Useful online resources

Dissemination of IT for the Promotion of Materials Science by Cambridge University

https://www.doitpoms.ac.uk/index.php

- ► Creep: Introduction
- Creep: Mechanisms
- Creep: Constitutive models
- Stress analysis and Mohr's circle
- Brittle fracture

- Rocks are elastic: they deform under loading and go back to the original shape (reference configuration) when the loading is removed.
- However, when the applied loading exceeds a certain critical value, rocks start to develop permanent deformation.
 - Linear and non-linear rheologies of rocks are used to describe permanent deformation as viscous flow.
 - Another mode of permanent deformation is fracturing: e.g., Faults and joints.
- ▶ We are going to study which mode becomes dominant under a given pressure and temperature condition. This understanding leads to a convenient way of representing both modes and required force in one plot: yield strength envelope.

- Another related issue is what is the stress required for fracturing a rock and for causing the fractured surface to slip. Naturally, this question is related to the studies on faulting.
- Let's first get some intuition from these movies:
 - Microscopic view to "permanent" deformation: http://www.youtube.com/watch?v=n7LXYyohmgg
 - 2. Tension tests on metals: http://www.youtube.com/watch?v=D8U4G5kcpcM

 Stress-strain curves for linear and non-linear elastic materials.

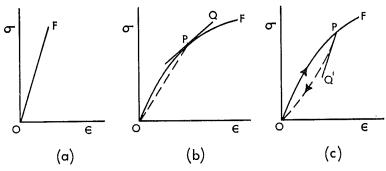


Fig. 4.2.1 (a) Linearly elastic material. (b) Perfectly elastic material showing tangent modulus PQ and secant modulus OP. (c) Elastic material with hysteresis, showing loading and unloading cycle.

(Ch. 4 in Fundamentals of Rock Mechanics, Jaeger, J. C. and N. G. W. Cook, 2nd ed., 1976, Chapman and Hall,

A typical stress-strain curve for rock.

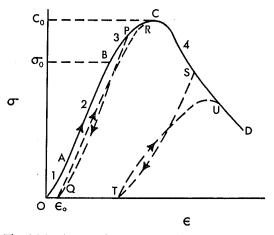


Fig. 4.2.2 The complete stress-strain curve for rock.

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- The curve has 4 regions
 - OA: Slightly concave upward. Nearly elastic (i.e. no permanent deformation when unloaded) although maybe with hysteresis.
 - AB: Almost linear. Nearly elastic although maybe with hysteresis.
 - 3. BC: Concave downward. Usually, $\sigma_0 \sim 2/3C_0$. Irreversible deformation starts in this region. Successive cycles of loading and unloading trace out different curves. For instance, paths PQ and QR leaves ϵ_0 .
 - 4. CD: Failling region. Starts from the stress maximum, C. Characterized by a negative slope. Unloading (ST) leaves a large permanent deformation and reloading (TU) approaches the curve CD at a lower stress U than S.

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- A material is said to be ductile under conditions in which it can sustain permanent deformation without losing its ability to resist load. The ductile state corresponds to the region BC.
- A material is said to be **brittle** under conditions in which its ability to resist load decreases with increasing deformation. The brittle state corresponds to the region CD.
- If loading is cyclic, a reloading will attain a higher stress (as in PQR) if the material is in a ductile state while a lower stress (as in STU) if in a brittle state.
- ► The process of **failure** is regarded as a continuous one which occurs progressively throughout the brittle region CD, in which the rock steadily deteriorates. The stress value at C, C₀ is called *uniaxial compressive strength*.

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- ▶ In actual testing, sudden failure often occurs at some point of the curve CD with complete loss of cohesion across a plane and this is known as brittle fracture. For instance (and for fun), watch
 - http://www.youtube.com/watch?v=PRF_ZJZksp4.
- ▶ The point B at which the transition from elastic to ductile behavior takes place is known as the **yield point** and the corresponding stress σ_0 as the **yield stress**.

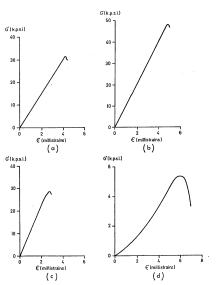


Fig. 4.2.3 Stress-strain curves for uniaxial compression in a stiff testing machine. (a) Solenhofen limestone, (b) Karroo dolerite, (c) Rand quartzite, (d) Gosford sandstone

- Some stress-strain curves of typical rocks for comparison with the idealized curve discussed earlier.
- These curves show how unimportant the regions OA and BC are in the practical cases.
- So, the assumption of linear elasticity up to failure is a good one in many cases.

< Effects of Confining Pressure >

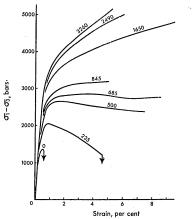


Fig. 4.3.2 Stress-strain curves for Carrara marble (after von Karman) at various confining pressures. The numbers on the curves are confining pressures in bars.

- Confining pressure up to 500 bars (50 MPa), brittle fracture occurs as before with an increase of strength and a small increase in permanent strain.
- The curve for 685 bars is completely different: It's a curve for a ductile state.
- The curve for 235 bars shows transitional behavior.
- ➤ Such a transition is called the **brittle-ductile**transition.

< Effects of Temperature >

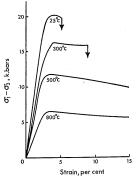


Fig. 4.3.3 Stress-strain curves for granite at a confining pressure of 5 kilobars and various temperatures (after Griggs, Turner, and Heard).

- The effect of increasing the temperature is to lower the brittle-ductile transition pressure.
- At room temperature, the brittle failure occurs.
- At higher temperatures, the substantial amount of permanent deformation is introduced without loss of load.

- Even the pervasively fractured rocks have a finite yield stress under confining pressure.
- ▶ Byerlee showed that the amount of shear stress (τ) required for the most favorably oriented fracture surface (or fault plane) is a linear function of the normal stress (σ_n) on that plane. This shear stress can be identified with the yield stress (τ_Y) .
- Moreoever, the linear relationship between τ_Y and σ_n holds regardless of rock type.
- Since the normal stress can be related to lithostatic pressure¹, ρgz , this fact leads to a linear relationship between the yield stress of pervasively fractured brittle rock and depth: i.e., $\tau_Y \propto z$

Ductile Deformations

We can apply a general power-law rheology to ductile deformation. Then, for a given geotherm and an assumed value of strain rate, the shear stress becomes a function of depth, too.

$$\dot{\epsilon} = A(\sigma_1 - \sigma_3)^n \exp\left(\frac{Q}{RT(z)}\right),\tag{1}$$

where $(\sigma_1 - \sigma_3)$ can be related to the amount shear stress required for the assumed strain rate.

- ▶ If both brittle and ductile yield stresses are identified with $(\sigma_1 \sigma_3)$, we can plot the stress difference $(\sigma_1 \sigma_3)$ as a function of depth.
- At each depth, two stress difference values will be available: one for the brittle strength and the other for ductile strength. By picking the lower one, we get a single profile of strength, which is called the **yield strength** envelope.

Ductile Deformations

 $\dot{arepsilon} = A_p (\sigma_1 - \sigma_3)^n \ e^{-Q_P/R_g \ T}$

Steady State Flow Properties for Selected Rocks and Minerals

Rock/Mineral	Exponent n	$(Pa^{-n}s^{-1})$	Q_p (kJ mol^{-1}	Primary Reference
Quartz	3.0	1.2×10^{-24}	92	Heard and Carter (1968)
Quartzite (dry)	3.0	6.1×10^{-24}	190	Brace and Kohlstedt (1980)
Quartzite (wet)	1.9	1.2×10^{-13}	173	Hansen (1982)
Granite (Westerly: wet)	1.9	7.9×10^{-16}	141	Hansen and Carter (1983)
Granite (Westerly: dry)	3.3	3.1×10^{-26}	186	Hansen and Carter (1983)
Anorthosite	3.2	3.2×10^{-22}	238	Shelton and Tullis (1981)
Diabase (dry)	3.05	3.1×10^{-20}	276	Caristan (1982)
Diabase (Columbia)	4.7	1.1×10^{-26}	488	Mackwell et al. (1998)
Diabase (Maryland)	4.7	5.0×10^{-28}	482	Mackwell et al. (1998)
Quartz Diorite	2.4	1.2×10^{-16}	212	Hansen (1982)
Orthopyroxene (wet)	2.8	1.0×10^{-19}	271	Rayleigh et al. (1971)
Orthopyroxene (dry)	2.4	1.2×10^{-15}	293	Ross and Nielsen (1978)
Clinopyroxene (wet)	3.3	2.3×10^{-14}	490	Boland and Tullis (1986)
Clinopyroxene (dry)	5.3	1.6×10^{-36}	380	Boland and Tullis (1986)
Olivine	3.0	7.0×10^{-14}	520	Goetze (1978)
Olivine (dry)	3.5	2.4×10^{-16}	540	Karato et al. (1986)
Olivine (wet)	3.0	1.9×10^{-15}	420	Karato et al. (1986)
Dunite(wet)	4.5	4.0×10^{-25}	498	Chopra and Paterson (1981)
Dunite(dry)	3.6	7.9×10^{-18}	535	Chopra and Paterson (1981)



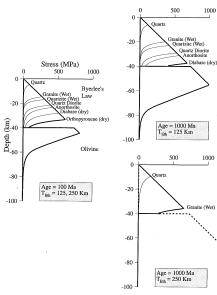
Yield Strength Envelope

< YSE for continental lithosphere with $\dot{\varepsilon} = 10^{-14} \text{ s}^{-1} >$

(Fig. 6.28 in Isostasy and Flexure of the Lithosphere, Watts, A. B., 2001, Cambridge University Press, Cambridge).

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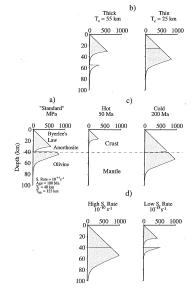
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Yield Strength Envelope

(cont'd) (Fig. 6.29 in Isostasy and Flexure of the Lithosphere, Watts, A. B., 2001, Cambridge University Press,

Cambridge)



Yield Strength Envelope

Strength of Oceanic Lithosphere

