

Final Project

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Task 1

1.1 Statement of Problem

The purpose of this assignment was to investigate the natural forces contributing to littoral transport and beach erosion for re-nourishment purposes. Using 20 years of hind-cast wave height data, 20 and 50 year return period extremes for three dominant wave directions were to be determined. The wave height extrema were also to be determined and fitted with a Gumbel probability distribution function. The results were to be plotted and tabulated.

1.2 Hypotheses and Theories

The Wave Information Studies (WIS) program is an Army Corps of Engineers project which consistently monitors hours, long term wave climatologies along all US coastlines. Wave predictions can be made based off this hind cast data in order to determine future wave characteristics and their effects on the coastline. For this study, weather station 63079 from the WIS program was selected for its close proximity to the Narragansett Town Beach in order to conduct the investigations. A map containing the location of weather station 63079 (outlined in red) can be seen below in Figure ??

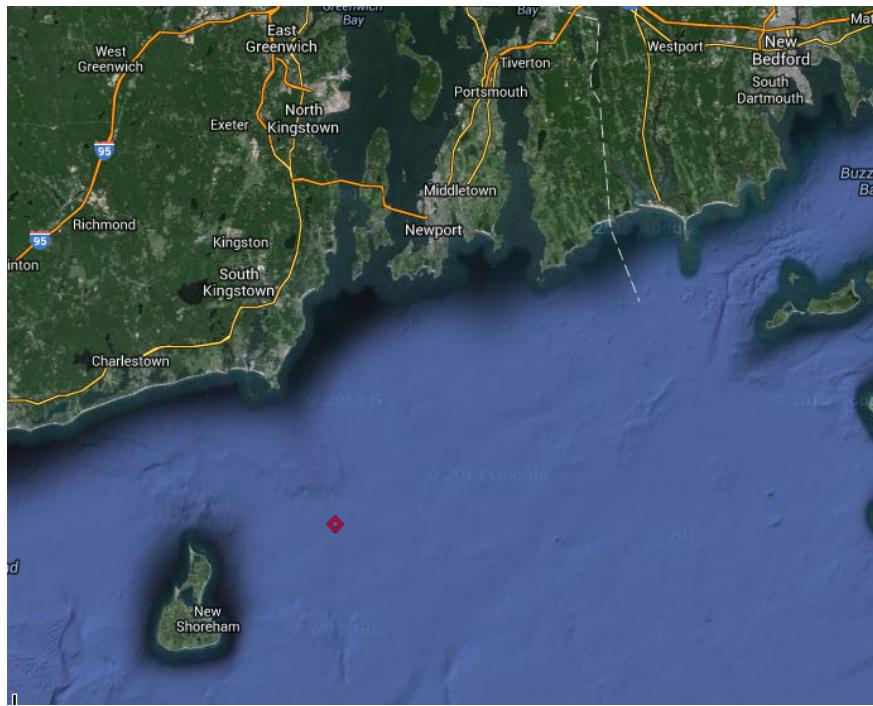


Figure 1.1: Location of WIS station 63079

A collection of functions were provided for the purpose of this study in order to extrapolate information from this weather station. This included the *wavedir.m* function which would extract the predominant wave directions from the hind-cast data. The *monthlyextrema.m* function could be used according to these dominant directions in order to determine the maximum wave height per each month in a 20 year hind-cast interval. These maximums could be processed in using a Gumbel distribution in the *extremeDist2new.m*.

A Gumbel distribution can be used in order to model the distribution of the maximums in a number of samples of various length. It can be used in order to model the distribution of maximum wave heights throughout a series of sample periods. A Gumbel distribution can also be used in order to make predictions of certain wave heights occurring in the future.

1.3 Solution of the Problem

20 years of hind-cast wave height data from the Rhode Island coastline was downloaded from the U.S. Army Core of Engineers Wave Information Studies (WIS) project. Due to the functions provided for this study being outdated, a parsing function was developed in order to convert the data from the WIS station in to a form which could be interpreted. This function would parse the date vector from the station data and arrange the array data in a way which interfaced with the provided functions.

Using the *wavedir.m* function, a rose plot consisting of 30 degree intervals was generated which displayed

the concentrations of wave direction present at the studied location. This rose can be seen in Figure ??.

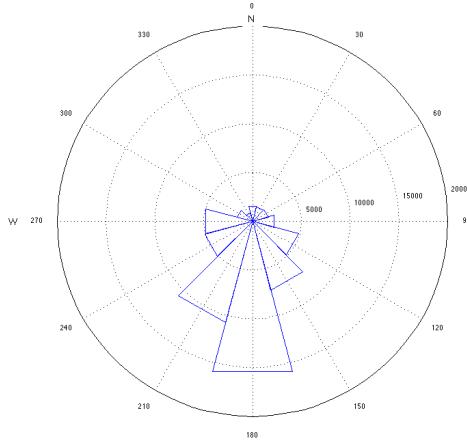


Figure 1.2: Wave Directions at Station 63079

From this rose plot it can be seen that the predominate wave directions were 150° , 180° , and 210° . The monthly maximums were then calculated for each of these three directions. This was done using the *monthlyExtrema_new.m*, which would output the extrema data for each month to a text file for each direction. An example of this output can be seen in Appendix SITE THIS.

Using the *extremeDist2_new.m* function it was possible to render Gumbel distributions based off the calculated monthly extrema data. The distributions for angles 150° , 180° , and 210° respectively can be seen below in Figures 1.3 through 1.8.

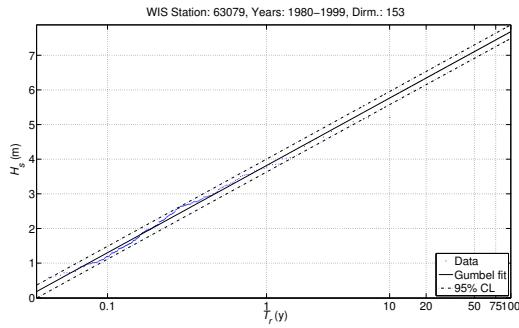


Figure 1.3: Gumbel distribution for Height at 150°

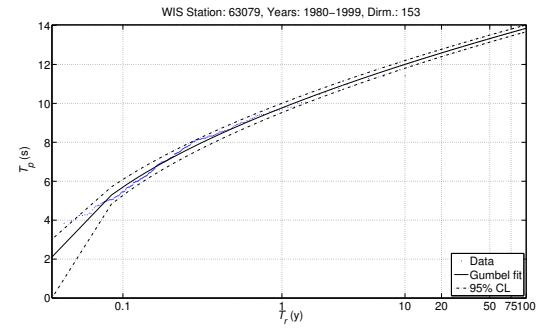
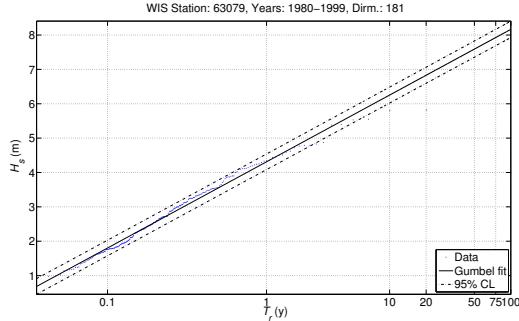
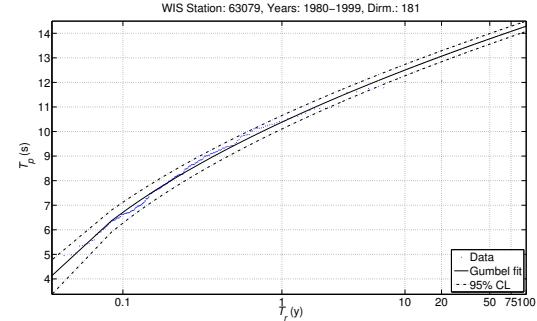
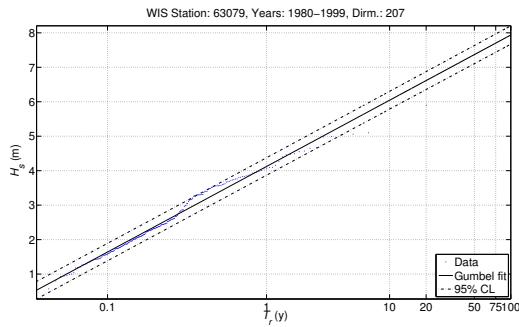
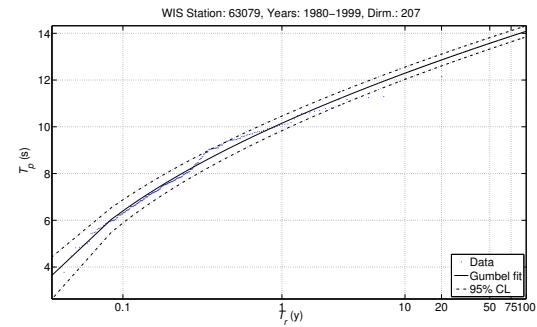


Figure 1.4: Gumbel distribution for Period at 150°

Figure 1.5: Gumbel distribution for Height at 180° Figure 1.6: Gumbel distribution for Period at 180° Figure 1.7: Gumbel distribution for Height at 210° Figure 1.8: Gumbel distribution for Period at 210°

The 20 - 100 year return parameters were also generated using the Gumbel distribution function. Tables containing this information for each dominant incident angle can be seen below in Tables 1.1 through 1.3.

Tr (y)	Hs (m)	Tp (s)	CL95 Hs(m)	CL95 Tp (s)
100.0	7.7	13.9	7.9	14.0
75.0	7.4	13.6	7.6	13.8
50.0	7.1	13.3	7.3	13.5
20.0	6.3	12.6	6.5	12.8

Table 1.1: 150 degree return characteristics

Tr (y)	Hs (m)	Tp (s)	CL95 Hs(m)	CL95 Tp (s)
100.0	8.2	14.3	8.4	14.5
75.0	7.9	14.1	8.2	14.3
50.0	7.6	13.8	7.8	14.0
20.0	6.8	13.1	7.1	13.3

Table 1.2: 180 degree return characteristics

Tr (y)	Hs (m)	Tp (s)	CL95 Hs(m)	CL95 Tp (s)
100.0	7.9	14.1	8.2	14.3
75.0	7.7	13.9	8.0	14.1
50.0	7.4	13.6	7.6	13.8
20.0	6.6	12.9	6.9	13.1

Table 1.3: 210 degree return characteristics

1.4 Conclusion

Task 2

2.1 Statement of Problem

Task 2 utilizes a wave ray tracing algorithm and local bathymetry information, provided for Narragansett Bay, in order to propagate storm waves determined from task 1. In order to complete this task, simulations are conducted for the 20 and 50 year storms along with creating rays that intersect along the shoreline of Narragansett Beach. For the 20 year storm simulations, use the ray spacing on the beach to calculate a refraction coefficient for each section of beach. Then, using bathymetry data from a chart near the beach, estimate the beach slope. This is used along with the deep water wave properties and refraction coefficient to estimate the shoaling coefficient, breaking depth, and breaker height.

2.2 Hypotheses and Theories

When waves propagate over a non uniform sea floor, they refract depending on how the sea floor depth changes along the wavefront. As water depth decreases wave fronts can focus or de-focus along the coastline. Seen in 2.1, the refraction coefficient can be determined with the distance between wave rays initially, and in the target location. A refraction coefficient less than one indicates de-focusing.

$$K_r = \sqrt{\frac{b_o}{b}} \quad (2.1)$$

In shallow water, waves slow down and experience an increase in wave height due to shoaling. The degree of wave refraction, and the shoaling coefficient dictate breaking wave height and water depth. Seen in 2.4, the ratio of phase speed and group celerity changes as water depth approaches the shallow water condition. With the deep water characteristics, and angle of incidence, breaker depth and wave height can be found iteratively.

$$[H]c = c_o * \tanh(kh); \quad (2.2)$$

$$c_g = \frac{c}{2} * \left(1 + \frac{2kh}{\sinh(2kh)} \right) \quad (2.3)$$

$$K_s = \sqrt{c/2c_g} \quad (2.4)$$

$$H_b = H_o K_{sb} K_{rb} = \kappa h_b \quad (2.5)$$

$$K_{sb} = F(L, h) \quad (2.6)$$

$$K_{rb} = F(L, h, \theta_o) \quad (2.7)$$

The programs supplied for the problem simulated waves propagating into the region of coastline surrounding Narragansett Beach. The programs required deep water wave characteristics, and latitude-longitude grid resolution.

2.3 Solution of the Problem

Using the supplied C functions and waveray.m, wave rays were propagated into Narragansett Beach under the conditions determined in Task 1. Twenty and fifty year predicted wave parameters were used to simulate the path lines extreme waves followed. Seen in CITE WAVEDIR FIGURE HERE, three predominant angles of incidence were determined from the supplied data. Waveray.m was non-intuitive, and didn't allow for significant manipulation of the rendered area without ruining the data. A consequence of this was that our breaking wave characteristics were evaluated manually, and were not based off the simulation data.

Seen below, the simulated wave rays from all three angles of incidence experienced de-focusing as they approached the beach. Significant focusing can be observed in 2.3 further south on the shoreline.

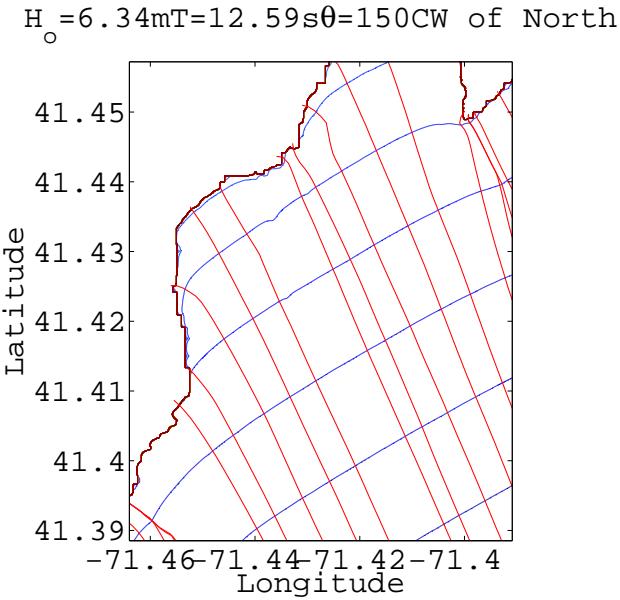


Figure 2.1: Wave Ray 20y Expected Wave Parameters at 150 deg

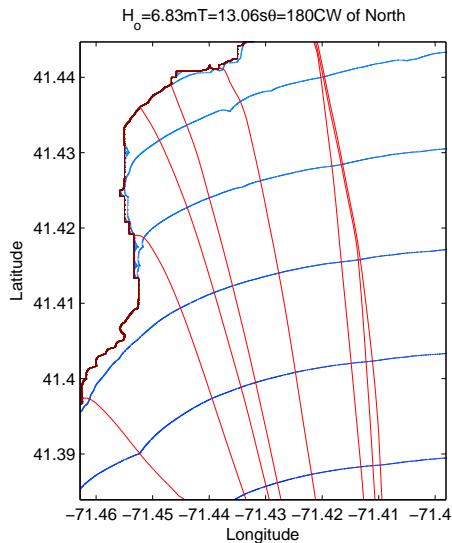


Figure 2.2: Wave Ray 20y Expected Wave Parameters at 180 deg

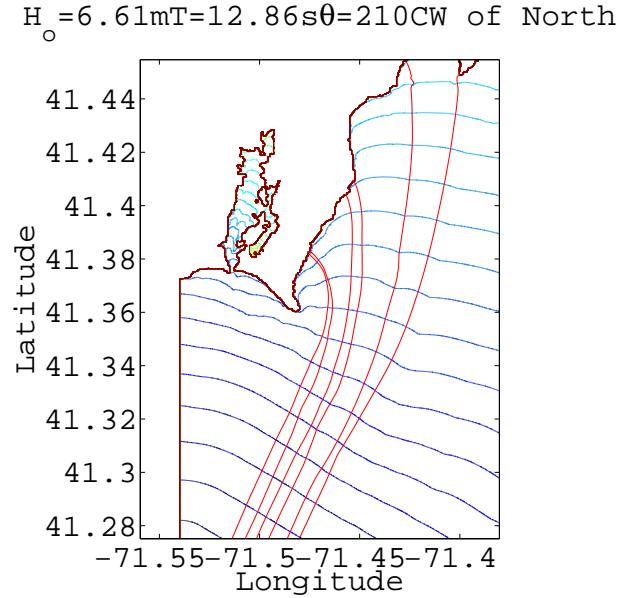


Figure 2.3: Wave Ray 20y Expected Wave Parameters at 210 deg

Seen below, the simulated wave rays for the 50 year extreme waves were very similar to their 20 year counterparts. Narragansett Beach de-focused the few rays that hit the beach, and the degree of de-focusing can be observed to increase as the angle of incidence approached 210° .

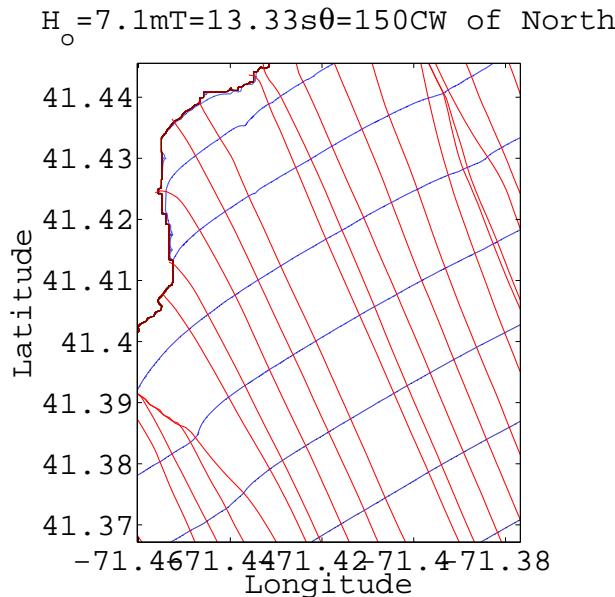


Figure 2.4: Wave Ray 50y Expected Wave Parameters at 150 deg

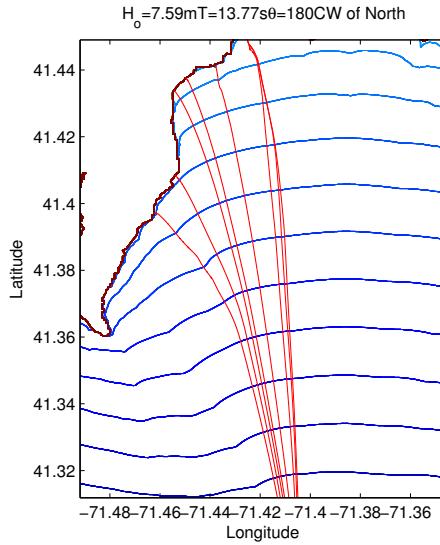


Figure 2.5: Wave Ray 50y Expected Wave Parameters at 180 deg

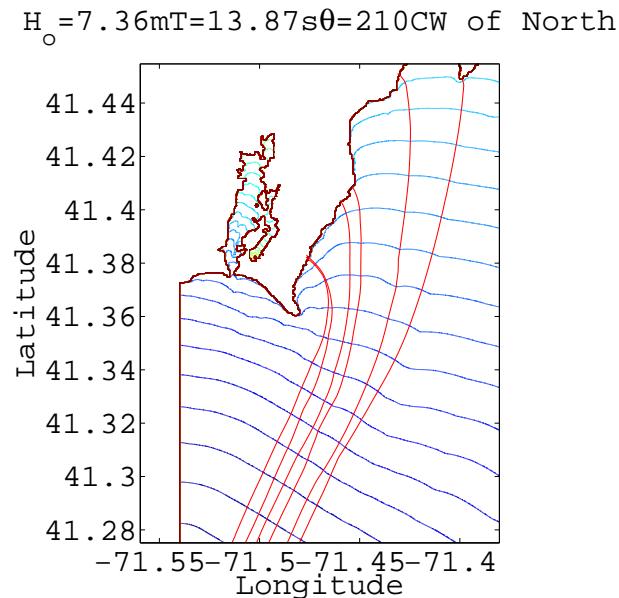


Figure 2.6: Wave Ray 50y Expected Wave Parameters at 210 deg

Sediment transport

Diffraction modeling would have influenced our 210° angle of incidence waves. Bloc

2.4 Conclusion

Appendices

.1 MATLAB Calculations