

**CE 3372 Water Systems Design**  
**Fall 2017**

1. Consider the two networks depicted in Figure 1. Both network designs serve the same junction nodes, have same nodal demands, and draw supply from the same reservoir.

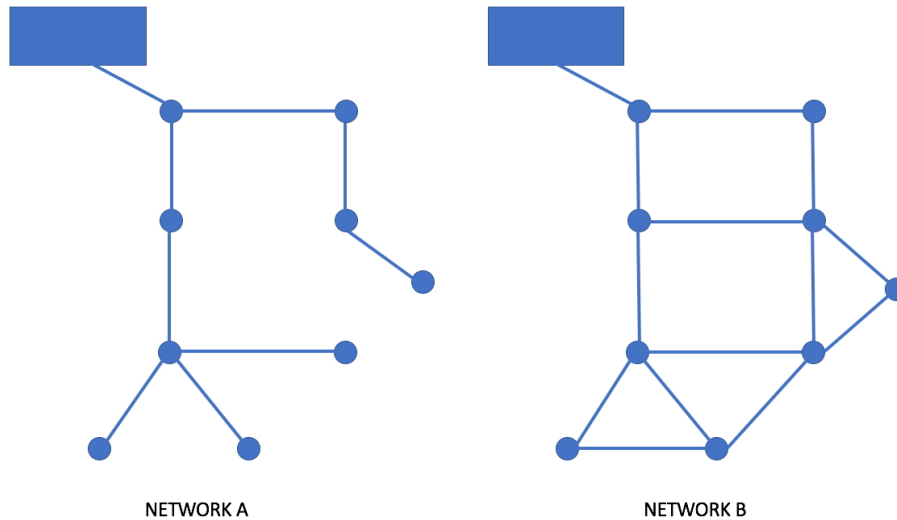
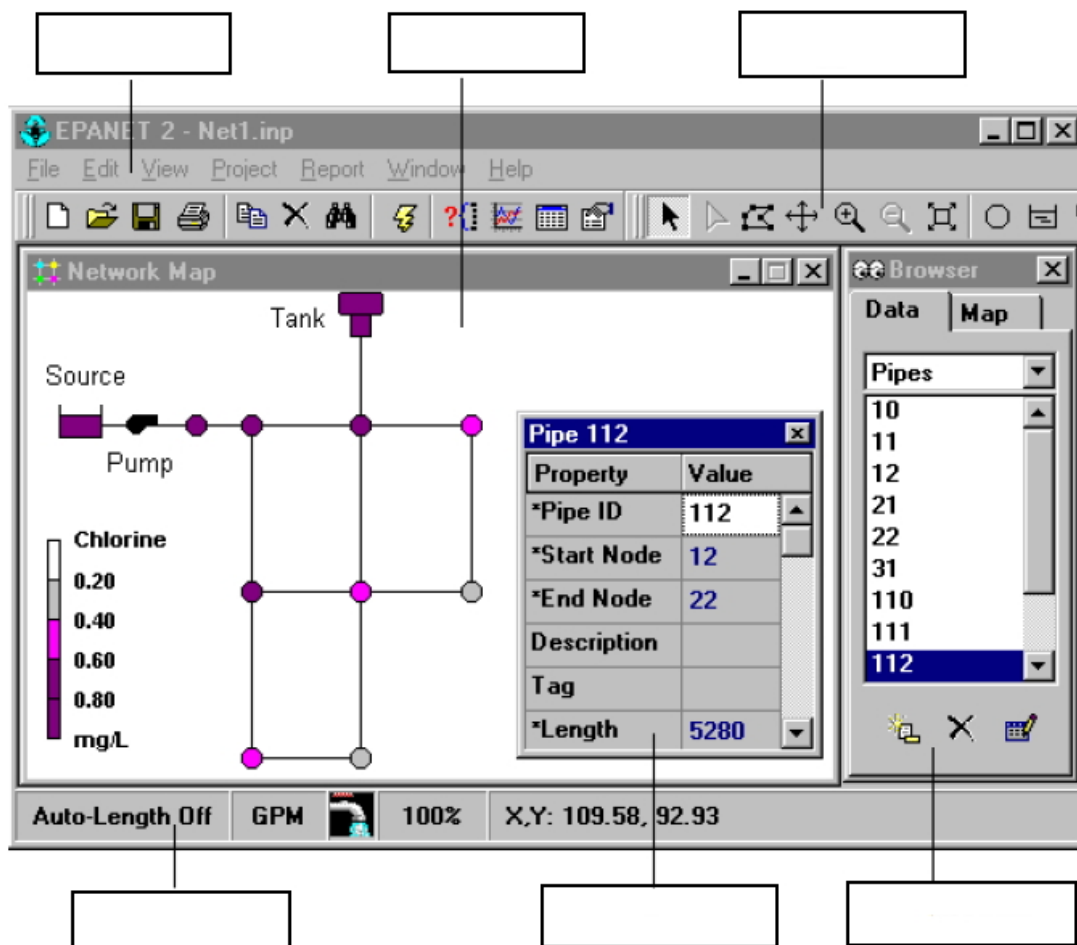


Figure 1: Two Network Designs

- a) Which network design would be superior in terms of reliability? Why?
- b) Which network design would cost less to build? Why?
- c) Which network would be superior in terms of disinfection residual? Why?

2. Figure 2 is a screen capture of the EPANET graphical user interface. Identify the following items in the workspace, and write the item names in the empty boxes on the figure.

- a) Browser
- b) Menu Bar
- c) Network Map
- d) Property Editor
- e) Status Bar
- f) Toolbars



- REVISION A #::

6. An EPA-NET simulation model for a reservoir-pump-network was constructed and operated for four (4) different operational scenarios. Figure 3 is a depiction of the network. The numbers next to the nodes are Node\_ID values in the reports that follow, and the numbers next to the pipes are the Link\_ID values. The network is supplied from a reservoir through a booster pump, both are depicted on Figure 3.

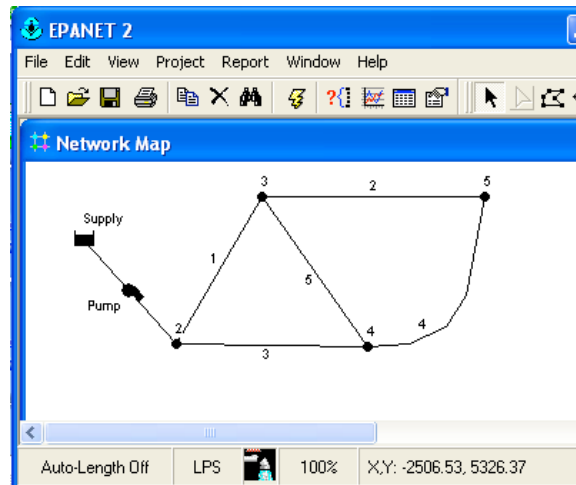


Figure 3: EPA-NET system topology.

Figure 4 is the a portion of the summary report for simulation 1. Figure 5 is the a portion of the summary report for simulation 2. Figure 6 is the a portion of the summary report for simulation 3. Figure 7 is the a portion of the summary report for simulation 4.

These four simulation represent different demand scenarios for the same system.

Interpret these reports, to answer the following questions:

- (a) Complete the table below.  $Q_{pump}$  is the discharge in liters-per-second through the pump station,  $H_{Supply}$  is the head at the supply reservoir,  $H_{Node2}$  is the head at Node 2, and  $\Delta H_{pump}$  is the added head supplied by the pump.

Table 1: Pump Discharge and Supplied Head

Simulation #	$Q_{pump}$	$H_{Supply}$	$H_{Node2}$	$\Delta H_{pump}$
1				
2				
3				
4				

- (b) Complete the table below.  $Q_{pump}$  is the discharge in liters-per-second through the pump station,  $\Delta H_{Node2-to-5}$  is head loss in the system from Node 2 to Node 5.

Table 2: System Discharge and Head Loss

Simulation #	$Q_{pump}$	$H_{Node2}$	$H_{Node5}$	$\Delta H_{Node2-to-5}$
1				
2				
3				
4				

- (c) If the pump performance curve has the mathematical structure:

$$\Delta H_{pump} = H_{shutoff} - C_{pump} \times Q^2, \text{ estimate the values of } H_{shutoff} \text{ and } C_{pump}.$$

- (d) If the system frictional loss curve has the mathematical structure:

$$H_{loss-system} = K_{network} \times Q^2, \text{ estimate the value of } K_{network}$$

- (e) What effect would removing the pipe joining nodes 3 and 4 have on the system performance? Explain your reasoning.
- (f) Estimate the flow distribution and head losses the the system if the the pipe joining nodes 3 and 4 are removed, and the pipe joining node 4 and 5 is removed if the nodal demands are the same as SIMULATION 2.

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*                               E P A N E T                               *
*                               Hydraulic and Water Quality                 *
*                               Analysis for Pipe Networks                   *
*                               Version 2.0                                *
*****

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Input File: SIMULATION #1

Link - Node Table:

Link ID	Start Node	End Node	Length m	Diameter mm
1	2	3	1000	124
2	3	5	1000	124
3	2	4	1000	124
4	4	5	1000	124
5	3	4	1400	124
7	6	2	#N/A	#N/A Pump

Node Results:

Node ID	Demand LPS	Head m	Pressure m	Quality
2	0.00	20.00	20.00	0.00
3	0.00	20.00	20.00	0.00
4	0.00	20.00	20.00	0.00
5	0.00	20.00	20.00	0.00
6	0.00	0.00	0.00	0.00 Reservoir

Link Results:

Link ID	Flow LPS	Velocity m/s	Unit Headloss m/km	Status
1	0.00	0.00	0.00	Open
2	0.00	0.00	0.00	Open
3	0.00	0.00	0.00	Open
4	0.00	0.00	0.00	Open
5	0.00	0.00	0.00	Open
7	0.00	0.00	-20.00	Open Pump

Figure 4: EPA-NET Summary Report, Simulation #1

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*                               Version 2.0                                *
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Input File: SIMULATION 2

Link - Node Table:

Link ID	Start Node	End Node	Length m	Diameter mm
1	2	3	1000	124
2	3	5	1000	124
3	2	4	1000	124
4	4	5	1000	124
5	3	4	1400	124
7	6	2	#N/A	#N/A Pump

Node Results:

Node ID	Demand LPS	Head m	Pressure m	Quality
2	0.00	19.28	19.28	0.00
3	1.00	19.03	19.03	0.00
4	1.00	19.03	19.03	0.00
5	1.00	18.99	18.99	0.00
6	-3.00	0.00	0.00	0.00 Reservoir

Link Results:

Link ID	Flow LPS	Velocity m/s	Unit Headloss m/km	Status
1	1.50	0.12	0.25	Open
2	0.50	0.04	0.03	Open
3	1.50	0.12	0.25	Open
4	0.50	0.04	0.03	Open
5	0.00	0.00	0.00	Open
7	3.00	0.00	-19.28	Open Pump

Figure 5: EPA-NET Summary Report, Simulation #2



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*                               Hydraulic and Water Quality                 *
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*                               Version 2.0                                *
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Input File: SIMULATION 3

Link - Node Table:

Link ID	Start Node	End Node	Length m	Diameter mm
1	2	3	1000	124
2	3	5	1000	124
3	2	4	1000	124
4	4	5	1000	124
5	3	4	1400	124
7	6	2	#N/A	#N/A Pump

Node Results:

Node ID	Demand LPS	Head m	Pressure m	Quality
2	0.00	17.12	17.12	0.00
3	2.00	16.16	16.16	0.00
4	2.00	16.16	16.16	0.00
5	2.00	16.04	16.04	0.00
6	-6.00	0.00	0.00	0.00 Reservoir

Link Results:

Link ID	Flow LPS	Velocity m/s	Unit Headloss m/km	Status
1	3.00	0.25	0.96	Open
2	1.00	0.08	0.12	Open
3	3.00	0.25	0.96	Open
4	1.00	0.08	0.12	Open
5	0.00	0.00	0.00	Open
7	6.00	0.00	-17.12	Open Pump

Figure 6: EPA-NET Summary Report, Simulation #3

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*                               Hydraulic and Water Quality                 *
*                               Analysis for Pipe Networks                   *
*                               Version 2.0                                *
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Input File: SIMULATION 4

Link - Node Table:

Link ID	Start Node	End Node	Length m	Diameter mm
1	2	3	1000	124
2	3	5	1000	124
3	2	4	1000	124
4	4	5	1000	124
5	3	4	1400	124
7	6	2	#N/A	#N/A Pump

Node Results:

Node ID	Demand LPS	Head m	Pressure m	Quality
2	0.00	13.52	13.52	0.00
3	3.00	11.40	11.40	0.00
4	3.00	11.40	11.40	0.00
5	3.00	11.15	11.15	0.00
6	-9.00	0.00	0.00	0.00 Reservoir

Link Results:

Link ID	Flow LPS	Velocity m/s	Unit Headloss m/km	Status
1	4.50	0.37	2.12	Open
2	1.50	0.12	0.25	Open
3	4.50	0.37	2.12	Open
4	1.50	0.12	0.25	Open
5	0.00	0.00	0.00	Open
7	9.00	0.00	-13.52	Open Pump

Figure 7: EPA-NET Summary Report, Simulation #4

7. The hydraulic radius in a conduit containing a flowing liquid is
- (A) the mean radius from the center of flow to the wetted side of the conduit
  - (B) the ratio of the cross-sectional area of the conduit and the wetted perimeter
  - (C) the ratio of the wetted perimeter and the cross-sectional area of the conduit
  - (D) the ratio of the cross-sectional area of flow and the wetted perimeter
8. A circular, 60-inch diameter, reinforced concrete sewer pipe ( $n = 0.013$ ) carries 50 MGD of wastewater to a lift station wet well. Average slope along the flow path is 1.0%.
- a) Sketch the cross section, indicate the pipe diameter.
  - b) For the conditions in the problem statement, what is the flow rate in cubic feet per second?
  - c) What is the diameter of the pipe, in feet?
  - d) Use Manning's equation ( $Q = \frac{1.49}{n} AR^{(2/3)} S^{(1/2)}$ ) and determine the **pipe-full** discharge in cubic feet per second.
  - e) What is the pipe-full discharge ( $Q_f$ ) in million gallons per day (MGD)?

f) Determine the discharge in the sewer when it is half-full, in million gallons per day (MGD).

g) Determine the discharge in the sewer when it is 90%full, in million gallons per day (MGD).

h) Determine the discharge in the sewer when it is 75%full, in million gallons per day (MGD).

i) Which of these conditions (50%,75%,90% of full) is closest to the actual flow of 50 MGD?

14. A smooth concrete channel is depicted in Figure 8. The channel's dimensionless slope in the direction of flow is 0.005. If the flow width at the surface is 2-meter, what is the flow rate in the channel?

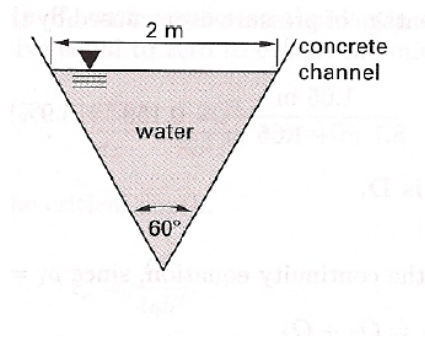


Figure 8: Triangular channel.

9. The rational runoff coefficient for a 14.81 acre parcel property is 0.35. The rainfall intensity is 4.56 inches per hour. Determine the size (diameter) of a reinforced concrete sewer pipe to convey the peak discharge, if the pipe is to be laid on a 0.5% slope, and the desired flow depth in the pipe is 1/2 full.

10. List the names of your water systems team members in the table below. Rate them in the categories indicated, from 1 to 4 with 1 being poor performance, and 4 being excellent performance.

Table 3: Mid-term Team Mate Assessment

Team Member Name	Reliability	Availability	Work Quality	Contribution(%)
1)				
2)				
3)				
4)				
5)				

Additional Comments Below: <sup>1</sup>

<sup>1</sup>This sheet will be removed from the exam and will not be returned, the feedback is used to assess team effectiveness, but does not affect the examination score.