

# CE 3372 WATER SYSTEMS DESIGN

LESSON 11 PART 1: STORAGE AND EXTENDED PERIOD SIMULATION FALL 2020

# OVERVIEW

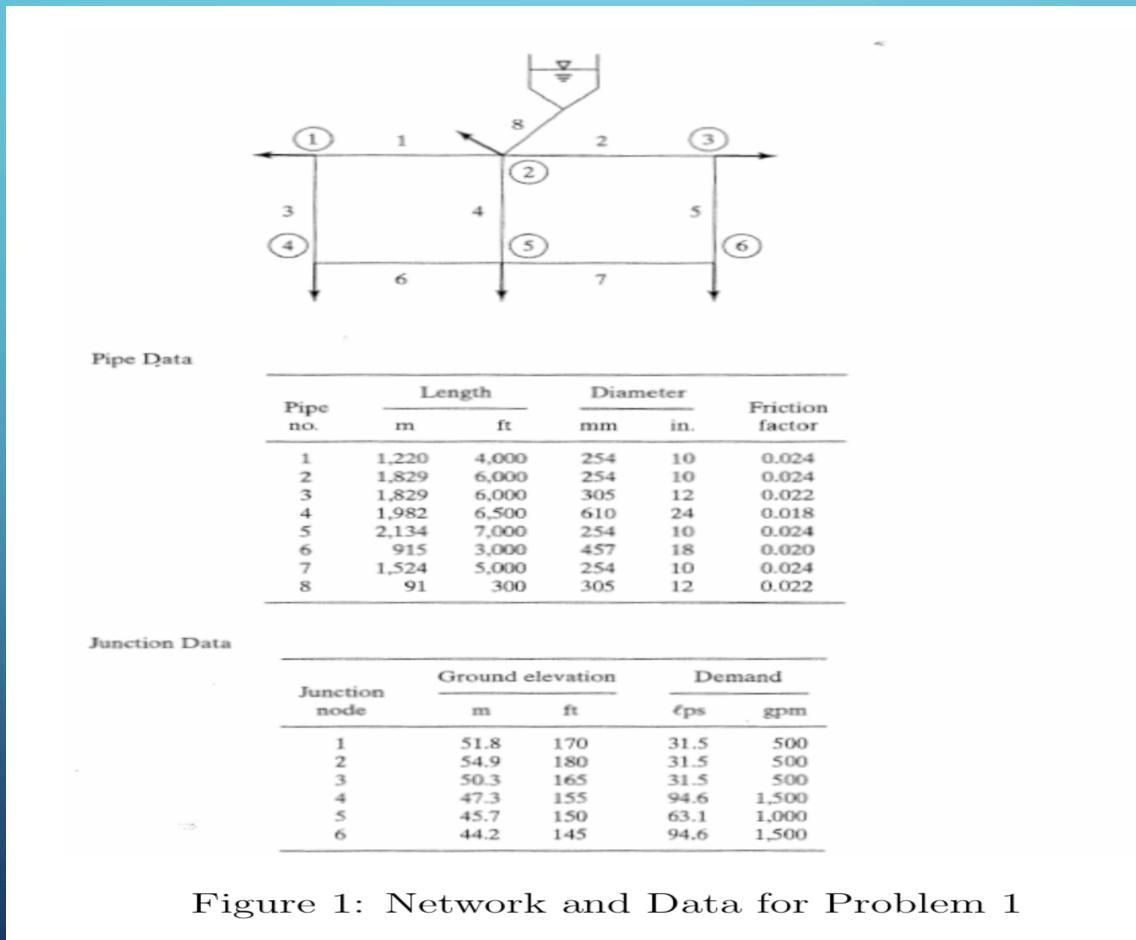
- EPANET Tank Model(s)
  - Single period simulation
- Multiple Period Simulation
  - Reasons
  - Multiplier Table

# READINGS

- EPA NET User Manual – how to model storage tanks in a water distribution system.
  - Interesting web-resources
  - <http://www.invisiblestructures.com/rainstore3.html>
  - <http://www.upout.com/blog/san-francisco-3/heres-what-it-looks-like-under-those-brick-circles-in-the-street>

# RECALL EARLIER EXAMPLE

Compute the discharge in each pipe and the pressure at each junction node for the 8-pipe system shown in Figure 1. The water surface elevation in the storage tank is 315.0 ft. Prepare your solution using EPA-NET. Report your results in U.S. Customary units. Identify the node with the lowest pressure in your solution. Include a transmittal letter with the solution.

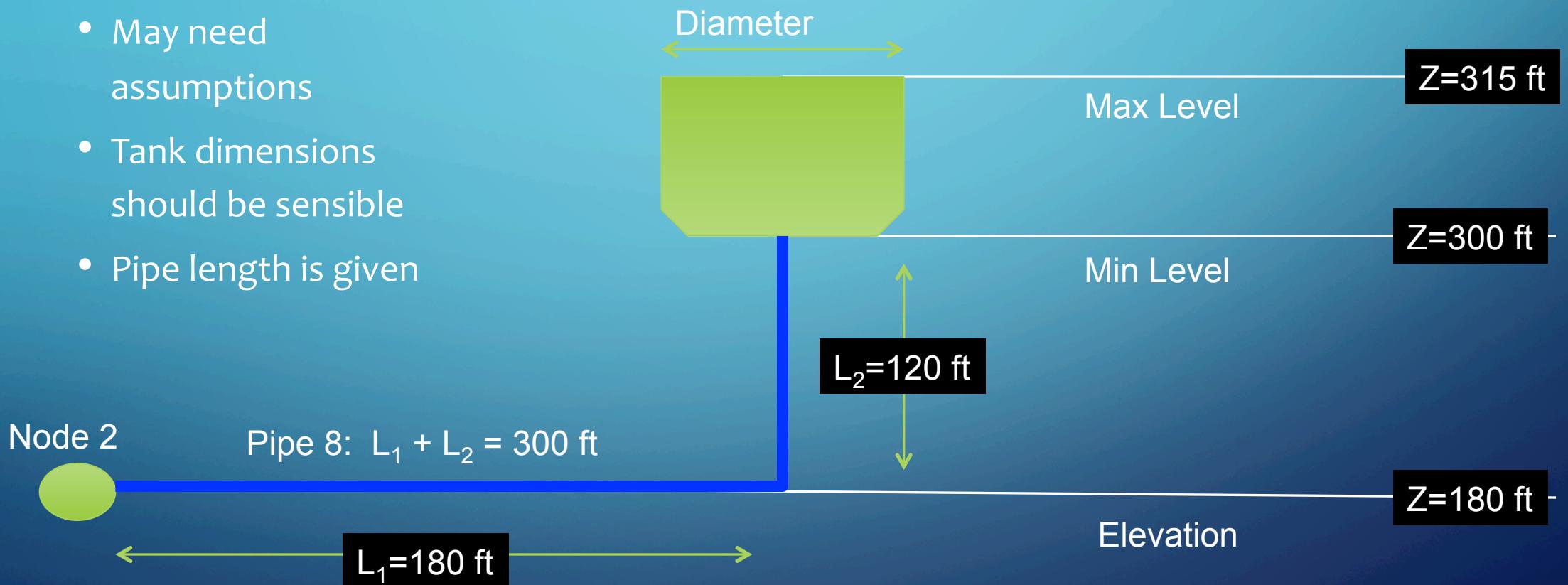


# MODELING PROTOCOL

- Sketch a layout on paper
- Identify pipe diameters; length; roughness values
- Identify node elevations; demands
- Supply reservoir (or tank); identify reservoir pool elevation
- Identify pumps; pump curve in problem units

# TANK

- Supply reservoir (or tank); identify reservoir pool elevation
  - May need assumptions
  - Tank dimensions should be sensible
  - Pipe length is given



# EXTENDED PERIOD SIMULATION

- EPANET and similar programs find steady-flow solutions
- Extended period simulation produces a sequence of steady states with approximations for:
  - Tanks drain and fill
  - Pressures can change at beginning and end of a time interval
  - Pump operating points moving along a pump curve

# USES

- Extended period simulation used for:

- Modeling pressure in systems during changing demand –usually at hourly time scale
- Storage tank operation and sizing
- Water quality simulation
  - EPANET can approximate water quality from multiple sources – has uses in
    - Water age in system
    - Detection of intrusions into a system
    - Severity of contamination (impact assessment)

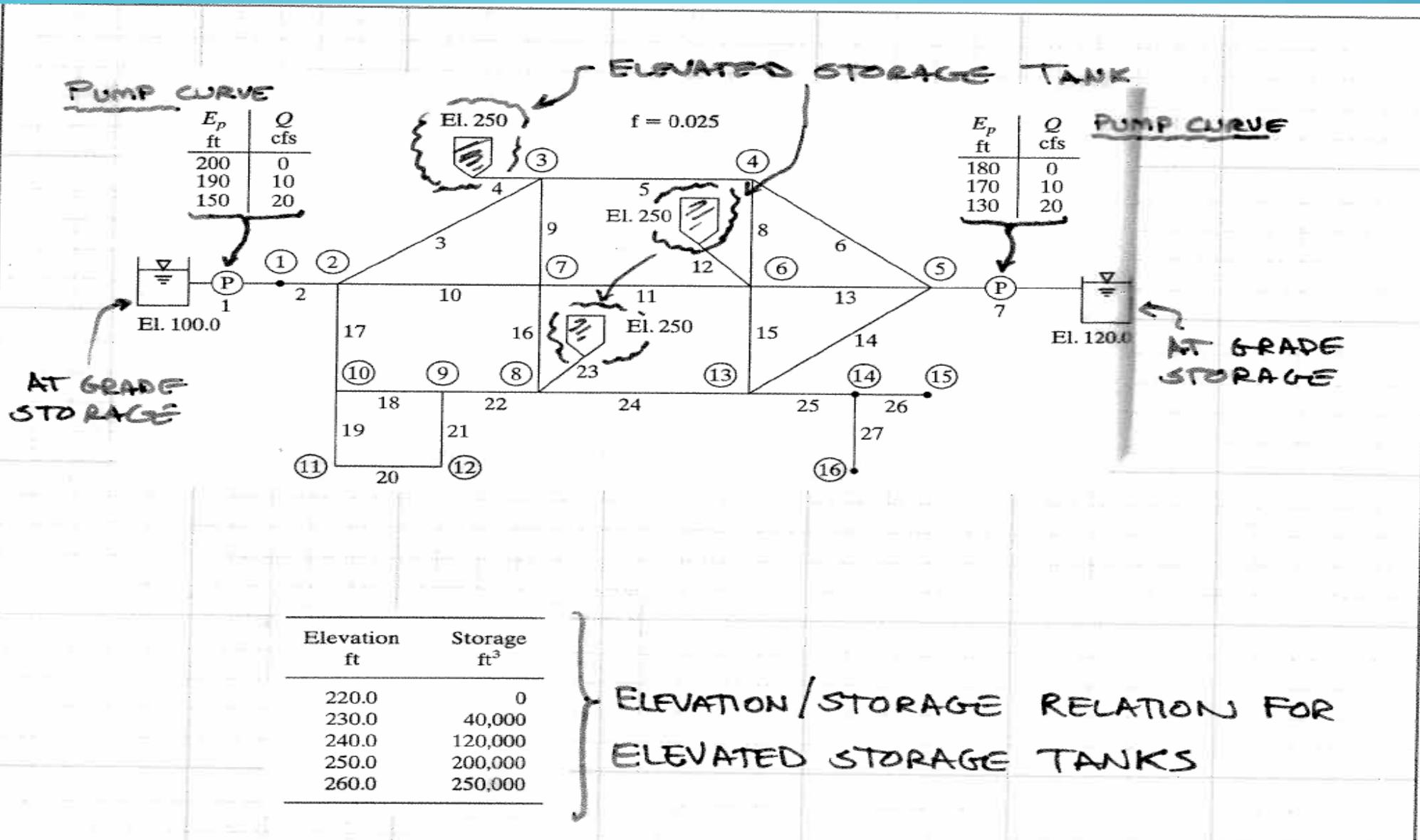
# HOW IMPLEMENTED?

- In EPANET assign a demand pattern to a node
- Set simulation times
- Program then follows the pattern

# MODELING PROTOCOL

- Sketch a layout on paper
- Identify pipe diameters; length; roughness values
- Identify node elevations; demands
- Supply reservoir (or tank); identify reservoir pool elevation
- Identify pumps; pump curve in problem units
- Identify demand pattern(s) and tank operating considerations

# EXAMPLE



# ILLUSTRATE BY EXAMPLE

Pipe no.	Node US	Node DS	Length (ft)	Diameter (in)	Minor loss coefficient	Fixed grade (ft)
1	0	1	2,000.0	24.0	0.5	100.0
2	1	2	800.0	24.0	0.0	
3	2	3	5,000.0	18.0	0.0	
4	3	0	700.0	18.0	0.5	250.0
5	3	4	3,700.0	12.0	0.0	
6	5	4	3,900.0	15.0	0.0	
7	0	5	2,100.0	24.0	0.5	120.0
8	6	4	2,500.0	10.0	0.0	
9	3	7	3,100.0	12.0	0.0	
10	2	7	5,500.0	18.0	0.0	
11	6	7	3,700.0	15.0	0.0	
12	0	6	900.0	18.0	0.5	250.0
13	5	6	2,900.0	15.0	0.0	
14	5	13	4,500.0	15.0	0.0	
15	6	13	2,500.0	15.0	0.0	
16	7	8	2,700.0	15.0	0.0	
17	2	10	3,100.0	18.0	0.0	
18	10	9	1,900.0	15.0	0.0	
19	10	11	1,600.0	8.0	0.0	
20	11	12	1,500.0	6.0	0.0	
21	9	12	1,650.0	8.0	0.0	
22	8	9	2,900.0	15.0	0.0	
23	0	8	1,900.0	18.0	7.5	250.0
24	13	8	3,100.0	15.0	0.0	
25	13	14	1,600.0	8.0	0.0	
26	14	15	1,750.0	6.0	0.0	
27	14	16	1,500.0	6.0	0.0	

PIPE CHARACTERISTICS

(ADJUST LOSS COEFF. TO GET  
 $f = 0.025$   
 (given) )

IN PRACTICAL CASE  
 USE PIPE MATERIAL  
 INFO.

# ILLUSTRATE BY EXAMPLE

Junction no.	Elevation (ft)	Demand (gpm)
1	90.00	0
2	110.00	694 1.54
3	95.00	694 1.54
4	105.00	2,083 4.64
5	100.00	694 1.54
6	103.00	2,428 5.40
7	97.00	2,083 4.64
8	103.00	1,044 2.32
9	107.00	0
10	112.00	0
11	115.00	350 0.77
12	112.00	350 0.77
13	110.00	0
14	120.00	0
15	135.00	175 0.39
16	130.00	175 0.39

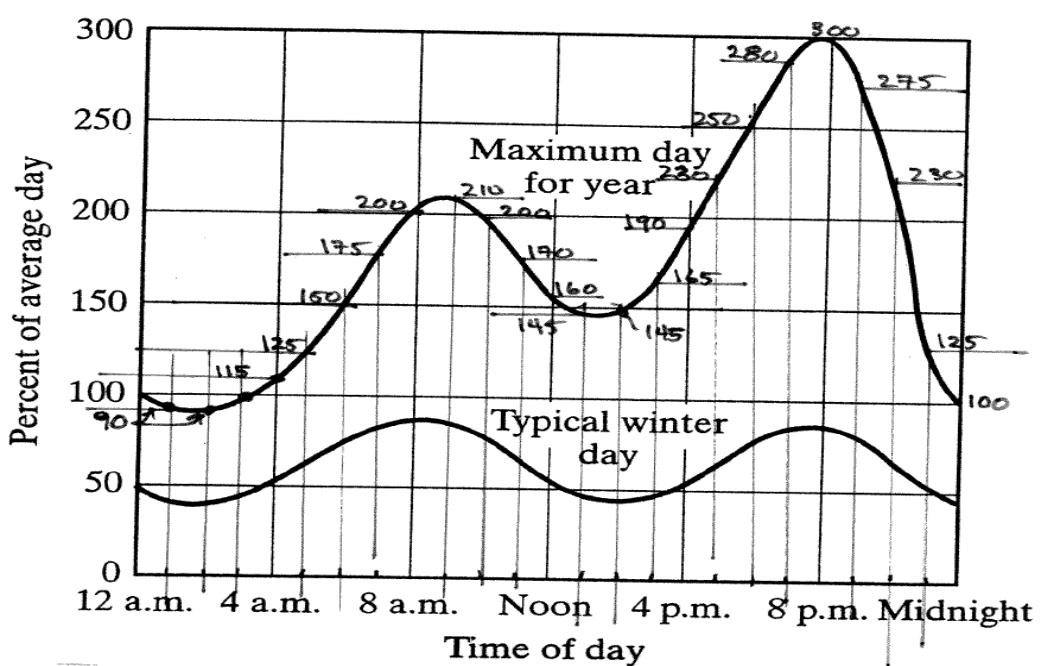
DEMAND (CFS)

0  
1.54  
1.54

$$\frac{1 \text{ gal}}{\text{min}} * \frac{\frac{1}{4} \text{ m}^3}{7.48 \text{ gal}} * \frac{1 \text{ min}}{60 \text{ sec}}$$

BASE DEMAND &  
NODE TOPOGRAPHY

# ILLUSTRATE BY EXAMPLE



DEMAND MULTIPLIERS - READ FROM CHART FOR HOUR OF DAY,  
BUILD MULTIPLIER TABLE



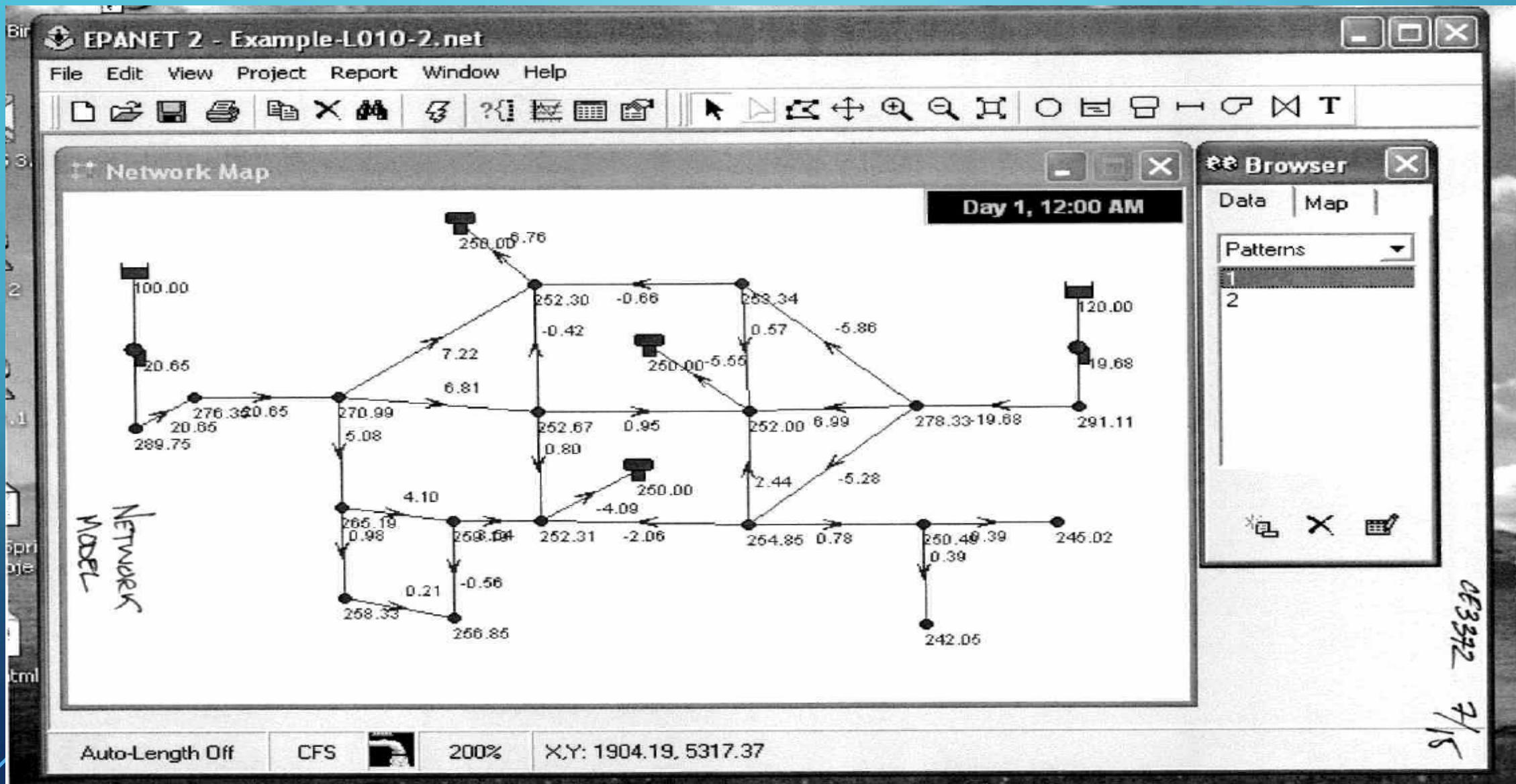
REPEATS CYCLE

Hour	Factor (Multiplier)	Clock Time
1	1.0	0:00
2	0.9	1:00
3	0.9	2:00
4	1.0	3:00
5	1.15	4:00
6	1.25	5:00
7	1.5	6:00
8	1.75	7:00
9	2.0	8:00
10	2.10	9:00
11	2.0	10:00
12	1.7	11:00
13	1.6	12:00
14	1.45	13:00
15	1.45	14:00
16	1.65	15:00
17	1.90	16:00
18	2.30	17:00
19	2.50	18:00
20	2.80	19:00
21	3.00	20:00
22	2.75	21:00
23	2.30	22:00
24	1.25	23:00

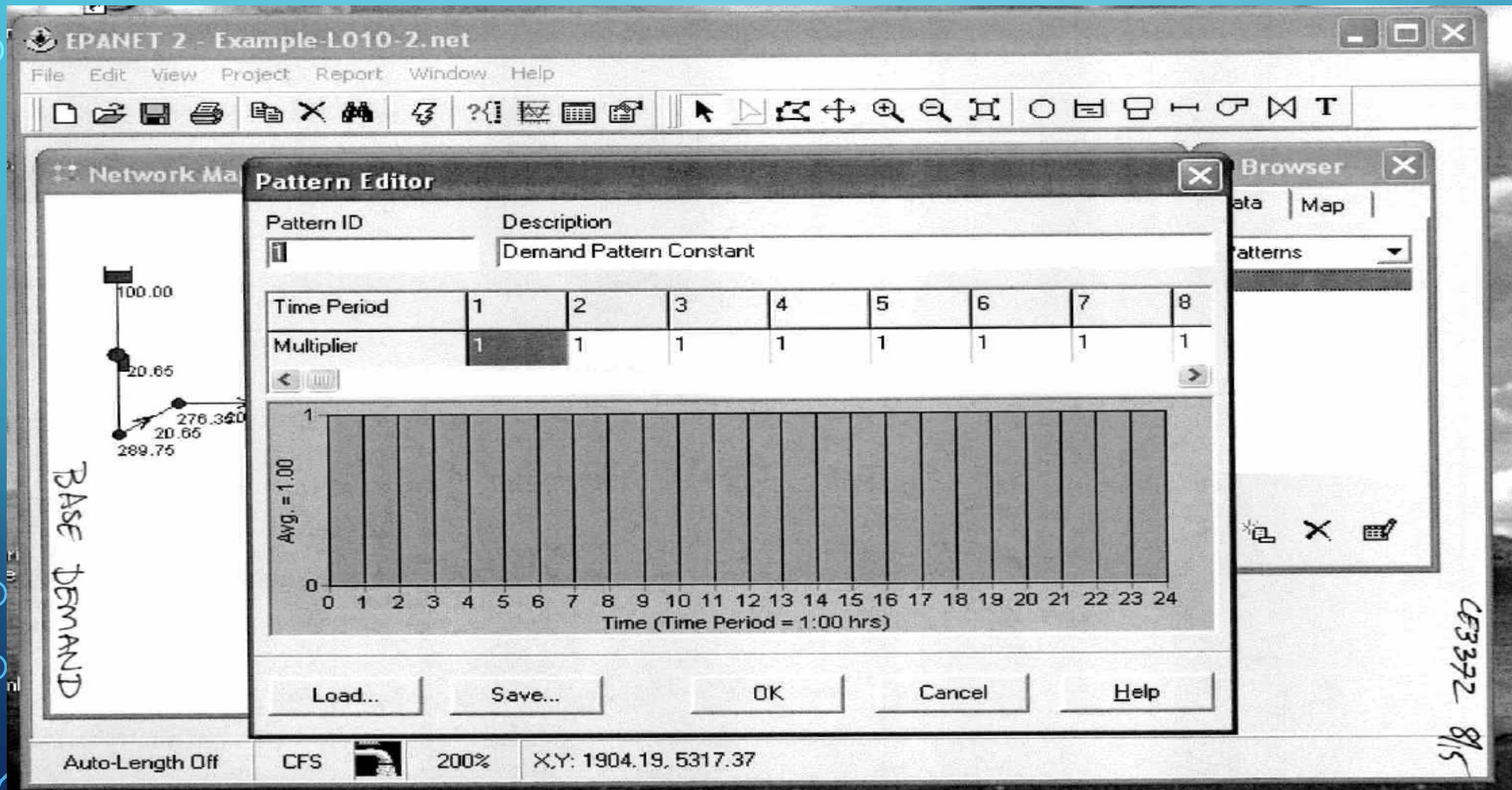
# ILLUSTRATE BY EXAMPLE

- Build network layout
  - Nodes (junctions, tanks, reservoirs)
  - Links (pipes, pumps, valves)
- Add pump curves
  - BROWSER/DATA/CURVES/ADD (TYPE=PUMP)
- Add storage curves
  - BROWSER/DATA/CURVES/ADD (TYPE=STORAGE)
- Add demand pattern(s)
  - BROWSER/PATTERNS/ADD

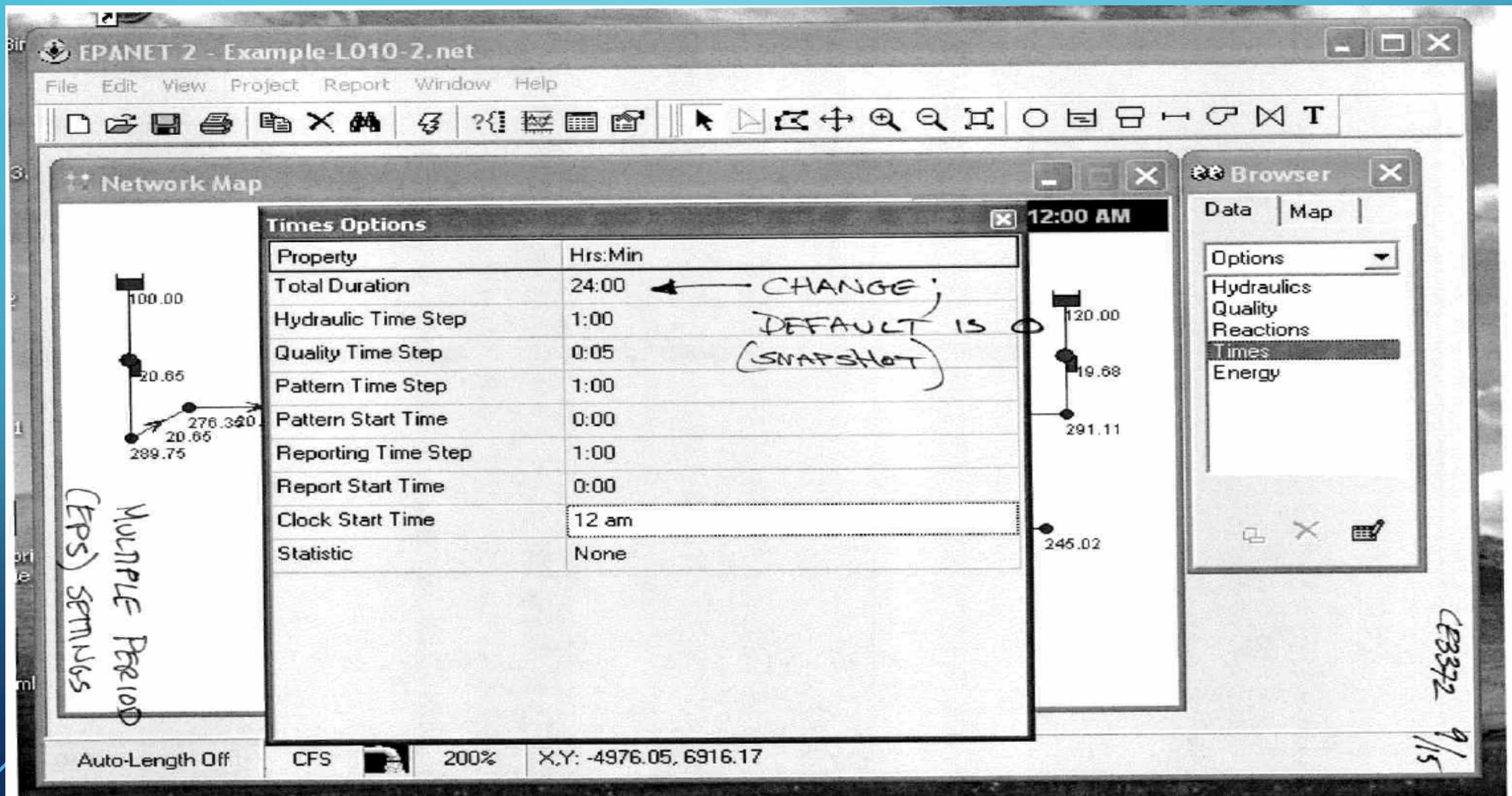
# ILLUSTRATE BY EXAMPLE



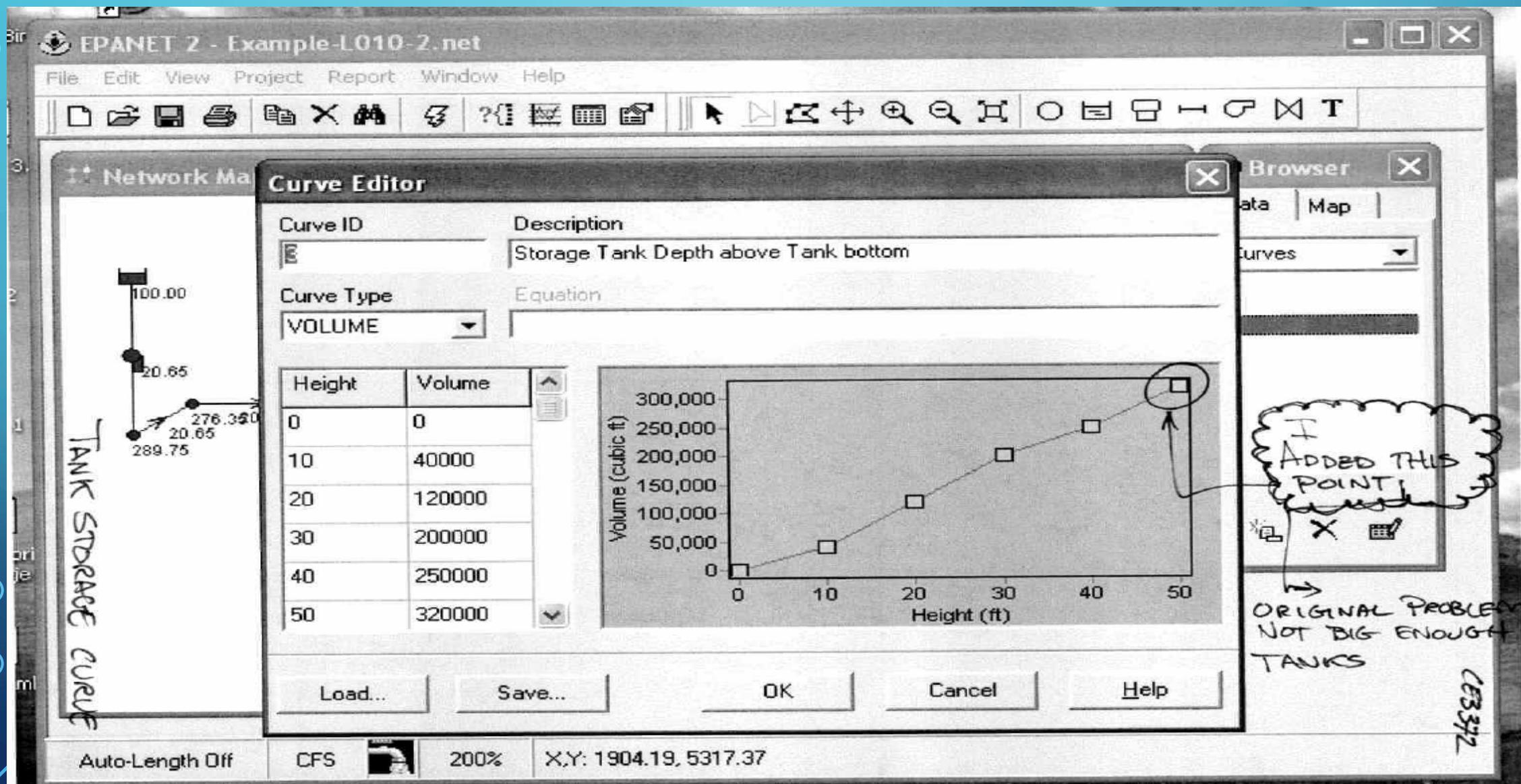
# ILLUSTRATE BY EXAMPLE



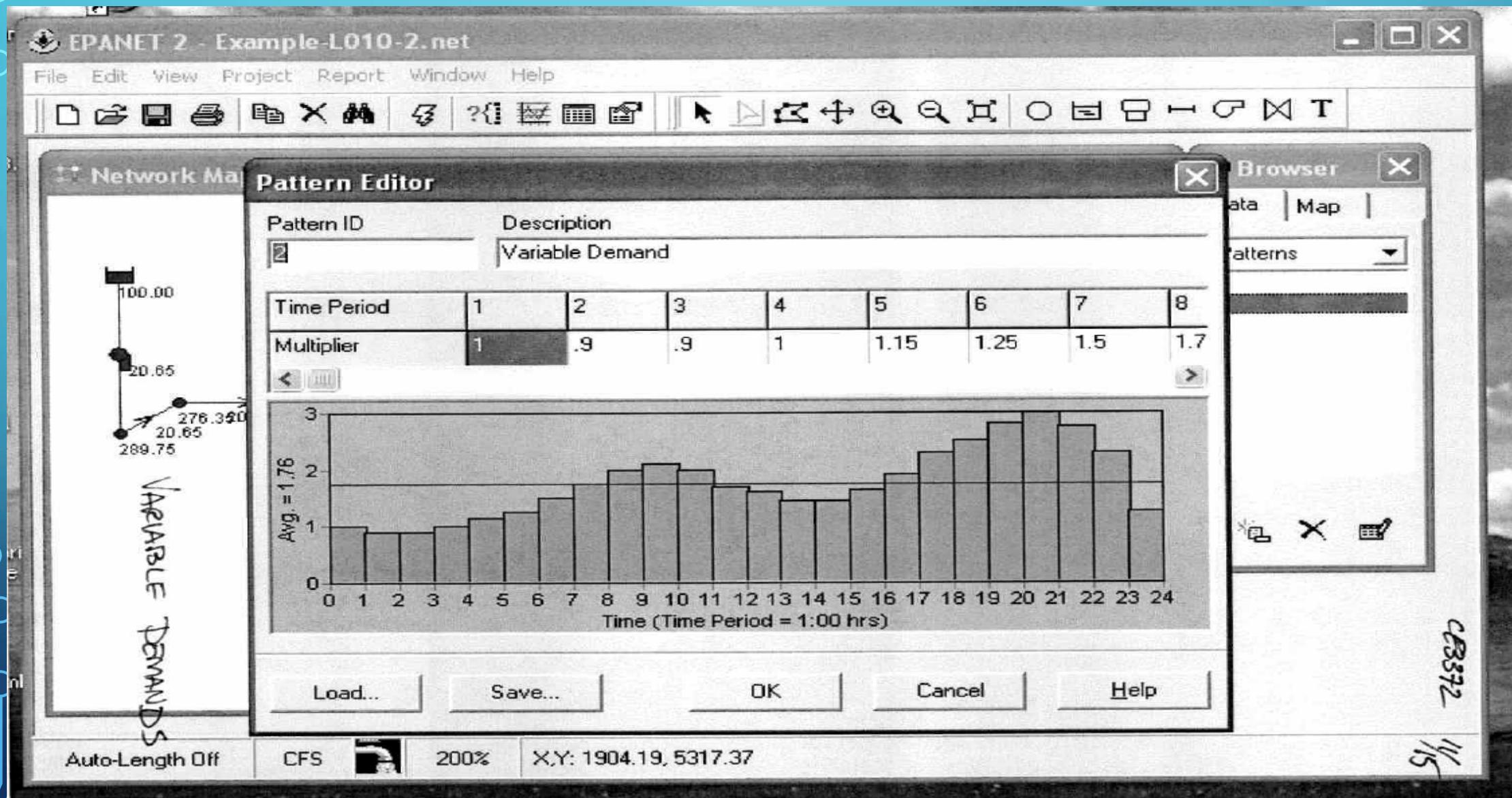
# ILLUSTRATE BY EXAMPLE



# ILLUSTRATE BY EXAMPLE



# ILLUSTRATE BY EXAMPLE



# REPORT OUTPUT

12/15

Page 1  
\*\*\*\*\*  
\* E P A N E T \*  
\* Hydraulic and Water Quality \*  
\* Analysis for Pipe Networks \*  
\* Version 2.0 \*  
\*\*\*\*\*

Input File: Example-L010-2.net

Link - Node Table:

Link ID	Start Node	End Node	Length ft	Diameter in
3	17	1	2000	24
4	1	2	800	24
5	2	3	5000	18
6	3	4	3700	12
7	4	5	3900	15
8	5	18	2100	24
9	5	6	2900	15
10	4	6	2500	10
11	3	7	3100	12
12	2	7	5500	18
13	7	6	3700	15
14	2	10	3100	18
15	10	9	1900	15
16	9	8	2900	15
17	8	13	3100	15
18	13	14	1600	8
19	14	15	1750	6
20	14	16	2700	6
21	13	6	2500	15
22	13	5	4500	15
23	10	11	1600	8
24	11	12	1500	6
25	12	9	1650	8
26	7	8	2700	15
29	StorageTank1	3	700	18
30	StorageTank2	6	900	18
31	StorageTank3	8	1900	18
1	Left_Reservoir	17	#N/A	#N/A Pump
2	Right_Reservoir	18	#N/A	#N/A Pump

FIRST FEW PAGES  
OF FULL STATUS REPORT

SKILLED USER CAN INFER NETWORK  
From START & END NODES

Topology

Pipe#



# REPORT OUTPUT

Page 2

Energy Usage:

Pump	Usage Factor	Avg. Effic.	Kw-hr /Mgal	Avg. Kw	Peak Kw	Cost /day
1	100.00	75.00	882.66	438.17	452.22	0.00
2	100.00	75.00	714.04	378.48	391.05	0.00
			Demand Charge: 0.00			
			Total Cost: 0.00			

Node Results at 0:00 Hrs:

Node ID	Demand CFS	Head ft	Pressure psi	Quality
1	0.00	276.35	80.75	0.00
2	1.54	270.99	69.76	0.00
3	1.54	252.30	68.16	0.00
4	4.64	253.34	64.27	0.00
5	1.54	278.33	77.27	0.00
6	5.40	252.00	64.56	0.00
7	4.64	252.67	67.45	0.00
8	2.32	252.31	64.70	0.00
9	0.00	259.19	65.94	0.00
10	0.00	265.19	66.38	0.00
11	0.77	258.33	62.11	0.00
12	0.77	256.85	62.76	0.00
13	0.00	254.85	62.76	0.00
14	0.00	250.49	56.54	0.00
15	0.39	245.82	47.67	0.00
16	0.39	242.85	48.55	0.00
17	0.00	289.75	82.22	0.00
18	0.00	291.11	74.14	0.00
Left_Reservoir	-28.65	100.00	0.00	0.00 Reservoir
Right_Reservoir	-19.68	120.00	0.00	0.00 Reservoir
StorageTank1	6.76	250.00	13.00	0.00 Tank

1<sup>ST</sup> STRESS PERIOD

Pressures in PSI

HEADS

# REPORT OUTPUT

StorageTank2	5.55	250.00	13.00	0.00	Tank
StorageTank3	4.09	250.00	13.00	0.00	Tank
Link Results at 0:00 Hrs:					
TANKS FILLING					
Link ID	Flow CFS	Velocity fps	Unit Headloss ft/Kft	Status	
3	20.65	6.57	6.70	Open	
4	20.65	6.57	6.70	Open	
5	7.22	4.09	3.74	Open	
6	-0.66	0.84	0.28	Open	
7	-5.86	4.78	6.41	Open	
8	-19.68	6.26	6.89	Open	
9	6.99	5.70	9.08	Open	
Page 3					
Link Results at 0:00 Hrs: (continued)					
Link ID	Flow CFS	Velocity fps	Unit Headloss ft/Kft	Status	
10	0.57	1.04	0.54	Open	
11	-0.42	0.54	0.12	Open	
12	6.81	3.85	3.33	Open	
13	0.95	0.77	0.18	Open	
14	5.08	2.88	1.87	Open	
15	4.10	3.34	3.16	Open	
16	3.54	2.89	2.37	Open	
17	-2.06	1.68	0.82	Open	
18	0.78	2.23	2.73	Open	
19	0.39	1.99	3.12	Open	
20	0.39	1.99	3.12	Open	
21	2.44	1.99	1.14	Open	
22	-5.28	4.31	5.22	Open	
23	0.98	2.82	4.29	Open	
24	0.21	1.09	0.99	Open	
25	-0.56	1.59	1.42	Open	
26	0.80	0.65	0.13	Open	
29	-6.76	3.82	3.28	Open	
30	-5.55	3.14	2.22	Open	
31	-4.09	2.31	1.22	Open	
1	20.65	0.00	-189.75	Open Pump	
2	19.68	0.00	-171.11	Open Pump	
Node Results at 1:00 Hrs:					
Node ID	Demand CFS	Head ft	Pressure psi	Quality	
1	0.00	280.04	82.34	0.00	
2	1.39	274.94	71.47	0.00	
3	1.39	257.10	70.24	0.00	
4	4.18	258.53	66.52	0.00	

NEXT STRESS PERIOD

# REPORT OUTPUT

30	-5.55	3.14	2.22	Open
31	-4.09	2.31	1.22	Open
1	20.65	0.00	-189.75	Open Pump
2	19.68	0.00	-171.11	Open Pump

Node Results at 1:00 Hrs:

Node ID	Demand CFS	Head ft	Pressure psi	Quality
1	0.00	280.04	82.34	0.00
2	1.39	274.94	71.47	0.00
3	1.39	257.18	78.24	0.00
4	4.18	258.53	66.52	0.00
5	1.39	281.90	78.82	0.00
6	4.86	256.42	66.48	0.00
7	4.18	257.20	69.41	0.00
8	2.09	256.32	66.43	0.00
9	0.00	263.43	67.78	0.00
10	0.00	269.35	68.18	0.00
11	0.69	263.37	64.29	0.00
12	0.69	261.74	64.88	0.00
13	0.00	259.08	64.59	0.00
14	0.00	255.52	58.72	0.00
15	0.35	251.06	58.29	0.00
16	0.35	248.63	51.40	0.00
17	0.00	292.78	83.53	0.00
18	0.00	294.01	75.40	0.00
Left_Reservoir	-20.13	100.00	0.00	0.00 Reservoir

Page 4

Node Results at 1:00 Hrs: (continued)

Node ID	Demand CFS	Head ft	Pressure psi	Quality
Right_Reservoir	-19.14	120.00	0.00	0.00 Reservoir
StorageTank1	6.66	254.87	15.11	0.00 Tank
StorageTank2	6.12	253.99	14.73	0.00 Tank
StorageTank3	4.95	252.94	14.27	0.00 Tank

Link Results at 1:00 Hrs:

Link ID	Flow CFS	Velocity fps	Unit Headloss ft/Kft	Status

PUMP BEHAVIOR

NEXT STRESS PERIOD

# REPORT OUTPUT

1	19.25	0.00	-197.62	Open Pump
2	18.32	0.00	-178.24	Open Pump

Node Results at 6:00 Hrs:

Node ID	Demand CFS	Head ft	Pressure psi	Quality
1	0.00	285.74	84.82	0.00
2	2.31	281.87	74.12	0.00
3	2.31	268.66	75.25	0.00
4	6.96	265.69	69.63	0.00
5	2.31	286.57	80.84	0.00
6	8.10	266.66	70.91	0.00
7	6.96	266.48	73.43	0.00
8	3.48	266.24	70.73	0.00
9	0.00	270.36	70.78	0.00
10	0.00	275.37	70.79	0.00
11	1.15	263.58	64.38	0.00
12	1.15	262.89	65.38	0.00
13	0.00	268.12	68.52	0.00
14	0.00	258.51	60.02	0.00
15	0.58	246.50	48.31	0.00
16	0.58	239.97	47.65	0.00
17	0.00	297.44	85.55	0.00
18	0.00	297.78	77.03	0.00
Left_Reservoir	-19.28	100.00	0.00	0.00 Reservoir
Right_Reservoir	-18.41	120.00	0.00	0.00 Reservoir
StorageTank1	1.37	268.55	21.04	0.00 Tank
StorageTank2	-1.24	266.76	20.26	0.00 Tank
StorageTank3	1.66	265.85	19.86	0.00 Tank

TANK DRAINING

# REPORT OUTPUT

23	1.30	2.72	7.30	Open
24	0.14	0.73	0.46	Open
25	-1.01	2.90	4.52	Open
26	0.64	0.52	0.09	Open
29	-1.37	0.78	0.15	Open
30	1.24	0.70	0.12	Open
31	-1.66	0.94	0.21	Open
1	19.28	0.00	-197.44	Open Pump
2	18.41	0.00	-177.78	Open Pump

) PUMPS PRODUCING less Q

Node Results at 7:00 Hrs:

Node ID	Demand CFS	Head ft	Pressure psi	Quality
1	0.00	285.05	84.51	0.00
2	2.69	280.32	73.80	0.00
3	2.69	269.26	75.51	0.00
4	8.12	262.59	68.28	0.00
5	2.69	284.82	80.08	0.00
6	9.45	265.78	70.53	0.00
7	8.12	265.85	73.16	0.00
8	4.06	266.43	70.81	0.00
9	0.00	269.68	70.49	0.00
10	0.00	274.44	70.39	0.00
11	1.35	259.39	62.56	0.00
12	1.35	258.86	63.63	0.00

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Node Results at 7:00 Hrs: (continued)

Node ID	Demand CFS	Head ft	Pressure psi	Quality
13	0.00	267.47	68.23	0.00
14	0.00	254.47	58.27	0.00
15	0.68	238.23	44.73	0.00
16	0.68	229.42	43.08	0.00
17	0.00	296.87	85.31	0.00
18	0.00	296.36	76.42	0.00
Left_Reservoir	-19.39	100.00	0.00	0.00 Reservoir
Right_Reservoir	-18.69	120.00	0.00	0.00 Reservoir
StorageTank1	-0.20	269.26	21.34	0.00 Tank
StorageTank2	-2.25	266.13	19.99	0.00 Tank
StorageTank3	-1.36	266.70	20.23	0.00 Tank

] TANKS DRAINING

Link Results at 7:00 Hrs:

Link ID	Flow CFS	Velocity fps	Unit Headloss ft/kft	Status
1	19.28	6.17	5.94	Open

# CE 3372 WATER SYSTEMS DESIGN

LESSON 11 PART 2 : WATER QUALITY IN EPANET FALL 2020

# OUTLINE

- EPA-NET Water Quality Models

- Theory:

- Advection Transport in Pipeline
- Decay

- Practice:

- Estimate water age in system
- Estimate concentration of constituent at different points in network
- Respond to intrusions into the system

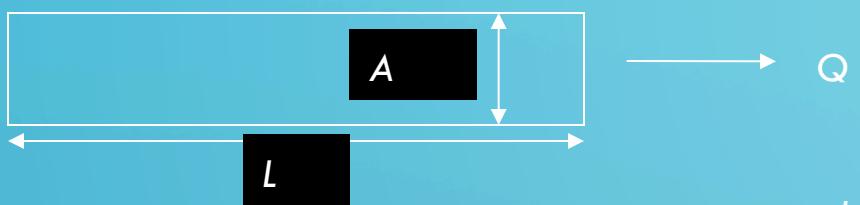
# WATER QUALITY IN EPANET

- Transport theory in EPANET
  - Lagrangian Approach  
(Discrete Parcel Advection)
  - Mixing Approach (in tanks)

# ADVECTIVE TRANSPORT

- Advection (convection) is the transport of dissolved or suspended material by motion of the host fluid.
- Requires knowledge of the fluid velocity field (the velocity of a fluid particle)
  - Velocity from EPANET hydraulics

# MEAN SECTION VELOCITY



Pipe Segment:  
Volume =  $L \cdot A$   
Flow Rate =  $Q$

Displacement of one segment volume takes a certain time,  $\Delta t$

$$\Delta t = L \cdot A / Q$$

Distance traveled by marker is segment length,  $L$ ;  
Marker velocity is distance/time

$$u = \frac{Q}{A}$$

$t=0$

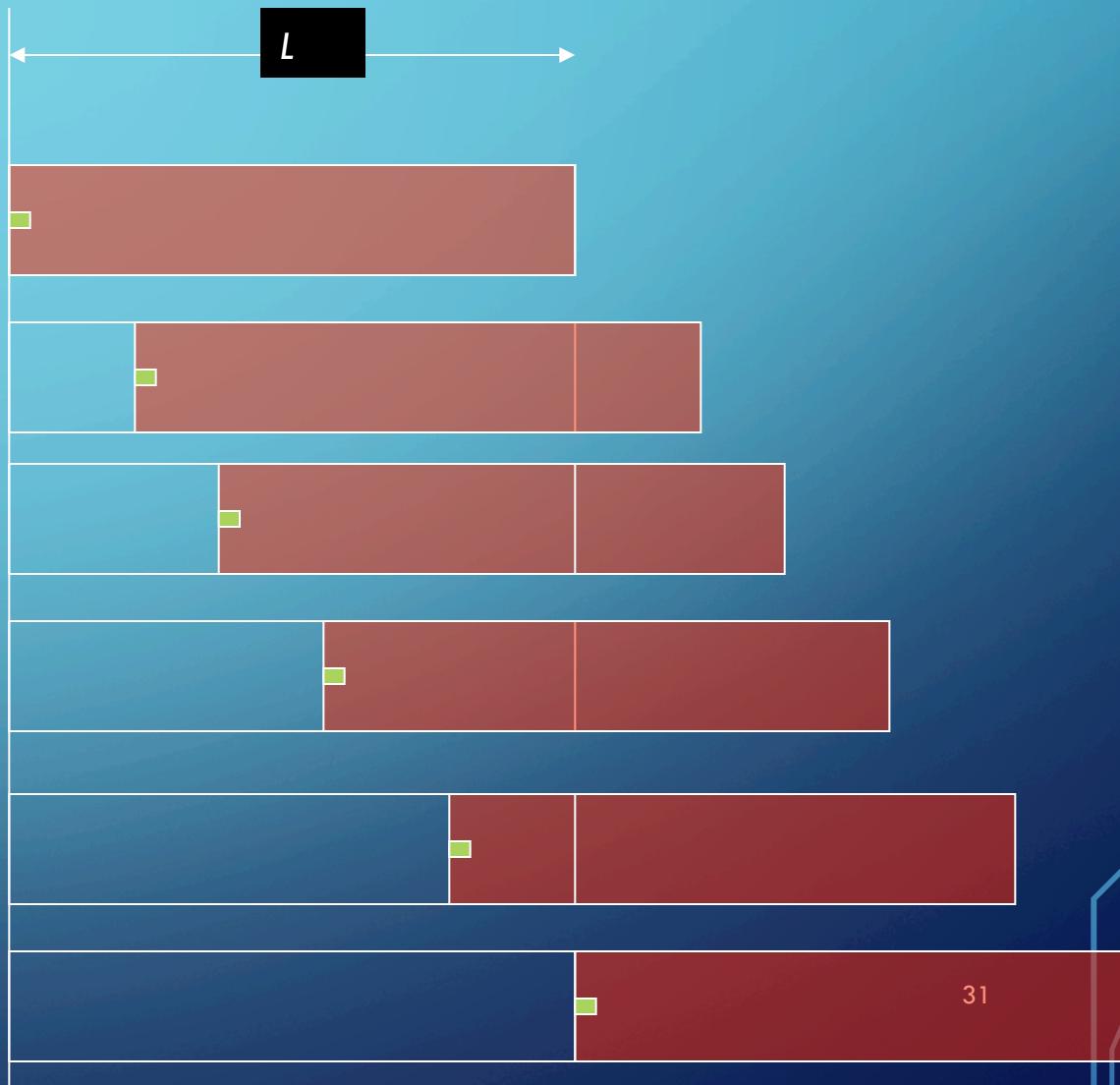
$t=0.2 \Delta t$

$t=0.4 \Delta t$

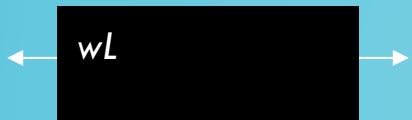
$t=0.6 \Delta t$

$t=0.8 \Delta t$

$t=1.0 \Delta t$



# MASS FLUX



Suppose one “blue” volume enters the pipe segment.



The mass of “blue” per unit volume is the concentration of blue.



Let one pipe volume enter the segment.



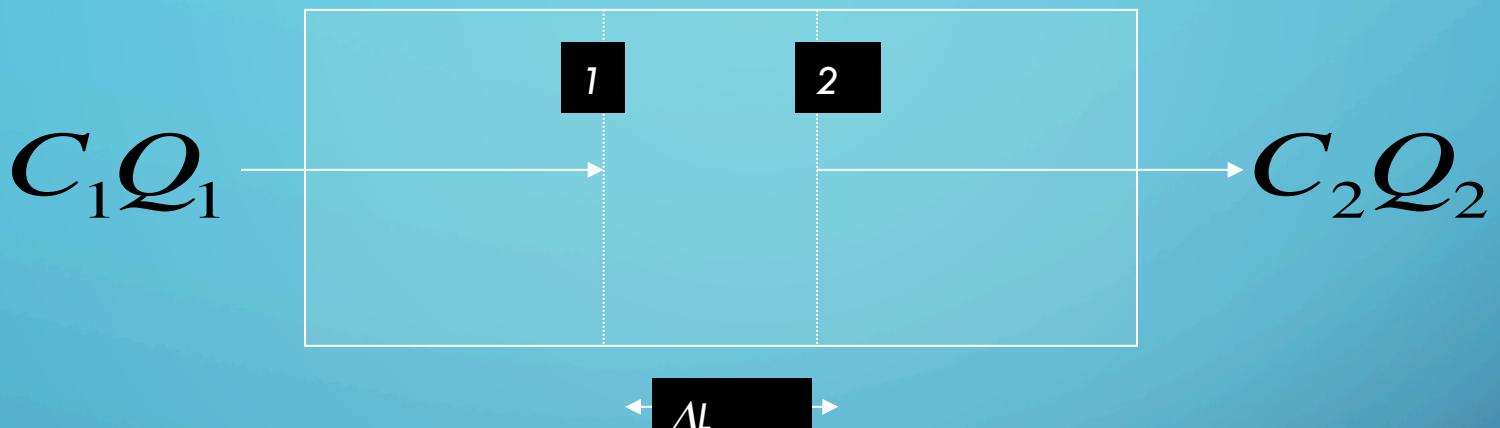
Total mass of blue in the segment is the concentration\*fluid volume

$$M_{blue} = ALC_{blue}$$

$$\frac{M_{blue}}{\Delta t} = \frac{ALC_{blue}Q}{AL} = c_{blue}Q$$

# MASS BALANCE

Now consider a small portion of the pipe.



Mass flow into segment.

$$C_1 Q_1$$

Mass flow out of segment.

$$C_2 Q_2$$

Rate of accumulation in segment.

$$= \frac{\partial}{\partial t} [CA\Delta L]$$

# BALANCE EQUATIONS

For a non-deforming medium this mass balance is expressed as:

$$\frac{C_1 Q_1 - C_2 Q_2}{A \Delta L} = \frac{\partial C}{\partial t}$$

Substituting the definition of average linear velocity:

$$\frac{C_1 u_1 - C_2 u_2}{\Delta L} = \frac{\partial C}{\partial t}$$

Taking the limit as  $\Delta L$  vanishes produces the fundamental equation governing convective transport.

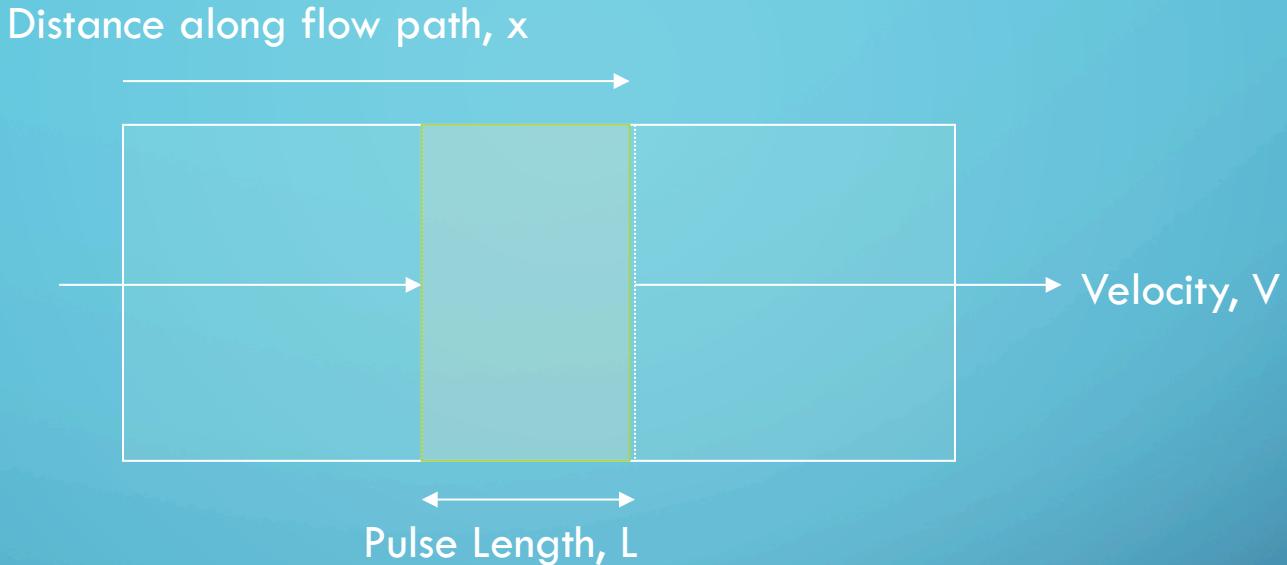
$$-\frac{\partial(uC)}{\partial L} = \frac{\partial C}{\partial t}$$

# GENERALIZATION

$$\frac{\partial C}{\partial t} = -\operatorname{div}(\vec{U}C)$$

- Express last term in more conventional form – divergence of the mass flux is equal to the rate of change of concentration at a point.
- Observe the obvious dependence on the velocity field  $(u, v, w)$ .
- In order to compute any mass fluxes we must first determine the velocity values in the domain of interest.

# ANALYTICAL MODEL



- Water at a constant velocity,  $V$ , is flowing through the zone carrying the dissolved component at a specific concentration,  $C_o$ .
- There is no degradation of the component, no dispersion of the component, nor is there any interaction with the solid phase (walls).
- The zone translates in space at a rate determined by the water velocity.
- The contaminant is dissolved, and does not alter the density of the flowing water.
- The contaminant is assumed to be uniformly mixed in contaminated zone.

# GOVERNING EQUATIONS, INITIAL, AND BOUNDARY CONDITIONS

The governing equation of mass transport for this case is:

$$\frac{\partial C}{\partial t} = -v \frac{\partial C}{\partial x}$$

The initial conditions throughout the pipe segment are:

$$C(x, t) = 0 \text{ for all } t \geq \frac{L}{v}, x = 0$$

$$C(x, t) = C_o \text{ for all } t \geq 0, t \leq \frac{L}{v} \text{ at } x = 0$$

$$C(x, t) = 0 \text{ for all } x > 0, t = 0$$

The boundary conditions at the source are:

$$C(x, t) = 0 \text{ for } t \leq 0; x \notin [-L, 0]$$

$$C(x, t) = 0 \text{ when } x \leq vt - L$$

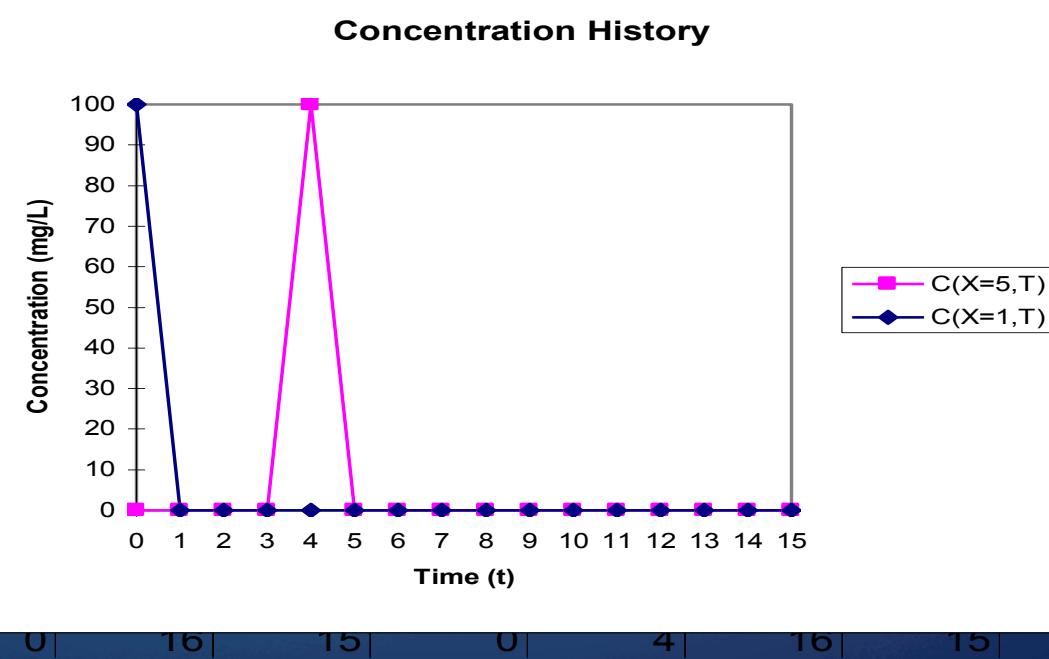
$$C(x, t) = C_o \text{ when } vt - L \leq x \leq vt$$

$$C(x, t) = 0 \text{ when } x > vt$$

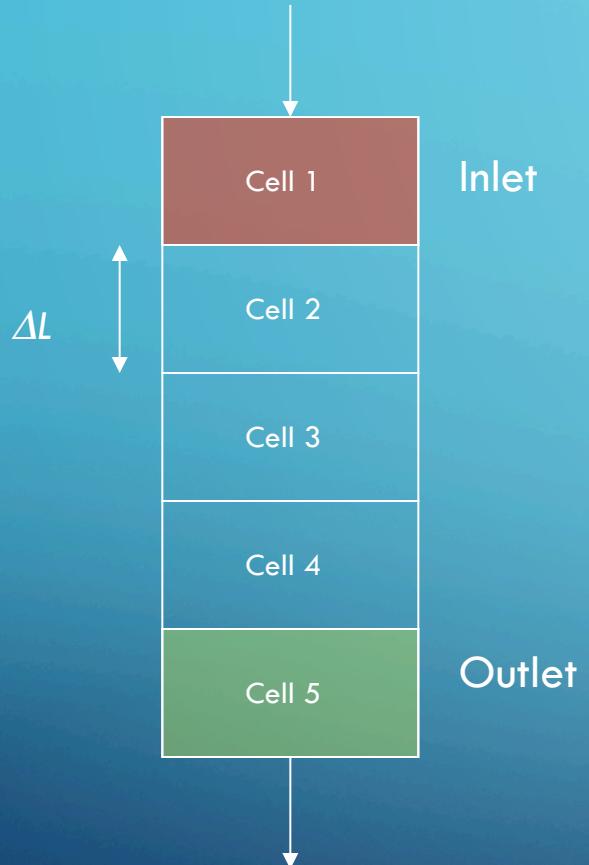
The solution for this case is:

	A	B	C	D	E	F	G	H	I
1	Co	100		$=\$B\$2*A6$		$=C6-\$B\$3$			
2	V	1							
3	L	1							
4									
5	T	X		VT	VT-L	$C(X=1,T)$			
6	0	0	0	-1	100	4	0	-1	0
7	1	0	1	0	0	4	1	0	0
8	2	0	2	1	0	4	2	1	0
9	3	0	3	2	0	4	3	2	0
10	4	0	4	3	0	4	4	3	100
11	5								0
12	6								0
13	7								0
14	8								0
15	9								0
16	10								0
17	11								0
18	12								0
19	13								0
20	14								0
21	15								0
22	16								0

$=IF(OR(B6<=D6,B6>C6),0,\$B\$$



# CELL BALANCE MODEL APPROACH

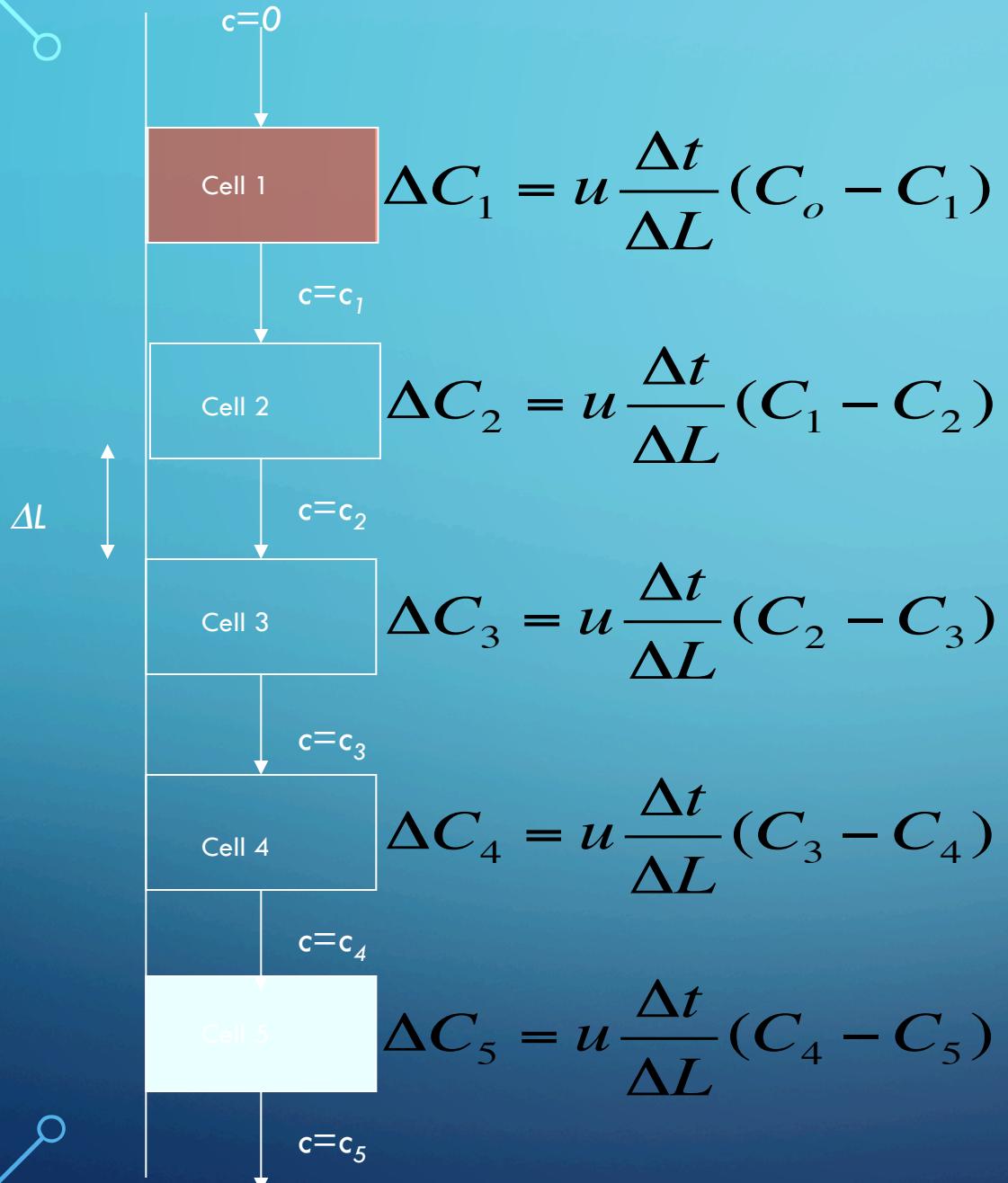


Suppose at  $t=0$  the concentration in the inlet cell is  $C_0$ . We want to determine the concentration in the pipe segment at future times.

We will assume the velocity is identical throughout the column.

A simple modeling approach is to treat each cell as completely mixed.

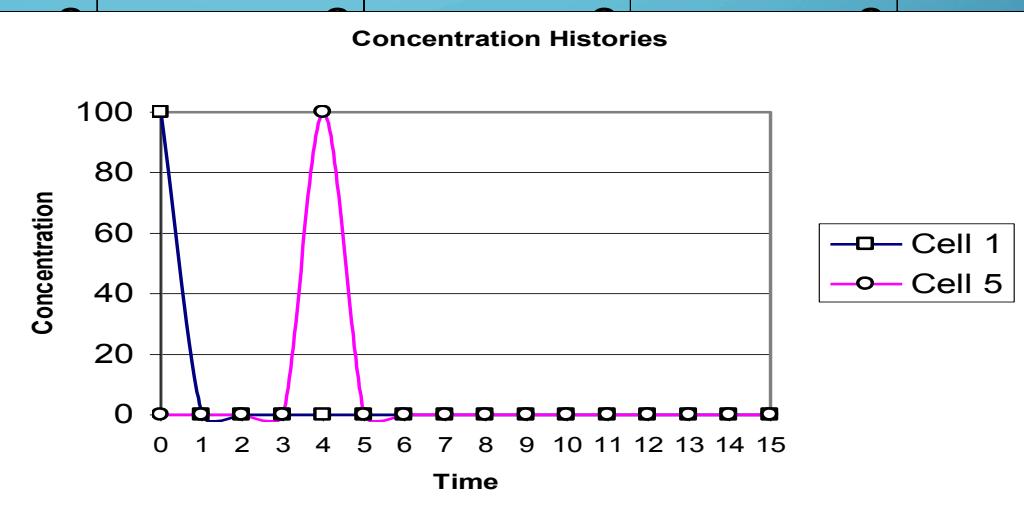
This means that the concentration at the cell exit is identical to the concentration in the cell.



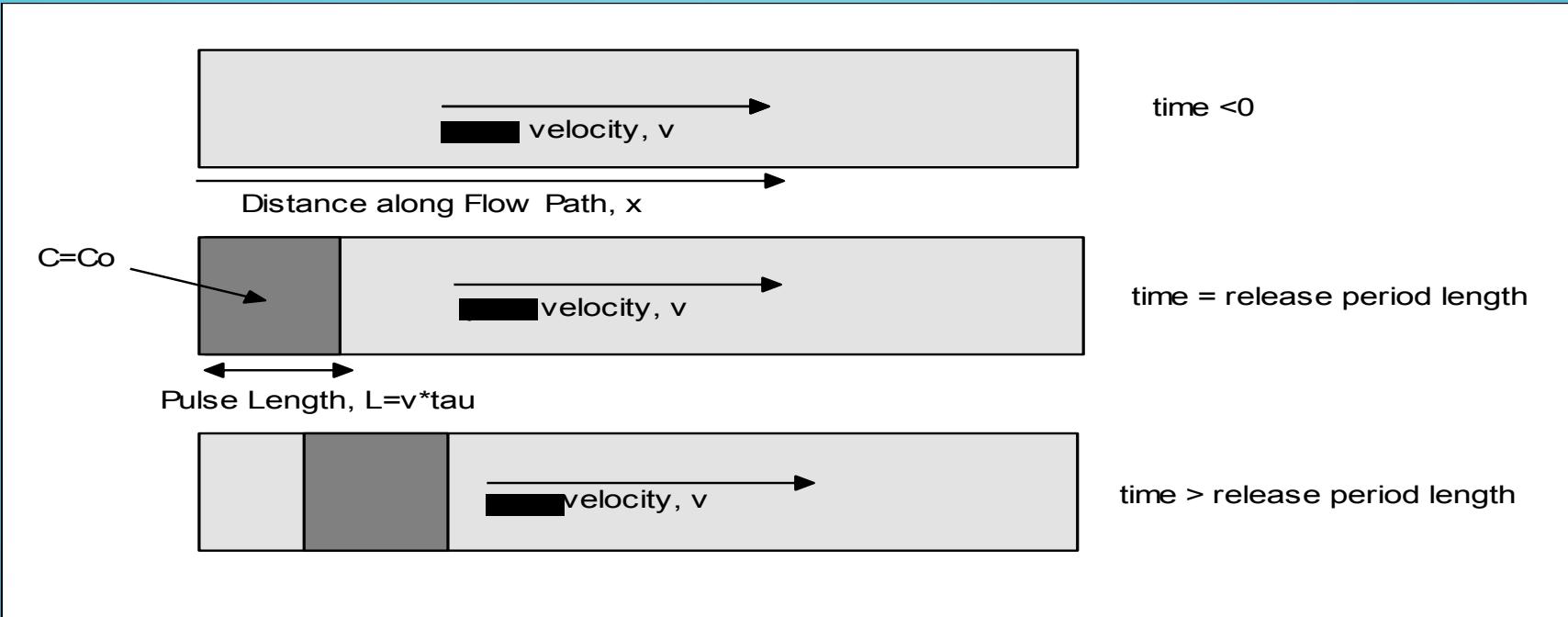
$$\frac{C_{in}u_{in} - C_{out}u_{out}}{\Delta L} = \frac{\Delta C}{\Delta t}$$

	A	B	C	D	E	F
1	$u$		1	$Co$	100	
2	$\Delta L$		1			
3	$\Delta t$		1			
4	$t$	Cell 1	Cell 2	Cell 3	Cell 4	Cell 5
5	0	100	0	0	0	0
6	1	0	100	0	0	0
7	2	0	0	100	0	0
8	3	0	0	0	100	0
9	4	0	0	0	0	100
10	5					0
11	6					0
12	7					0
13	8					0
14	9					0
15	10					0
16	11					0
17	12					0
18	13	0	0	0	0	0
19	14	0	0	0	0	0
20	15	0	0	0	0	0

=C5+(\$B\$1\*\$B\$3/\$B\$2)\*(B5-C5)



# TIMED RELEASE CASE



- At the origin ( $x=0$ ) a contaminant is added to the flowing water at fixed concentration  $C_0$  for a period of time  $t$ .
- At the end of the time period the contaminant addition is stopped.
- By the end of the time period a “parcel” of contaminated water is created.
- Mechanism of release does not disturb the local flow field in any fashion.
- Contaminant is assumed to be uniformly mixed in the parcel (zone)

# SOLUTION

$C(x, t) = 0$  when  $x \leq vt - v\tau$

$C(x, t) = C_o$  when  $vt - v\tau \leq x \leq vt$

$C(x, t) = 0$  when  $x > vt$

- Solution identical to first case.
- Substitute  $v\tau = L$  into the previous solution.

# IN EPANET HOW THESE ARE IMPLEMENTED

## Basic Transport

EPANET's water quality simulator uses a Lagrangian time-based approach to track the fate of discrete parcels of water as they move along pipes and mix together at junctions between fixed-length time steps. These water quality time steps are typically much shorter than the hydraulic time step (e.g., minutes rather than hours) to accommodate the short times of travel that can occur within pipes.

## MIXING AT NODE

- When parcels (concentration) reaches a node where there are multiple mass fluxes, a flow-weighted mixing model is used to compute the concentration at that node (which will become a new  $C_0$  for any downstream links)

$$C_{out} = \frac{\sum C_{in} Q_{in}}{\sum Q_{in}}$$

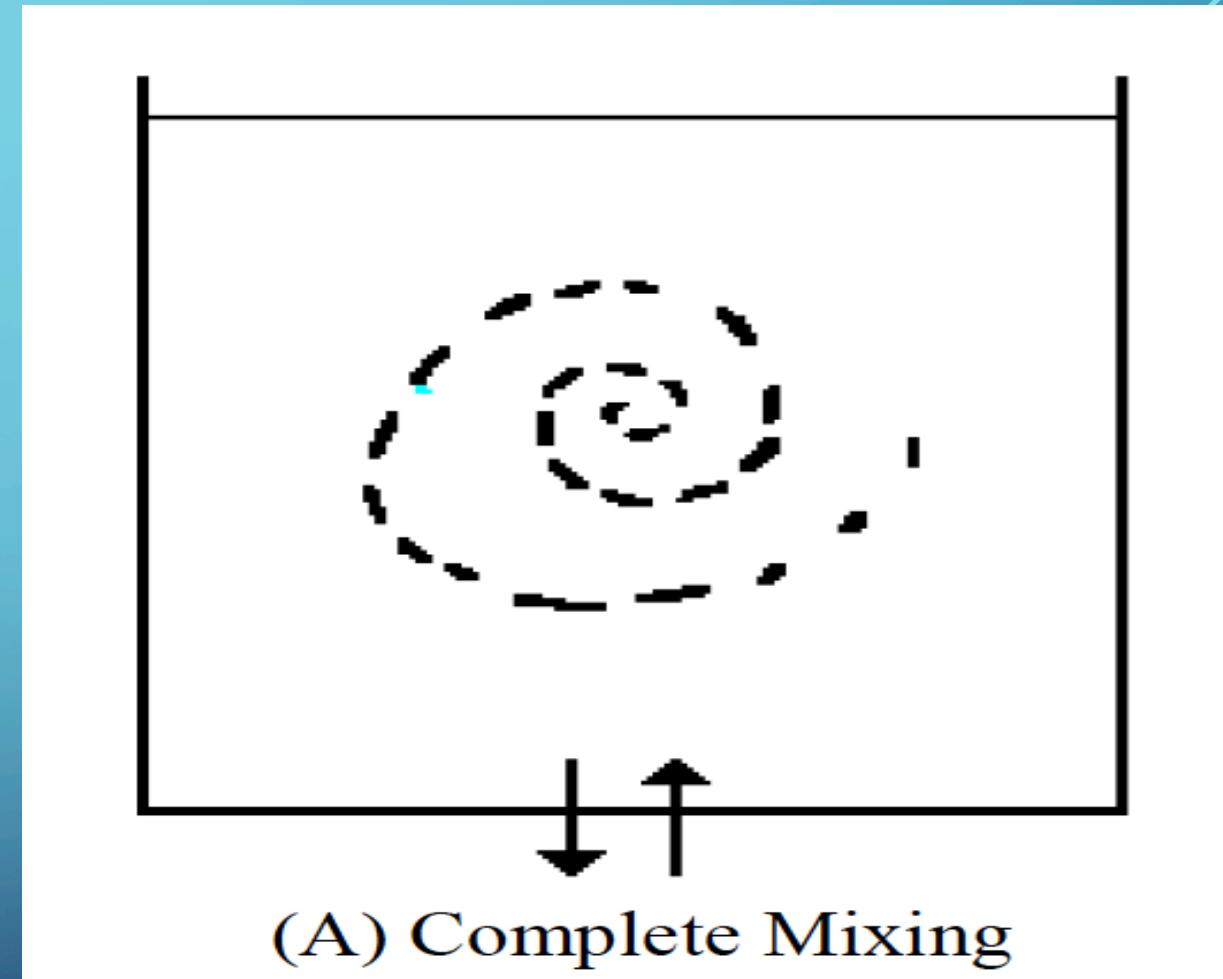
# MIXING IN A TANK

- Tank mixing is handled by four possible models:

- Completely mixed (CFSTR)
- Two-Compartment Mixing
- FIFO Plug Flow
- LIFO Plug Flow

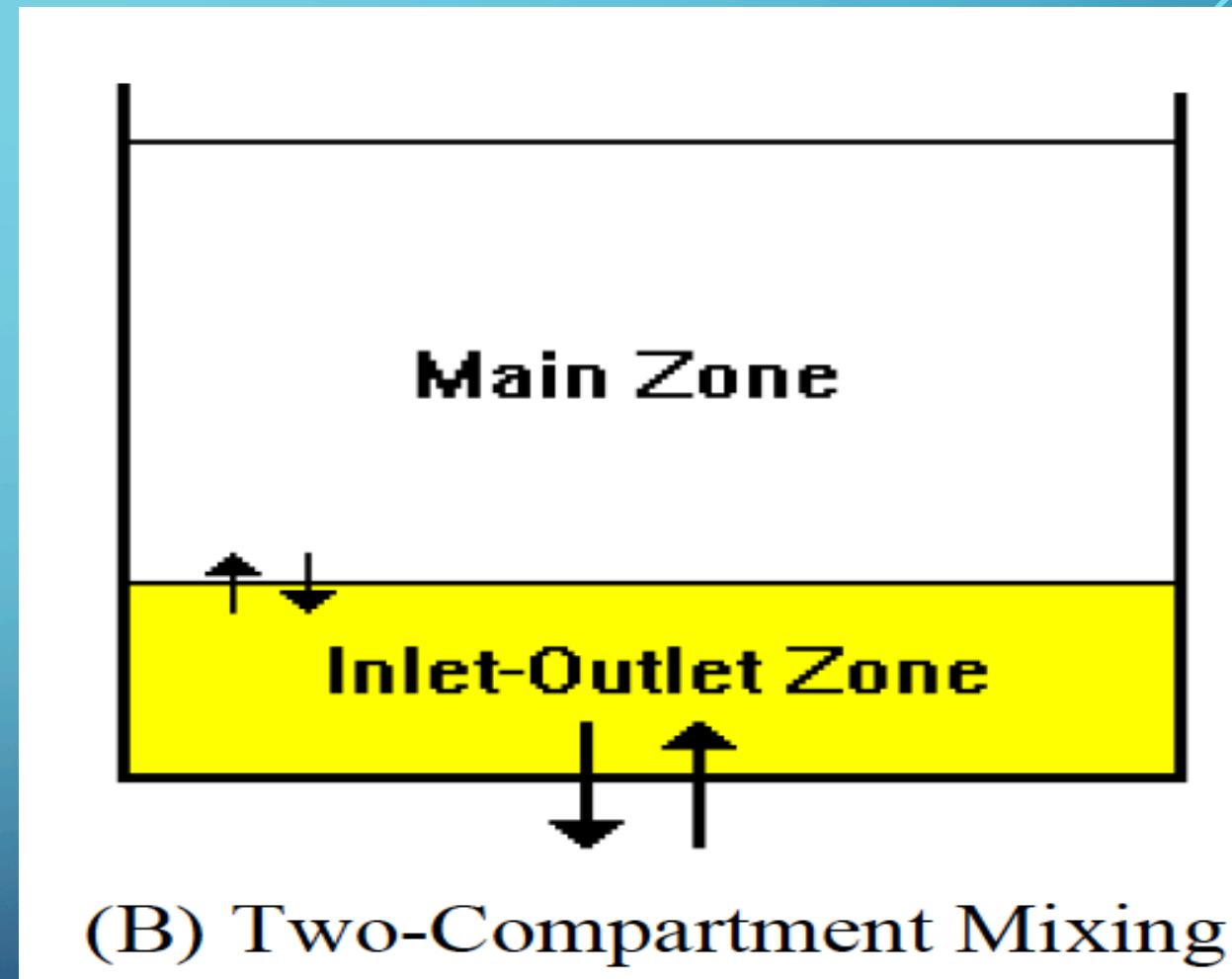
## COMPLETELY MIXED

- All water entering tank instantly and completely mixes
- Reasonable for small tanks, or hydraulic time steps that are long compared to transport time steps



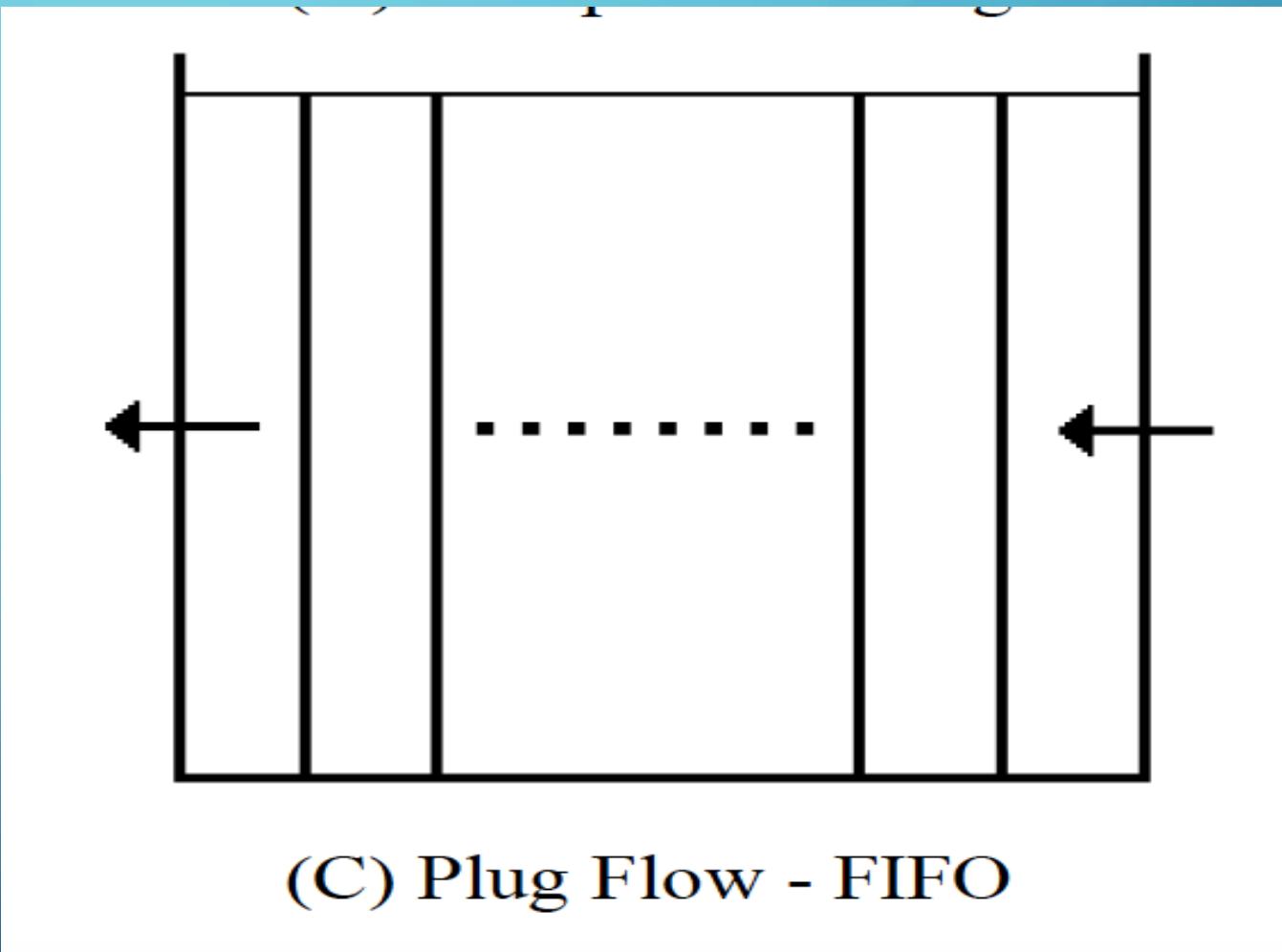
# TWO-COMPARTMENT MIXING

- Tank storage divided into two compartments
  - Inlet/Outlet zone
  - Main Zone
- When Inlet/Outlet zone is filled, then spills into main zone



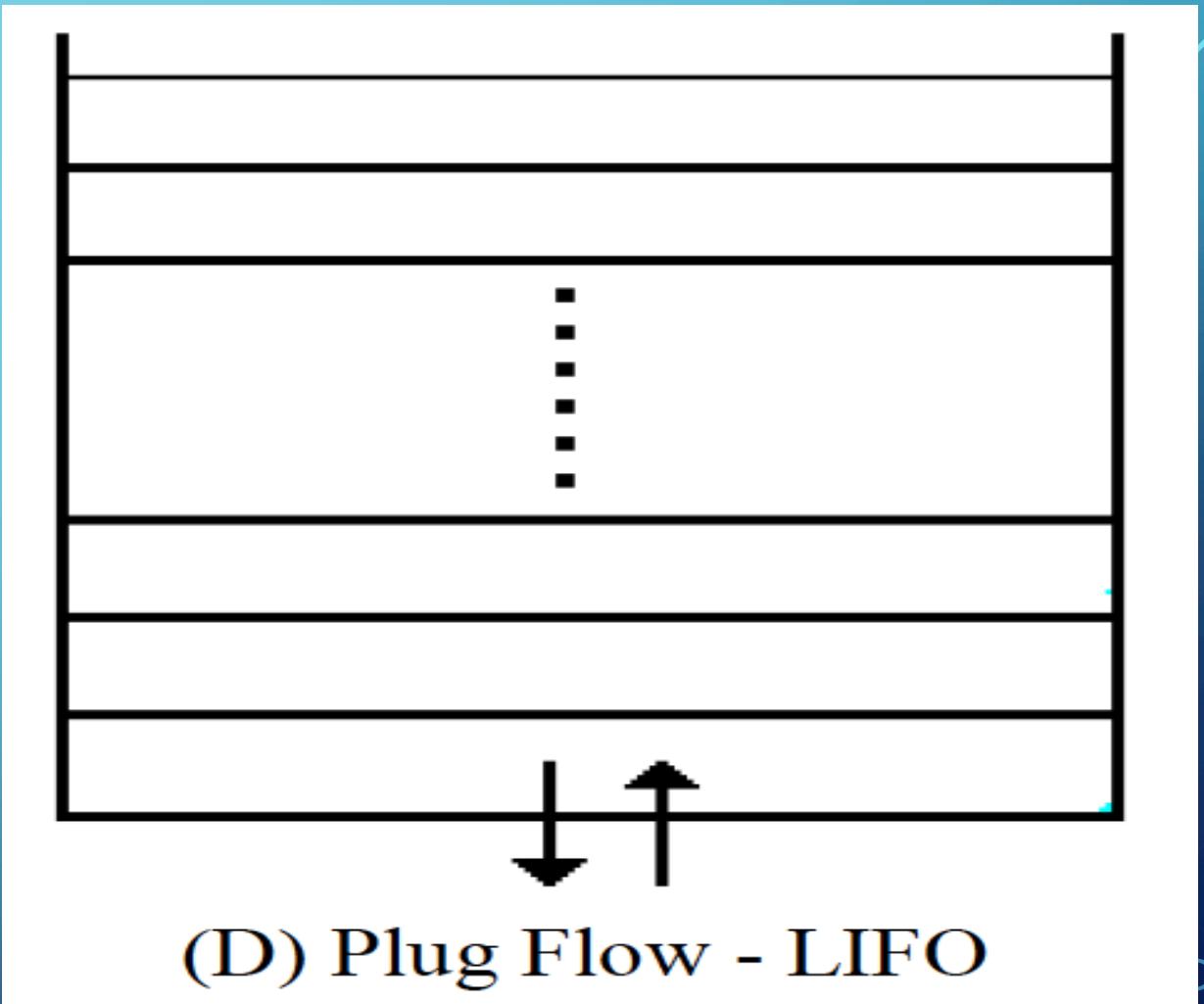
# FIFO MIXING

- The first parcel (volume) of water to enter the tank, is first parcel to leave
- Essentially plug-flow in the tank



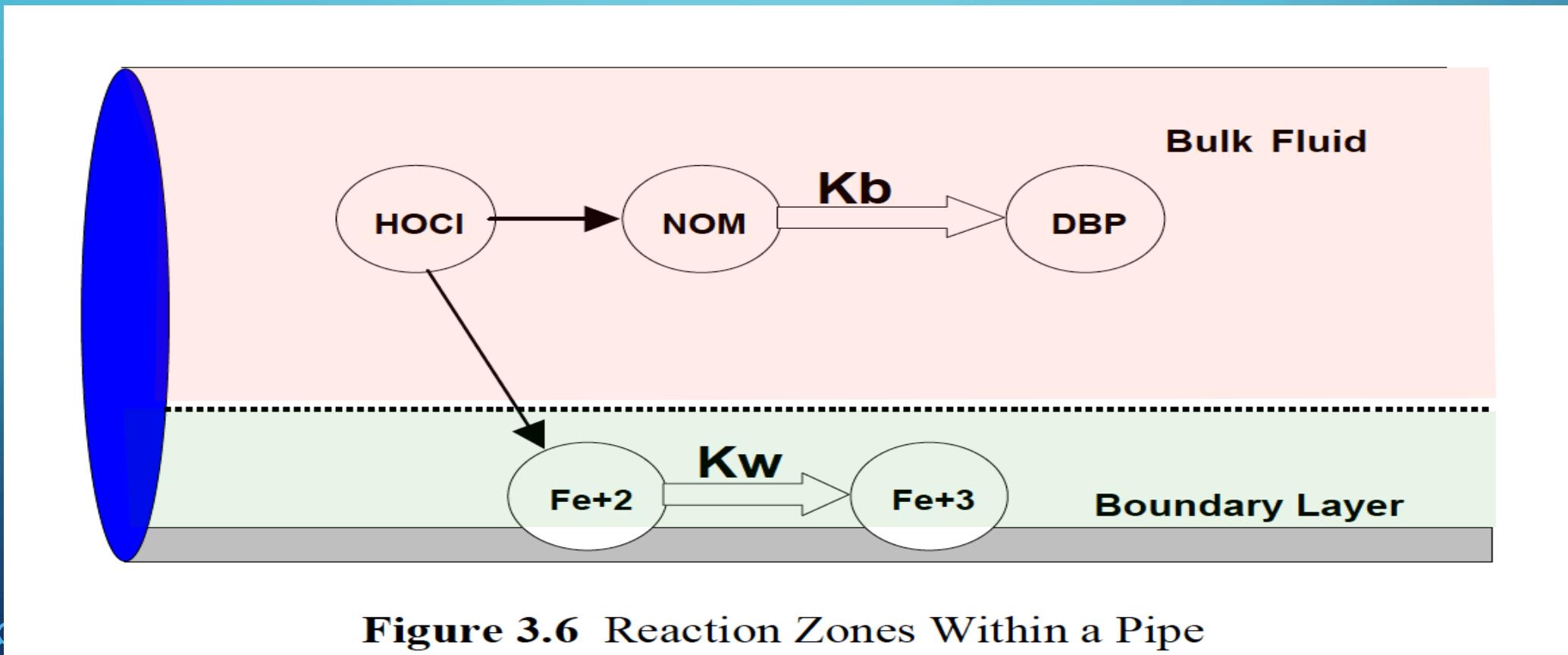
# LIFO MIXING

- The last (most recent) parcel (volume) of water to enter the tank, is first parcel to leave
- Essentially stratified-flow in the tank



# WATER QUALITY REACTIONS

- Bulk reactions (in the parcel)
- Wall reactions (at the parcel, pipe-wall interface)



# BULK REACTIONS

- Growth and decay of constituent in the bulk phase (parcel)
- Uses choice of
- No reaction
- Zero-Order kinetics
- 1-st Order Decay
- 1-st Order Saturation

$$\frac{\partial C}{\partial t} = -\operatorname{div}(\vec{U}C) + r(C)$$

$$r(C) = \begin{cases} K_b C^n \\ K_b(C_L - C) \\ K_b C(C - C_L) \\ \frac{K_b C}{C_L - C} \end{cases}$$

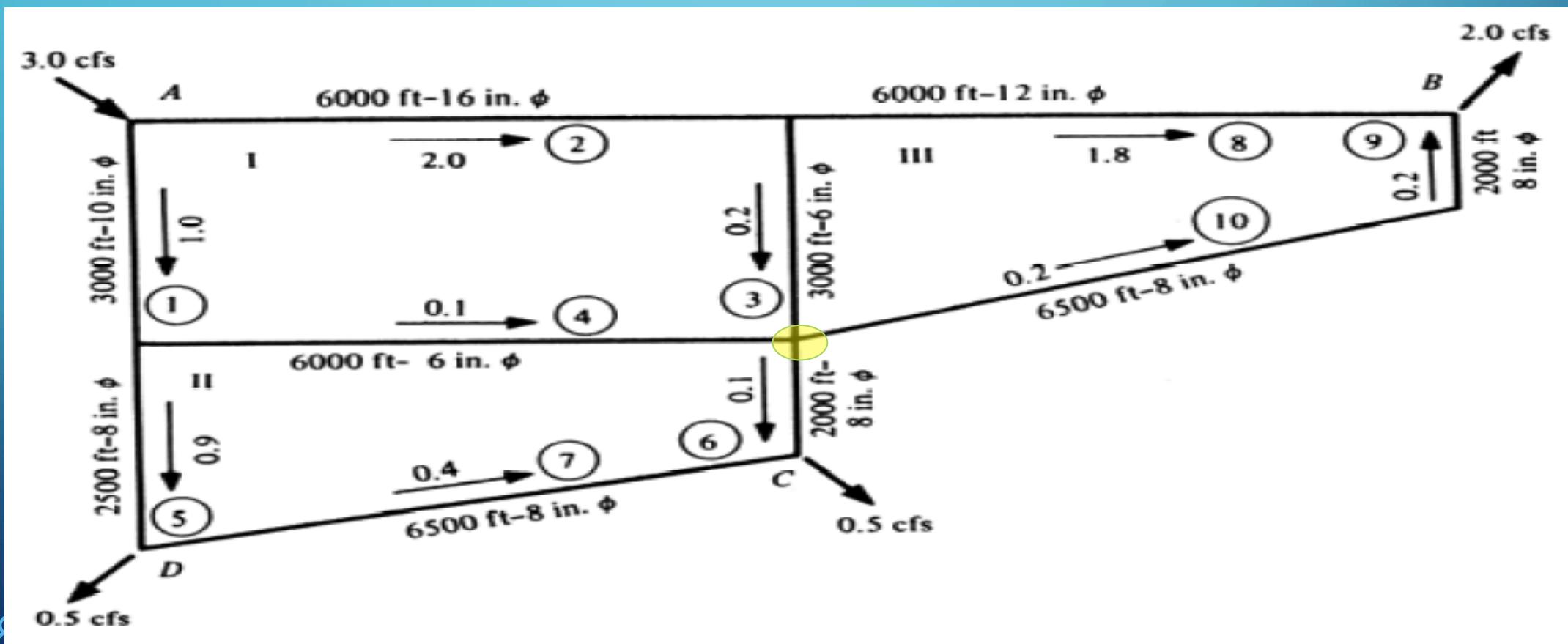
<i>Model</i>	<i>Parameters</i>	<i>Examples</i>
First-Order Decay	$C_L = 0, K_b < 0, n = 1$	Chlorine
First-Order Saturation Growth	$C_L > 0, K_b > 0, n = 1$	Trihalomethanes
Zero-Order Kinetics	$C_L = 0, K_b \neq 0, n = 0$	Water Age
No Reaction	$C_L = 0, K_b = 0$	Fluoride Tracer

# WALL REACTIONS

- Handled in similar fashion – generally a secondary reaction term based on location in the network

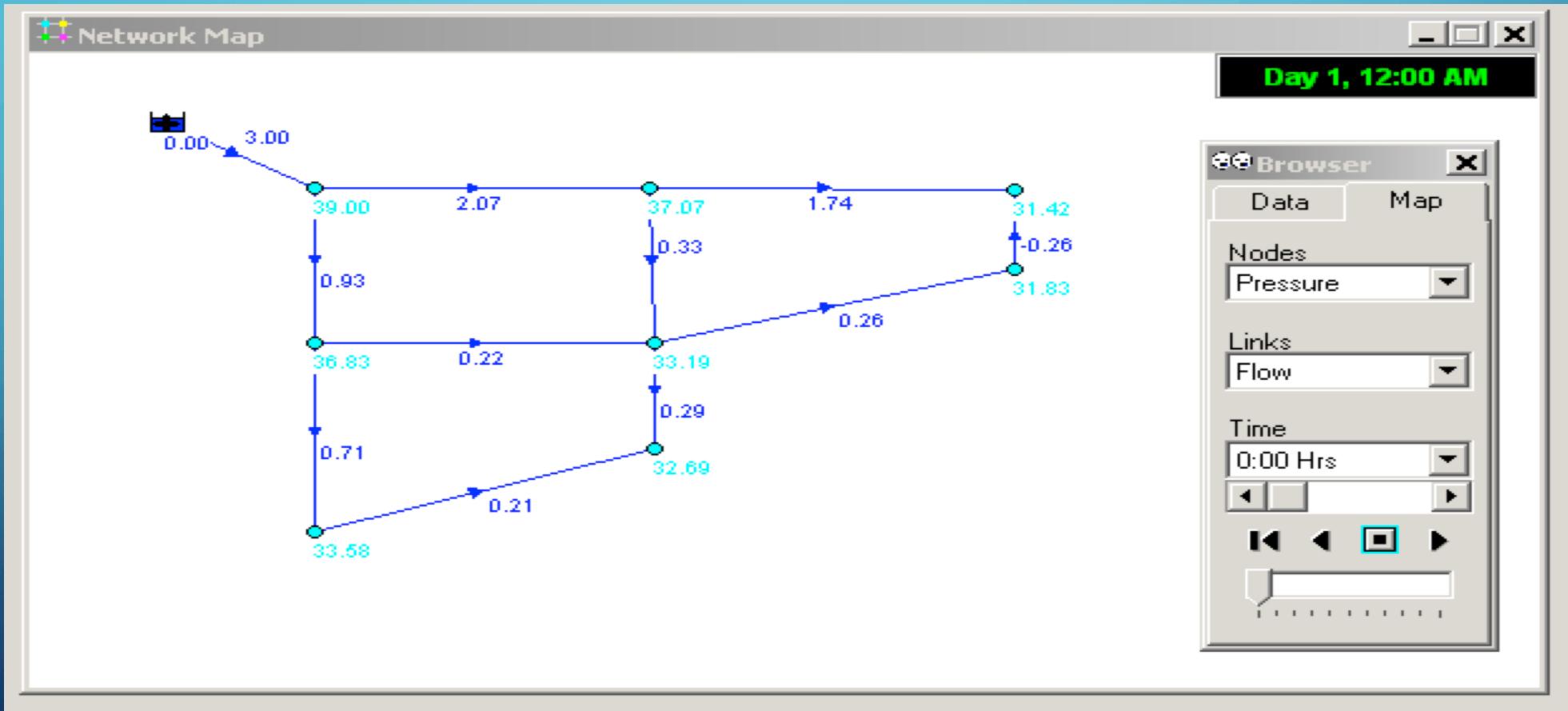
# EXAMPLE

- What is the disinfection residual in the system below if the source water has chloramine at 10 mg/L and the first-order decay mass transfer coefficient ( $K_b$ ) is -5?

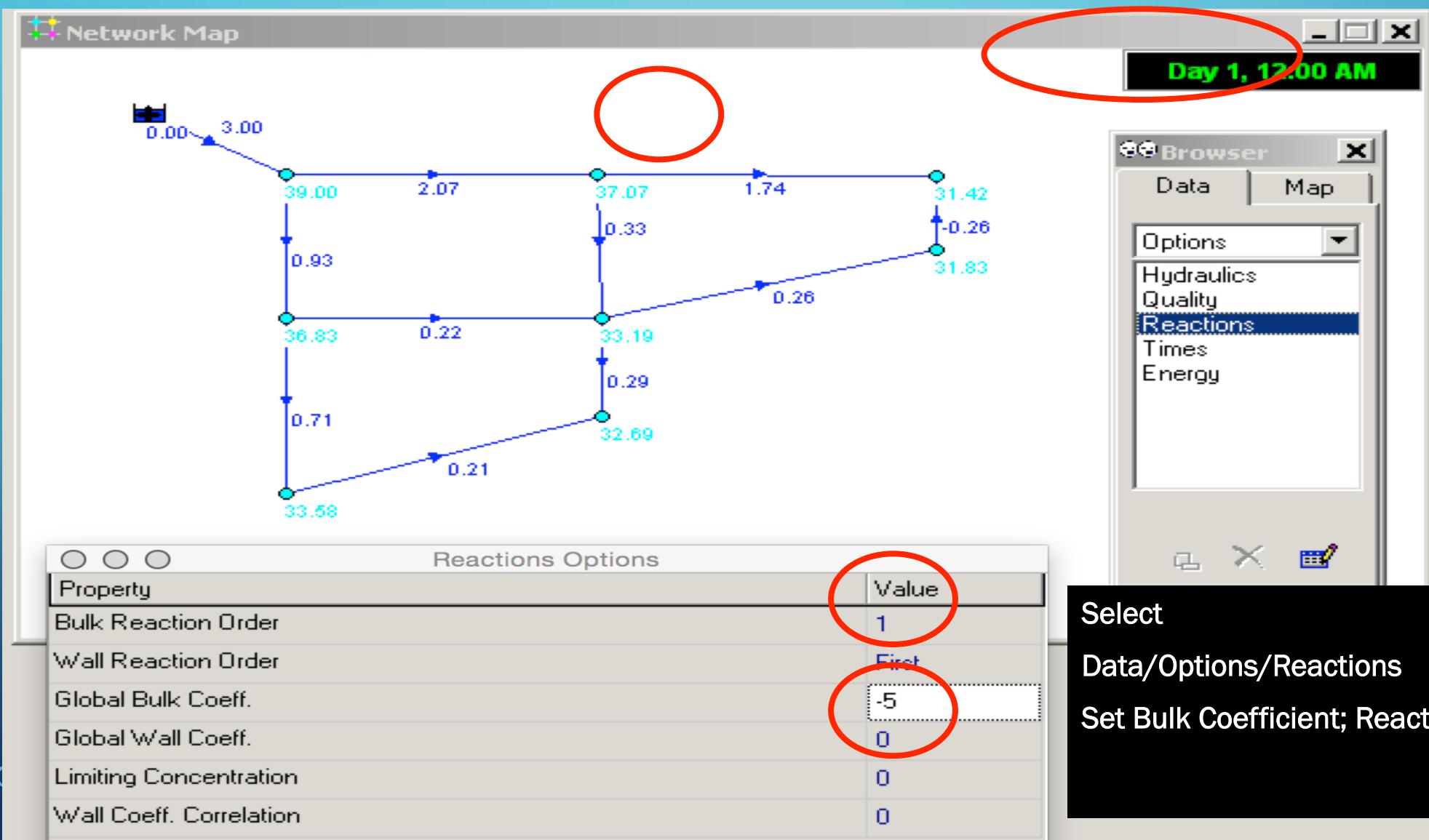


# EXAMPLE

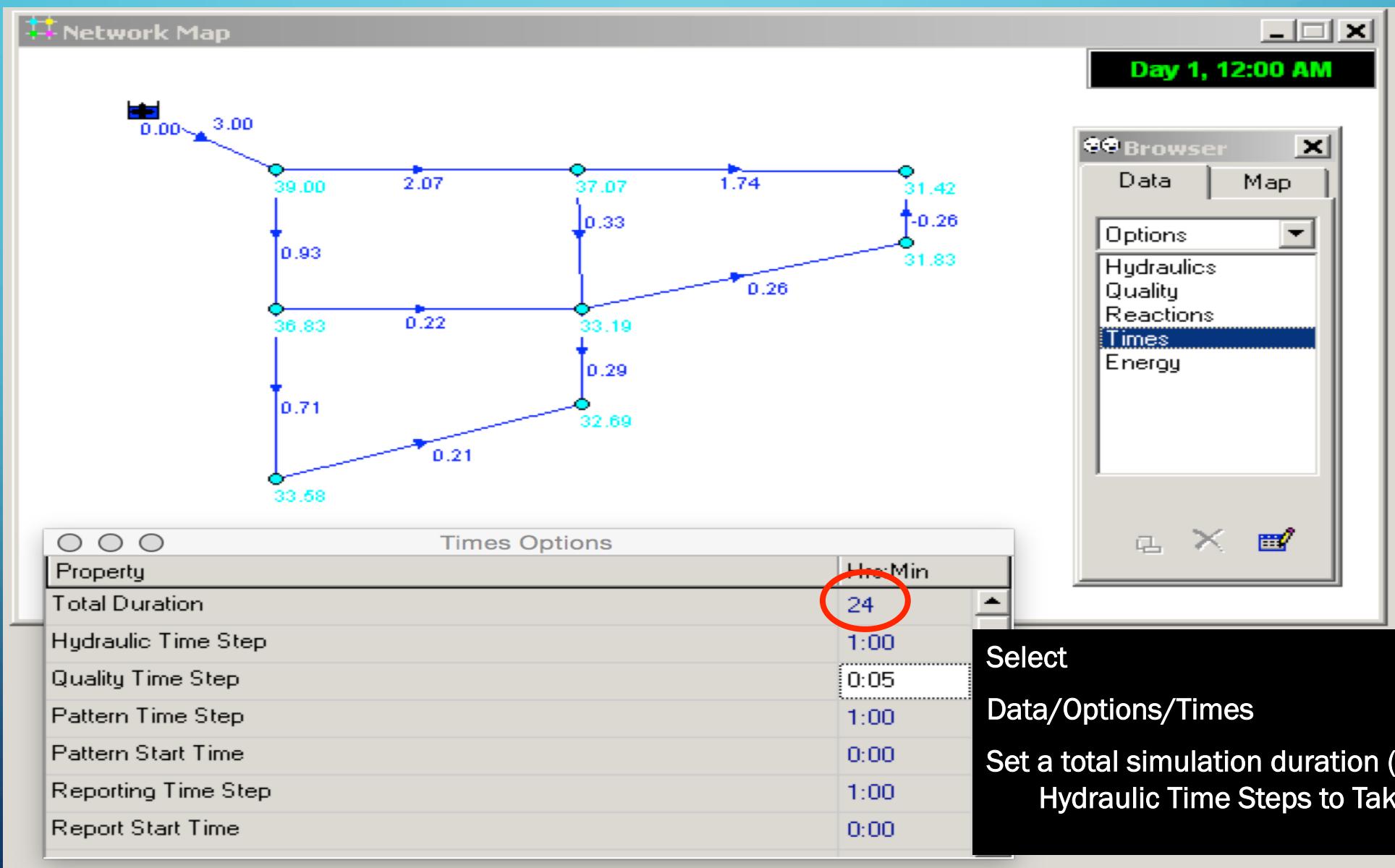
- What is the disinfection residual in the system below if the source water has chloramine at 10 mg/L and the first-order decay mass transfer coefficient ( $K_b$ ) is -5



# EXAMPLE



# EXAMPLE



# EXAMPLE

Network Map

Reservoir 9

Property	Value
*Reservoir ID	9
X-Coordinate	-2321.43
Y-Coordinate	8898.81
Description	
Tag	
*Total Head	90
Head Pattern	
Initial Quality	
Source Quality	10
Net Inflow	-3.00
Elevation	90.00
Pressure	0.00
Quality	0.00

Day 1, 12:00 AM

Browser

Select  
Data/Reservoirs/...  
Set a source quality and source type

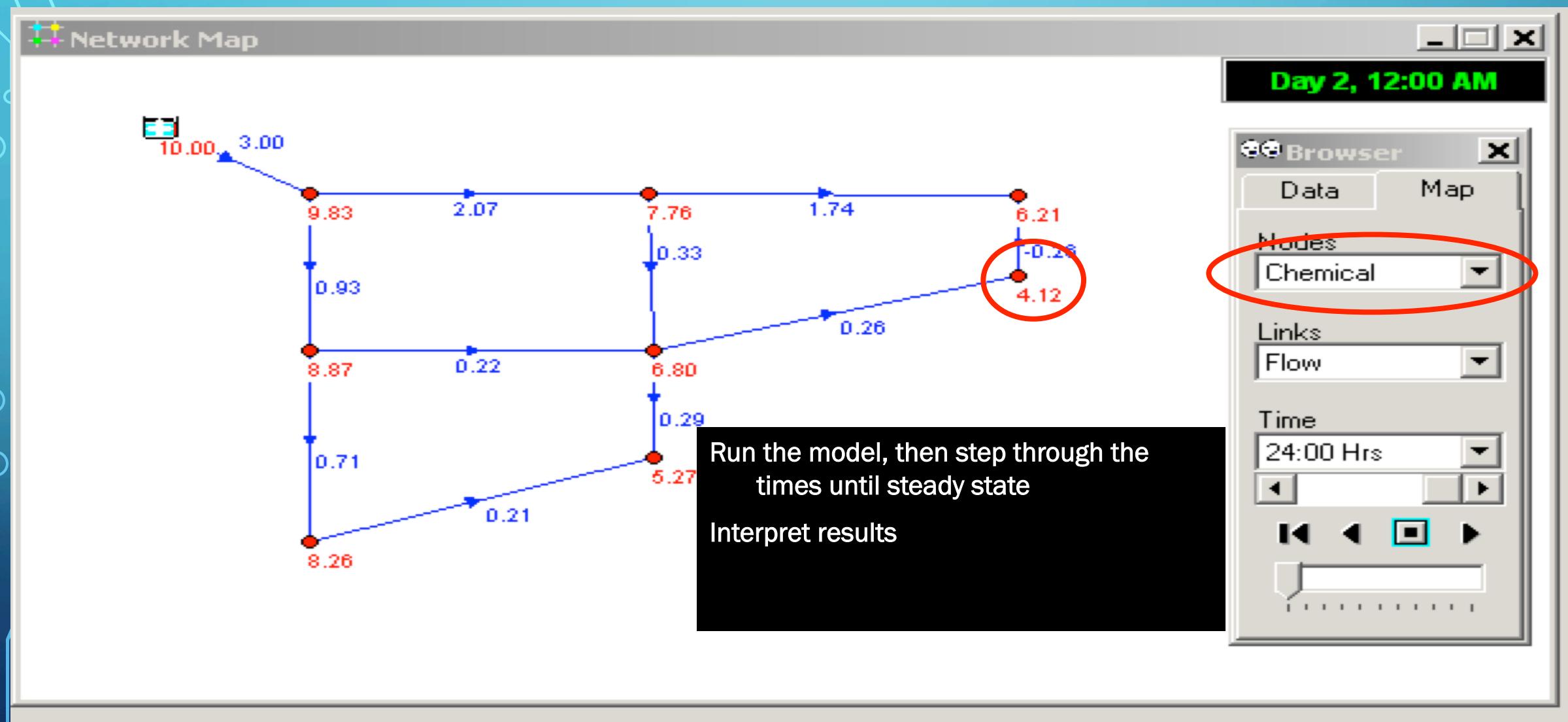
Source Editor for Node 9

Source Quality: 10 (circled)  
Time Pattern: (empty)

Source Type:  
 Concentration  
 Mass Booster  
 Setpoint Booster  
 Flow Paced Booster

OK Cancel Help

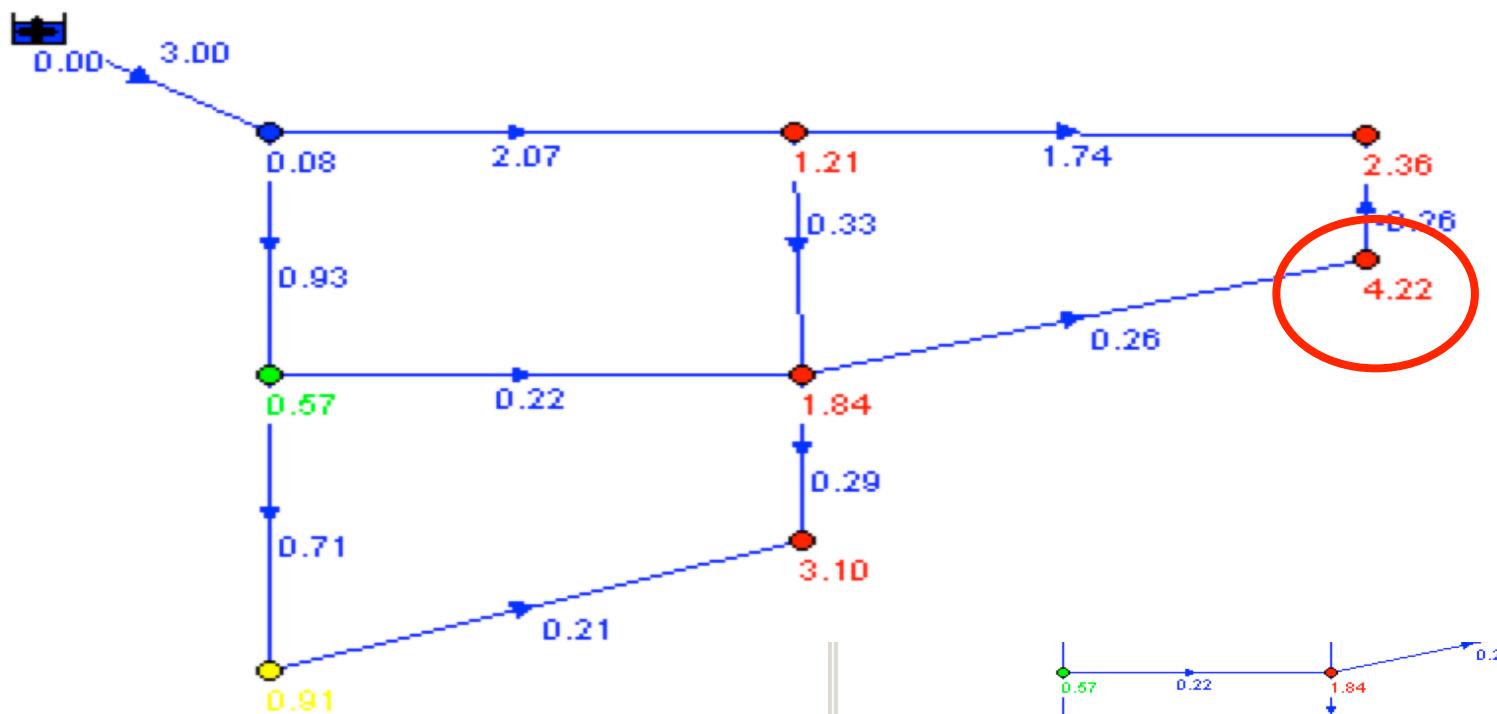
# EXAMPLE



# ADDITIONAL CONCEPTS

- A “tracer” can be used to estimate water age in the system (its treated as a different constituent)
- Use Zero-Order reaction with  $K_b = 1$ ; resulting “concentration” is water age in Hydraulic Time Steps
- Multiple sources can be used to estimate mixing in a system (homework)
- Intrusions of contaminants can be modeled (inject a dose at a node, and see where it arrives).

# EXAMPLE



Run the model, then step through the times until steady state

Interpret results

Property	Value
Bulk Reaction Order	0
Wall Reaction Order	First
Global Bulk Coeff.	1
Global Wall Coeff.	0
Limiting Concentration	0
Wall Coeff. Correlation	0

Day 2, 12:00 AM

Browser

Data Map

Nodes Age

Links Flow

Time 24:00 Hrs

Reactions Options

Reactions Times Energy

## NEXT TIME

- Open Channel Flow
  - Uniform flow
  - Gradually Varied Flow
  - Hydraulic Elements