08 — Hazen-Williams & Equivalent Pipe Water Resources, CIVL318

Last revision on October 10, 2018

Introduction

We begin this module by looking at a variety of pipes that are common in installed systems or are increasingly common in new systems.



Cast Iron Pipe

Cast iron pipe showing bell end. It is coated with yellow plastic to minimize corrosion.





Cast Iron Pipe

Cast iron pipe used under 16th Avenue, Bowness. Note the connection guides for correct overlap at joints.





PVC Plastic Pipe Connector

The rubber O-ring visible inside the bell is used to seal pipe connections





PVC Plastic Pipe Connector

This "Blue Brute" pipe has a maximum operating pressure of $150\ psi$ or $1000\ kPa$





Asbestos Cement Pipe and Connector

Not used in new installations





Inside a water distribution plant





Welded steel header in a filter backwash system — Bearspaw Water Treatment Plant





Welded steel pipe fittings and connections





South Feeder from Bearspaw Plant





Banff Middle Springs Plant; three pumps are to satisfy fire flow requirements





 ${\sf Fittings-Benchlands},\ {\sf Canmore}$





New Hyprescon installation, 14th St SW.





River crossing, 14th St SW





Hyprescon steel pipe with cement lining and mortar coating





Mortar coating





Cement lining





Custom welded section



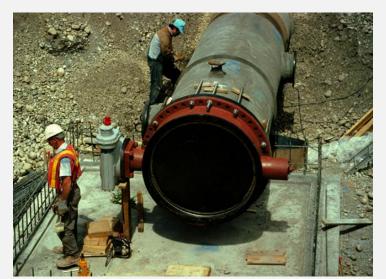


Custom welded section in place





Butterfly valve in Hyprescon pipe





Butterfly valve in Hyprescon pipe





Concrete inspection chamber for valve maintenance





Glenmore Water Treatment Plant





We have used Darcy's equation to analyze the flow of liquids in pipes, where the volume flow rate has been provided.

It is more complicated to apply Darcy's equation to calculate flow, requiring an iterative process to find the friction factor and associated headloss.

We will now continue our analysis using the Hazen-Williams equation to solve for head loss, pipe diameter or flow in water systems.



Darcy's equation, $h_L=f\cdot \frac{L}{D}\cdot \frac{v^2}{2g}$, works for all liquids and for laminar and turbulent flow conditions.

Darcy's equation is not directly suited to calculations to determine flow rates or diameters. (We have been provided with both for all the questions we have done using Darcy's equation.)

The Hazen-Williams equation is widely used for calculations relating to the flow of water.

It is also more practical for the analysis of flow through parallel pipe systems.



Hazen-Williams Equation

$$Q = 0.849 ACR^{0.63} s^{0.54}$$

where:

Q is the volume flow rate in m^3/s

A is the pipe area in m^2

C is a dimensionless roughness coefficient

R is the hydraulic radius of the pipe (R=D/4 for circular pipes)

s $\,$ is the hydraulic slope, the energy loss per length of pipe, h_L/L

We shall use the formula is a slightly different form...



Since we are only concerned with circular pipes, we shall use D, the diameter of the pipe, instead of A and R:

$$Q = 0.849ACR^{0.63}s^{0.54}$$

$$= 0.849\left(\frac{\pi D^2}{4}\right)C\left(\frac{D}{4}\right)^{0.63}s^{0.54}$$

$$= 0.27842CD^{2.63}s^{0.54}$$



Since we are only concerned with circular pipes, we shall use D, the diameter of the pipe, instead of A and R:

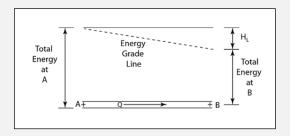
$$\begin{split} Q &= 0.849 A C R^{0.63} s^{0.54} \\ &= 0.849 \left(\frac{\pi D^2}{4}\right) C \left(\frac{D}{4}\right)^{0.63} s^{0.54} \\ &= 0.27842 \, C \, D^{2.63} s^{0.54} \end{split}$$

It is more useful to have Q in L/s and D in mm:

$$Q = 1000 \times 0.278 C \left(\frac{D}{1000}\right)^{2.63} s^{0.54}$$
$$= 0.0000035867 C D^{2.63} s^{0.54}$$
$$= \frac{C D^{2.63} s^{0.54}}{279000}$$



Energy Grade Line



Consider a horizontal pipe from A to B; energy is lost due to friction as water flows along the pipe such that the total energy at any point is shown by the dotted line – the energy grade line.

Total energy loss between A and B is h_L , in length L of pipe, so the slope of the energy grade line is h_L/L

We will replace the s term in the Hazen-Williams with h_L/L :





The Hazen-Williams is an empirical formula, derived from experimental results. The exponents 2.63 and 0.54 are to make observed results fit the formula more consistently.

Units are not consistent on sides of the equation.

$$Q = \frac{C D^{2.63} \left(\frac{h_L}{L}\right)^{0.54}}{279000}$$

C is a unitless coefficient so units are L/s on the left and ${\rm mm}^{2.63}$ on the right.



Approximate Roughness Coefficients, C

New Pipe	Design C
150	150
140	140
140	100
150	140
130	100
	110
	95
	85
	75
	150 140 140 150



Restrictions on Hazen-Williams

The Hazen-Williams equation works under the following conditions:

- Water flowing at approximately 10°C 15°C in fully pressurised pipe
- ► Flow velocity not in excess of 3.0 m/s
- ► Flow is in pipe with a diameter larger than 50 mm and smaller than 2.0 m

Use of the Hazen-Williams under other conditions from those specified above will result in some error.

(In practice, there is always error. ${\cal C}$ is an estimated value; pipe does not stay new for long!)



Alternative Form - Solving for Head Loss

$$Q = \frac{C D^{2.63} \left(\frac{h_L}{L}\right)^{0.54}}{279000}$$

$$\Rightarrow \left(\frac{h_L}{L}\right)^{0.54} = \frac{279000 Q}{C D^{2.63}}$$

$$\Rightarrow \frac{h_L}{L} = \left(\frac{279000 Q}{C D^{2.63}}\right)^{\frac{1}{0.54}}$$

$$\Rightarrow h_L = L \left(\frac{279000 Q}{C D^{2.63}}\right)^{1.852}$$



Alternative Form - Solving for Diameter

$$Q = \frac{C D^{2.63} \left(\frac{h_L}{L}\right)^{0.54}}{279000}$$

$$\Rightarrow D^{2.63} = \frac{279000 Q}{C \left(\frac{h_L}{L}\right)^{0.54}}$$

$$\Rightarrow D = \left(\frac{279000 Q}{C \left(\frac{h_L}{L}\right)^{0.54}}\right)^{\frac{1}{2.63}}$$

$$\Rightarrow D = \left(\frac{279000 Q}{C \left(\frac{h_L}{L}\right)^{0.54}}\right)^{0.386}$$



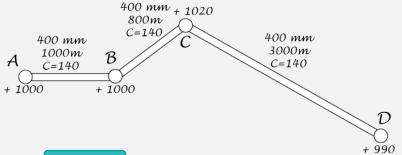
Hazen-Williams Various Forms

Hazen-Williams Formulæ

$$\begin{split} Q &= \frac{C \, D^{2.63} \left(\frac{h_L}{L}\right)^{0.54}}{279000} \\ h_L &= L \, \left(\frac{279000 \, Q}{C \, D^{2.63}}\right)^{1.852} \\ D &= \left(\frac{279000 \, Q}{C \, \left(\frac{h_L}{L}\right)^{0.54}}\right)^{0.3802} \end{split}$$

where Q is in L/s, D is in mm, h_L and L are in m.

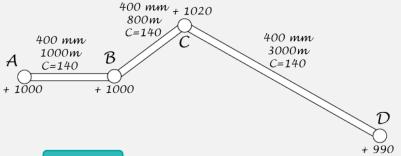




The pipes are cement-lined Hyprescon with a diameter of $400\,\mathrm{mm}$ and a roughness coefficient of C=140 and flow is 200 L/s. Elevations are as indicated.

Calculate the pressure at B given that the pressure at A is $700\,\mathrm{kPa}$.



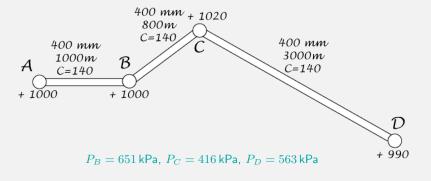


Exercise 1

The pipes are cement-lined Hyprescon with a diameter of $400\,\mathrm{mm}$ and a roughness coefficient of C=140 and flow is 200 L/s. Elevations are as indicated.

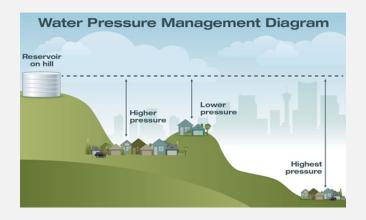
Calculate the pressures at C and D, given that the pressure at A is $700\,\mathrm{kPa}$.





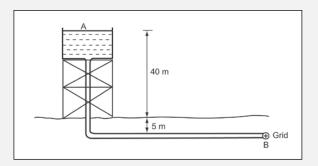
Pressures are lower at points farther away from the point of delivery (A is the point of delivery in this case) but the main pressure determinant is elevation, with higher elevations in a distribution system having lower pressures.





http://www.calgary.ca/UEP/Water/Pages/Drinking-water/Water-quality/Water-Pressure.aspx





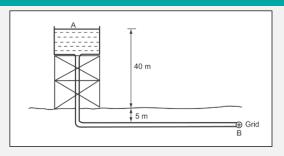
Water flows from a storage tank through a welded steel pipe that is $1200\,\mathrm{m}$ long and $350\,\mathrm{mm}$ in diameter, entering a distribution grid at point B. Assume C=100.

Determine:

- 1 The pressure at B when the flow is $150\,\mathrm{L/s}$
- ho The maximum flow rate into the grid when the minimum allowable pressure at B is $400\,\mathrm{kPa}$.



Overview...



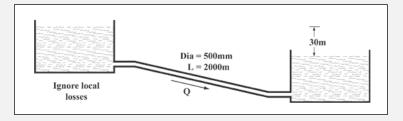
Velocity $v_B=1.5591\,\mathrm{m/s}$ in part (1) and velocity head was small (0.12389 m).

Omitting the velocity head term altogether would have changed P_B by about 0.3% of the calculated $317\,\mathrm{kPa}$ value. This is well within the error to be expected in estimation of the C-value.

In water distribution systems, velocities are normally designed to be less than $2\,\mathrm{m/s}$ since faster flow causes excessive head loss and may cause noise and vibration.



It is generally acceptable to ignore velocity head in cases where diameter or flow are unknown; this greatly simplifies some calculations.



Exercise 2

Water flows from one reservoir down to another, through a $500\,\mathrm{mm}$ diameter pipe that is $2000\,\mathrm{m}$ in length. The difference in elevation between the surfaces of the two reservoirs is $30\,\mathrm{m}$.

Determine:

- I The flow with high density polyethylene pipe (HDPE) with C=140
- **2** The flow with welded steel with C=100
- \blacksquare The diameter of HDPE pipe required for a flow of $1200\,\mathrm{L/s}$

Neglect minor losses.



Hazen-Williams and Minor Losses

In our earlier study of minor losses, we calculated minor head losses through valves using

$$h_L = k \cdot \frac{v^2}{2g}$$

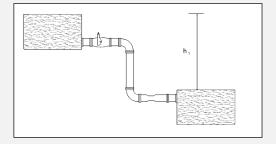
which required knowledge of both Q and D to calculate v.

We defined the 'equivalent length ratio,' L_e/D , of a valve or fitting where the equivalent length, L_e , is defined as the length of pipe that would produce the same (frictional) loss as the valve or fitting (minor) losses.

Thus, to determine the losses due to a fully open globe valve (with $L_e/D=340$), we can instead calculate the frictional losses due to length L_e of pipe.

(Note that we will still need the diameter D of the pipe)





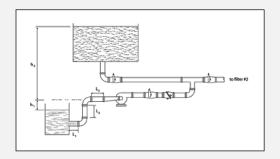
In a water treatment plant, water flows from a filter down to a clear well through the pipe system shown. The pipe is welded steel with a diameter of $300 \,\mathrm{mm}$ and roughness coefficient C=130. The total length of pipe is $50 \,\mathrm{m}$. Elevation difference h_1 between the tanks is $5.0 \,\mathrm{m}$.

Equivalent length ratios, L_e/D , are:

Entrance and exit losses: 50 Butterfly valve: 35 Venturi meter: Large radius elbows: 25 100

Determine the flow through the system.





In a water treatment plant, backwash water is pumped from the clear well through the pipe system shown to the filter. The required backwash flow is $10 \, \text{L/s}$ per square meter of filter area (the filter dimensions are $10 \,\mathrm{m}$ by $15 \,\mathrm{m}$. The inlet pipe is made of welded steel (C = 130), has a diameter of $1000 \,\mathrm{mm}$ and a total length $(L_1 + L_2 + L_3)$ of $10 \,\mathrm{m}$.

The outlet pipe, from the pump to the filter, is also welded steel, has a diameter of $700 \, \text{mm}$ and a length of $70 \, \text{m}$.

The two elevation differences are $h_1 = 2 \,\mathrm{m}$ and $h_2 = 10 \,\mathrm{m}$.

Cont'd...

Equivalent length ratios, L_e/D , are:

Entrance: 10 Elbow (inlet): 25

Eccentric Reducer: 2 Butterfly Valve: 40 Check Valve: 120 Elbow (outlet): 35

Tee Connection (through): 60



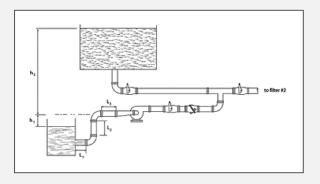
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Determine:

- 1 The head losses on the inlet side (clear well to pump)
- The head losses on the outlet side (pump to filter)

Neglect exit losses into the filter.



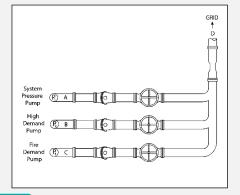


Exercise 3

This exercise is a continuation of the previous example. Determine:

- The head added by the pump
- The pressure at the pump outlet





The pumps and piping system are used to supply a municipal grid. Pump P_1 (P_2 and P_3 are not running at this time) runs continuously and maintains the basic pressure in the distribution grid beyond point D. The elevations are the same at the pump and the discharge point D.

The outlet pipe, from the pump to point D, is welded steel (C=130) with a diameter of $200\,\mathrm{mm}$ and a total length between fittings of $10\,\mathrm{m}$.

The minimum pressure required at D is $500\,\mathrm{kPa}$ for a design flow of $150\,\mathrm{L/s}$.



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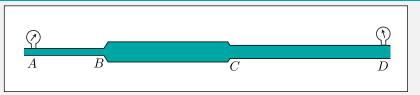
Check Valve: 120 Gate Valve: 15
Tee Connection (through): 60 Venturi Meter: 100

There is no flow from pumps P_2 and P_3 . Determine:

- lacktriangledown the head losses between A and D
- f 2 the pressure at A required for the required pressure and flow at D



Determining Flow Through Pipes in Series



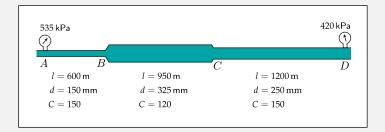
We have used the Hazen-Williams Equation to find flow through a single pipe but how can we find the flow through a system of pipes in series (such as pipes AB, BC and CD shown above)?

Note that, **for pipes in series**, the volume flow rate is constant through each pipe and that the head loss is cumulative.

Pipes in Series $Q_{AB} = Q_{BC} = Q_{CD}$ $h_{AD} = h_{AB} + h_{BC} + h_{CD}$



These are **important** properties of pipes in series!



Determine Q, the volume flow rate from A to D, through the system shown. Ignore minor losses and assume that A and D are at the same elevation.



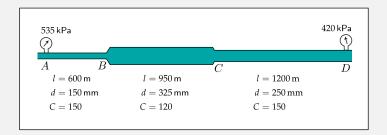
Equivalent Pipes

- Two pipes and/or pipe systems are hydraulically equivalent if each pipe or system has the same headloss at the same flow.
- An imaginary, or virtual, pipe that is hydraulically equivalent to a real pipe or system is known as an equivalent pipe.
- An equivalent pipe is determined by three properties (length, diameter, resistance coefficient). By setting two of these to convenient values (e.g., length $L=1000\,\mathrm{m}$ and resistance coefficient C=100), the diameter of the equivalent pipe is uniquely defined.
- An equivalent pipe does not physically exist; it is a tool to simplify pipe network analysis.



- Determine the diameter of a pipe with length $L=1000\,\mathrm{m}$ and resistance coefficient C=100 that is equivalent to $1685\,\mathrm{m}$ of new Schedule $40\,18$ -in steel pipe $(d=428.7\,\mathrm{mm}, C=130).$
- Verify that the equivalent pipe is hydraulically equivalent at flows of





Use the equivalent pipe technique to determine Q, the volume flow rate from A to D, through the system shown. Ignore minor losses and assume that A and D are at the same elevation.

