



4.121 Given: Trapezoidal channel  $Q = 3.75 \text{ m}^3/\text{sec}$  lined w/ grass  
Class C Rutterdence  $S = 0.004$ ,  $b = 2.0 \text{ m}$ ,  $m = 3$   
Find: Depth of design flow. Is channel stable?

①  $VRC = C \rightarrow \text{Table 4.4} \rightarrow \tau_p = 1.05 \text{ psf} = 48 \text{ N/m}^2$  [11]

② Estimate flow depth  $b = 2.0 \text{ m}$ ,  $m = 3$   
Guess  $y_p = 1.0 \text{ m}$

$A_1 = y_{o1} (b + m y_{o1}) = 1 \text{ m} (2 \text{ m} + 3(1 \text{ m})) = 5.0 \text{ m}^2$  [13]

$P_1 = b + 2 y_{o1} \sqrt{1 + m^2} = 2 \text{ m} + 2(1 \text{ m}) \sqrt{1 + 3^2} = 8.32 \text{ m}$

$R_1 = \frac{A_1}{P_1} = \frac{5.0 \text{ m}^2}{8.32 \text{ m}} = 0.60 \text{ m}$

③ Table 4.4  $\rightarrow MEI = 0.5 \text{ N/m}^2$ ,  $h_s = 0.2 \text{ m}$

$\tau_o = \gamma R S = (9810 \text{ N/m}^3)(0.60 \text{ m})(0.004) = 24 \text{ N/m}^2$

$\frac{k}{h_s} = 0.1 + \left[ \frac{(MEI/\tau_o)^{0.25}}{h_s} \right]^{1.89} = 0.1 + \left[ \frac{(0.5 \text{ N/m}^2 / 24 \text{ N/m}^2)^{0.25}}{0.2 \text{ m}} \right]^{1.89}$   
 $= 0.39 \Rightarrow k = 0.39(0.2 \text{ m}) = 0.078 \text{ m}$

$u_* = (\gamma R S)^{1/2} = [(9810 \text{ N/m}^3)(0.60 \text{ m})(0.004)]^{1/2} = 0.65 \text{ m/s}$

$u_{*c} = \min \left[ 0.028 + 6.35 MEI^2, 0.23 MEI^{0.106} \right]$   
 $= \min \left[ 0.028 + 6.35 (0.5 \text{ N/m}^2)^2, 0.23 (0.5 \text{ N/m}^2)^{0.106} \right]$   
 $= \min [1.61, 0.21] = 0.21 \text{ m/s}$  [12]

$\frac{u_*}{u_{*c}} = \frac{0.65 \text{ m/s}}{0.21 \text{ m/s}} = 0.71 \rightarrow \text{Table 4.3 } q = 0.15, b = 1.85$

$\frac{n}{k^{1/6}} = \frac{K_n}{\sqrt{g}} \frac{(R/k)^{1/6}}{(a + b \log \frac{R}{k})} = \frac{1}{(59.61 \text{ m/s})^{1/6}} \frac{(0.60 \text{ m} / 0.078 \text{ m})^{1/6}}{(0.15 + 1.85 \log \frac{0.60}{0.078})}$

$= 0.089$   
 $n = 0.089 (0.078)^{1/6} = 0.058$

④  $Q = \frac{K_n}{n} A_1 R_1^{2/3} S^{1/2} = \frac{1}{0.058} \frac{A_2^{5/6}}{P_2^{2/3}} S^{1/2}$   
 $3.75 \text{ m}^3/\text{s} = \frac{1}{0.058} \frac{y_{o2}^5 [2m + 3y_{o2}]^{5/6}}{(2m + 2y_{o2} \sqrt{10})^{2/3}} (0.004)^{1/2}$  [13]

Good Luck  $y_{o2} = 0.984 \text{ m}$

See spread sheet for next iterations

$y_o = 0.987 \text{ m}$   $n = 0.058$

⑤  $\tau_{max} = \gamma y_o S = (9810 \text{ N/m}^3)(0.987 \text{ m})(0.004) = 38.8 \text{ N/m}^2 < \tau_p = 48 \text{ N/m}^2$   
Stable [1]

First trial from first yo guess

b = 2 m MEI = 0.5 N m

m = 3  $h_s = 0.2$  m

$S_o = 0.004$   $\gamma = 9810$  N/m<sup>3</sup>

n = 0.058  $g = 9.81$  m/s<sup>2</sup>

| Q (m <sup>3</sup> /s) | $y_o$ (m) | A (m <sup>2</sup> ) | P (m) | RHS (m <sup>3</sup> /s) | by Goal Seek |
|-----------------------|-----------|---------------------|-------|-------------------------|--------------|
| 3.75                  | 0.984     | 4.87                | 8.22  | 3.75                    |              |

Next trial

| $y_o$ (m) | A (m <sup>2</sup> ) | P (m) | R (m) | $\tau_o$ (N/m <sup>2</sup> ) | $k/h_s$ | k (m) | $u_*$ (m/s) | $u_{*c}$ (m/s) | $u_*/u_{*c}$ | a    | b    | $n/k^{1/6}$ | n      | Q (m <sup>3</sup> /s) |
|-----------|---------------------|-------|-------|------------------------------|---------|-------|-------------|----------------|--------------|------|------|-------------|--------|-----------------------|
| 0.984     | 4.87                | 8.22  | 0.593 | 23.3                         | 0.393   | 0.08  | 0.1525      | 0.214          | 0.714        | 0.15 | 1.85 | 0.0892      | 0.0584 | 3.73                  |
| 0.987     | 4.89                | 8.24  | 0.594 | 23.3                         | 0.393   | 0.08  | 0.1527      | 0.214          | 0.714        | 0.15 | 1.85 | 0.0891      | 0.0583 | 3.75                  |

Last by Goal Seek



4.17 Given: Riprap-lined channel  $Q = 1000 \text{ cfs}$   $S_0 = 0.0005$   
Crushed rock  $b \leq 15 \text{ ft}$

Find: Riprap size,  $m$ ,  $y_0$  design depth

① Choose  $d_{50} = 100 \text{ mm} = 10 \text{ cm} \left( \frac{1 \text{ ft}}{30.5 \text{ cm}} \right) = 0.33 \text{ ft}$

⑤  $F_g 4.13 \quad \phi = 41.8^\circ, \theta = 23^\circ, m = 2.3$

② Critical bed & wall shear stresses

$\tau_{oc} (\text{psf}) = 4 d_{50} (\text{ft}) = 4 (0.33 \text{ ft}) = 1.32 \text{ psf}$

⑤  $K_r = \frac{\tau_{oc}^w}{\tau_{oc}} = \left[ 1 - \frac{\sin^2 \theta}{\sin^2 \phi} \right]^{1/2} = \left[ 1 - \frac{\sin^2 (23^\circ)}{\sin^2 (41.8^\circ)} \right]^{1/2} = 0.81$

$\tau_{oc}^w = K_r \tau_{oc} = 0.81 (1.32 \text{ psf}) = 1.07 \text{ psf}$

② ③  $n = 0.04 d_{50}^{1/4} = 0.04 (0.33 \text{ ft})^{1/4} = 0.033$

④ Find  $y_0$  for given  $b, Q, S$

I'll choose  $b = 12 \text{ ft}, m = 2.3$

$Q = \frac{K_u}{n} A R^{2/3} S_0^{1/2}$

$A R^{2/3} = \frac{A^{5/3}}{P^{2/3}} = \frac{Q n}{K_u S_0^{1/2}}$

$\frac{[y_0(b + m y_0)]^{5/3}}{[b + 2 y_0(1 + m^2)^{1/2}]^{2/3}} = \frac{[y_0(12 \text{ ft} + 2.3 y_0)]^{5/3}}{[12 \text{ ft} + 2 y_0(1 + 2.3^2)^{1/2}]^{2/3}} = \frac{(1000 \text{ cfs})(0.033)}{(1.49)(0.0005)^{1/2}} = 990$

using trial seek  $y_0 = 9.5 \text{ ft}, R = 5.4 \text{ ft}$

⑤  $(\tau_{oc})_{\max} = 1.58 R S = 1.5 (62.4 \text{ lb/ft}^3) (5.4 \text{ ft}) (0.0005) = 0.25 \text{ psf}$

$(\tau_{oc}^w)_{\max} = 1.2 R S = 1.2 (62.4 \text{ lb/ft}^3) (5.4 \text{ ft}) (0.0005) = 0.20 \text{ psf}$

④  $(\tau_{oc})_{\max} = 0.25 \text{ psf} < \tau_{oc}^c = 1.32 \text{ psf}$  ✓

$(\tau_{oc}^w)_{\max} = 0.20 \text{ psf} < \tau_{oc}^w = 1.07 \text{ psf}$  ✓

Channel  
is  
stable  
so ok

Your answers may vary, but  
procedure should agree



4.19 Given: Rectangular channel.  $b = 10\text{ ft}$   $n = 0.020$

Find: Slope such that uniform flow will always have  $Fr \leq 0.5$

Limit Slope  $S_L = \frac{2.67 n^2}{K_n^2} \frac{1^2}{b^{1/3}}$

$$S_L = \frac{2.67 (32.2 \text{ ft/s}^2)}{1.49^2} \frac{(0.020)^2}{(10 \text{ ft})^{1/3}}$$

$$S_L = 0.0072$$

$$Fr_{\max} = \left[ \frac{S_0}{S_L} \right]^{1/2}$$

$$Fr_{\max}^2 = \frac{S_0}{S_L}$$

$$S_0 = Fr_{\max}^2 S_L = (0.5)^2 (0.0072)$$

$$\boxed{S_0 = 0.0018}$$

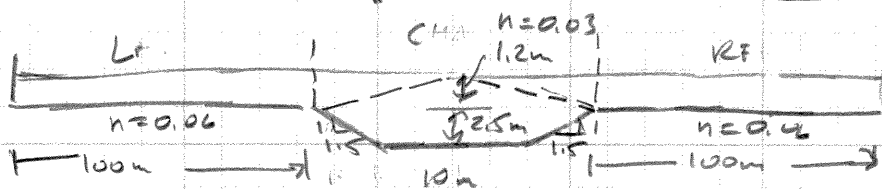
4.13 | Given: Compound channel shown below

$$s_0 = 0.001, y_0 = 3.7 \text{ m}$$

Prnd:  $Q$  for uniform flow

(i) vertical interface w/ depth P for main channel

(ii) diagonal interface  $u' \quad u \quad u \quad u \quad u \quad u \quad u$



(i) With vertical interface included

$$\Sigma Q = Q_L + Q_C + Q_R = 2Q_L + Q_C \quad \text{since } Q_L = Q_R$$

$$Q_L = \frac{1}{0.06} A_L R_L^{4/3} S_0^{1/2}$$

$$A_L = (100\text{m})(1.2\text{m}) = 120\text{m}^2$$

$$P_1 = 100m + 2(1.2m) = 102.4m$$

$$R_L = \frac{120 \text{ m}^2}{102.4 \text{ m}} = 1.17 \text{ m}$$

$$Q_L = \frac{1}{0.06} (120m^2) (1617m)^{2/3} (0.001)^{1/2}$$

$$Q = 70.2 \text{ m}^3/\text{sec} = Q_R$$

$$B_y = 10m + 2(2.5m)(15) = 17.5m$$

$$A_c = (1.2m)(17.5m) + \frac{10m + 17.5m}{2} (2.5m)$$

$$= 21 \text{ m}^2 + 34.4 \text{ m}^2 = 55.4 \text{ m}^2$$

$$P_c = 10m + 2(25m)(1+15^2)^{1/2} + 2(1.2m) = 19.0m + 2.4m = 21.4m$$

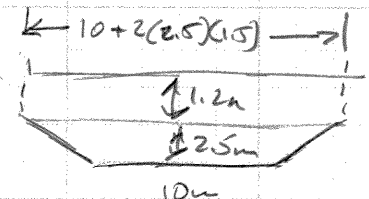
$$R_c = \frac{A_c}{P_c} = \frac{55.4 \text{ m}^2}{21.4 \text{ m}} = 2.59 \text{ m}$$

$$Q_c = \frac{1}{0.03} (554 \text{ m}^2) (2.59 \text{ m})^{2/3} (0.001)^{1/2}$$

$$= 110 \text{ m}^3/\text{sec}$$

$$\sum Q = 2(70.2 \text{ m}^3/\text{sec}) + 110 \text{ m}^3/\text{sec}$$

$$\Sigma Q = 250 \text{ m}^3/\text{sec}$$



• w/o channel interface P included

A stays same, P changes

$$P_L = 100m + 1.2m = 101.2m$$

$$R_L = \frac{A_L}{P_L} = \frac{120m^2}{101.2m} = 1.19m$$

$$Q_L = \frac{1}{0.06} (120m^2) (1.19m)^{2/3} (0.001)^{1/2}$$

$$Q_L = 71 m^3/sec$$

$$P_C = 21.4m - 2(1.2m) = 19.0m$$

$$R_C = \frac{A_C}{P_C} = \frac{55.4m^2}{19.0m} = 2.92m$$

$$Q_C = \frac{1}{0.03} (55.4m^2) (2.92m)^{2/3} (0.001)^{1/2}$$

$$= 119 m^3/sec$$

$$\Sigma Q = 2(71 m^3/sec) + 119 m^3/sec$$

$$\boxed{\Sigma Q = 261 m^3/sec}$$

(ii) Diagonal interfaces w/o P for main channel

$$A_L = (100m)(1.2m) + \frac{1}{2} (17.5m)(1.2m)$$

$$= 125.3m^2$$

$$P_L = 1.2m + 100m = 101.2m$$

$$R_L = \frac{A_L}{P_L} = \frac{125.3m^2}{101.2m} = 1.24m$$

$$Q_L = \frac{1}{0.06} (125.3m^2) (1.24m)^{2/3} (0.001)^{1/2}$$

$$\boxed{Q_L = 76.2 m^3/sec}$$

$$A_C = 34.4m^2 + \frac{1}{2} (17.5m)(1.2m) = 34.4m^2 + 10.5m^2$$

$$= 44.9m^2$$

$P_C = 19.0m$  as in (i) w/o channel interface P

$$R_C = \frac{44.9m^2}{19.0m} = 2.36m$$

$$Q_C = \frac{1}{0.03} (44.9m^2) (2.36m)^{2/3} (0.001)^{1/2}$$

$$Q_C = 83.9 m^3/sec$$

$$\Sigma Q = 2(76.2 m^3/sec) + 83.9 m^3/sec$$

$$\boxed{\Sigma Q = 236 m^3/sec}$$

