

## CE 3305 – Fluid Mechanics Exam 3

# Purpose

Demonstrate ability to apply fluid mechanics and problem solving principles covering topics such as: Dimensional analysis and similitude; turbulent flow in closed conduits; pump system performance.

### Instructions

- 1. Choose any **four (4)** of the six (6) problems. You do **not** need to complete all six problems.
- 2. Put your name on each sheet you submit.
- 3. Use additional sheets as needed.
- 4. Begin each problem on a separate page. Ok to disassemble the exam to keep pages in order.
- 5. Do not write on the back of sheets (I don't even look)
- 6. Use the **problem solving protocol** in the class notes. The discussion section can simply be the word "discussion"
- 7. Label and/or underline answers, be sure to include units.

# Allowed Resources

- 1. Your notes
- 2. Your textbook
- 3. The mighty Internet with following proviso:

You may not communicate with other people during the exam

- 1. A smooth pipe designed to carry crude oil is to be modeled with a smooth pipe 0.75 inches in diameter carrying water ( $T = 60^{\circ}F$ ). The protoype properties are:
  - D = 47 inches
  - $\rho = 1.75 \text{ slugs}/ft^3$
  - $\mu = 4 \times 10^{-4} \frac{lbf \ s}{ft^2}$

#### Determine:

(a) The mean velocity of the water in the model to ensure dynamically similar conditions, if the mean velocity in the prototype is to be 2 ft/s,?

MODEL Pm = 1.94 sluss/fx3  $N = 2.36 \times 0^{-5} 16.5$ 

GOVERNING PRINCIPLES (+1

SULUTION Rep = MN Dp Rem = pm Vm Dm

Vm (+ SET EGAL; SOLVE FOR UNKNOWN  $\frac{\mathcal{L}_{P} \mathcal{V}_{P} \mathcal{D}_{P}}{\mathcal{N}_{o}} = \frac{\mathcal{L}_{m} \mathcal{V}_{m} \mathcal{D}_{m}}{\mathcal{N}_{m}}$ Nm yp Vp Dp = Vm (+1) Np pm Dn JUBSTITUTE NUMERICAL VALUES (4.10-4)(1.94)

DIRECT APPLICATION OF RE SIMILARITY & ARITHMETIC 2. Flow around a bridge pier is studied using a  $\frac{1}{12}$  scale model. The approach velocity in the model is  $0.9 \frac{m}{s}$  and at this speed the standing wave at the bridge pier nose is measured to be 2.5 cm in height (above the undisturbed water surface).

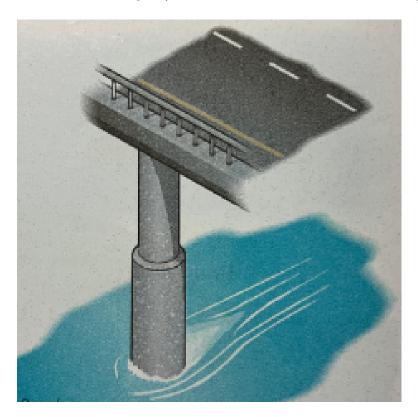


Figure 1:

#### Determine:

- (a) The approach velocity in the prototype using Froude number matching  $(Fr = \frac{V}{\sqrt{aL}})$ .
- (b) The wave height in the prototype.

SKETCH

Wp
->Vp
KNOWN
1:12 GEOMETRIC SCALE Vm = 0.9 m/s
$W_{\rm M} = 0.025  \text{m}$
UNKNOWN
$V_P$ , $W_P$
GOVERNING PRINCIPLES
Fr SIMILARITY
SOLUTION
Tglp = Vm Lp = 12 Lm 1 (GIVEN)
$V_{p} = \frac{V_{m} \sqrt{L_{p}}}{\sqrt{L_{m}}} = \frac{V_{m} \sqrt{12}}{\sqrt{1}} = \frac{0.9 \sqrt{12}}{\sqrt{1}} = \frac{3.1 \text{ m}}{\sqrt{1}}$
$W_{p} = 12 W_{m} = 0.3 m (30 cm)$

# DISCUSSION

DIRECT APPLICATION
DEFN. Fr MATCHING &
GEMETRIC SCAUNG



- 3. A prototype littoral frigate-class vessel has a length of 421 ft and is designed to travel on water at 75 ft/s<sup>1</sup>. A 4.21-ft-long model is tested in oil to maintain the same Froude number  $(Fr = \frac{V}{\sqrt{gL}})$  and Reynolds number  $(Re = \frac{\rho VD}{\mu})$  as the prototype. Determine:
  - (a) The geometric scaling factor
  - (b) The speed of model  $(V_m)$
  - (c) The required kinematic viscosity of oil  $(\mathcal{N}_m)$ .  $\mathcal{N}_m = \mathcal{N}_m$

SKETCH

2 42/ ->
2 75 FH/s

1 = 1.66.10 42/S

PROTOTYPE

2011 = ? MODEL

KNOWN

PROTOTYPE VELOCITY
PROTOTYPE LIQUID

UNKNOWN
GEOMETRIC SCALE
Vmodel

1

 $<sup>^1\</sup>mathrm{Roughly}$  the specifications of the USS Independence (LCS-2) Littoral Combat Ship

For scaling desired

Re scaling desired

Experimental controls of Voil & Vineckel

SOLUTION

GEOMETRIC SCAUNG:

$$\frac{L_{\text{partitype}}}{L_{\text{partitype}}} = \frac{4.21}{421} = \frac{1}{100}$$

SO MODEL IS 1:100 GEOMETRIC

$$Re_p = \frac{V_p L_p}{v_p}$$
 $Re_m = \frac{V_m L_m}{v_m}$ 

WANT Rep = Rem (PROSIEM STATEMENT)

$$F_p = \frac{V_p}{\sqrt{g}L_p}$$
  $F_m = \frac{V_m}{\sqrt{g}L_m}$ 

WE WANT

$$F_{r_p} = F_{r_m} \Rightarrow \frac{V_p}{f_p L_p} = \frac{V_m}{f_p L_m}$$

$$\frac{V_{p}L_{p}}{V_{p}} = \frac{V_{m}L_{m}}{V_{m}}$$
 () (FRUM PRIOR)

$$\mathcal{O} \quad \mathcal{V}_{m} = \frac{V_{m} L_{m} \mathcal{V}_{p}}{V_{p} L_{p}}$$

SUBSITIUIE NUMBERS

$$V_{m} = \frac{\sqrt{4.21} + 75 \text{ H/s}}{\sqrt{421}} = 7.5 \text{ H/s}$$

$$V_{on} = (7.5 \text{ H/s}) (4.21 \text{ H}) (1.66.10^{-5} \text{ H/s})$$

$$(75 \text{ H/s}) (421 \text{ H})$$

$$= 1.66 \cdot 10^{-8} \text{ H/s}$$
[No such liquid substance on Earth)

DISCUSSION

READ PP. 432-433 FOR SUGGESTED METHOD METHOD FOR SUCH EXPERIMENT. 4. In the design of a lift station, a bypass line is often installed parallel to the pump so some liquid recirculates as shown on Figure 2. The bypass valve then controls the flow rate in the system.

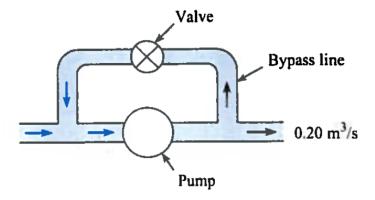


Figure 2:

The pump performance function is

$$h_p = 100 - 100Q$$

where  $h_p$  is in meters, and Q is in  $\frac{m^3}{sec}$ . The bypass line is 10 cm in diameter. The valve setting produces a fitting loss coefficient of K=0.2 and this valve loss is the only meaningful head loss at the lift station. For a discharge leaving the lift station of  $0.2 \frac{m^5}{sec}$ 

#### Determine:

- (a) The discharge through the pump
- (b) The discharge through the bypass line



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JKETCH b → Pour= 0.20m3/sec hp = 100 - 100 Q Qin m3/sec KNOWN: Qut = 0.2 m3/s (+1) DB1985 = 0.10 m ABYPASS = TT (0.10)2 = 0.6078 m2 NOTE LINEAR hp = 100-100 Q Ky= 0.2 9 RUMP GOVERNING PRINCIPLES CONTINUDITY: Par = QIH = PP - QBY (had loss in bypass) ENERGY  $h_B - h_A = K \frac{V_{BY}}{Zq}$ (added head in pump) NA + NP = NB

SOLUTION

SUBSTITUTE DUMP CURVE INTO; SUBSTITUTE &

1NTO (1)

$$V_{A}(100-100Q) = V_{A} = \frac{KV_{BV}^{2}}{23}$$

$$V_{BY} = \frac{Q_{OUT} - Q_{P}}{A_{BY}} = \frac{0.2 - Q_{P}}{0.0078}$$

$$100-100 P_P = \frac{K}{29} \left( \frac{0.2-P_P}{0.0078} \right)^2$$

$$100 - 100 P_P = \frac{0.2}{2(9.8)} \left( \frac{0.2 - P_P}{0.0078} \right)^2$$

$$100 - 100 P_p = 0.0102 \left( 0.2 - P_p \right)^{\frac{1}{2}}$$

$$1-Q_{p} = 1.6765(0.2 - Q_{p})^{2}$$

+2 PALGEBRA + KRITHMETIC

Qp=1-1.6765 (0.2 - Qp 1-1.6765(0.2-\$p) Qp 0.2 1.0 0,683 0.983 0:3 -0.721 0,178 0,9 0,131 0,731 0,6 -0,203 0,527 0.731 0.028 0.636 6,665 -0.014 0.645 0,66 -0.007 0,657 0.649 8000,0 0.6545 0.6536 Qp = 0.6545 m3/sex +2 = 0.4545 m<sup>3</sup>/ POUT DISCUSSION BYPASS CHECK HEAD LOSS  $h_{\text{Loss}} = \frac{6.2 \left(0.4545\right)^2}{2(9.8) 0.0078} = 34.6 \text{m}$ 

CHECK ADDED HEAD  $h_p = 100 - 100 Q + 1)$  = 100 - 100 (0.6545) = 34.55m

ha+34.6-34.55 HEADS

5. The figure below is a schematic of a pumped-storage system. Water is pumped from the lower reservoir in a pipeline with the following characteristics: D = 300 mm, L = 150 m, f = 0.029,  $\Sigma K = 5.0$ . The radial-flow pump characteristic curve for a single-stage pump is  $H_p = 22.9 + 10.7Q - 109Q^2$  where  $H_p$  is in meters and Q is in  $\frac{m^3}{sec}$ .

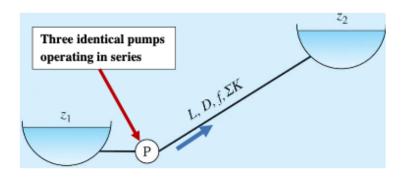
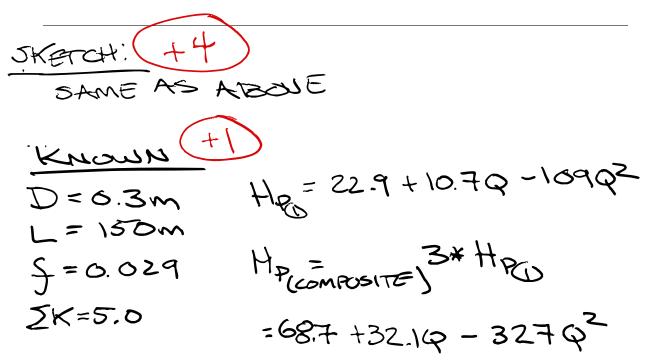


Figure 3:

#### Determine:

- (a) Plot the lift station composite pump curve, and the system curve on the same graph.
- (b) The discharge  $Q_D$  and pump added head  $H_D$  if the lift  $(z_2 z_1)$  is 40 m using a three-stage pump (treat as three identical pumps operating in series).



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UNKNOWN +3 PUMP CURVE (PLOT) SYSTEM CURVE (PLOT) Po Ho for 22-21=40m PRINCIPLES (+3) CONTINUNITY ENERGY; PIPE WSS + MINOR RUMPS IN SERLES Ogage

Ogage

Ogage

Ogage

Oyage

The thirty of the thirt Z, + Hp = Zz + HL Hp= (22-21) + H(+1) GIVEN AS 40 M Hp = AO + HL (+1) Now LOSSES HL = 8FLQ= + 2K 8Q + 1)

$$H_{L} = 8f_{L}Q^{2} + 2K \frac{8Q^{2}}{\Pi^{2}Q}Q^{4} + 1$$

$$H_{P} = 68.7 + 32.1Q - 327Q^{2}$$

$$H_{P} = 40 + H_{L} \qquad H_{P}$$

$$Q \quad | 40 + H_{L} | H_{P}$$

$$Q \quad | 40 + H_{L} | H_{P}$$

$$Q \quad | 40 + H_{L} | H_{P}$$

$$Q \quad | 41.9Q \quad | 68.6 \quad | -26.6 \quad | -26.6 \quad | -14.07 \quad | 51.90 \quad | \\ | -0.2 \quad | 47.96 \quad | 62.04 \quad | -14.07 \quad | 51.90 \quad | \\ | -0.3 \quad | 57.9 \quad | 48.9 \quad | 9.02 \quad | 140.07 \quad | 140.07 \quad | \\ | -0.2 \quad | 49.63 \quad | 59.9 \quad | -10.2Q \quad | \\ | -0.24 \quad | 51.9 \quad | 57.5 \quad | -6.0Q \quad | \\ | -1.48 \quad | 53.96 \quad | 54.94 \quad | -1.48 \quad | \\ | -0.17 \quad | 54.51 \quad | 53.52 \quad | 0.787 \quad | \\ | -0.2644 \quad | 54.13 \quad | 54.04 \quad | 0.09 \quad | \\ | -0.2644 \quad | 54.13 \quad | 54.04 \quad | 0.09 \quad | \\ | -0.2644 \quad | 54.13 \quad | 54.04 \quad | 0.09 \quad | \\ | -0.2644 \quad | 54.13 \quad | 54.04 \quad | 0.09 \quad | \\ | -0.2644 \quad | 54.13 \quad | 54.04 \quad | 0.09 \quad | \\ | -0.2644 \quad | 54.13 \quad | 54.04 \quad | 0.09 \quad | \\ | -0.2644 \quad | 54.13 \quad | 54.04 \quad | 0.09 \quad | \\ | -0.2644 \quad | 54.13 \quad | 54.04 \quad | 0.09 \quad | \\ | -0.2644 \quad | 54.13 \quad | 54.04 \quad | 0.09 \quad | \\ | -0.2644 \quad | 54.13 \quad | 54.04 \quad | 0.09 \quad | \\ | -0.2644 \quad | 54.13 \quad | 54.04 \quad | 0.09 \quad | \\ | -0.2644 \quad | 54.13 \quad | 54.04 \quad | 0.09 \quad | \\ | -0.2644 \quad | 54.13 \quad | 54.04 \quad | 0.09 \quad | \\ | -0.2644 \quad | 54.13 \quad | 0.09 \quad | \\ | -0.2644 \quad | 0.09 \quad | 0.09 \quad | \\ | -0.2644 \quad | 0.09 \quad | \\ | -0.2644$$

75 60 53 50 HET STATION PLMN CURVE 45 SYSTEM CURVE CH\_+40 30 + 15 0.05 0.1 0.15 0.2 0.25 0.3 Q(m3/s) IF HAVE PLOTITING SOFTWARE MAKE PRETTIER.

DILUSSION

BRUTE FORCE GIVES INFO FOR PLOTS. (+1)

6. The figure below is a schematic of a parallel pipe system. Flow occurs from A to B as shown. To augment the flow a pump is located between C and C'. The network is on a plane (flat) surface; all the junction elevations are the same. The pipes are commercial steel.

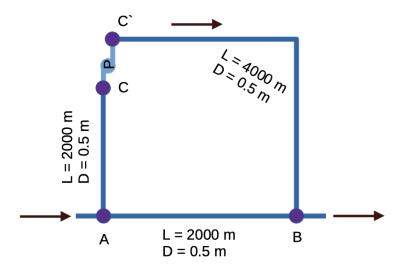


Figure 4:

The pump characteristic curve is shown below:

Total discharge is 0.60  $\frac{m^3}{sec}$ 

#### Determine:

- (a) The division of flow between pipes A-B and A-C-B
- (b) The head loss in pipe A-B
- (c) The head loss in pipe A-C
- (d) The head loss in pipe C'-B
- (e) The pump operating conditions.

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SKETCH 4000m long U.5m diameter 2000mlang O.Smdan > 0.6 m3/sec 2000m long 0.5m dlamater KNOWN DIPE LENGTHS DIAMETER MATERIAL RUMP CURVE (GIVEN) PUMP CURUE TOTAL Q 40. UNKNOWN + GAB, PACI, ShaB & hac, She'B PRINCIPLES/+ .2 O CONTINUNITY ENERGY HOAD LOSS MODEL SOLUTION = 8 f L QAB (head luss AB) (+/ 8 f L PACB (head loss C'B) +1

$$\Delta h_{AC} = \frac{8fL}{\pi^2 g} \frac{Q_{ACB}^2}{D^5} \text{ (head loss AC)} + 1$$

$$Q_{AB} + Q_{ACB} = Q_{TOTAL} \text{ (continuity whole }) + 1$$

$$= FROM \text{ pump curve}$$

$$\Delta h_{AB} = \Delta h_{AC} - h_p + \Delta h_{C'B} \text{ (Expressed as loss)}$$

$$Will need f from Re & Moocky Chut$$

$$\mathcal{E}_{Shool} = 0.045$$

$$OR \text{ Use tabulated factor factor for } SCH 40 \text{ Steel Pipe } f = 0.012 + 1$$

$$\text{Use & check Re later an.}$$

PAB	PACB	ShaB	Shac+shors-shas	+2
0.1 0.2 0.3 0.4	0.5 0.4 0.3 0.2	0.6 2.53 5.71 10.15	46 95	Than Plot Pump Curve

PAB	PACB	ShAB	Shac+shc'B-shAB	
0.3	0.3	0.6 2.53 5.71	46.95 27.921 11.42	Then Plot on Pump Curve
0.4	<b>9. 2</b> 60 -	10.15	-2.5	
	Total Head, m - 05 - 05	-		- PUMP PATH SY:TEM CURVE -hp 2/6

 $Q_{ACB} = 0.330 \text{ m} \%$  Figure 5:  $Q_{AB} = 0.27 \text{ m} \%$  Scoring NEXT PAGE

Discharge, m³/s

PAB PACB ShAB ShAC+ShEB-ShAB 1 46.95 Than 0.5 0.6 0.4 27.921 2.53 11.42 5.71 Pump Curve 10.15 -2.5 4.626 16.1055 Check 0.27 0.33 Check nydraulics Path A >> B, let h\_ = 100 ha - sha = hB 100-4.626 = 95.374 check Optronde Path A>C->C-B ha-shae +hp-shes = hB 100-6.9105+16.105=13.821 = 95.374 : QAR = 0.27 m3/5 hp = 16.1055 PACB = 0.33 m3/s hAB = 4.626 M hAc = 6.9105 m = 13.82/mhc's

DISCUSSION (+1



DIRECT APPLICATION PIPE HYDRAULICS; BUT HAVE TO LOOK UP hp FROM CURVE, PLUT LOSS EQN ONTO CHART TO GET CLOSE

\* CHECK 
$$f$$
 $V = \frac{Q}{4(0.33)} = 1.68 \, \text{m/sec}$ 
 $T = \frac{4(0.33)}{7(0.5)^2} = 1.68 \, \text{m/sec}$ 
 $T = \frac{4(0.33)}{7(0.5)^2} = 1.68 \, \text{m/sec}$ 

$$Re = \frac{\rho VD}{\nu} = \frac{(998 \text{kg/m}^3)(1.68)(0.5)}{0.001 (N^{-5}/\text{m}^2)}$$

2838320

$$\frac{\mathcal{E}}{D} = \frac{0.045 \cdot 10^{-3}}{0.5} = 4.10^{-5}$$

$$f = 0.0135$$

50 quess using 0.012 is good

838320