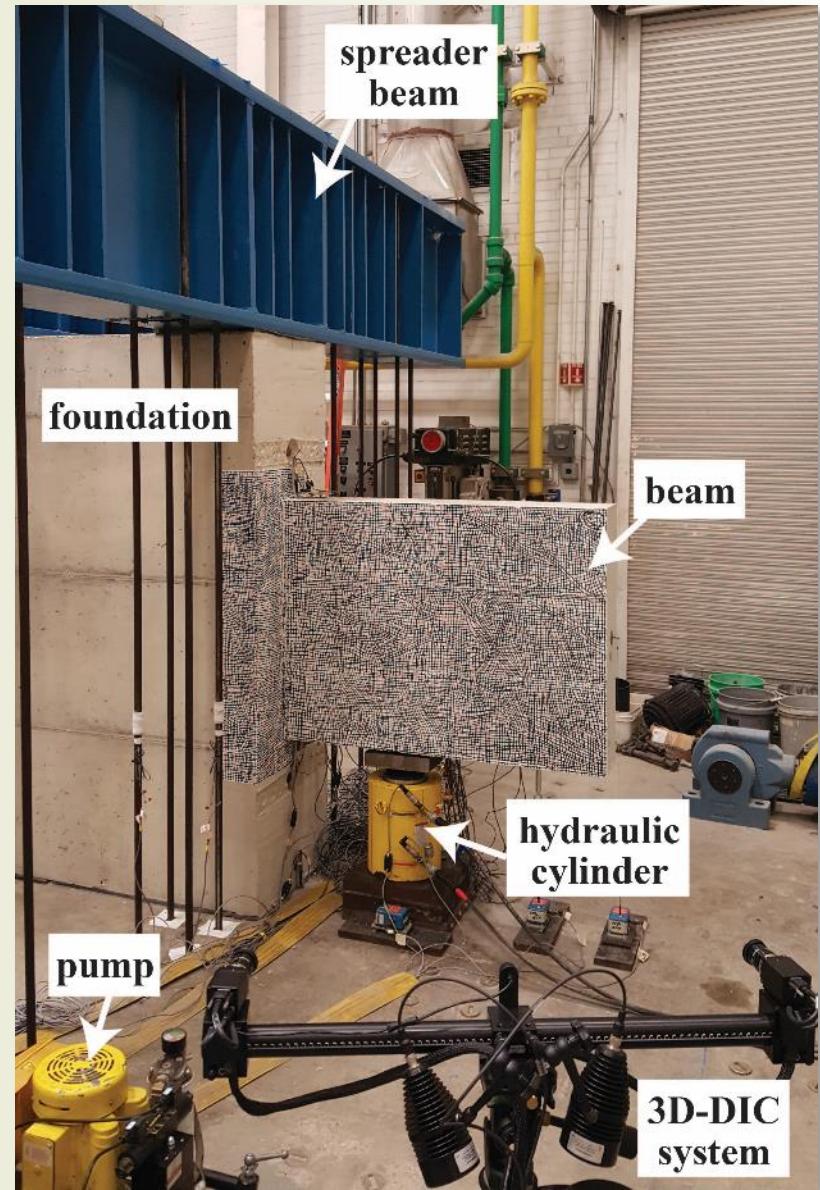
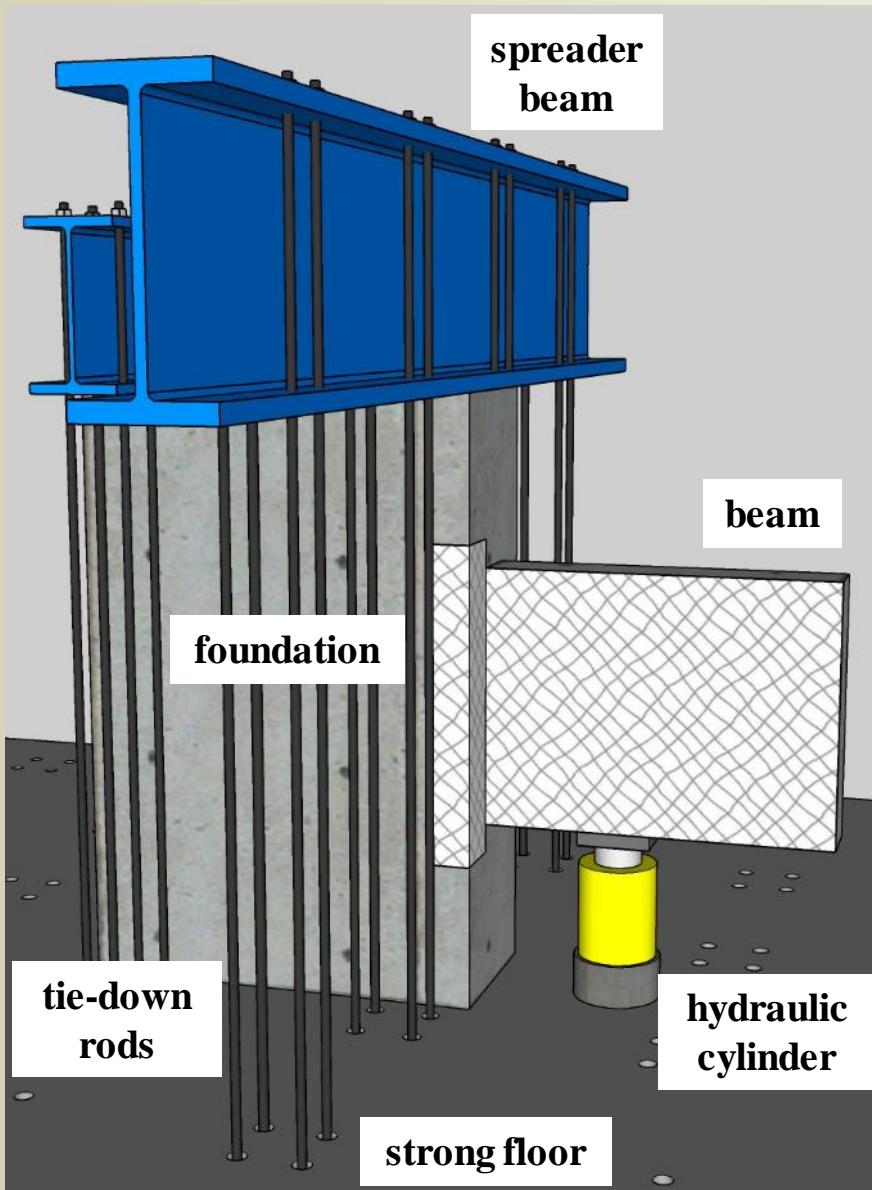
ABAQUS

# 4. Experimental Testing

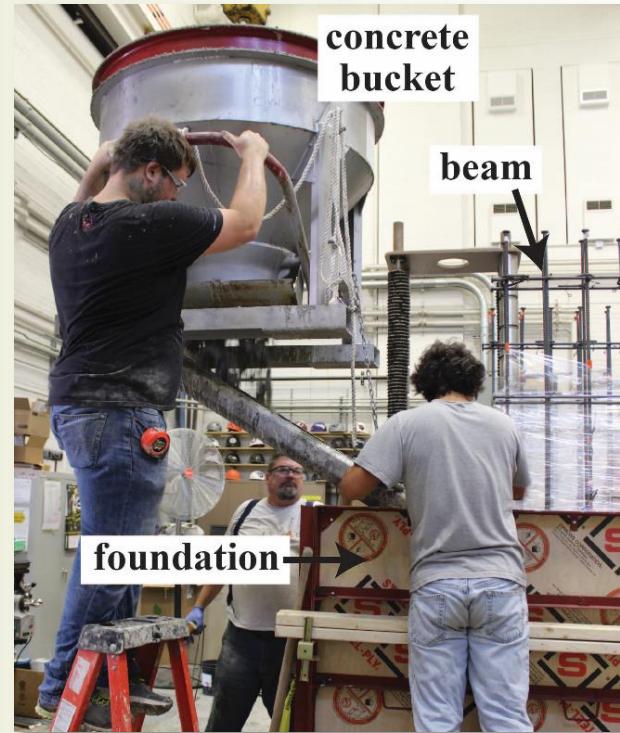
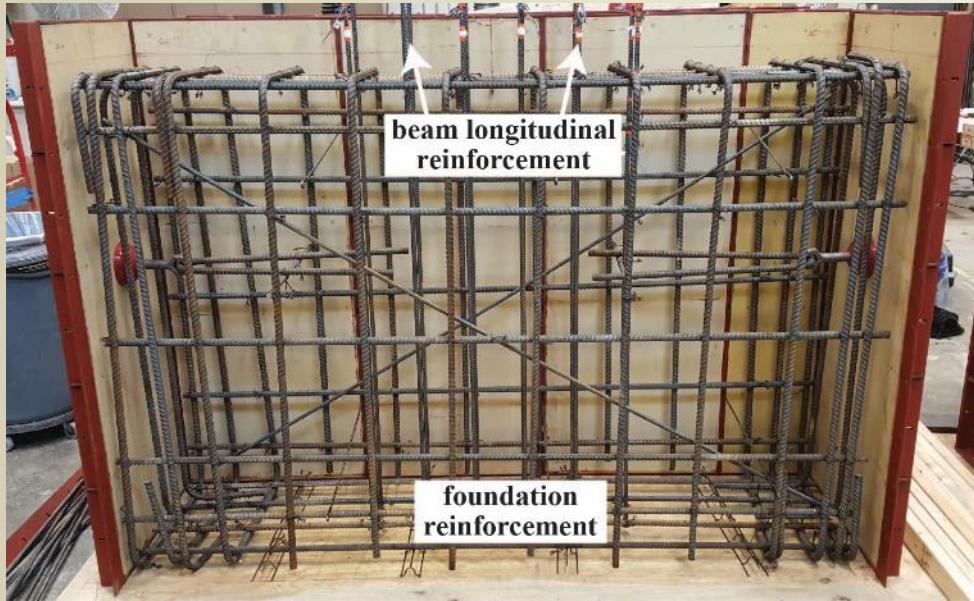
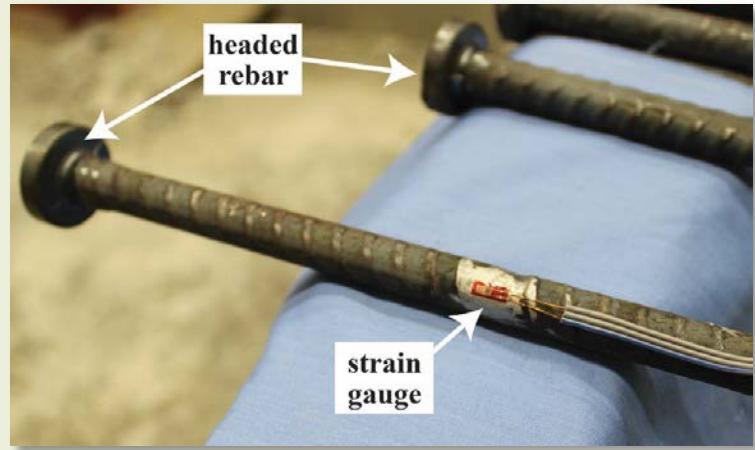
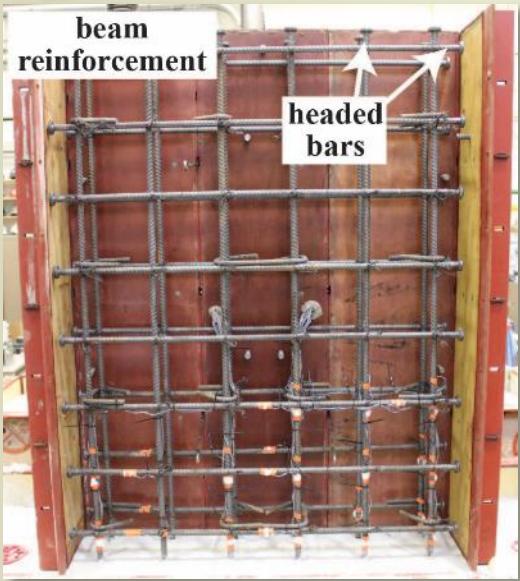
- “Generic wall” dimensions determined using publicly-available design control documents
- Provides basis for future deep beam and shear wall tests



# Deep Beam Test Setup



# Deep Beam Construction



# Deep Beam Construction



Normal-Strength Concrete

$f'_c = 6500 \text{ psi}$

slump = 8 in.



High-Strength Concrete

$f'_c = 14690 \text{ psi}$

slump = 8.75 in.

# Deep Beam Test Parameters

Specimen	$f'_c$ (psi)	$f_y$ (ksi)	$\rho_s$ (%)	$M/(Vl_w)$
DB1	6500	70	0.833	0.5
DB2	6500	133	0.833	0.5
DB3	14960	70	0.833	0.5
DB4	14960	133	0.833	0.5

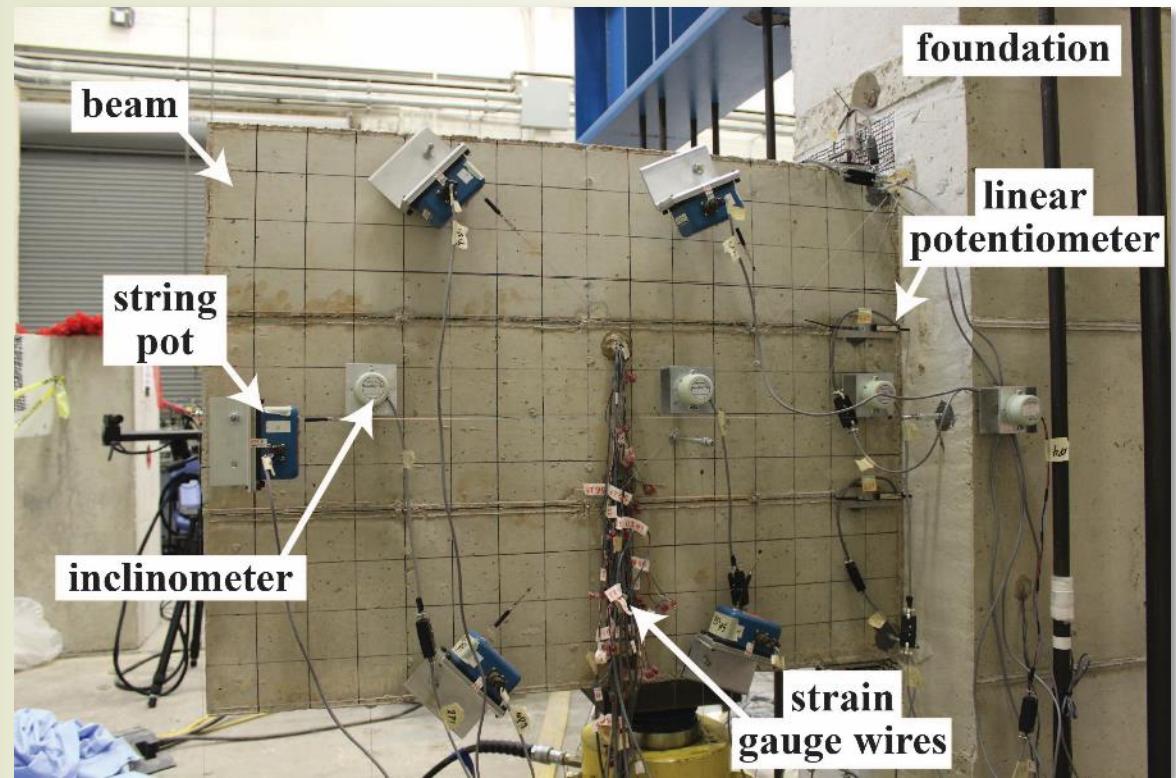
Definitions:  $f'_c$  – concrete 28 day compressive strength

$f_y$  – rebar yield strength, determined by tensile tests and 0.2% offset method

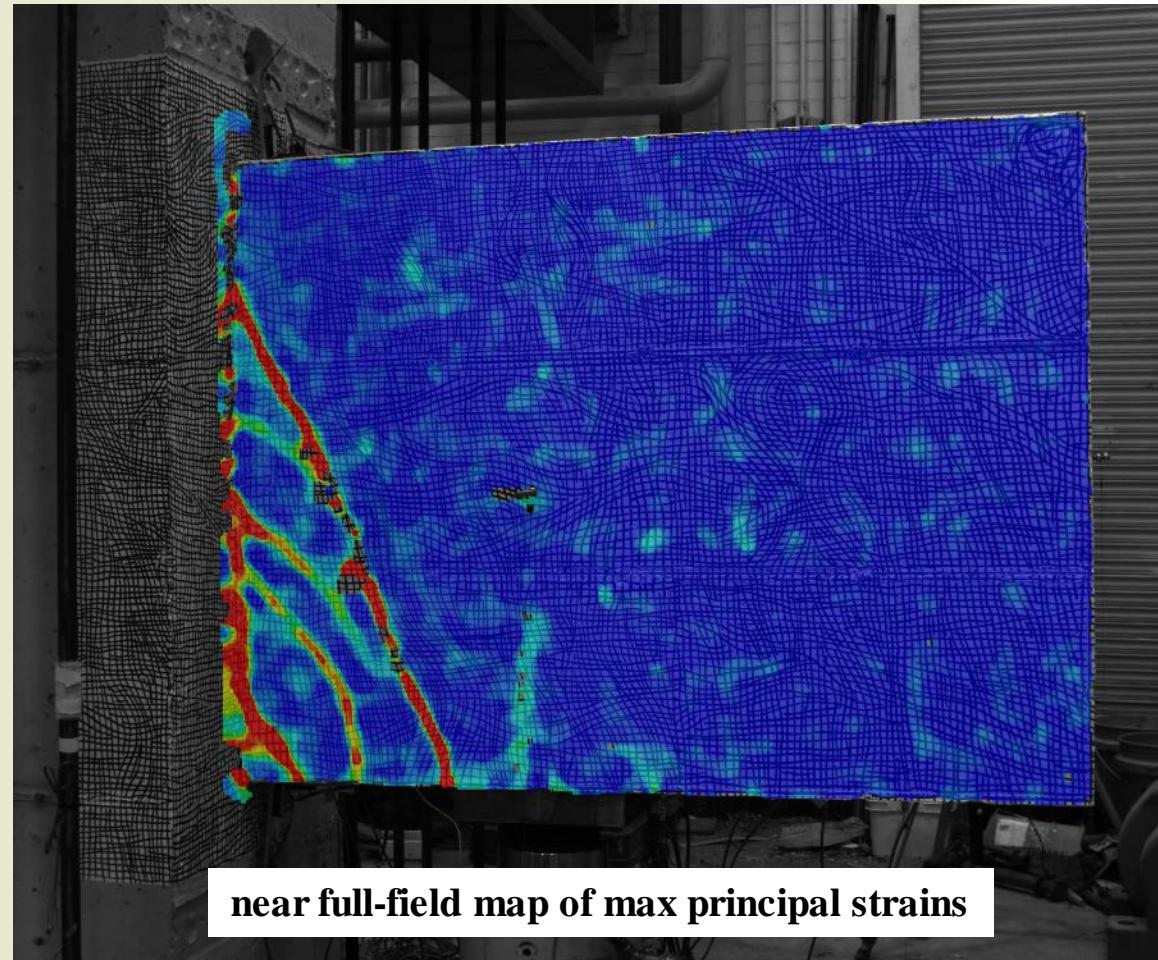
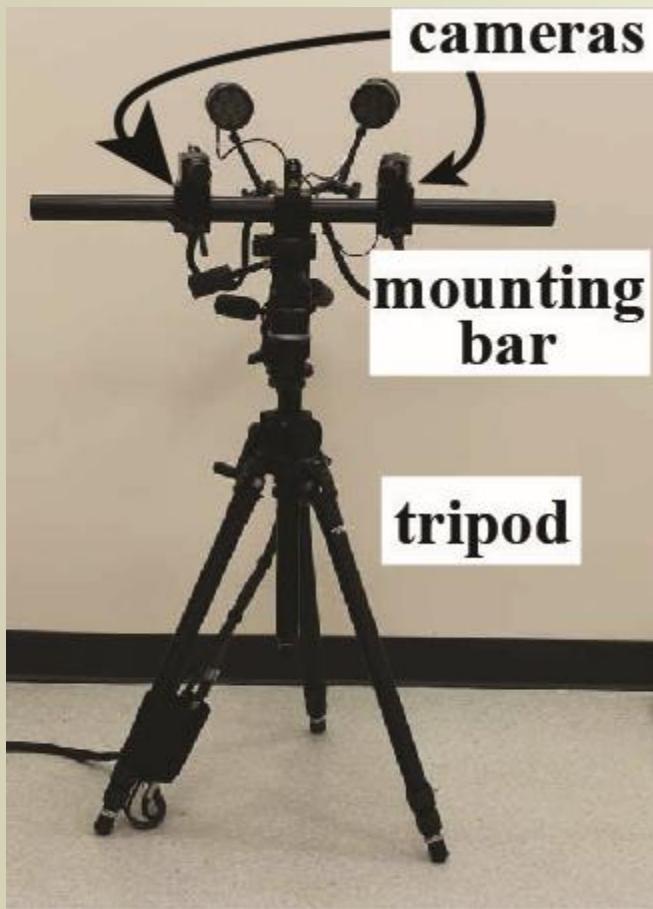
$\rho_s$  – reinforcement ratio, symmetric for longitudinal and transverse rebar

# Deep Beam Instrumentation

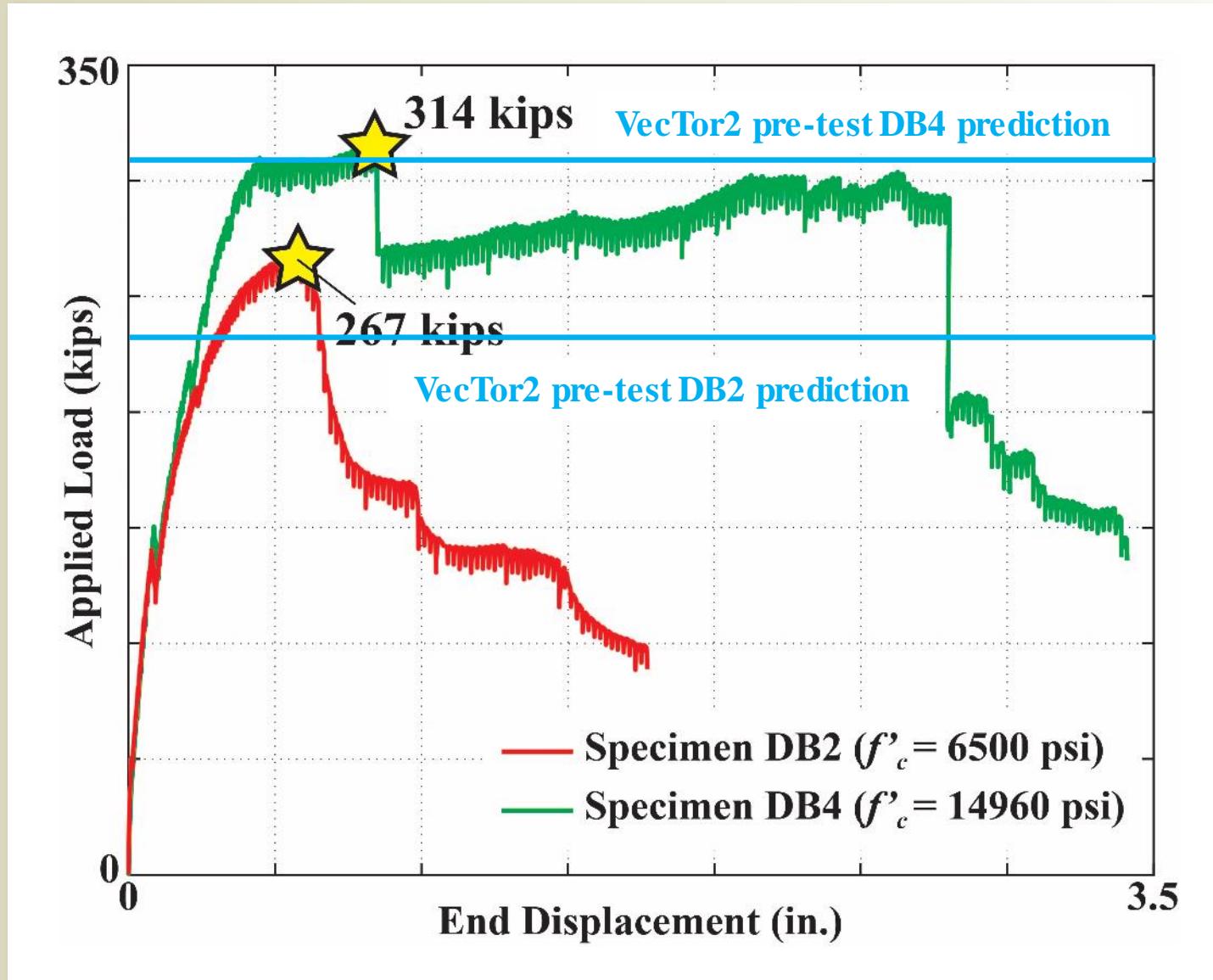
Type	Number
pressure transducer	2
string potentiometer	9
linear potentiometer	8
inclinometer	4
strain gauge	42
<b>TOTAL</b>	<b>65</b>



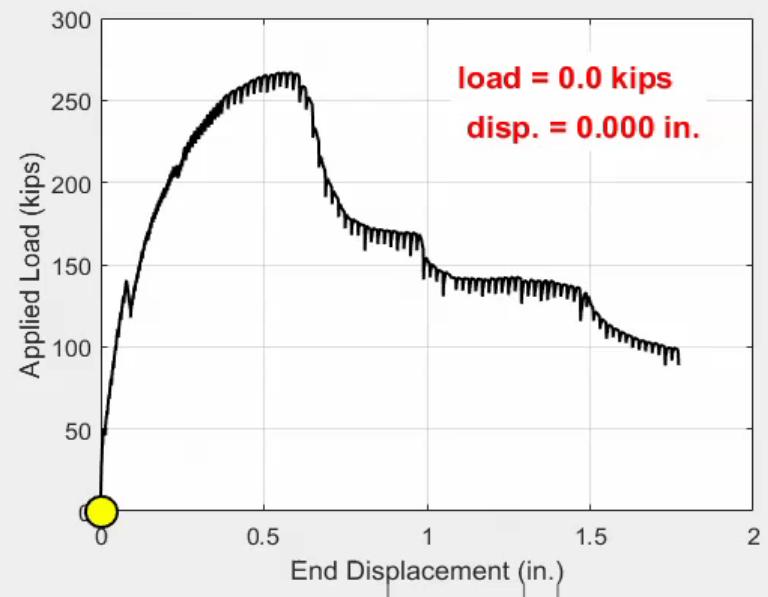
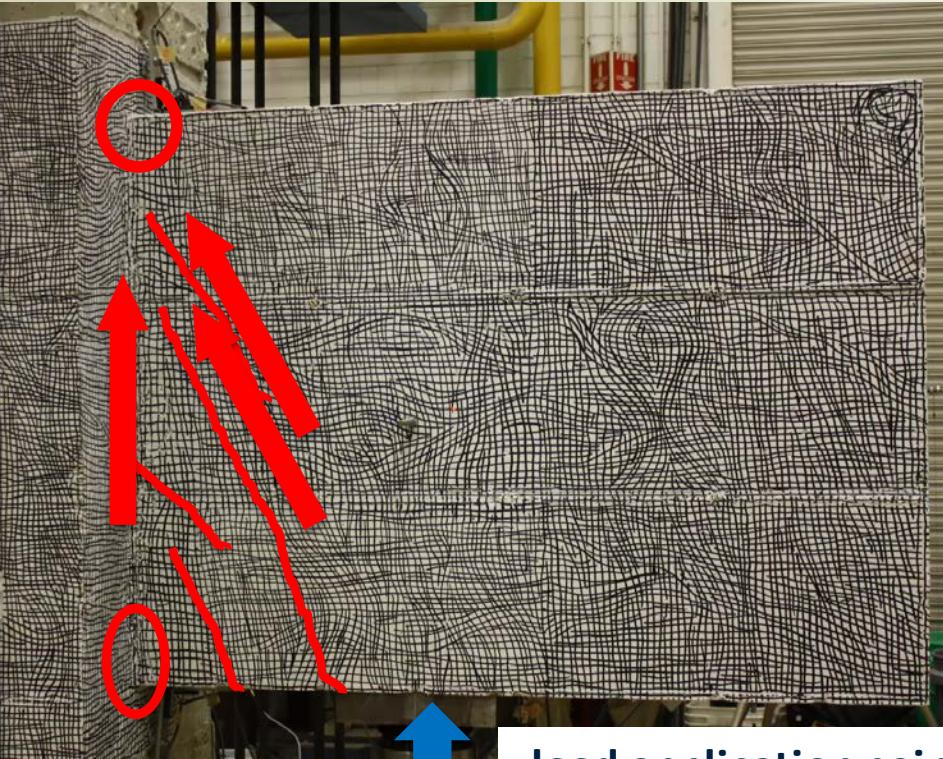
# 3D Digital Image Correlation



# Specimens DB2 and DB4 Response

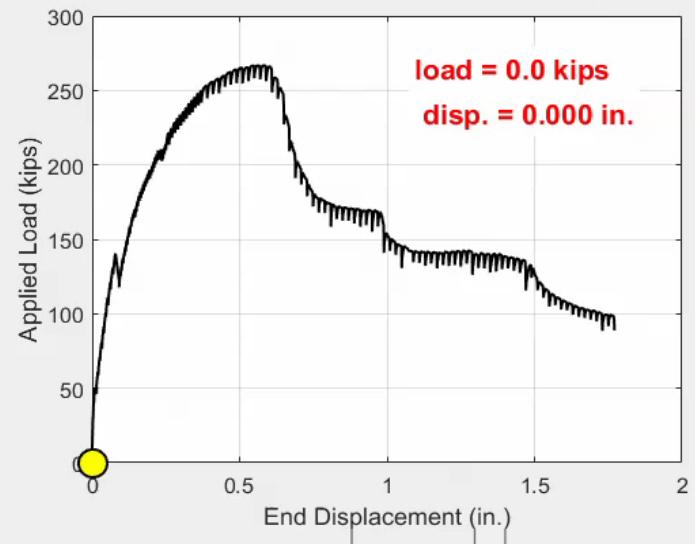
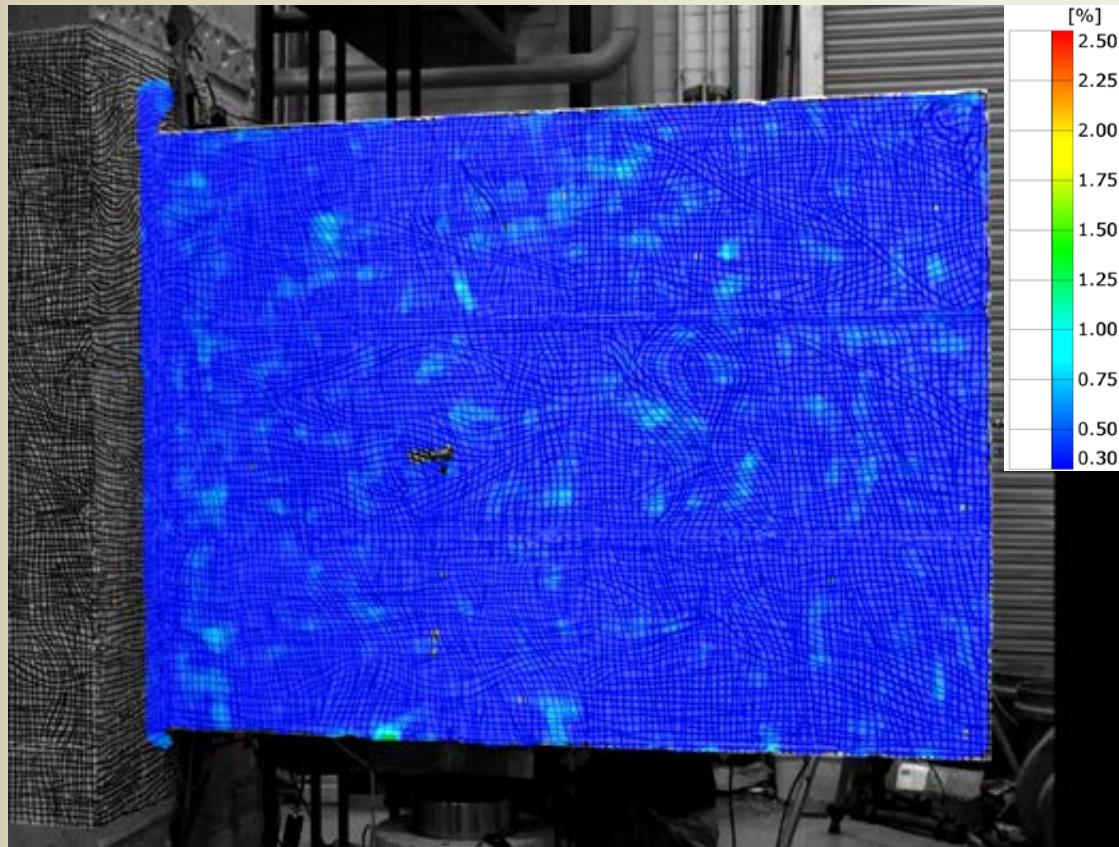


# DB2 ( $f'_c = 6500$ psi, $f_y = 133$ ksi)

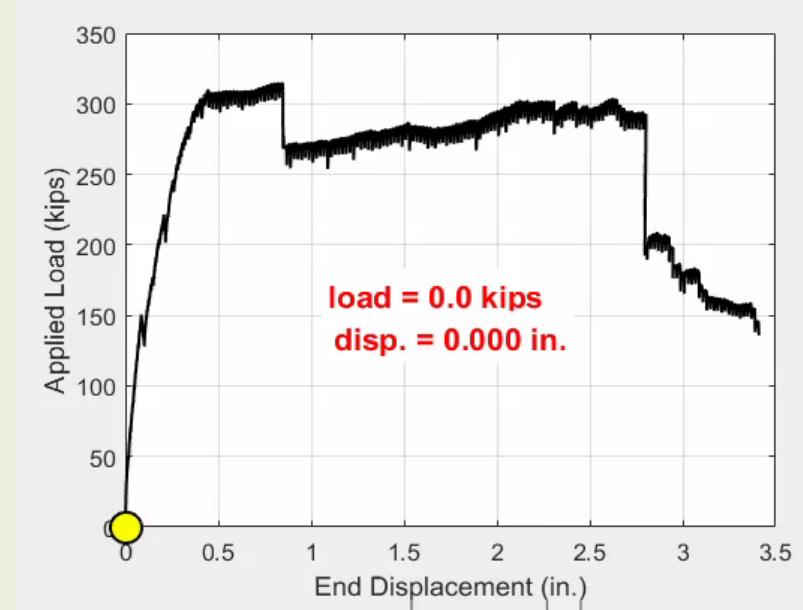
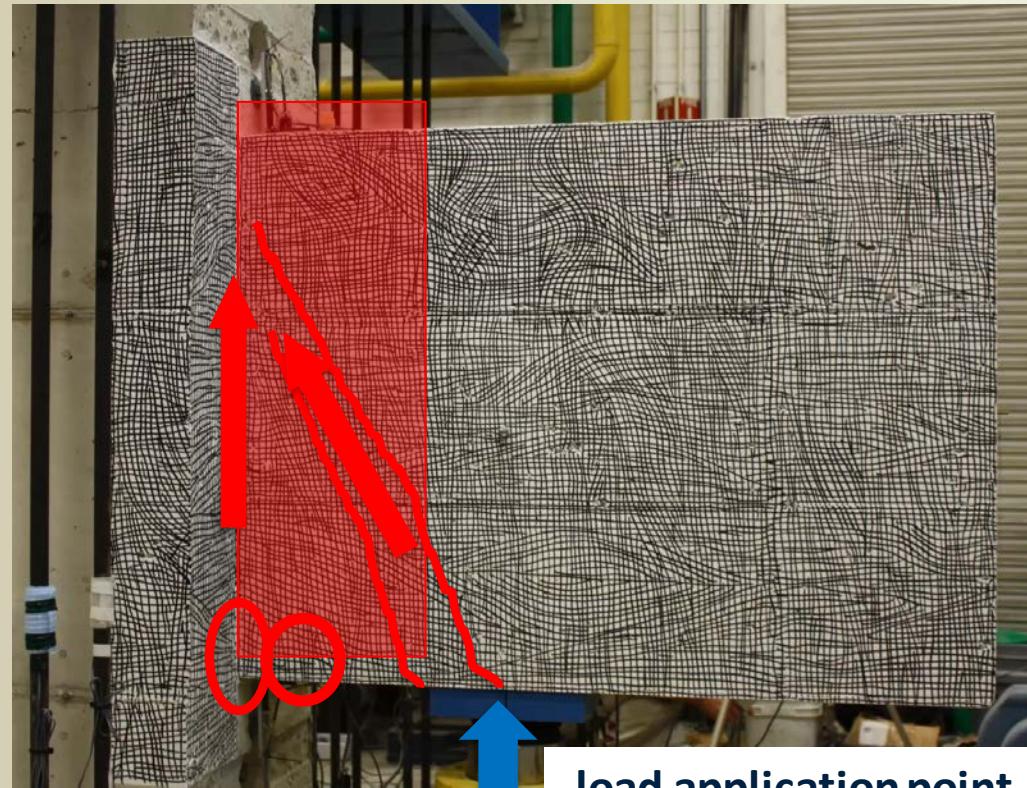


Slipping along 2<sup>nd</sup> diagonal crack

# DB2 ( $f'_c = 6500$ psi, $f_y = 133$ ksi)

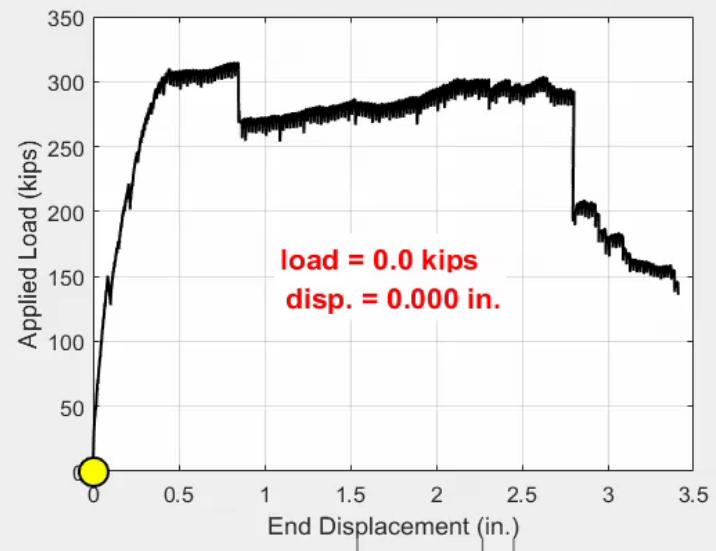
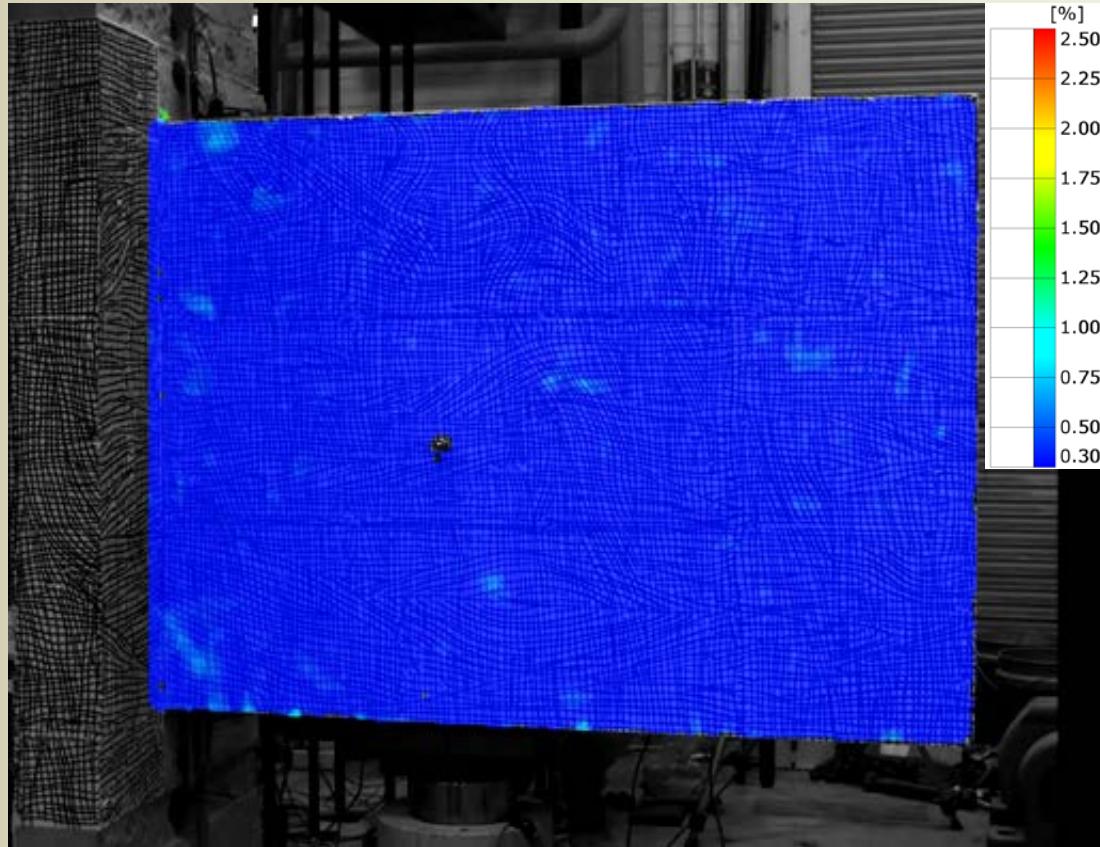


# DB4 ( $f'_c = 14960$ psi, $f_y = 133$ ksi)

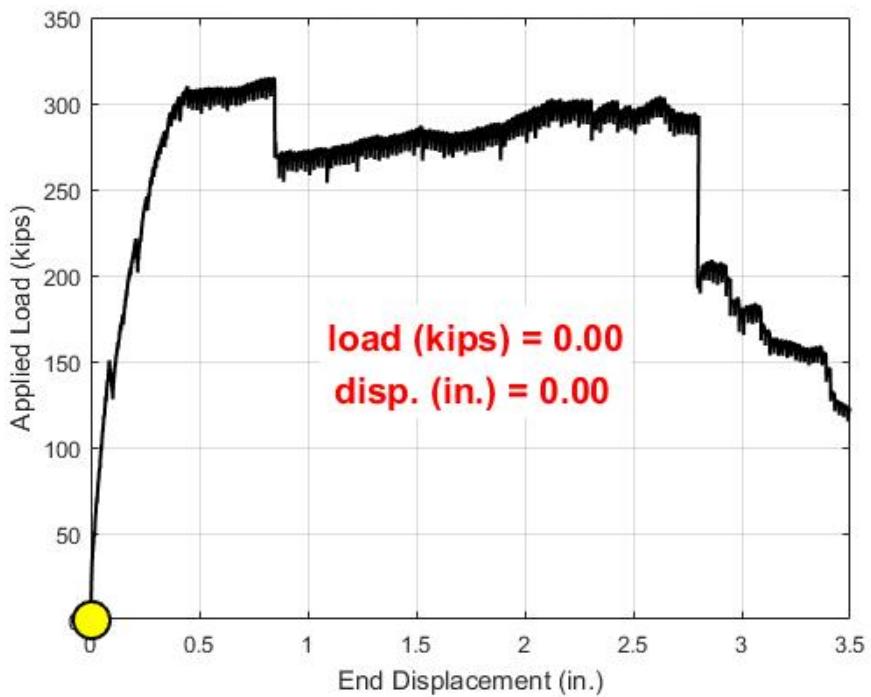
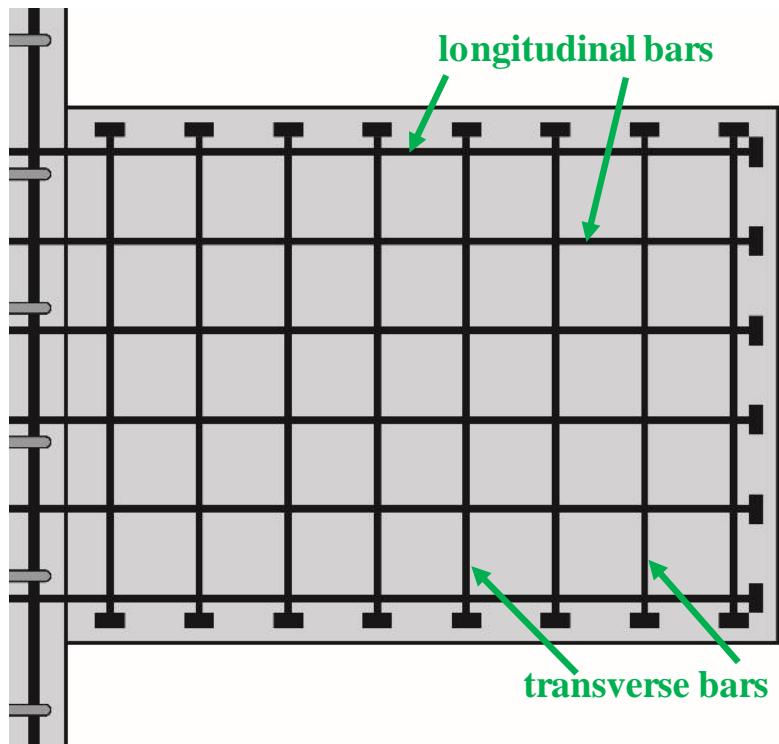


Significant concrete degradation through beam depth

# DB4 ( $f'_c = 14960$ psi, $f_y = 133$ ksi)



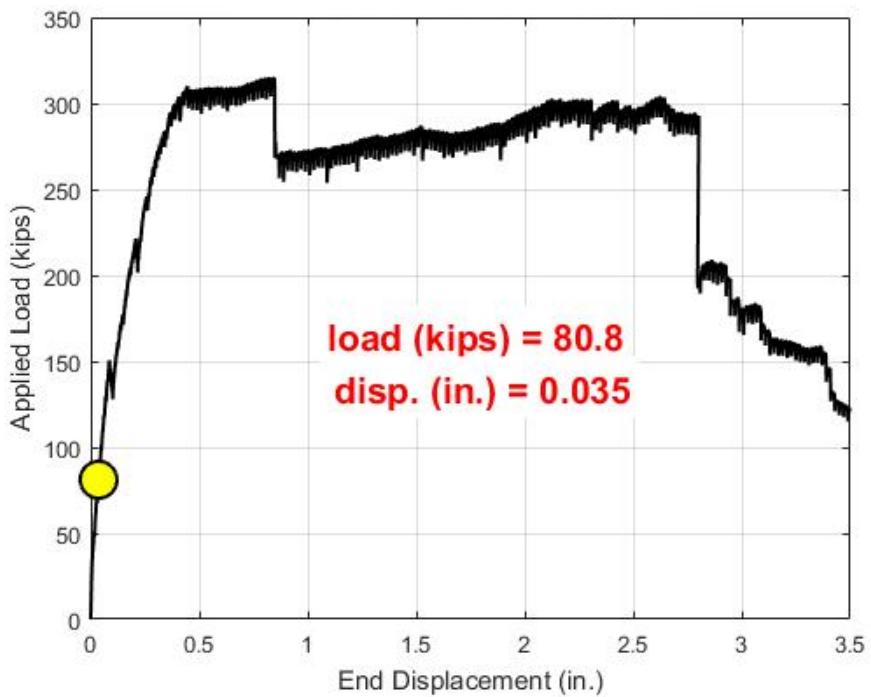
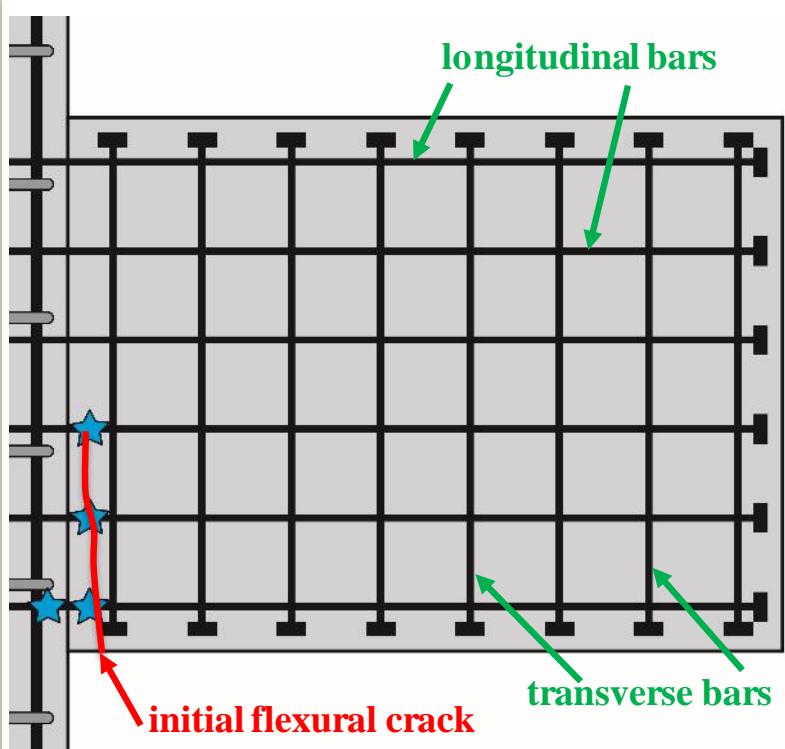
# DB4 ( $f'_c = 14960$ psi, $f_y = 133$ ksi)



★ active tension strain

★ tension yield ( $6.85 m\varepsilon$ )

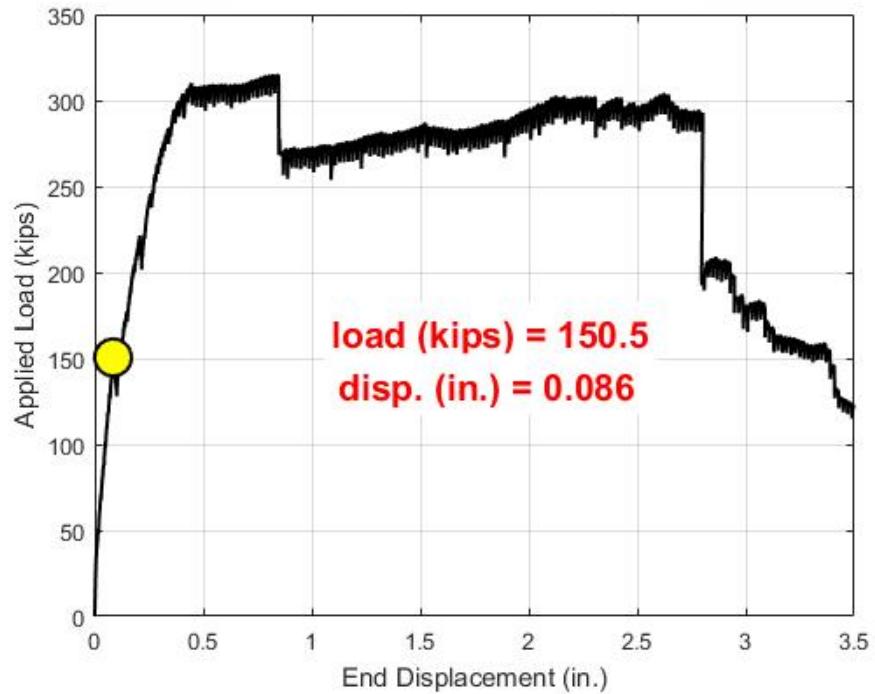
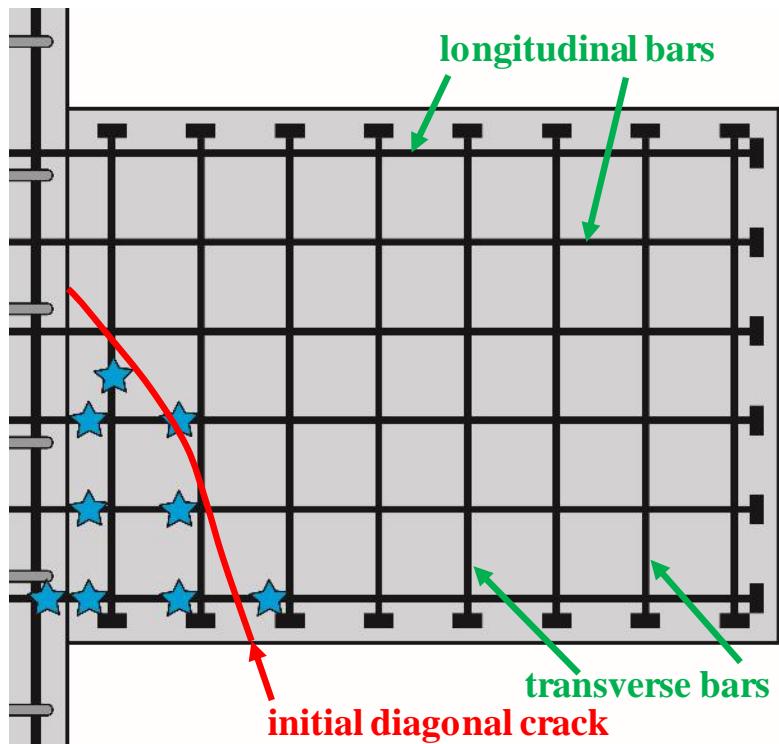
# DB4 ( $f'_c = 14960$ psi, $f_y = 133$ ksi)



- ★ active tension strain
- ★ tension yield ( $6.85 \text{ me}$ )

Initial flexural cracking, bottom three longitudinal layers  
active in tension

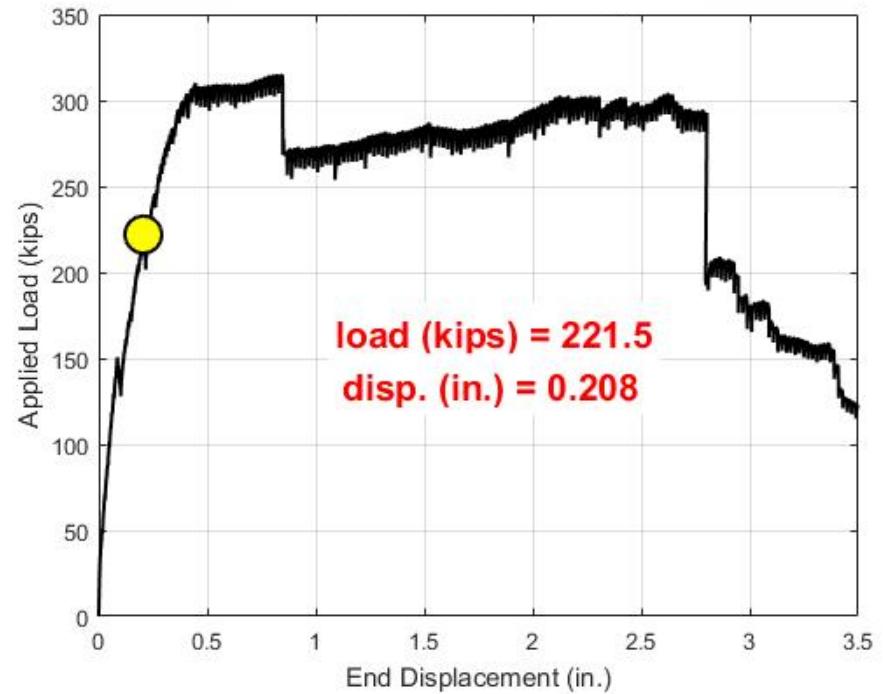
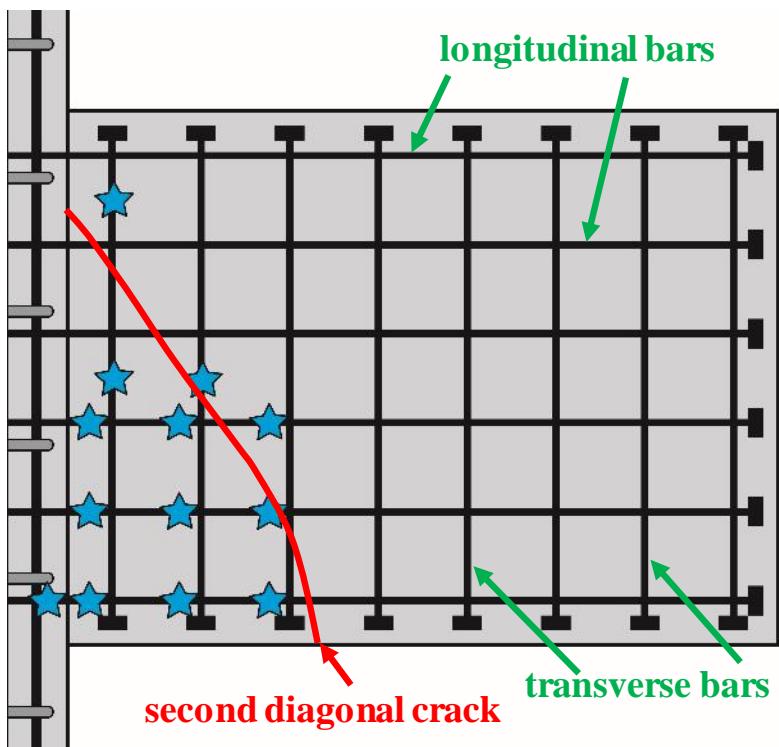
# DB4 ( $f'_c = 14960$ psi, $f_y = 133$ ksi)



- ★ active tension strain
- ★ tension yield ( $6.85 \text{ me}$ )

Bottom three longitudinal layers and closest transverse layer to foundation strain to arrest diagonal crack

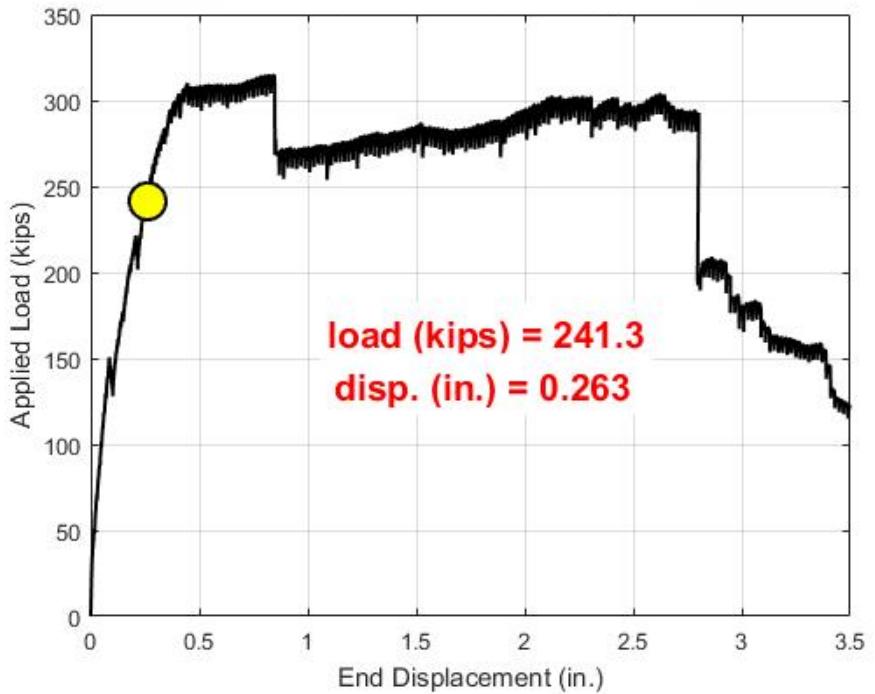
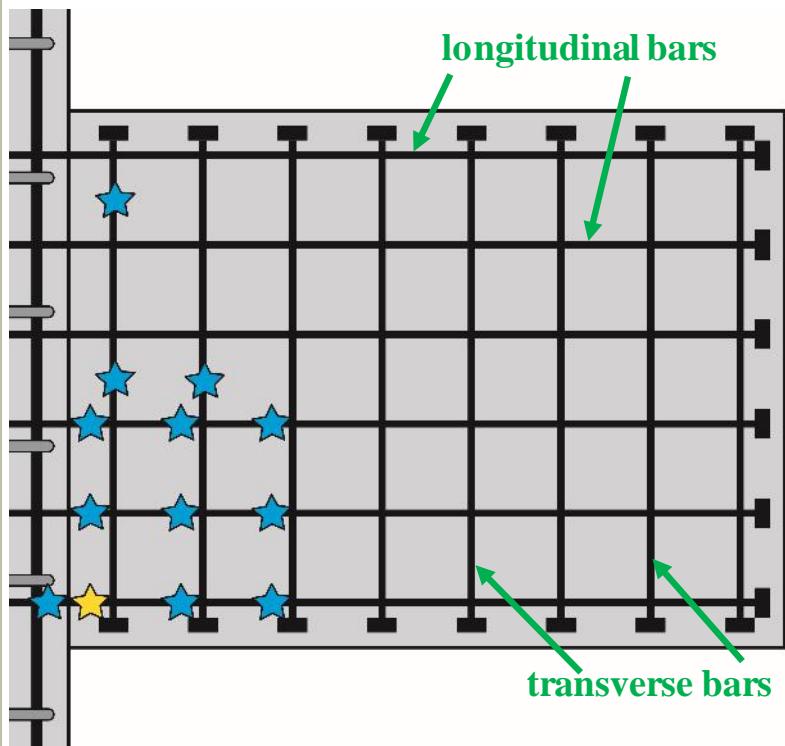
# DB4 ( $f'_c = 14960$ psi, $f_y = 133$ ksi)



★ active tension strain  
★ tension yield ( $6.85 \text{ me}$ )

Two transverse bar layers and two longitudinal bar layers  
above the bottom experience strain increase

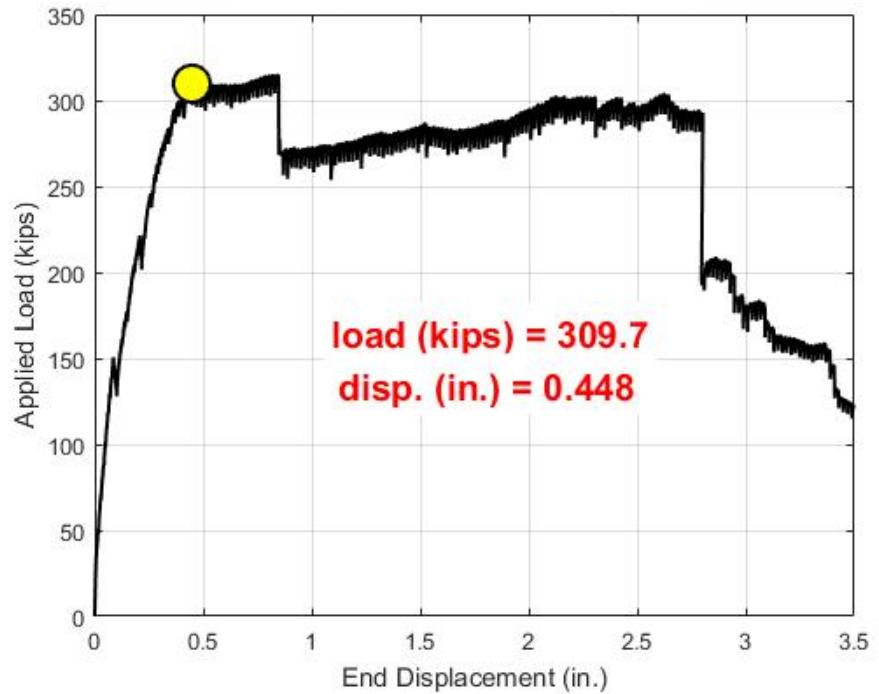
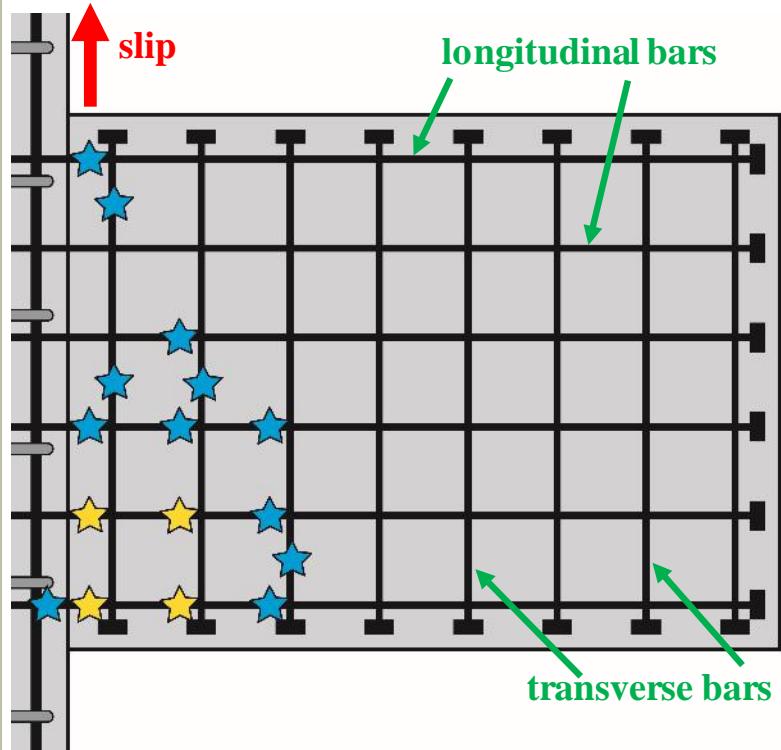
# DB4 ( $f'_c = 14960$ psi, $f_y = 133$ ksi)



- ★ active tension strain
- ★ tension yield (6.85 me)

Initiation of longitudinal reinforcement yielding

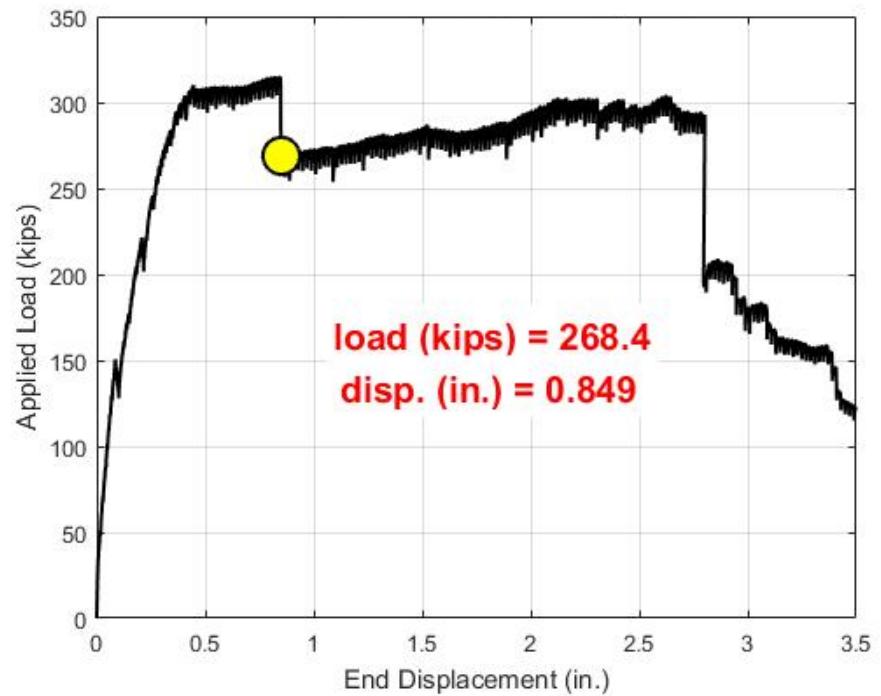
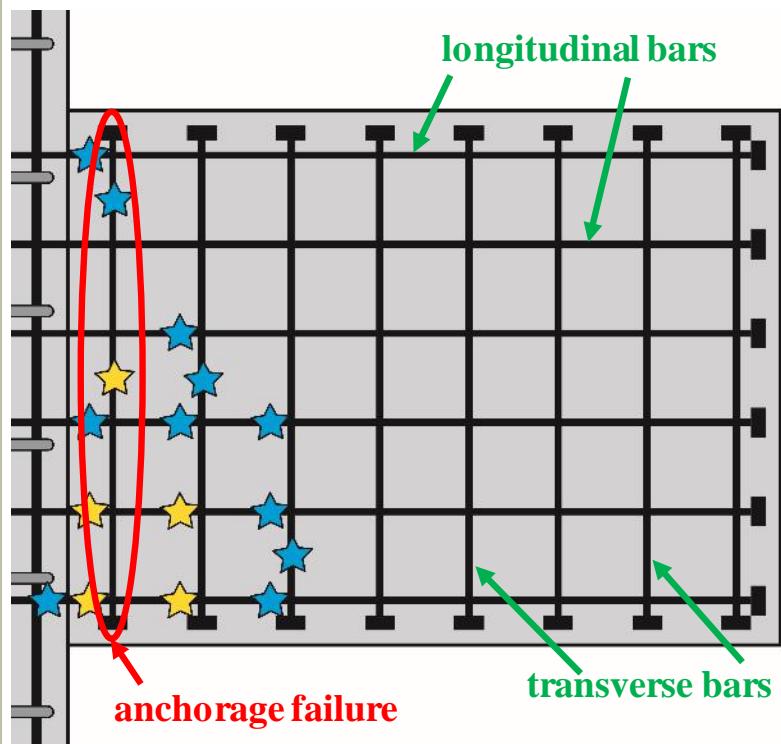
# DB4 ( $f'_c = 14960$ psi, $f_y = 133$ ksi)



- ★ active tension strain
- ★ tension yield (6.85  $m\epsilon$ )

Slip at foundation interface  
Extensive yielding of longitudinal reinforcement

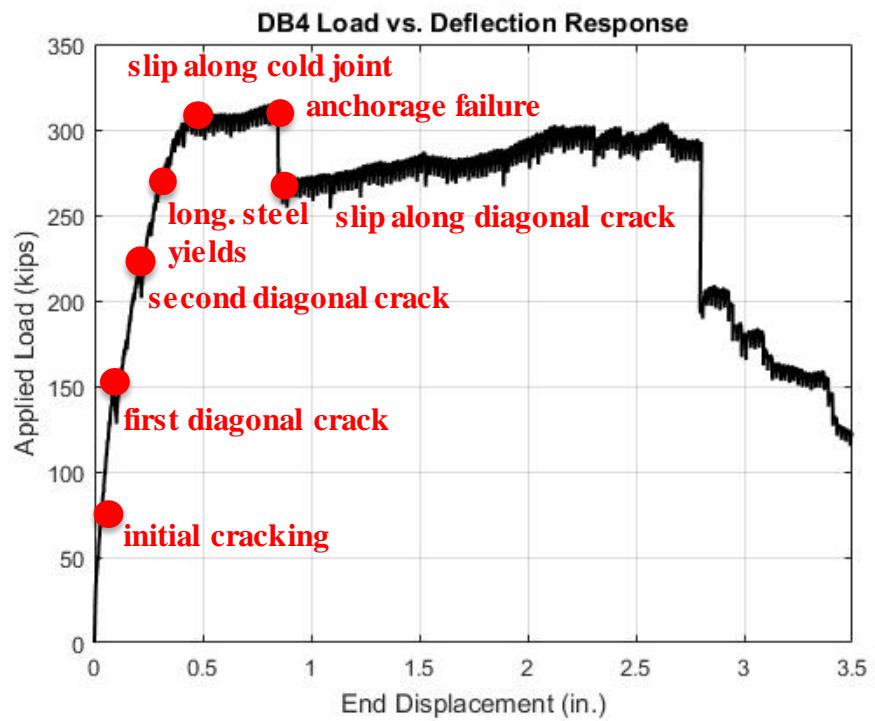
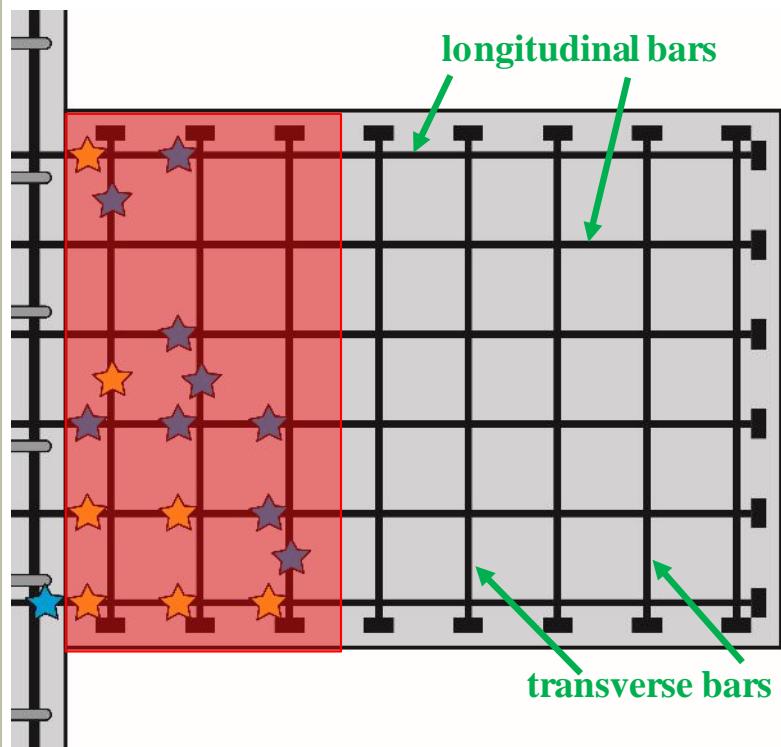
# DB4 ( $f'_c = 14960$ psi, $f_y = 133$ ksi)



- ★ active tension strain
- ★ tension yield (6.85 me)

Anchorage failure of first transverse bar after yielding to arrest diagonal cracks

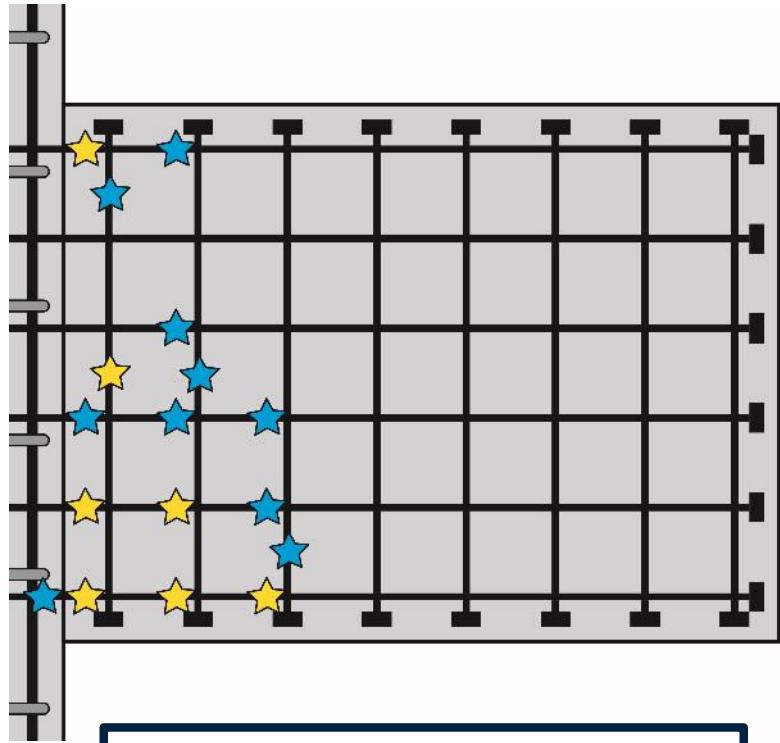
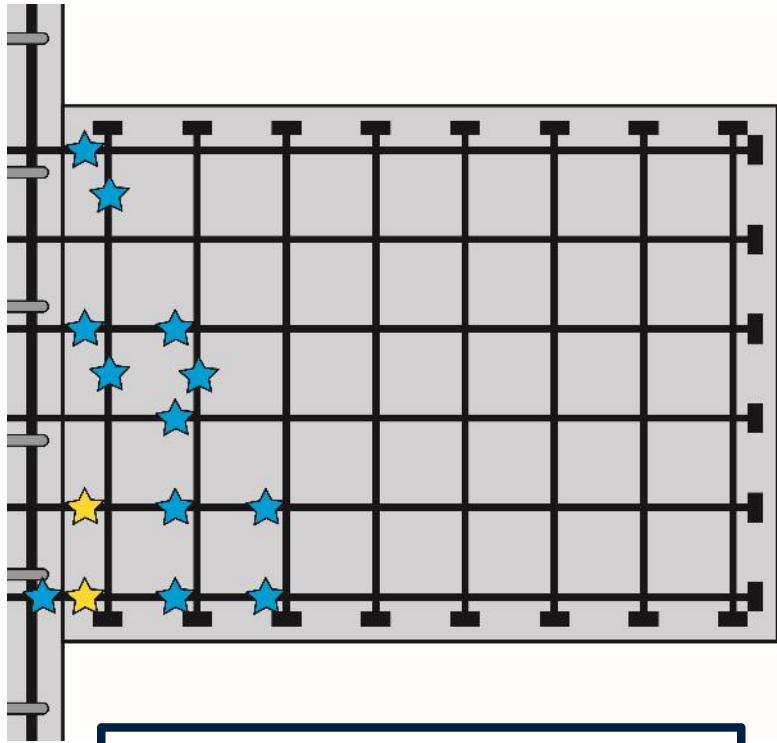
# DB4 ( $f'_c = 14960$ psi, $f_y = 133$ ksi)



★ active tension strain  
★ tension yield ( $6.85 \text{ me}$ )

Extensive concrete degradation

# DB2 and DB4 Strain Comparisons



★ active tension strain  
★ tension yield (6.85 me)

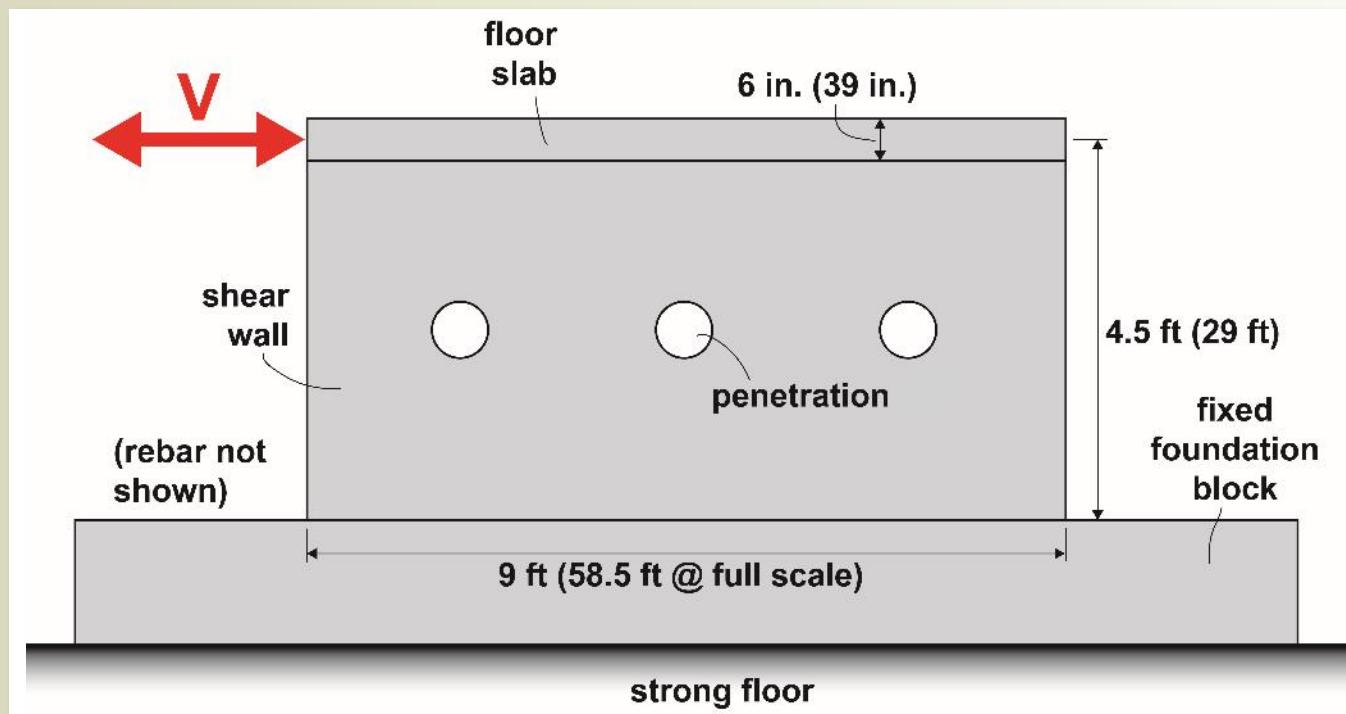
High-strength concrete able to better take advantage of  
higher yield strengths of reinforcement

# Summary of Deep Beam Tests

- 17.6% increase in peak shear strength when increasing  $f'_c$  from 6500 psi to 14960 psi
- Significant increase in ductility due to increase in  $f'_c$
- Pre-test analyses provided reasonable predictions for peak strength

# Future Reduced-Scale Shear Wall Tests

- 1:6.5 scale of “generic wall”
- $M/(VI_w) = 0.50$
- Tested under cyclic and accidental thermal loads
- High-strength steel and concrete



# Conclusions to Date

- High-strength steel more effective when combined with high-strength concrete
  - Numerically demonstrated (economics and peak strength)
  - Measured experimentally
- Greatest benefit for walls with low moment-to-shear ratios and large reinforcement ratios; typical of nuclear concrete shear walls
- Largest economic and structural benefits when using Grade 100 rebar together with 10 ksi concrete
- Project tasks on schedule



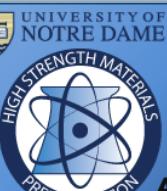
# Research Products

- Journal Paper (submitted):
  - “Effect of High-Strength Materials on Lateral Strength of Shear-Critical Reinforced Concrete Walls,” *ACI Structural Journal*.
- Presentations:
  - Presentation, 2015 Fall ACI Convention, Denver, CO.
  - Poster, 2015 Energy Week, Center for Sustainable Energy, University of Notre Dame, Notre Dame, IN.
  - Presentation, 2016 Fall ACI Convention, Philadelphia, PA.
  - Presentation, 2016 American Nuclear Society Winter Meeting and Nuclear Technology Expo, Las Vegas, NV.



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- Federal Point of Contact: Alison Hahn
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  - MMFX Steel
  - Dayton Superior Corp.
  - HRC, Inc.
  - Sika Corp. U.S.



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# Questions?

*<http://phsrc-nuclearwalls.nd.edu>*

