

CE 3354 Engineering Hydrology

Lecture 23: Transient (Time-Varying)
Well Hydraulics; Superposition

Outline

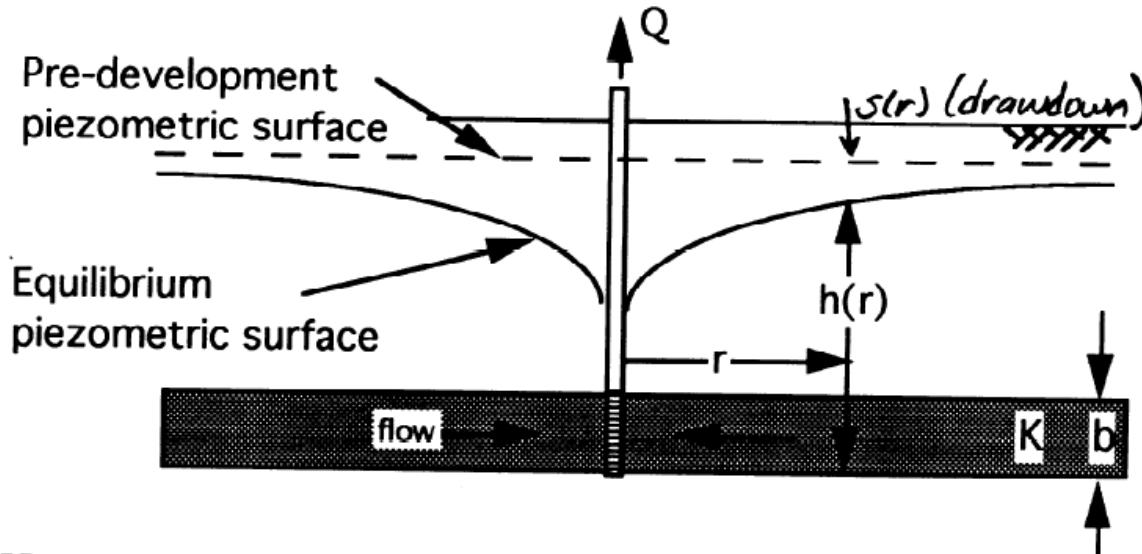
- ✿ Steady flow to well
 - ✿ Confined
 - ✿ Unconfined
- ✿ Superposition to represent
 - ✿ Multiple wells
 - ✿ Aquifer Boundaries

Outline

- ✿ Unsteady flow to a well
 - ✿ Confined (Theis Solution)
- ✿ Superposition
 - ✿ Multiple wells
 - ✿ Aquifer Boundaries
- ✿ Convolution
 - ✿ Time-varying pumping rates
- ✿ Leaky (Hantush Solution)
- ✿ Spreadsheets

Confined Aquifer

Theim solution



Homogeneous, isotropic, confined aquifer. Flow in radial direction only.
Steady state. No internal sources/sinks.

$$h(r) = h_o + \frac{Q}{2\pi T} \ln\left(\frac{r}{R}\right)$$

$$s(r) = h_o - h(r) = -\frac{Q}{2\pi T} \ln\left(\frac{r}{R}\right) = \frac{Q}{2\pi T} \ln\left(\frac{R}{r}\right)$$

Confined Aquifer

- ✿ Theim solution
 - ✿ Derivation in attached reading

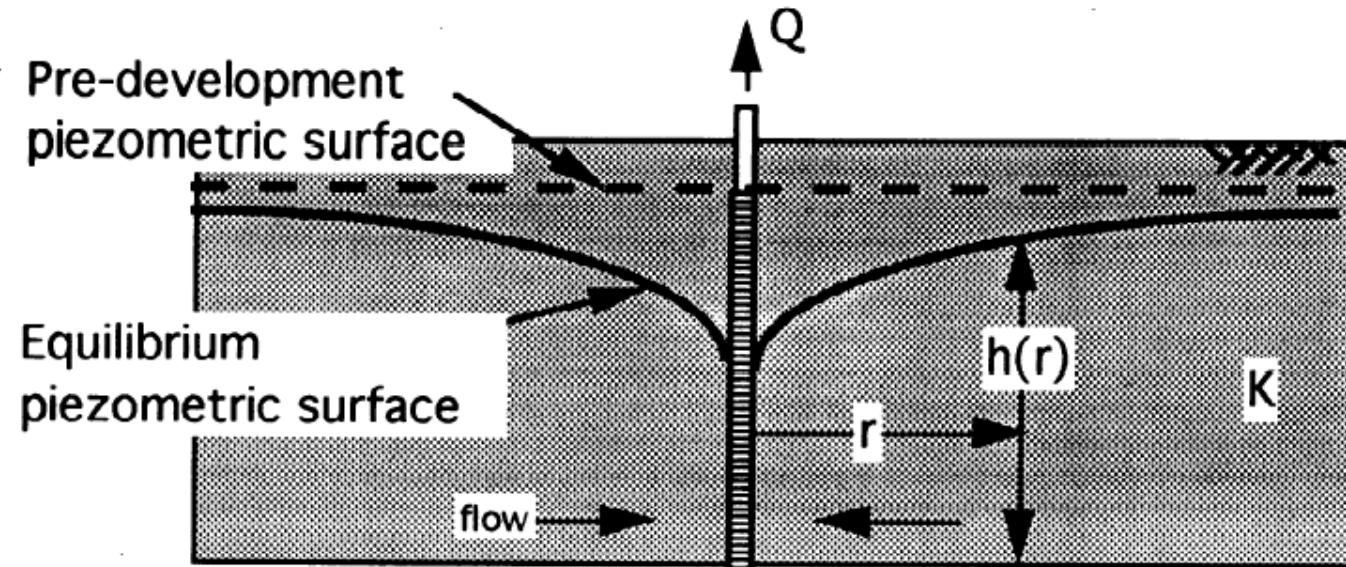
$s(r)$ is a solution to

$$\frac{1}{r} \frac{d}{dr} \left(r \frac{\partial s}{\partial r} \right) = 0.$$

Theim equation $s_1 - s_2 = \frac{Q}{2\pi T} \ln \left(\frac{r_2}{r_1} \right)$ used to
inter hydraulic properties.

Unconfined Aquifer

❖ Sketch



Homogeneous, isotropic, confined aquifer. Flow in radial direction only.
Steady state. No internal sources/sinks.

$$h^2(r) = \frac{Q}{\pi K} \ln\left(\frac{r}{R}\right) + h_0^2$$

Unconfined Aquifer

- Several solutions:

Corrected drawdown term treats unconfined aquifer as if sat. thickness is $b = h_0$

$$s'(r) = \frac{Q}{2\pi K h_0} \ln\left(\frac{R}{r}\right)$$

Use this one

and

$$\begin{aligned}s(r) &= h_0 - h(r) = h_0 - \sqrt{h_0^2 + \frac{Q}{\pi K} \ln\left(\frac{r}{R}\right)} = \\ &= h_0 \left(1 - \sqrt{1 + \frac{2s'}{h_0}}\right)\end{aligned}$$

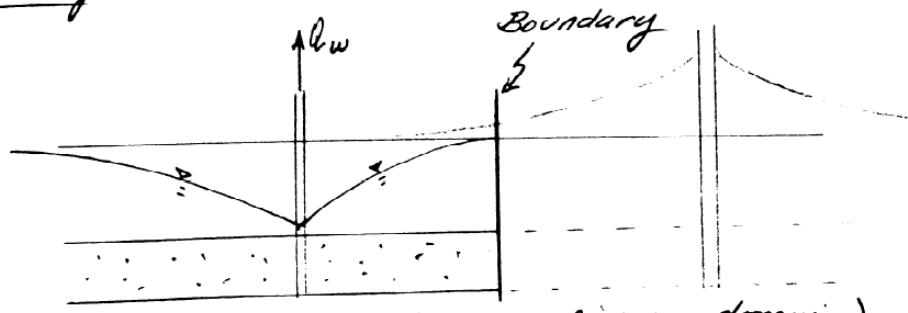
Superposition

- ✿ Linear combination of solutions to model effect of
 - ✿ Multiple wells
 - ✿ Boundaries

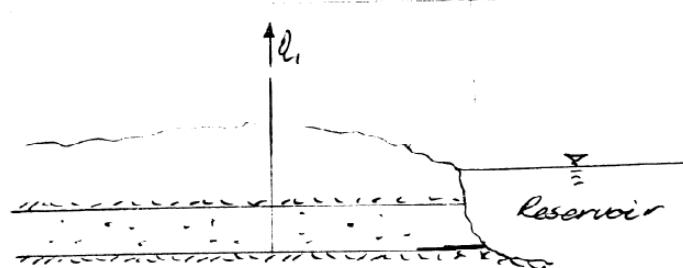
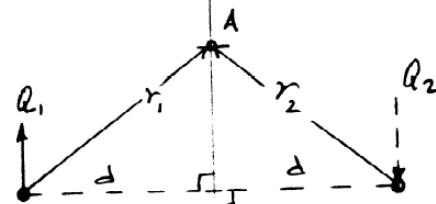
Superposition

Single Well near a constant head boundary

Boundary



(Model System)



(Physical System)

Superposition

$$S_A = S_A(\text{from well \#1}) + S_A(\text{from well \#2})$$

But A is a constant head boundary $\therefore S_A = 0$

$$S_A = \frac{Q_1}{2\pi K b} \ln\left(\frac{R}{r_1}\right) - \frac{Q_2}{2\pi K b} \ln\left(\frac{R}{r_2}\right)$$

" - " because well #2
is "modeled" as injection
to produce zero drawdown
at A (anywhere along
boundary)

$$|Q_1| = |Q_2| = |Q_w|$$

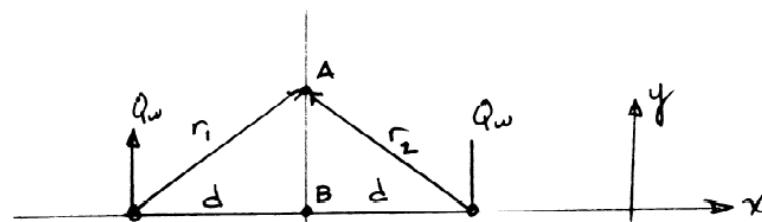
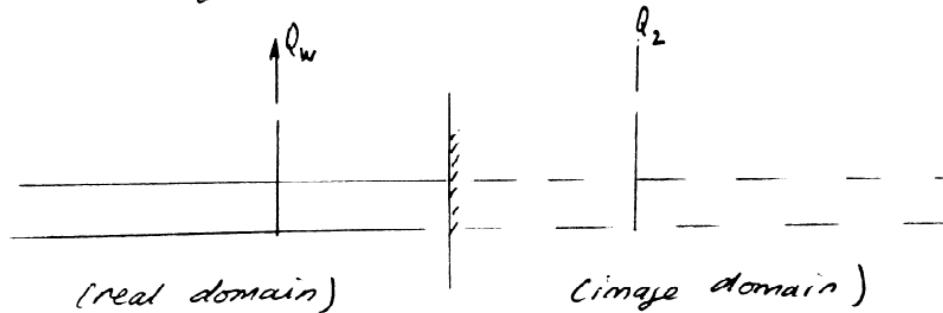
Superposition

$$\begin{aligned}S_A &= \frac{Q_1}{2\pi K b} \ln \left| \frac{R}{r_1} \right| - \frac{Q_2}{2\pi K b} \ln \left| \frac{R}{r_2} \right| \\&= \frac{Q_1}{2\pi K b} \ln \left| \frac{r_2}{r_1} \right|\end{aligned}$$

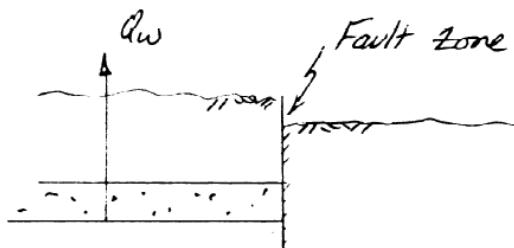
Now if Q_2 is located the same distance from the boundary as Q_1 , $r_2 = r_1$
 $\Rightarrow S_A = 0$ (as expected)

Superposition

No Flow Boundary



Model Systems



Superposition

$$S_A = \frac{Q_w}{2\pi K b} \ln\left(\frac{R}{r_1}\right) + \frac{Q_w}{2\pi K b} \ln\left(\frac{R}{r_2}\right) = \frac{Q_w}{2\pi K b} \ln\left(\frac{R^2}{r_1 r_2}\right)$$

$$S_B = \frac{Q_w}{2\pi K b} \ln\left(\frac{R^2}{d^2}\right)$$

$$\left. \frac{dS_B}{dx} \right|_{\text{Well } 1} = \frac{Q_w}{2\pi K b d} \quad ; \quad \left. \frac{dS_B}{dx} \right|_{\text{Well } 2} = -\frac{Q_w}{2\pi K b d}$$

$$\left. \frac{dS_B}{dx} \right|_{\text{both wells}} = \frac{Q_w}{2\pi K b d} - \frac{Q_w}{2\pi K b d} = 0$$

(This will be the result for all points along the boundary)

Superposition

Summary

- ① drawdown in confined aquifer due to well (steady flow)

$$S(r) = \frac{Q_w}{2\pi K b} \ln \left| \frac{R}{r} \right|$$

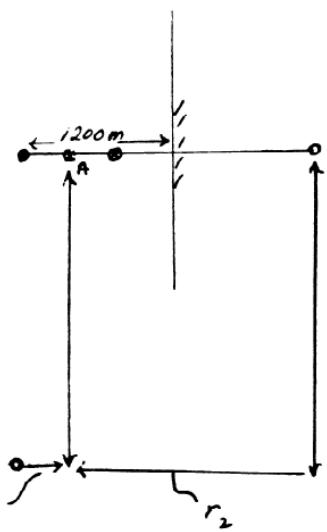
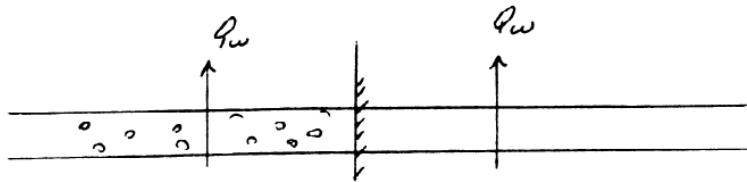
- ② constant head boundary:

a) locate image well same distance from boundary as real well, opposite sense on Q_w

- ③ no flow boundary

b) locate image well same distance from boundary as real well, same sense on Q_w

Superposition



$$Q_w = 100 \text{ m}^3/\text{d}$$

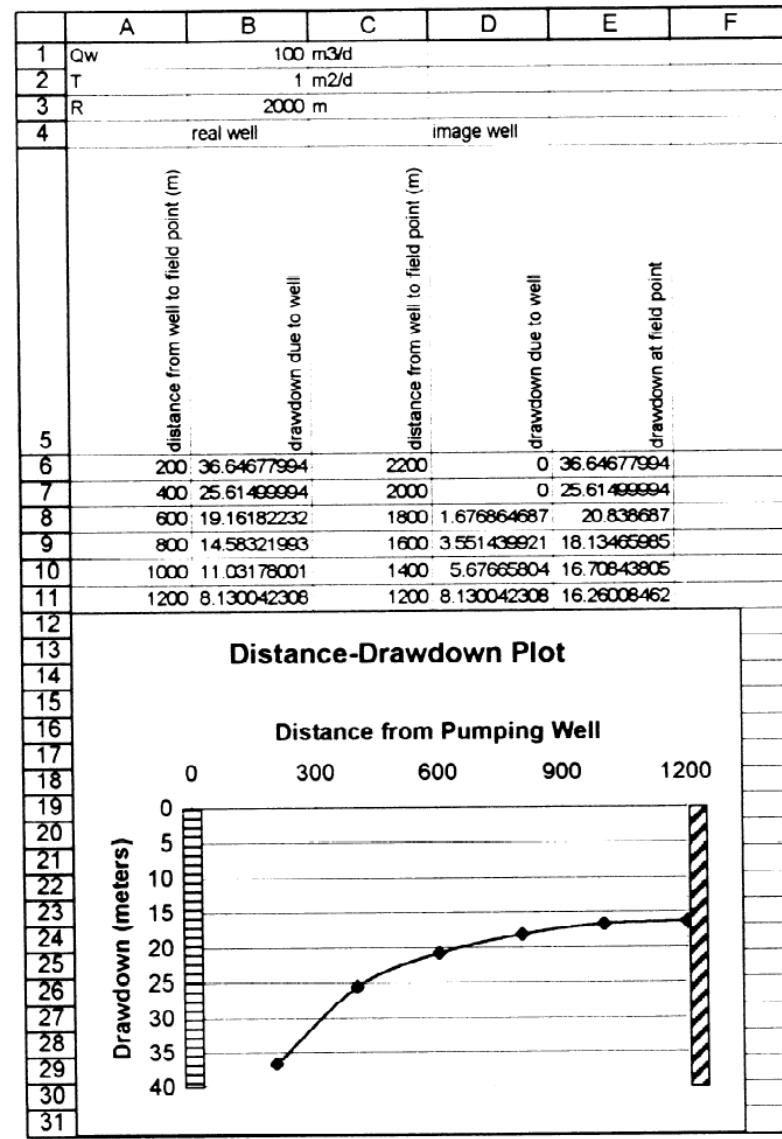
$$Kb = 1 \text{ m}^2/\text{d}$$

$$R = 2000 \text{ m}$$

Plot distance -
drawdown
profile between
well and boundary

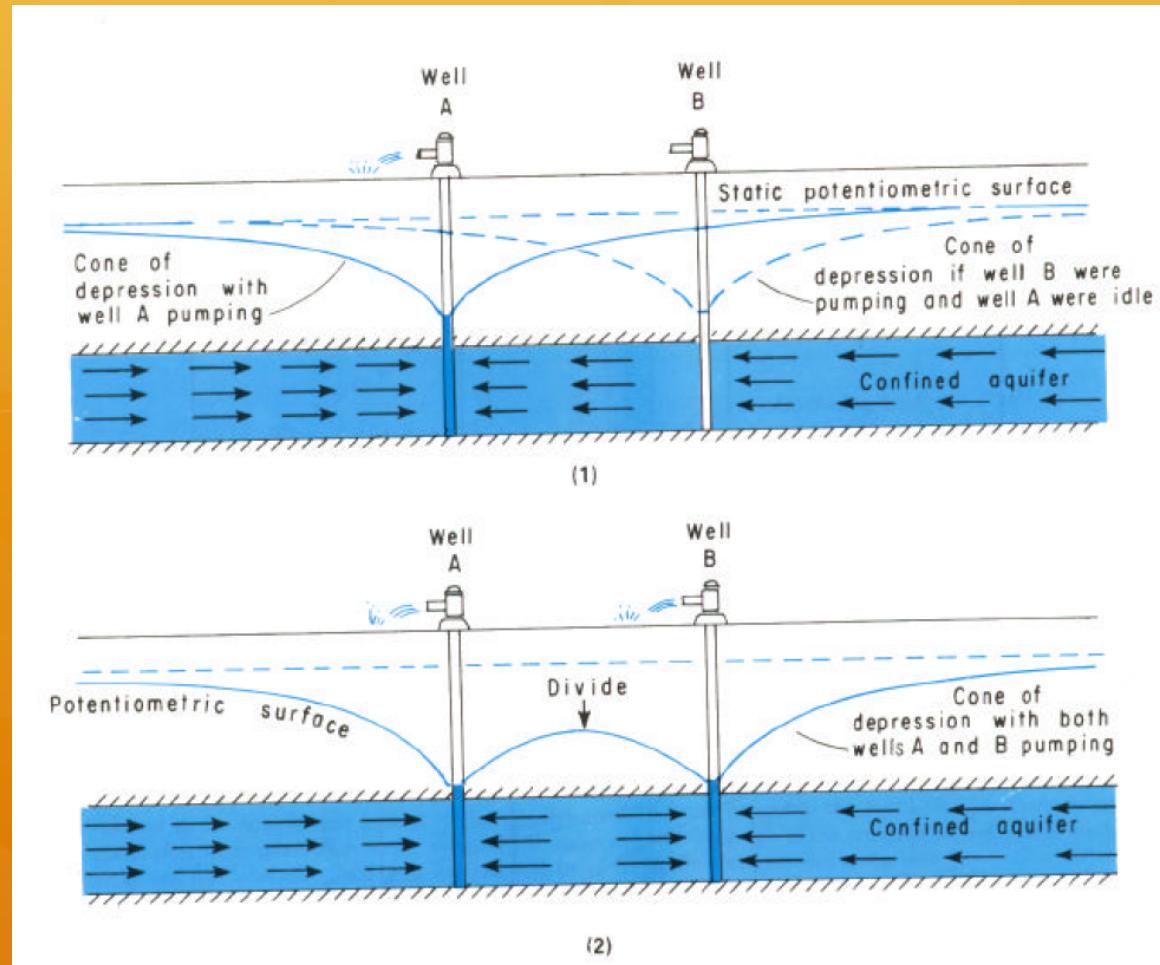
$$S_A = \frac{Q_w}{2\pi Kb} \ln\left(\frac{R}{r_1}\right) + \frac{Q_w}{2\pi Kb} \ln\left(\frac{R}{r_2}\right)$$

Superposition



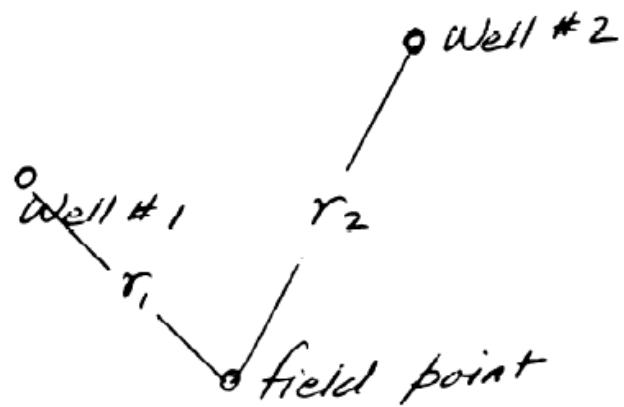
Well Interference

- Two (or more) wells operating near each other produce a combination drawdown that might affect operation of the wells
- If the pump impellers are not deep enough, the well may not produce because of drawdown caused by nearby wells



Well Interference

- ❖ Superposition is used to model such situations



$$\psi_{\text{field point}} = \frac{\varrho_1}{2\pi K b} \ln \left| \frac{R}{r_1} \right| + \frac{\varrho_2}{2\pi K b} \ln \left| \frac{R}{r_2} \right|$$

Well Interference

- ✿ Superposition is used to model such situations

Example

$$T = 1 \text{ m}^2/\text{d}$$

$$Q_1 = 100 \text{ m}^3/\text{d}$$

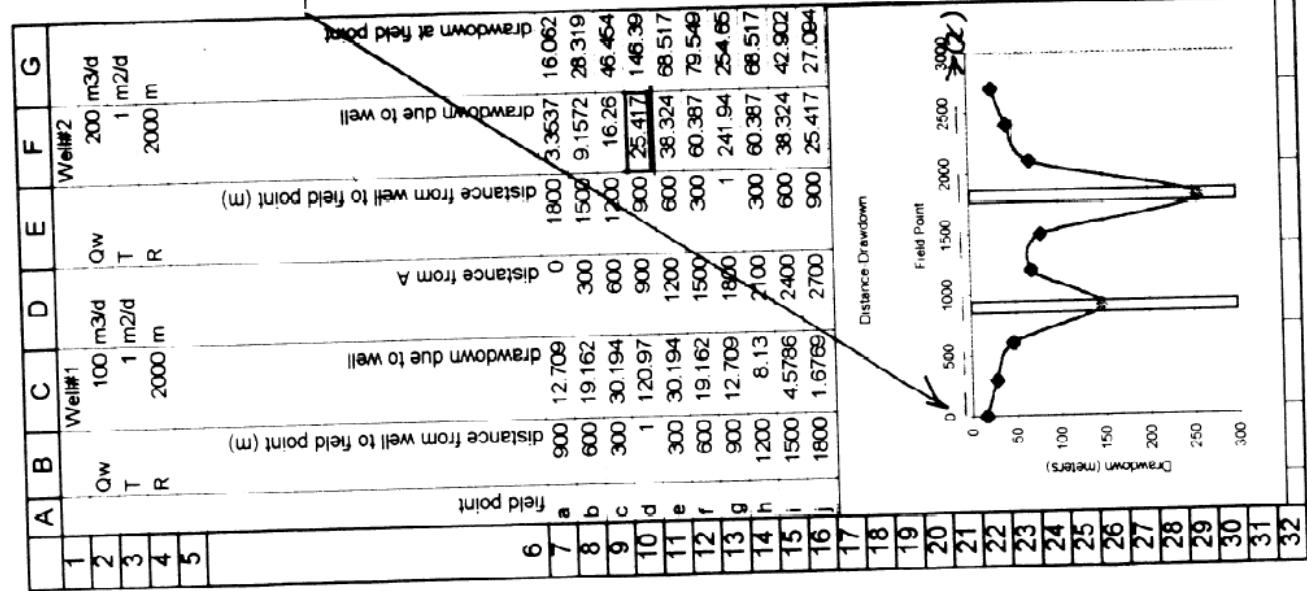
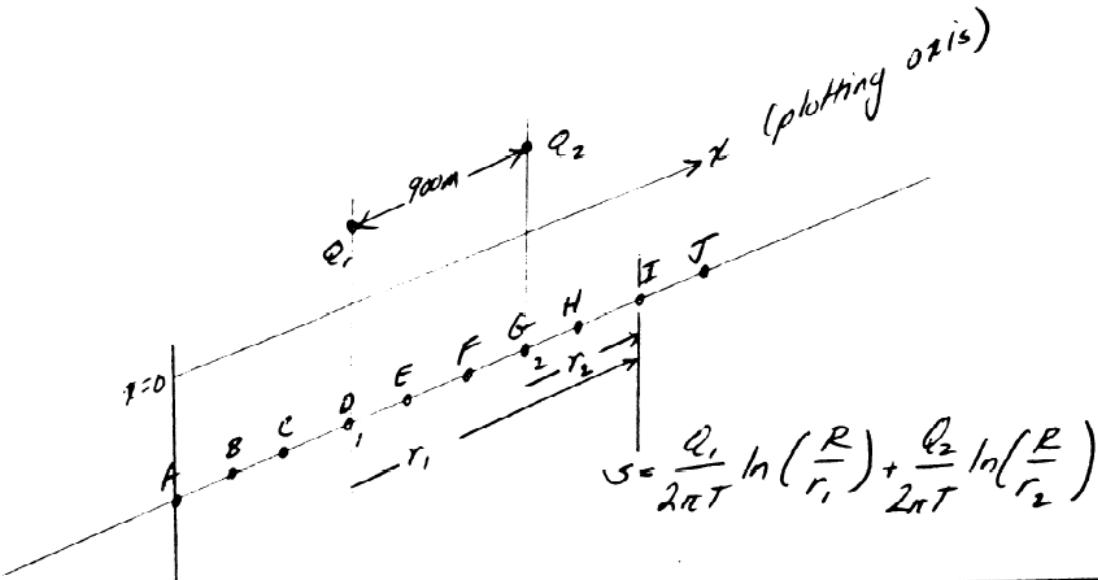
$$Q_2 = 200 \text{ m}^3/\text{d}$$

$$R = 2000 \text{ m}$$

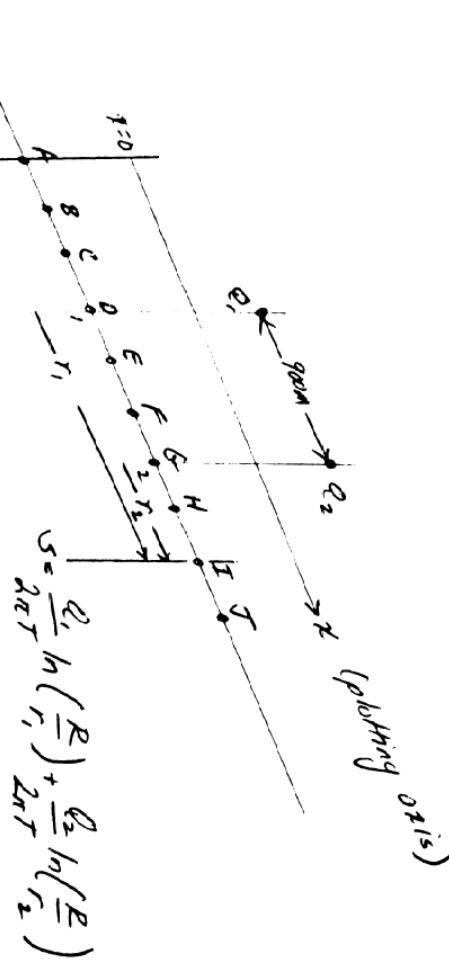
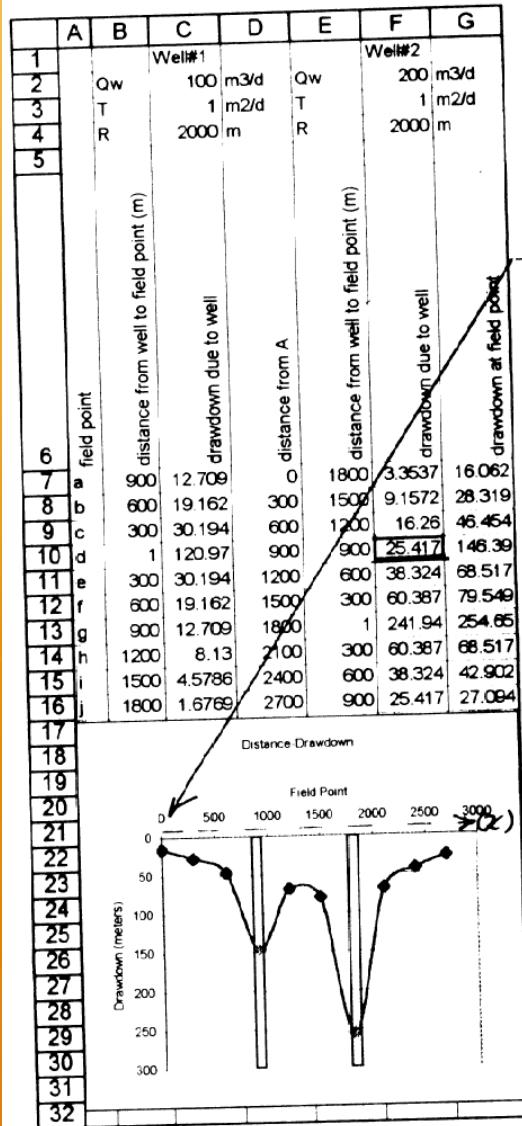
Wells located 900m apart. Show distance-drawdown profile.

Determine "interference" at well #1 due to operations of well #2

Well Interference

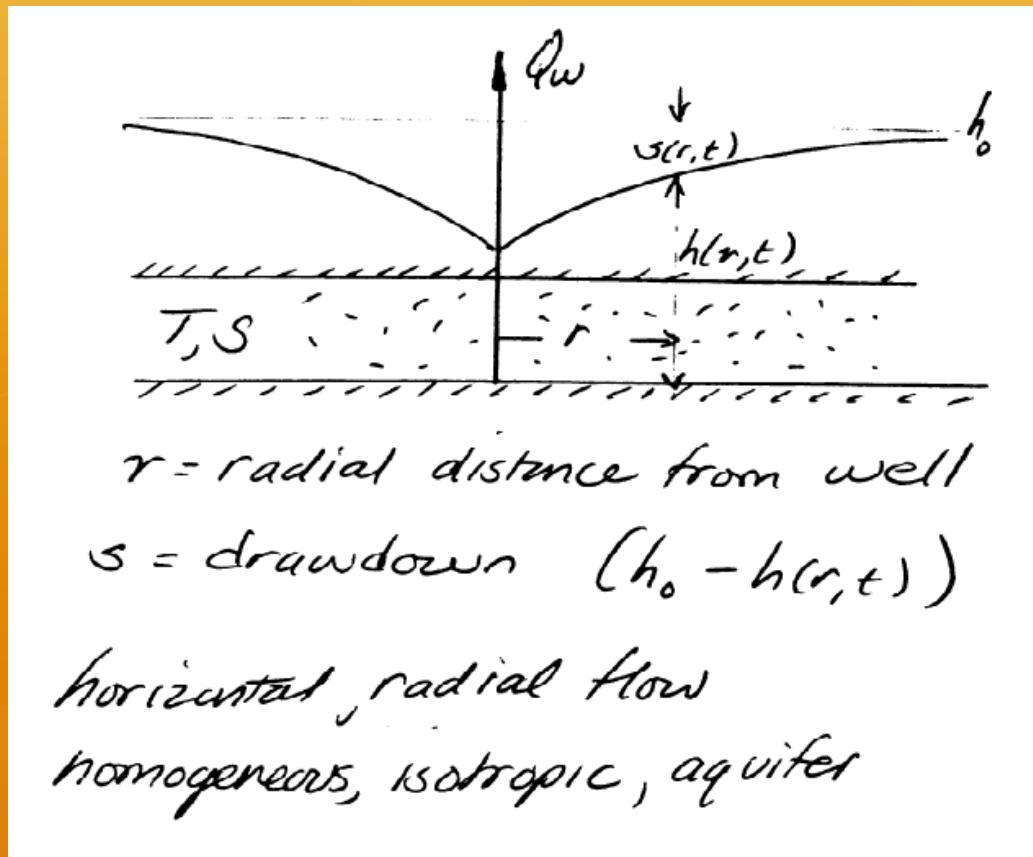


Well Interference



Confined Aquifer

- ✿ Transient flow to a well
(sketch of derivation – more in readings)



Confined Aquifer

Governing PDE and BCs

- Oddly enough Drawdown is lower case “s” and storativity is upper case “S” – need to be aware when reading.

$$\operatorname{div}(\mathbf{T} \operatorname{grad}(h)) = S \frac{\partial h}{\partial t}$$

$$s = h_0 - h(r, t)$$

$$\frac{\partial s}{\partial t} = - \frac{\partial h}{\partial t} \quad \frac{\partial^2 s}{\partial r^2} = - \frac{\partial^2 h}{\partial r^2} \quad \frac{\partial s}{\partial r} = - \frac{\partial h}{\partial r}$$

Storage Coefficient

$$T \frac{\partial^2 s}{\partial r^2} + \frac{T}{r} \frac{\partial s}{\partial r} = S \frac{\partial s}{\partial t} \quad \text{or}$$

Drawdown

$$\frac{\partial^2 s}{\partial r^2} + \frac{1}{r} \frac{\partial s}{\partial r} = \frac{S}{T} \frac{\partial s}{\partial t}$$

Confined Aquifer

- ✿ Governing PDE and BCs

Boundary Conditions

$$r = \infty, s = 0; r \rightarrow 0, \lim_{r \rightarrow 0} \left(2\pi r T \frac{\partial s}{\partial r} \right) = -Q_w$$

Initial Conditions

$$t = 0, s = 0$$

Confined Aquifer

- Solving the PDE – apply a Boltzman Transformation

Obtaining a Solution

$$\text{let } U = \frac{r^2 S}{4\pi t}$$

$$t \rightarrow 0, S \rightarrow 0 \Rightarrow U \rightarrow \infty$$

$$r \rightarrow \infty, S \rightarrow 0 \Rightarrow U \rightarrow \infty$$

$$\lim_{r \rightarrow 0} r \frac{\partial S}{\partial r} = - \frac{Q_w}{2\pi T}$$

$$\lim_{U \rightarrow 0} r \frac{\partial S}{\partial r} = \lim_{U \rightarrow 0} r \frac{\partial S}{\partial U} \frac{\partial U}{\partial r} = \lim_{U \rightarrow 0} \frac{\partial U \partial S}{\partial U} = - \frac{Q_w}{2\pi T}$$

$$\therefore \lim_{U \rightarrow 0} U \frac{\partial S}{\partial U} = - \frac{Q_w}{4\pi T}$$

Confined Aquifer

- ✿ Solving the PDE – apply a Boltzman Transformation
- ✿ Convert PDE into an ODE

Now transform governing equations into an ODE

$$\frac{\partial s}{\partial t} = \frac{\partial s}{\partial u} \frac{\partial u}{\partial t} = - \frac{v}{t} \frac{\partial s}{\partial u}$$

$$\frac{\partial s}{\partial r} = \frac{\partial s}{\partial u} \frac{\partial u}{\partial r} = \frac{2u}{r} \frac{\partial s}{\partial u}$$

$$\frac{\partial^2 s}{\partial r^2} = \frac{2u}{r^2} \frac{\partial s}{\partial u} + \frac{4u^2}{r^2} \frac{\partial^2 s}{\partial u^2}$$

Substitute into PDE

$$-\frac{v}{t} \frac{\partial s}{\partial u} \frac{1}{T} = \frac{4u^2}{r^2} \frac{\partial^2 s}{\partial u^2} + \frac{2u}{r^2} \frac{\partial s}{\partial u} + \frac{2u}{r} \frac{\partial s}{\partial u}$$

Confined Aquifer

Solving the PDE – Algebra and Calculus

Multiply by $v^{1/4}$, divide by v^2 , rearrange:

$$\frac{d^2s}{dv^2} + \left(\frac{v+1}{v}\right) \frac{ds}{dv} = 0 \quad \text{let } x = \frac{ds}{dv}$$

$$\frac{dx}{dv} = -\left(\frac{v+1}{v}\right)x \Rightarrow -\int \frac{dx}{x} = \int \frac{v+1}{v} dv$$
$$-\ln|x| = \ln|v| + v + \ln|c|$$

$$v = -\ln|x| - \ln|v| - \ln|c| = -\ln|xvc|$$

$$e^{-v} = xvc \quad \text{but } x = \frac{ds}{dv}$$

so:

$$\frac{ds}{dv} = \frac{1}{c} \frac{e^{-v}}{v}$$

Confined Aquifer

- Apply IC and BCs

Apply B.C. $v \frac{ds}{du} \rightarrow -\frac{Q_w}{4\pi T} v \rightarrow 0$

$$\therefore \frac{1}{C} = -\frac{Q_w}{4\pi T}$$

$$\frac{ds}{du} = -\frac{Q_w}{4\pi T} \frac{e^{-u}}{u} \rightarrow \int_0^s ds = -\int_{\infty}^u \frac{Q_w}{4\pi T} \frac{e^{-u}}{u} du$$

$$s(u) = -\frac{Q_w}{4\pi T} \underbrace{\int_{\infty}^u \frac{e^{-v}}{v} dv}_{-Ei(v)} \quad \text{Exponential integral}$$

Confined Aquifer

- Apply IC and BCs

Apply B.C. $v \frac{ds}{du} \rightarrow -\frac{Q_w}{4\pi T} v \rightarrow 0$

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$$s(u) = -\frac{Q_w}{4\pi T} \underbrace{\int_{\infty}^u \frac{e^{-v}}{v} dv}_{-Ei(v)} \quad \text{Exponential integral}$$

Confined Aquifer

- ❖ Now we have a solution, but how to evaluate the integral?
 - ❖ Once upon a time you would look up values in a table (readings)
 - ❖ Alternatively, you can apply a series expansion of the integrand and find a series solution

Confined Aquifer

- Now we have a solution, but how to evaluate the integral?

EVALUATE $Ei(v)$ BY

- SERIES EXPANSION:

$$e^x = 1 + x + \frac{x^2}{2!} + \frac{x^3}{3!} + \dots$$

$$Ei(v) = \int_v^\infty \frac{1 + x + \frac{x^2}{2!} + \frac{x^3}{3!} + \dots}{x} dx = \left[\ln|x| - x + \frac{x^2}{2 \cdot 2!} - \frac{x^3}{3 \cdot 3!} + \dots \right]_v^\infty$$

NOTE: $\left[\ln|x| - x + \frac{x^2}{2 \cdot 2!} - \frac{x^3}{3 \cdot 3!} + \dots \right] \Big|_{x=\infty} = \gamma \approx -0.5772$

$$\therefore Ei(v) = \gamma - \ln|v| + v - \frac{v^2}{2 \cdot 2!} + \frac{v^3}{3 \cdot 3!} + \dots$$

Confined Aquifer

- Now we have a solution, but how to evaluate the integral?

- POLYNOMIAL APPROXIMATION

$$Ei(v) \approx -\ln|v| + a_0 + a_1 v + a_2 v^2 + a_3 v^3 + a_4 v^4 + a_5 v^5$$

$$a_0 = -0.57721566$$

$$a_3 = 0.05519963$$

$$a_1 = 0.99999193$$

$$a_4 = -0.00976004$$

$$a_2 = -0.24991055$$

$$a_5 = 0.00107857$$

FOR $0 < v \leq 1$

$$Ei(v) \approx \left(\frac{v^4 + a_1 v^3 + a_2 v^2 + a_3 v + a_4}{v^4 + b_1 v^3 + b_2 v^2 + b_3 v + b_4} \right) \left(\frac{1}{v \exp(v)} \right)$$

$$a_1 = 8.5733287401$$

$$b_1 = 9.5733223454$$

$$a_2 = 18.0590169730$$

$$b_2 = 25.6329561486$$

$$a_3 = 8.6347608925$$

$$b_3 = 21.0996530827$$

$$a_4 = 0.2677737343$$

$$b_4 = 3.9584969228$$

FOR $1 \leq v \leq \infty$

Confined Aquifer

- Now we have a solution, but how to evaluate the integral?

- POLYNOMIAL APPROXIMATION

$$Ei(v) \approx -\ln|v| + a_0 + a_1 v + a_2 v^2 + a_3 v^3 + a_4 v^4 + a_5 v^5$$

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FOR $0 < v \leq 1$

$$Ei(v) \approx \left(\frac{v^4 + a_1 v^3 + a_2 v^2 + a_3 v + a_4}{v^4 + b_1 v^3 + b_2 v^2 + b_3 v + b_4} \right) \left(\frac{1}{v \exp(v)} \right)$$

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$$b_4 = 3.9584969228$$

FOR $1 \leq v \leq \infty$

Confined Aquifer



VBA Code (to evaluate the well function)

```
TheisModel.xlsxm - Module1 (Code)
(General) W

Function W(U) As Double
'Theis Well Function -- actually the exponential integral
If U <= 1 Then
    A0 = -0.57721566
    A1 = 0.99999193
    A2 = -0.24991055
    A3 = 0.05519968
    A4 = -0.00976004
    A5 = 0.00107857
    W = (-Log(U) + A0 + A1 * U + A2 * U ^ 2 + A3 * U ^ 3 + A4 * U ^ 4 + A5 * U ^ 5)
    Exit Function
Else
    A1 = 8.5733287401
    A2 = 18.059016973
    A3 = 8.6347608925
    A4 = 0.2677737343
    B1 = 9.5733223454
    B2 = 25.6329561486
    B3 = 21.0996530827
    B4 = 3.9584969228
    W = ((U ^ 4 + A1 * U ^ 3 + A2 * U ^ 2 + A3 * U + A4) / (U ^ 4 + B1 * U ^ 3 + B2 * U ^ 2 + B3 * U + B4)) / (U * Exp(U))
    Exit Function
End If
End Function
```

Confined Aquifer

TheisModel.xlsm

Search in Sheet

Calibri (Body) 12 B I U

Home Layout Tables Charts SmartArt Formulas Data Review Developer

Font Alignment Number Format Cells Themes

Fill Calibri (Body) 12 abc Wrap Text General Conditional Formatting Styles Insert Delete Format Themes

Paste Clear B I U Merge

T29

1	Model Name	1D_radial_flow_confined_aquifer_transient					
2	Model Type:	Hydraulic Model					
3	References:	Theis, C.V. 1935. The relation between the lowering of the piezometric surface and the rate and duration of discharge of a well using ground-water storage: American Geophysical Union Transactions, 16th Annual Meeting, v. 16, pt. 2, p. 519-524.					
4		Walton, W. C. 1989. Analytical Groundwater Modeling, Flow and Contaminant Migration, Lewis Publishers, Chelsea, MI.					
5	Method:	Polynomial approximation of exponential integral					
6	Author:	Dr. T.G. Cleveland for CIV6361/7332 Students; Spring 1995					
7	Rewrite in VBA Mac Excel 2011 Fall 2015						
8	Notes:	Infinite Confined Aquifer					
9		Steady Pumping at Origin					
10		Computes Drawdown at radial distance, r, for different time values t					
11		Uses Polynomial approximation to well function (see references)					
12	Macros:	W(U) Well Functions contained in Module1 as local to this worksheet macros					
13							
14		Conversion Calculator (Use GoalSeek to find U.S Customary values for SI Units)					
15	Q	25 <=gpm 3.342246 <=ft^3/min 4812.8342 <=ft^3/day 0.094715 <=m^3/min 136.389 <=m^3/day					
16	T	0.76 <=ft^2/min 1094.4 <=ft^2/day 5.6848 <=gpm/ft 101.7252 <=m^2/day 0.070642 <=m^2/min					
17							
18		MODEL INPUT VALUES					
19		Input Data (must use consistent length and time units!)					
20	Item	Value	Units	Description			
21	Q	11	ft^3/min	Pumping well discharge (L^3/t)			
22	T	0.76	ft^2/min	Aquifer transmissivity (L^2/t)			
23	S	0.0005		Aquifer storage coefficient			
24	r	96		Radial distance of observation well from pumping well (L)			
25		Computed Constants					
26	$Q/(4\pi T)$	1.15178					
27	Chart Title	Drawdown history at 96 (feet) from pumping well					
28	Time	Drawdown					
29	Elapsed Time (min)	r (ft)	$r^2 S / 4 \pi T t$	W(u)	Drawdown(ft)	Observed Drawdown (ft)	Sq. Error
30	5	96	0.30316	0.897932	1.034219	0.9615503	0.005281
31	28	96	0.05414	2.392464	2.75559	2.6659802	0.00803
32	41	96	0.03697	2.757051	3.175514	3.0855088	0.008101
33	60	96	0.02526	3.126297	3.600804	3.5109705	0.00807
34	75	96	0.02021	3.34445	3.852062	3.7625233	0.008017
35	244	96	0.00621	4.510219	5.194776	5.1081853	0.007498
36	493	96	0.00307	5.210429	6.001264	5.9170095	0.007099
37	669	96	0.00227	5.514896	6.351943	6.2687644	0.006919
38	958	96	0.00158	5.873277	6.764719	6.6828359	0.006705
39	1129	96	0.00134	6.037278	6.953611	6.872329	0.006607
40	1185	96	0.00128	6.085625	7.009296	6.9281921	0.006578
41				SSE =>	0.078904		

Diagram illustrating the flow system in a confined aquifer. A pumping well is located at the center of a circular domain. An observation well is located at a radial distance 'r' from the pumping well. The aquifer has a thickness 'T.S.' and storage coefficient 'S'. The initial piezometric surface is shown as a horizontal line. The drawdown at the observation well is indicated by a vertical arrow labeled 'Drawdown'. The final piezometric surface is shown as a lower horizontal line. The difference between the initial and final piezometric surfaces is labeled 'Q (discharge)'.

Drawdown history at 96 (feet) from pumping well

Elapsed Time (min)	Drawdown(t) (ft)	Observed Drawdown (ft)
0	0.0	0.0
50	3.0	3.0
100	3.5	3.5
200	4.5	4.5
400	5.5	5.5
600	6.0	6.0
800	6.5	6.5
1000	6.8	6.8
1200	7.0	7.0

Superposition

ADDITIONAL SOLUTIONS BY SUPERPOSITION

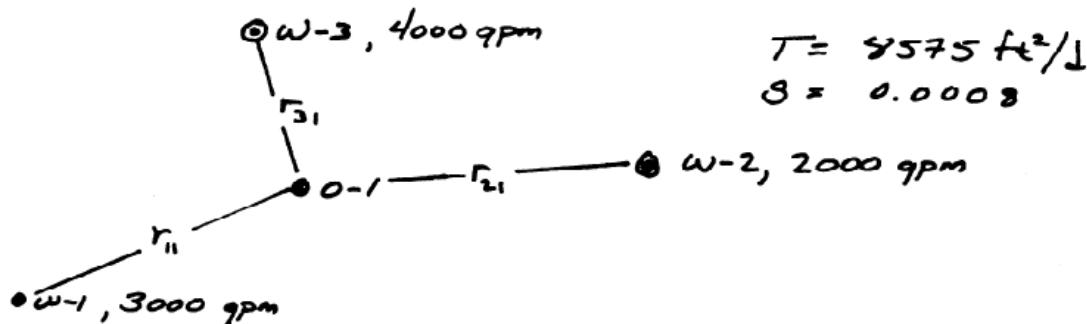
$$\frac{S}{T} \frac{ds}{dt} = \frac{\partial^2 s}{\partial r^2} + \frac{1}{r} \frac{\partial s}{\partial r} \quad \text{IS LINEAR IN } S \& T$$

∴ CAN DEVELOP ADDITIONAL SOLUTIONS BY
SUPERPOSITION

EXAMPLE

SUPPOSE WELLFIELD BELOW IS PLANNED
TO OPERATE AS SHOWN.

WHAT IS THE DRAWDOWN AT O-1 AFTER
365 DAYS OF PUMPING?



Superposition

SOLUTION:

- FIND DRAWDOWN AT O-Y FROM EACH PUMPING WELL
- TOTAL DRAWDOWN IS SIMPLY SUM OF INDIVIDUAL DRAWDOWNS

SUPPOSE:

$$r_1 = 1500 \text{ ft}$$

$$t = 365 \text{ day}$$

$$r_{21} = 1470 \text{ ft}$$

$$r_{31} = 1000 \text{ ft}$$

COMPUTE:

$$U_{11} = \frac{r_{11}^2 S}{4Tt} = 0.000144$$

$$U_{21} = \frac{r_{21}^2 S}{4Tt} = 0.000138$$

$$U_{31} = \frac{r_{31}^2 S}{4Tt} = 0.000064$$

Superposition

EVALUATE $E_i(u)$

$$E_i(u_1) = 8.270182$$

	$Q(\text{gpm})$	$Q(\text{ft}^3/\text{day})$
;	3000	577540

$$E_i(u_2) = 8.310582$$

;	2000	385027
---	------	--------

$$E_i(u_3) = 9.081032$$

;	4000	770053
---	------	--------

Superposition

COMPUTE INDIVIDUAL DRAWDOWNS

$$s_{11} = \frac{Q_1}{4\pi T} E_i(v_{11}) = \frac{577540}{4\pi(8575)} 8.270182 = 44.325$$

$$s_{21} = \frac{Q_2}{4\pi T} E_i(v_{21}) = 29.694$$

$$s_{31} = \frac{Q_3}{4\pi T} E_i(v_{31}) = 64.895$$

TOTAL DRAWDOWN

$$s = \sum_{i=1}^3 s_{i1} \quad \Sigma = 139'$$

∴ TOTAL PREDICTED DRAWDOWN AT 0-1 FROM
THE PUMPING WELL ENSEMBLE IS 139'
AFTER 365 DAYS OF PUMPING

Superposition

GENERAL FORM:

$$S_j = \sum_{i=1}^{Nw} \frac{Q_i}{4\pi T} Ei\left(\frac{r_{ij}^2 s}{4Tt}\right)$$

r_{ij} IS RADIUS FROM
i-th WELL TO FIELD
POINT j

Q_i IS PUMPING RATE
OF j-th WELL

Convolution

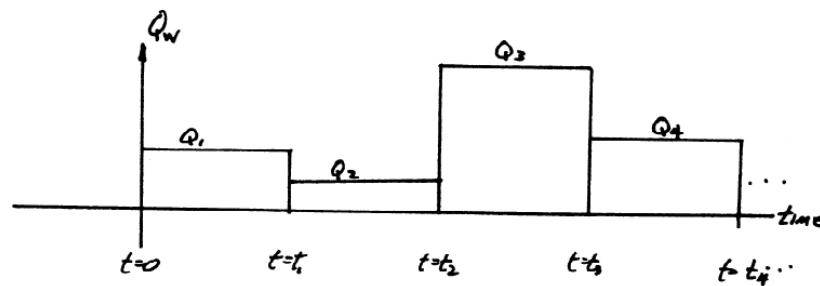
- Convolution == Superposition in Time

VARIABLE PUMPING RATES

USE CONVOLUTION IN TIME:

$$s(r, t) = \frac{Q}{4\pi T} E_i(v); \quad v = \frac{r^2 s}{4Tt}$$

ESTIMATE RESPONSE OVER SEVERAL PLANNING PERIODS (SEQUENTIAL) WITH DIFFERENT PUMP RATES:



From $t=0$ to $t=t_1$; $Q_w = Q_1$
 $t=t_1$ to $t=t_2$; $Q_w = Q_2$

Convolution

- Convolution == Superposition in Time

RESPONSE AT SOME ARBITRARY FIELD POINT:

$$0 \leq t \leq t_1 ; \quad s = \frac{Q_1}{4\pi T} Ei\left(\frac{r^2 S}{4Tt}\right)$$

$$t_1 \leq t \leq t_2 ; \quad s = \frac{Q_1}{4\pi T} Ei\left(\frac{r^2 S}{4Tt}\right) + \frac{Q_2 - Q_1}{4\pi T} Ei\left(\frac{r^2 S}{4T(t-t_1)}\right)$$

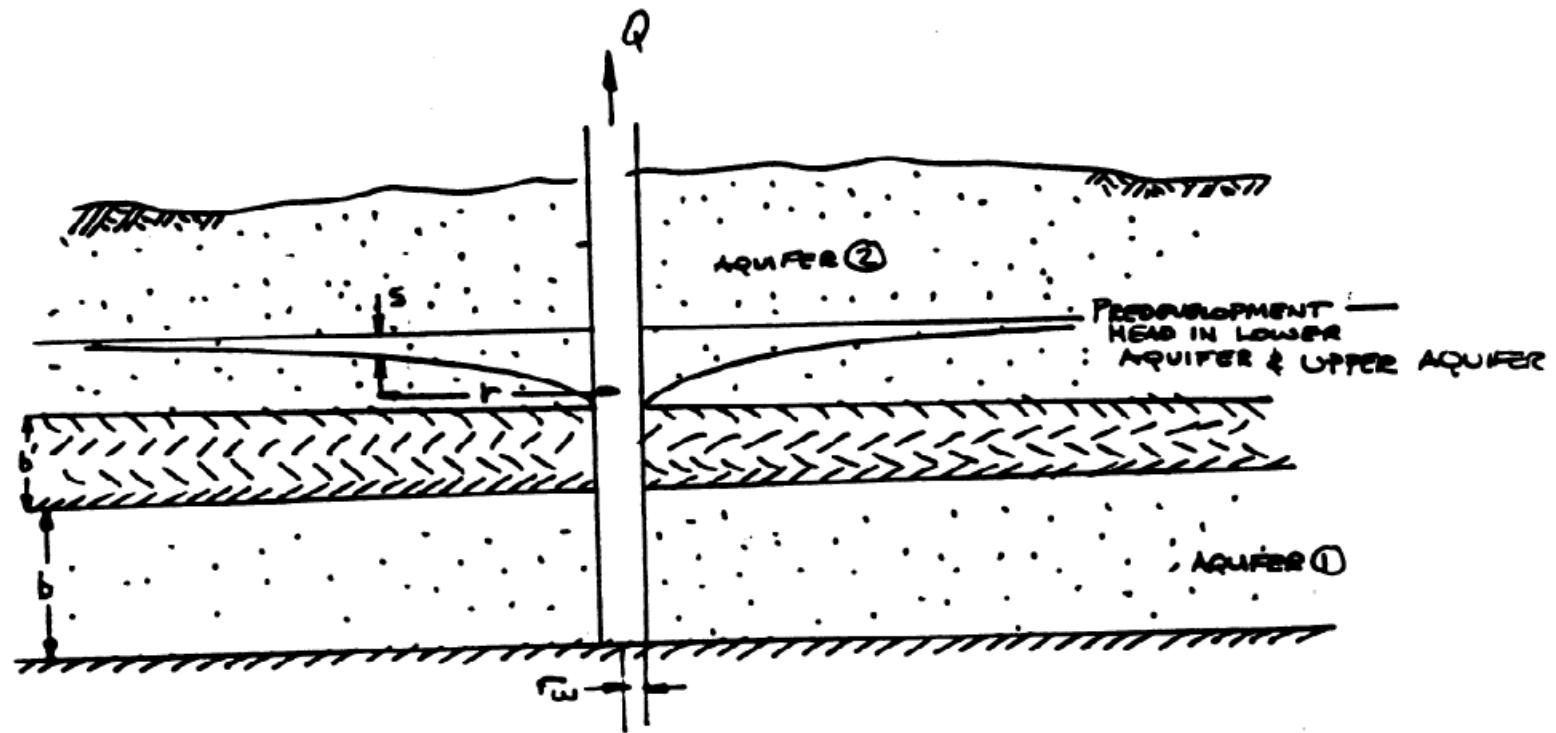
$$\begin{aligned} t_2 \leq t \leq t_3 ; \quad s &= \frac{Q_1}{4\pi T} Ei\left(\frac{r^2 S}{4Tt}\right) + \frac{Q_2 - Q_1}{4\pi T} Ei\left(\frac{r^2 S}{4T(t-t_1)}\right) \\ &\quad + \frac{Q_3 - Q_2}{4\pi T} Ei\left(\frac{r^2 S}{4T(t-t_2)}\right) \end{aligned}$$

$$t_3 \leq t \leq t_4 ; \quad s = \frac{Q_1}{4\pi T} Ei\left(\frac{r^2 S}{4Tt}\right) + \frac{Q_2 - Q_1}{4\pi T} Ei\left(\frac{r^2 S}{4T(t-t_1)}\right) + \frac{Q_3 - Q_2}{4\pi T} Ei\left(\frac{r^2 S}{4T(t-t_2)}\right) + \frac{Q_4 - Q_3}{4\pi T} Ei\left(\frac{r^2 S}{4T(t-t_3)}\right)$$

Leaky Aquifer

❖ Hantush Model

FULLY PENETRATING WELL IN A LEAKY AQUIFER



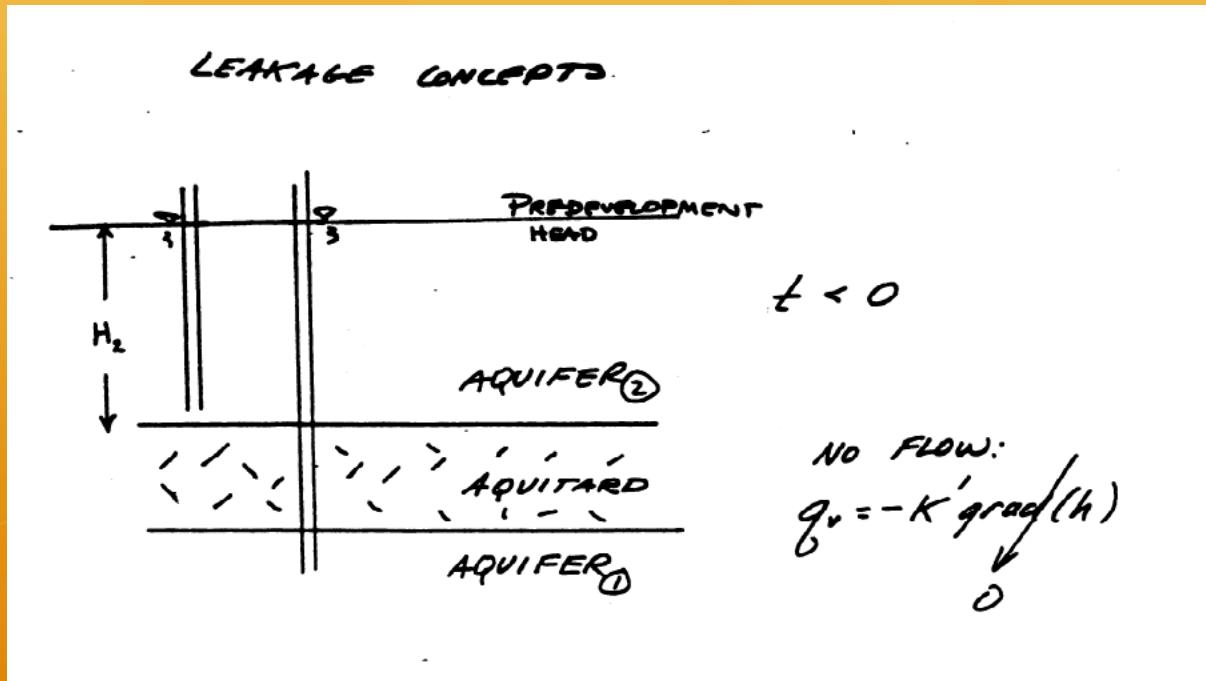
Leaky Aquifer

❖ Hantush Model

- WELL DISCHARGES AT CONSTANT RATE Q
- INFINITESMAL WELL DIAMETER
- AQUIFER ① OVERLAIN BY CONFINING BED (AQUITARD) OF THICKNESS b' , HYDRAULIC CONDUCTIVITY K'
- AQUIFER ② OVERLIES AQUITARD AND HAS CONSTANT HEAD
- HYDRAULIC GRADIENT ACROSS CONFINING BED CHANGES INSTANTLY — NO STORAGE IN AQUITARD
- AQUIFER FLOW IS 2D HORIZONTAL, AQUITARD FLOW IS VERTICAL

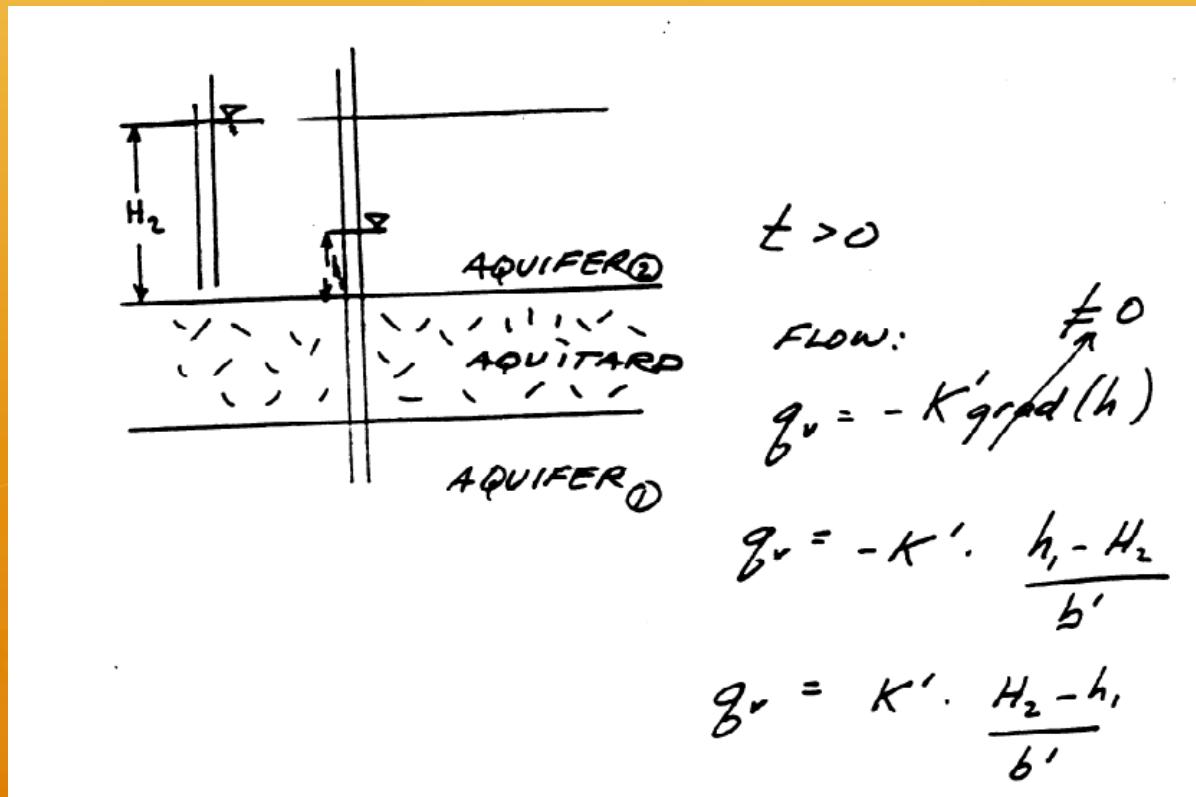
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- ✿ Leakage prior to pumping



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- Leakage after pumping begins



Leaky Aquifer

- ✿ Leakage – drawdown relationship

But: $H_2 - h_1 = s$ (DRAWDOWN)

$$\therefore g' = \frac{k's}{b'}$$

- NO STORAGE IN AQUITARD \Rightarrow CHANGE IN HEAD CAUSES INSTANTANEOUS CHANGE IN FLOW
- $H_2 = \text{CONSTANT}$ (ASSUMPTION 4)

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- ✿ Governing PDE and BCs

BASIC EQUATIONS

$$S \frac{\partial h}{\partial t} = \operatorname{div}(\mathbf{T} \operatorname{grad}(h)) \pm \text{sources}$$

OR

$$S \frac{\partial h}{\partial t} = T \frac{\partial^2 h}{\partial r^2} + \frac{T}{r} \frac{\partial h}{\partial r} - \frac{(H_2 - h)K'}{b'}$$

Leaky Aquifer

✿ Governing PDE and BCs

IN TERMS OF DRAWDOWN:

$$\frac{S}{T} \frac{\partial s}{\partial t} = \frac{\partial^2 s}{\partial r^2} + \frac{1}{r} \frac{\partial s}{\partial r} - \frac{s k'}{T b'}$$

SUBJECT TO:

$$s(r, 0) = 0$$

$$s(\infty, t) = 0$$

$$\lim_{r=r_w \rightarrow 0} \frac{\partial s}{\partial r} = - \frac{Q}{2\pi T}$$

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• Solution(s)

SOLUTION BY

LAPLACE TRANSFORM:

$$S = \frac{Q_w}{4\pi T} \int_0^\infty \frac{e^{-u - \frac{v^2}{u}}}{u} du$$

WHERE

$$U = \frac{r^2 S}{4T t}$$

$$V^2 = \frac{r^2 K'}{4T b'}$$

THE EXPONENTIAL INTEGRAL:

$$\int_U^\infty \frac{e^{-u - \frac{v^2}{u}}}{u} du = L(u, v)$$

Leaky Aquifer

❖ Solution(s)

$L(v, v)$ IS SOMETIMES DENOTED BY

$w(v, r/B)$.

THE TERM $B = \sqrt{\frac{T_b}{K'}}$ IS CALLED

THE LEAKAGE FACTOR

OBSERVE: $w(v, r/B) = L(v, 2v)$

Leaky Aquifer

❖ Solution(s)

$L(v, v)$ is sometimes denoted by

$w(v, r/B)$.

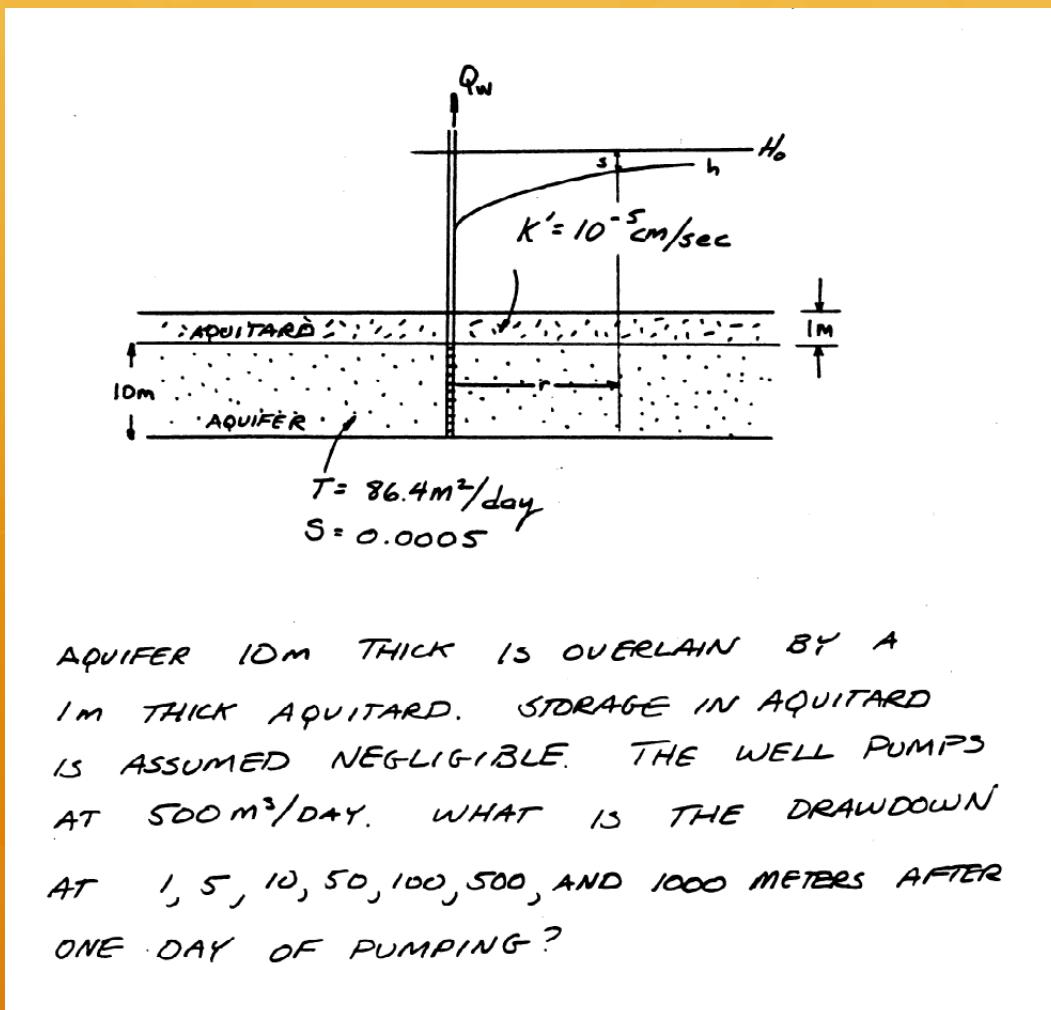
THE TERM $B = \sqrt{\frac{T_b}{K'}}$ IS CALLED

THE LEAKAGE FACTOR

OBSERVE: $w(v, r/B) = L(v, 2v)$

Leaky Aquifer

Example



Leaky Aquifer

✿ Example

① MODEL: $s(r, t) = \frac{Q_w}{4\pi T} L(u, v)$

$$u = \frac{r^2 s}{4T t}$$

$$v^2 = \frac{r^2 k'}{4T b'}$$

Leaky Aquifer

Example

② REDUCE DATA

$$t = 1 \text{ day}$$

$$Q_w = 500 \text{ m}^3/\text{d}$$

$$K' = 8.64 \cdot 10^{-3} \text{ m/d}$$

$$T = 86.4 \text{ m}^2/\text{d}$$

$$S = 0.0005$$

$$b' = 1 \text{ m}$$

$$v = \frac{r^2 (0.0005)}{4(86.4)(1)} = 1.45 \cdot 10^{-6} r^2$$

$$v^2 = \frac{r^2 (8.64 \cdot 10^{-3})}{4(86.4)(1)} = 2.5 \cdot 10^{-5} r^2$$

$$\frac{Q_w}{4\pi T} = \frac{500}{4(\pi)(86.4)} = 0.46$$

Leaky Aquifer

Example

③ MAKE A TABLE

r	v	v^2	v	$2v (= \frac{r}{B})$	$w(v, \frac{r}{B})$
1 m	$1.45 \cdot 10^{-6}$	$2.5 \cdot 10^{-5}$	$5.0 \cdot 10^{-3}$	0.01	9.44
5 m	$3.63 \cdot 10^{-5}$	$6.25 \cdot 10^{-4}$	$2.5 \cdot 10^{-2}$	0.05	6.23
10 m	$1.45 \cdot 10^{-4}$	$2.5 \cdot 10^{-3}$	$5.0 \cdot 10^{-2}$	0.1	4.85
50 m	$3.63 \cdot 10^{-3}$	$6.25 \cdot 10^{-2}$	$2.5 \cdot 10^{-1}$	0.5	1.85
100 m	$1.45 \cdot 10^{-2}$	$2.5 \cdot 10^{-1}$	$5.0 \cdot 10^{-1}$	1	0.842
500 m	$3.63 \cdot 10^{-1}$	6.25	2.5	5	0.007
1000 m	1.45	25	5.0	10	0.0001

Leaky Aquifer

Table-look up

u	r/B							
	0.001	0.003	0.01	0.03	0.1	0.3	1	3
1×10^{-6}	13.0031	11.8153	9.4425	7.2471	4.8541	2.7449	0.8420	0.0695
2	12.4240	11.6716	9.4425	7.2471	4.8541	2.7449	0.8420	0.0695
3	12.0581	11.5098	9.4425	7.2471	4.8541	2.7449	0.8420	0.0695
5	11.5795	11.2248	9.4413	7.2450	4.8530	2.7448	0.8419	0.0694
7	11.2570	10.9951	9.4361	7.2450	4.8530	2.7448	0.8419	0.0694
1×10^{-5}	10.9109	10.7228	9.4176	7.2471	4.8541	2.7449	0.8420	0.0695
2	10.2301	10.1332	9.2961	7.2471	4.8541	2.7449	0.8420	0.0695
3	9.8288	9.7635	9.1499	7.2470	4.8540	2.7448	0.8419	0.0694
5	9.3213	9.2818	8.8827	7.2450	4.8530	2.7448	0.8419	0.0694
7	8.9863	8.9580	8.6625	7.2371	4.8478	2.7350	0.8360	0.0634
1×10^{-4}	8.6308	8.6109	8.3983	7.2122	4.8292	2.7104	0.8190	0.0534
2	7.9390	7.9290	7.8192	7.0685	4.7079	2.7449	0.8420	0.0695
3	7.5340	7.5274	7.4534	6.9068	4.8541	2.7448	0.8419	0.0694
5	7.0237	7.0197	6.9750	6.6219	4.8530	2.7448	0.8419	0.0694
7	6.6876	6.6848	6.6527	6.3923	4.8478	2.7350	0.8360	0.0634
1×10^{-3}	6.3313	6.3293	6.3069	6.1202	4.8292	2.7104	0.8190	0.0534
2	5.6393	5.6383	5.6271	5.5314	4.7079	2.7449	0.8420	0.0695
3	5.2348	5.2342	5.2267	5.1627	4.5622	2.7448	0.8419	0.0694
5	4.7260	4.7256	4.7212	4.6829	4.2960	2.7428	0.8360	0.0634
7	4.3916	4.3913	4.3882	4.3609	4.0771	2.7350	0.8360	0.0634
1×10^{-2}	4.0379	4.0377	4.0356	4.0167	3.8150	2.7104	0.8190	0.0534
2	3.3547	3.3546	3.3536	3.3444	3.2442	2.5688	0.6950	0.0210
3	2.9591	2.9590	2.9584	2.9523	2.8873	2.4110	0.8420	0.0695
5	2.4679	2.4679	2.4675	2.4642	2.4271	2.1371	0.8409	0.0681
7	2.1508	2.1508	2.1506	2.1483	2.1232	1.9206	0.8360	0.0634
1×10^{-1}	1.8229	1.8229	1.8227	1.8213	1.8050	1.6704	0.8190	0.0534
2	1.2226	1.2226	1.2226	1.2220	1.2155	1.1602	0.7148	0.0695
3	.9057	.9057	.9056	.9053	.9018	.8713	.6010	.0694
5	.5598	.5598	.5598	.5596	.5581	.5453	.4210	.0681
7	.3738	.3738	.3738	.3737	.3729	.3663	.2996	.0639
1×10^0	.2194	.2194	.2194	.2193	.2190	.2161	.1855	.0534
2	.0489	.0489	.0489	.0489	.0488	.0485	.0444	.0210
3	.0130	.0130	.0130	.0130	.0130	.0130	.0122	.0071
5	.0011	.0011	.0011	.0011	.0011	.0011	.0011	.0008
7	.0001	.0001	.0001	.0001	.0001	.0001	.0001	.0001

LOG SCALE USE LINEAR INTERPOLATION FOR MISSING VALUES

Leaky Aquifer

✿ Table-look up

④ APPLY: $s = \frac{Q_w}{4\pi T} W(0, \frac{r}{B})$

(5) values

<u>r (meters)</u>	<u>s (meters)</u>
1	4.34
5	2.87
10	2.23
50	0.85
100	0.39
500	0.003
1000	0.000046

Leaky Aquifer

- ❖ More Modern Approach:
- ❖ VBA Script in Excel to evaluate $W(u,r/B)$
 - ❖ Complex uses:
 - ❖ ERFC (complimentary error function)
 - ❖ BESSELI (Bessel Function Type I)
 - ❖ BESSELJ (Bessel Function Type J)
- ❖ Script too complex to display – but ultimately it is just a function that can be evaluated just like $SQRT(Z)$.

Leaky Aquifer

HantushLeakyModel.xlsxm

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Home Layout Tables Charts SmartArt Formulas Data Review Developer

B4

1	Model Name:	1D_radial_flow_leaky_aquifer_transient_hantush								
2	Model Type:	Hydraulic Model								
3	References:	Hantush								
4										
5	Method:									
6	Author:	Dr. T.G. Cleveland for CIVE6361/7332 Students; Spring 1995 Rewrite in VBA Mac Excel 2011 Fall 2015								
7	Notes:	Infinite Confined Aquifer Steady Pumping at Origin Drawdown at radial distance, r , for different time values t in leaky aquifer system Uses Polynomial approximation to well function (see references)								
12	Macros:	W(U), LEAKY(U,V) Well Functions contained in Module1 as local to this worksheet macros								
13										
14										
15	Conversion Calculator (Use GoalSeek to find U.S Customary values for SI Units)									
16	Q	25 csgpm 3.34225 cft³/min 4812.83 cft³/dia 0.09736 m³/min 140.201 m³/day								
17	T	0.1722 ft²/min 247.968 cft²/min 1.28806 csgpm/ft 23.0488 m²/min 0.01601 m²/day								
18										
19	Input Data (must use consistent length and time units)									
20	Item	Value	Units	Description						
21	Q	3.34	cft³/min	Pumping well discharge (L^3/t)						
22	T	0.17	ft²/min	Aquifer transmissivity (L^2/t)						
23	S	0.00017		Aquifer storage coefficient						
24	K	5.7E-06	ft/min	Aquitard vertical hydraulic conductivity (L/t)						
25	b'	14	ft	Thickness of aquitard (L)						
26	r	96	ft	Radial distance of observation well from pumping well (L)						
27	Computed Constants									
28	B	648.55								
29	r/B	0.14802								
30	Q/4πT	1.56346								
31	Chart Title	Drawdown history at 96 (feet) from pumping well								
32	Time									
33	Elapsed Time (min)	5	Radial Distance (ft)	96	β	r/B	$N(u/r)$	Drawdown (ft)	Observed Drawdown (ft)	Error ²
34		0.044898	0.14802	0.62239	0.97309	0.96	0.00017			
35		0.08017	0.14802	1.97392	3.08616	3.3	0.04573			
36		0.05475	0.14802	2.30223	3.59945	3.59	8.9E-05			
37		0.03741	0.14802	2.62436	4.10309	4.08	0.00053			
38		0.02993	0.14802	2.80751	4.38944	4.39	3.2E-07			
39		0.0092	0.14802	3.62976	5.67499	5.47	0.04202			
40		0.04055	0.14802	3.92868	6.14234	5.96	0.03325			
41		0.00336	0.14802	4.00373	6.25968	6.11	0.0224			
42		0.00234	0.14802	4.055	6.33984	6.27	0.00488			
43		0.00199	0.14802	4.06801	6.36019	6.4	0.00159			
44		0.00189	0.14802	4.07089	6.36468	6.42	0.00306			
45					SSE	0.15372				

1DRFLAH-HYDM

Drawdown history at 96 (feet) from pumping well

Drawdown (ft) vs Elapsed Time (min)

Elapsed Time (min)	Drawdown (ft)	Observed Drawdown (ft)
0	0	0
200	4.5	4.5
400	5.5	5.5
600	6.0	6.0
800	6.2	6.2
1000	6.3	6.3
1200	6.4	6.4

Next Time

- ✿ Contaminant Transport Concepts
 - ✿ Advection, Dispersion, Retardation, Decay
- ✿ Aquifer Numerical Modeling
 - ✿ Flow Nets