

Computational Structural Analysis

2D Direct Stiffness Code

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1 Benchmarks

The accuracy and robustness of the code is tested by solving some sample problems, the exact solutions of which were computed apriori via the classical beam theory. The results are presented in this section using the following scheme:

- Demonstration of the problem
- Input file that addresses the problem
- Output file generated by the program after analysis
- Diagrams obtained by postprocessing
- Comparison of the results of numerical and analytical solutions

Each problem considered in the first part comprise a single or continous beam subjected to a certain external loading and support condition. The second part deals with a one-bay one-storey frame under different loads and restraints, and the third and final part is devoted to problems involving a prestressed beam, with each problem having different tendon layouts and/or support conditions.

1.1 Single Beam Under Various Loading and Support Conditions

1.1.1 Problem 1: Simple Beam - Uniformly Distributed Load

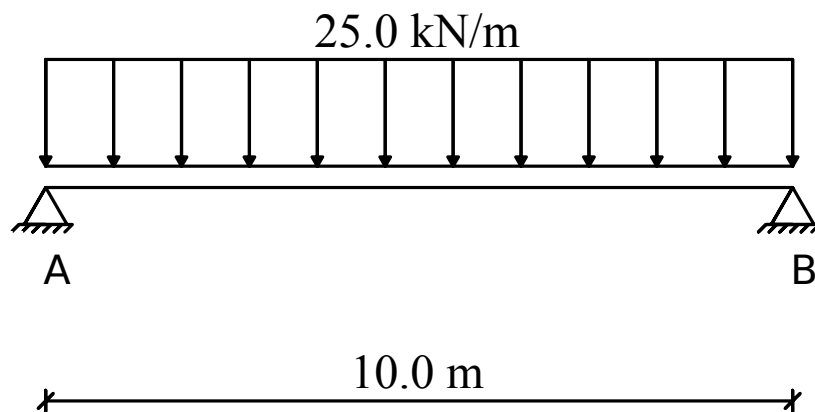


Figure 1: Problem 1: Loading, geometry and supports

```

# Default units - kN/m/C
# -----
# NODES <number of nodes> <x coord, y coord>
2
0.0 0.0
10.0 0.0
# ELEMENTS <number of elements> <start node, end node>
1
1 2
# SECTIONS <number of sections> <A, I, h, X>
1
0.32 0.01706666666666667 0.8 0.0
# SECTION INCIDENCES
1
# MATERIALS <number of materials> <E, v, alpha, gamma>
1
30.e6 0.2 0.0 0.0
# MATERIAL INCIDENCES
1
# RESTRAINTS <number of restraints> <node number, direction>
4
1 1
1 2
2 1
2 2
# LINKS <number of links> <master node, slave node, direction>
0
# ELASTIC RESTRAINTS <number of el. restraints> <node number, direction, k>
0
# ELASTIC LINKS <number of links> <node1, node2, direction, k>
0
# NODAL LOADS <number of nodal loads> <node, direction, magnitude>
0
# ELEMENT LOADS <number> <Element, Px, Py, Py2, DTtop, DTbottom>
1
1 0.0 -25.0 -25.0 0.0 0.0
# PRESTRESSING <number> <Element, e1, em, e2, P>
0

```

```

=====
INPUT DATA: [kN/m/C]
=====

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```

# NODE COORDINATES
Node      x-coord      y-coord
1          0.00        0.00
2          10.00        0.00

# SECTIONS

```

Sec.No	A	I	h
X			
1	.3200D+00	.1707D-01	0.8000
0.0000			

MATERIALS

Mat.No	E	v	alpha
gamma			
1	.3000D+08	0.20	.0000D+00
0.0000			

ELEMENTS

El.No	Start Node	End Node	Section	Material
1	1	2	1	1

RESTRAINTS

Restraint No	Node	Direction
1	1	1
2	1	2
3	2	1
4	2	2

LINKS

Link No	Master Node	Slave Node	Direction
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ELASTIC RESTRAINTS

E. Restr. No	Node	Direction	K
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ELASTIC LINKS

E. Link. No	Node 1	Node 2	Direction
K			

NODAL LOADS

Node	Direction	Magnitude
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ELEMENT LOADS

Element	Px	Py1	Py2	DTtop	DTbottom
1	0.00	-25.00	-25.00	0.00	0.00

PRESTRESSING

Element	e1	em	e2	P
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===== ANALYSIS RESULTS =====

>> NODAL DISPLACEMENTS

Node	DX	DY
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1	0.0000000000D+00	0.0000000000D+00
2	0.0000000000D+00	0.0000000000D+00

>> MEMBER END FORCES

Member	N1	T1	M1	N2	T2
M2					
1	0.00	125.00	-0.00	0.00	125.00
0.00					