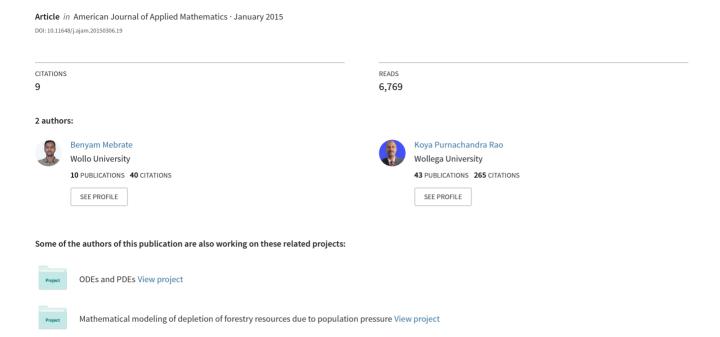
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Numerical Solution of a Two Dimensional Poisson Equation with Dirichlet Boundary Conditions

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Abstract: In this paper we have introduced Numerical techniques to solve a two dimensional Poisson equation together with Dirichlet boundary conditions. Specifically two methods are used for the purpose of numerical solution, viz. Finite difference method and Finite element method. The implementation of the solutions is done using Microsoft Office Excel worksheet or spreadsheet. The numerical solutions obtained by these two methods are also compared with each other graphically in two and three dimension.

Keywords: Dirichlet Boundary Conditions, Finite Difference Method, Finite Element Method, Poisson Equation, Spreadsheet

1. Introduction

Differential equations arise in many areas of applications such as in Sciences and Engineering. Most of the problems in almost all the scientific and engineering areas are modeled using partial differential equations. Partial differential equations arise when a dependent variable depends on two or more independent variables. Usually analytic solutions for these partial differential equations are difficult to compute and hence alternative methods are used. Numerical methods are widely used to solve partial differential equations within the given domain together with boundary conditions.

In general the domain is divided in to finite number of elements or the domain is made discretization into small triangles or rectangles. The values of the dependent variable at the nodes or grid points of these triangles or rectangles are computed. In literature finite difference and finite element methods are applied to solve two dimensional Laplace equations with Dirichlet boundary conditions using spreadsheet [8]. This motivates us to use spreadsheet implementation for solving two dimensional Poisson equations with Dirichlet boundary condition.

In Section 2, we introduced and discussed the two dimensional Poisson equation with Dirichlet boundary conditions. In Section 3, the finite difference method is applied for solving Poisson equation with Dirichlet boundary conditions. Also, we have made the use of spreadsheet

implementation. In Section 4, the finite element method is applied for solving Poisson equation with Dirichlet boundary conditions. Also, we have made the use of spreadsheet implementation. In Section 5, we have compared the results obtained in Sections 3 and 4. The paper ends with concluding remarks in Section 6.

2. Poisson Equation and the Problem Definition

The Poisson equation is given by [1-2, 4, 6, and 10]

$$\frac{\partial^2 u}{\partial x^2} + \frac{\partial^2 u}{\partial y^2} = f(x, y) \text{ or } \Delta u = f(x, y)$$
 (1)

Here the Laplace operator denotes $\Delta = \frac{\partial^2}{\partial x^2} + \frac{\partial^2}{\partial y^2}$. Equation (1) is elliptic second order linear partial differential equation. In (1), the dependent variable u(x,y) is a function of its arguments and depends on the independent variables x and y. The function f(x,y) is known as a source function. For the purpose of numerical solution of the Poisson equation (1) we consider a plane region defined by $R = \{(x,y): 0 \le x \le 1 \text{ and } 0 \le y \le 1\}$. We will also impose Dirichlet boundary conditions for the dependent variable u(x,y) on the region R defined as

$$u(0, y) = 0, u(x, 0) = 0,$$

$$u(x, 1) = 0, u(1, y) = 0$$
 (2)

The region R together with the Dirichlet boundary conditions (2) is illustrated in Figure 1. The x - variable increases along the positive horizontal axis while the y - variable increases along the positive vertical axis.

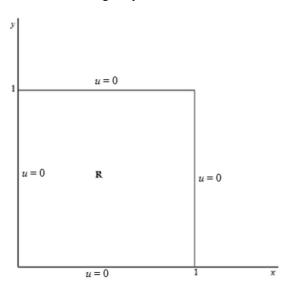


Figure 1. Rectangular region R with boundary conditions.

We now split the region R in Figure 1 in to a finite number of rectangular elements. The selection of step lengths h = (1/4) in x – direction and k = (1/4) in y – direction will split the plane region R into 16 rectangular elements.

The nodes and the sides in and on the region R can be classified in to two groups, viz. interior and exterior. The interior nodes and sides lie inside the region R while the exterior nodes and sides lie on the boundary of the region R. Further all the interior nodes and sides are common to adjacent rectangular elements. Of course this fact is exempted for the exterior nodes and sides. The nodes of the region R are numbered and shown in Figure 2.

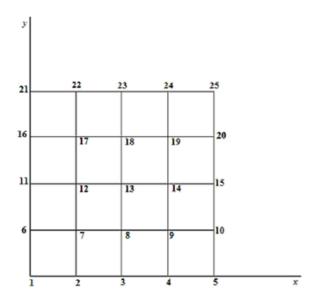


Figure 2. Rectangular elements with node numbers.

Here our problem is to find numerical values of the function u(x,y) at the interior node points of the region R provided that the Poisson equation (1) and the Dirichlet conditions given in (2) are satisfied.

3. Finite Difference Method

The finite difference techniques are based on the replacement of differential equations with approximately equivalent finite difference equations. Whenever solving differential equations analytically is not easy then the differential equations are replaced by corresponding finite difference equations and those will be solved. Further, the differential equations provide exact values while the difference equations provide approximate values for the variables. Approximation of the solution relates to the number of grids given region is divided into. More are the grids and better is the approximation.

The finite difference equations are algebraic in nature and their approximate solutions are related to number of grid points of the region R. A finite difference solution basically involves the following three steps:

- (i) Divide the solution region using grids and nodes into small rectangles
- (ii) Approximate the given differential equation by equivalent finite difference equations that relate the solutions to the grid points.
- (iii) Solve the difference equations subject to the prescribed boundary and/or initial conditions.

To find the solution of the function u(x, y) on the region R, we divide the solution region R of xy – plane into equal rectangles or meshes of sides h and k along x and y directions respectively, as shown in Figure 2. We now construct a finite difference equation to represent equation (1) and boundary conditions (2). This will help us to calculate the values of u(x, y) at the nodes of the rectangles in the region R. The detailed derivation of finite difference equation from (1) is given in Appendix 1. Here we directly consider the difference equation as

$$u_{i,j} = \frac{1}{4} \left(u_{i-1,j} + u_{i+1,j} + u_{i,j-1} + u_{i,j+1} - h^2 f_{i,j} \right),$$

$$i, j = 2, 3, 4$$
 (3)

Also, the boundary conditions as

$$u(x_i, y_1) = 0, u(x_i, y_5) = 0, u(x_1, y_j) = 0, u(x_5, y_j) = 0,$$

 $i, j = 1, 2, 3, 4, 5$ (4)

Equation (3) represents a difference equation and (4) represents the boundary conditions equivalent respectively to the differential equation (1) and the boundary conditions (2). Here in (3) we have used the notations $asu_{i,j} = u(x_i, y_j)$ and $f_{i,j} = f(x_i, y_j)$. Also, the set $\{x_1, x_2, x_3, x_4, x_5\}$ is the set of partition points of the interval [0, 1] along x - axis with step length h. Similarly, the set $\{y_1, y_2, y_3, y_4, y_5\}$ is the set of partition points of the interval [0, 1] along y - axis with step length k. Of course in our present study we choose k - k. The

numerical solutions for u(x, y) is computed at all the nine free nodes of the region Rlocated at the locations $\{(i, j): i = 2, 3, 4 \text{ and } j = 2, 3, 4\}$. These nine free nodes are numbered as shown in Figure 3.

Gauss Seidel method is applied to solve (3) numerically for the specific value of the function f(x,y) = 1. The function f(x,y) in fact admits any constant or expression in terms of x and y. For simplicity and to illustrate the procedure of numerical solution here we have selected that f(x,y) = 1. Other expressions for f(x,y) will be considered as and when it demands in our further work.

At the nodal points as shown in Figure 3 we compute the successive approximate solutions for u(x,y) using spreadsheet. The iterations and the approximations of the solutions are provided in a tabular form in Table 1.The Gauss – Seidel iterations for every node are continued till the approximate solution values converge to a constant.

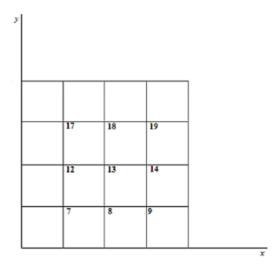


Figure 3. Numberedfree nodes of the region R.

Table 1. Solution using Gauss Seidel method.

T44'	Node numl	bers							
Iterations	7	8	9	12	13	14	17	18	19
1	0	0	0	0	0	0	0	0	0
2	-0.0156	-0.0195	-0.0205	-0.0195	-0.0254	-0.0271	-0.0205	-0.0271	-0.0292
3	-0.0254	-0.0334	-0.0308	-0.0334	-0.0459	-0.0421	-0.0308	-0.0421	-0.0367
4	-0.0323	-0.0429	-0.0369	-0.0429	-0.0581	-0.0485	-0.0369	-0.0485	-0.0399
5	-0.0371	-0.0486	-0.0399	-0.0486	-0.0642	-0.0516	-0.0399	-0.0516	-0.0414
6	-0.0399	-0.0516	-0.0414	-0.0516	-0.0673	-0.0532	-0.0414	-0.0532	-0.0422
7	-0.0414	-0.0532	-0.0422	-0.0532	-0.0688	-0.0539	-0.0422	-0.0539	-0.0426
8	-0.0422	-0.0539	-0.0426	-0.0539	-0.0695	-0.0543	-0.0426	-0.0543	-0.0428
9	-0.0426	-0.0543	-0.0428	-0.0543	-0.0699	-0.0545	-0.0428	-0.0545	-0.0429
10	-0.0428	-0.0545	-0.0429	-0.0545	-0.0701	-0.0546	-0.0429	-0.0546	-0.0429
11	-0.0429	-0.0546	-0.0429	-0.0546	-0.0702	-0.0546	-0.0429	-0.0546	-0.0429
12	-0.0429	-0.0546	-0.0429	-0.0546	-0.0703	-0.0547	-0.0429	-0.0547	-0.0430
13	-0.0429	-0.0547	-0.0430	-0.0547	-0.0703	-0.0547	-0.0430	-0.0547	-0.0430
14	-0.0430	-0.0547	-0.0430	-0.0547	-0.0703	-0.0547	-0.0430	-0.0547	-0.0430
15	-0.0430	-0.0547	-0.0430	-0.0547	-0.0703	-0.0547	-0.0430	-0.0547	-0.0430

4. Finite Element Method

Finite element methods are essentially methods for finding approximate solutions of partial differential equations defined in a finite region or domain. Finite element method involves the following steps in solving a partial differential equation together with boundary conditions:

- i. Divide the domain in to a finite number of elements.
- ii. Drive the weak formulation corresponding to the given problem.
- Calculate the stiffness matrix and load vector for each element in the domain.
- iv. Calculate global stiffness matrix and assemble. Calculate global load vector and assemble
- v. Solve the resulting system of algebraic linear equations subject to the satisfaction of the boundary conditions.

The given domain R is divided into 32 congruent right angled and isosceles triangles. The nodes of the triangles are represented using unenclosed numbers. These unenclosed numbers are called global node numbers. Similarly, the triangles are represented by en-rectangle numbers. These

enclosed numbers are called element numbers. The enrectangle number is a number that is enclosed by a rectangle. These details are shown in Figure 4.

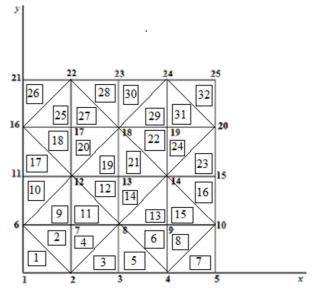


Figure 4. Division of domain R into isosceles and right angled triangles.

The weak formulation of the problem defined in (1) and (2) requires that the function u(x,y) (i) is a member of Sobolev space $H_0^1(R)$ i.e., $u \in H_0^1(R)$ and (ii) has to satisfy the condition $\left[-\int_R \nabla u \nabla v dR\right] = \left[\int_R f v dR\right]$, $\forall v \in H_0^1(R)$. Here Sobolev space $H_0^1(R)$ is defined as a set of functions w satisfying the conditions (i) $\int_R w^2 dR < \infty$ (ii) $\int_R (w_x)^2 dR < \infty$ (iii) $\int_R (w_y)^2 dR < \infty$ and (iv) w = 0 on the boundary of the region w. The detailed derivation of weak formulation of equation (1) is given in appendix 2.

The finite element problem for the approximation of Poisson equation (1) together with boundary condition (2) requires finding $u_d \in V_d$ satisfying the following condition:

$$-\int_{R} \nabla u_d \cdot \nabla v_d dR = \int_{R} f v_d dR \forall v_d \in V_d. \tag{5}$$

Here in (5) the space V_d approximates $H_0^1(R)$ and defined as

$$V_d = \begin{cases} v_d \in C(\bar{R}) \colon v_d|_{K_m} \in \mathbb{p}_r, v_d|_{\partial R} = 0, \\ \forall m \in \{1, 2, \dots, 32\} \end{cases}$$

Where $C(\bar{R})$ = the space of continous function on the closure of R, that is \bar{R} .

 ∂R = boundary of the region R

 p_r = the space of plynomials of degree less than or equal tor

 K_m = the m^{th} triangular elements of the region R

$$d = \max_{m \in \{1, 2, \dots, 32\}} diameter(K_m), diameter(K_m)$$

$$= \max_{x,y \in K_m} |x - y|$$

Furthermore, each function $v_d \in V_d$ is characterized, uniquely, by the values it takes at the nodes N_j , $\forall j = 1,2,3$ of the triangular elements. A basis in the space V_d can be the set of the characteristic Lagrangian functions $\varphi_i \in V_d$, i = 1,2,3, such that

$$\varphi_i(N_j) = \begin{cases} 0, i \neq j \\ 1, i = j \end{cases} i, j = 1,2,3 \text{ and } N_j = \text{node } j \text{ of } K_m$$

The function φ_i is called shape function. The function $v_d \in V_d$ can be expressed through a linear combination of the basis functions of V_d in the following way

$$v_d(x,y) = \sum_{i=1}^{3} v_i \varphi_i(x,y), v_i = v_h(N_i)$$
 (6)

By expressing the discrete solution u_d in terms of the basis $\{\varphi_i\}$, we have

$$u_d(x,y) = \sum_{i=1}^{3} u_i \varphi_i(x,y), v_i = v_h(N_i)$$
 (7)

We now assign numbers 1, 2, and 3 to the nodes of each triangle element. Starting with right angled vertex, number the nodes in anti clockwise direction using 1, 2, and 3 respectively. This numbering procedure will help us in having all elements the same stiffness matrix since these elements have the same geometry. The detailed derivation of equation of stiffness matrix and load vector is given in appendix 3. Here we consider only equitation of stiffness matrix and load vector. The stiffness matrix for each triangular element is calculated using the relation

$$A_{i,j}^{K_m} = -\int_{K_m} \nabla \varphi_i \, \nabla \varphi_j dx dy \, i, j = 1,2,3$$

 $K_m = \text{The } m^{th}$ triangular element in the domain R. Here m = 1, ..., 32.

 A^{K_m} = Stiffness matrix of the m^{th} triangular element.

 $A_{i,j}^{K_m}$ = The (i,j) entry of the stiffness matrix A^{K_m} .

 φ_l = Shape functions at the nodes of the element K_m . Here l=1,2,3.

Thus the stiffness matrix for each element is

Table 2. Stiffness matrix representing every triangular element of the region R.

Local node numbers Local node numbers	1	2	3
1	-1	0.5	0.5
2	0.5	-0.5	0
3	0.5	0	-0.5

The (p,q) entry of the global matrix of element K_m takes the (i,j) entry of the stiffness matrix of element K_m where p, q = 1,2,...,25 are global node numbers, and i,j = 1,2,3 are corresponding local node numbers for the global node numbers and m = 1,2,...,32. The sum of the 32 global matrices gives the assembled global matrix and the assembled global matrix is given in Table 3.

Table 3. Assembled global stiffness matrix.

	Glo	bal no	de nu	ımber	s																					
		1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25
	1	-1	0.5	0	0	0	0.5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	2	0.5	-2	0.5	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	3	0	0.5	-2	0.5	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Global	4	0	0	0.5	-2	0.5	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
node numbers	5	0	0	0	0.5	-1	0	0	0	0	0.5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1141110-015	6	0.5	0	0	0	0	-2	1	0	0	0	0.5	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7	0	1	0	0	0	1	-4	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
	8	0	0	1	0	0	0	1	-4	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0
	9	0	0	0	1	0	0	0	1	-4	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0

10	0	0	0	0	0.5	0	0	0	1	-2	0	0	0	0	0.5	0	0	0	0	0	0	0	0	0	0
11	0	0	0	0	0	0.5	0	0	0	0	-2	1	0	0	0	0.5	0	0	0	0	0	0	0	0	0
12	0	0	0	0	0	0	1	0	0	0	1	-4	1	0	0	0	1	0	0	0	0	0	0	0	0
13	0	0	0	0	0	0	0	1	0	0	0	1	-4	1	0	0	0	1	0	0	0	0	0	0	0
14	0	0	0	0	0	0	0	0	1	0	0	0	1	-4	1	0	0	0	1	0	0	0	0	0	0
15	0	0	0	0	0	0	0	0	0	0.5	0	0	0	1	-2	0	0	0	0	0.5	0	0	0	0	0
16	0	0	0	0	0	0	0	0	0	0	0.5	0	0	0	0	-2	1	0	0	0	0.5	0	0	0	0
17	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	-4	1	0	0	0	1	0	0	0
18	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	-4	1	0	0	0	1	0	0
19	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	-4	1	0	0	0	1	0
20	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.5	0	0	0	1	-2	0	0	0	0	0.5
21	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.5	0	0	0	0	-1	0.5	0	0	0
22	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0.5	-2	0.5	0	0
23	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0.5	-2	0.5	0
24	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0.5	-2	0.5
25	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.5	0	0	0	0.5	-1

For the constant f(x, y) = 1, the load vector for each triangular element K_m is denoted by F^{K_m} and calculated from the relation

$$F_j^{K_m} = \int_{K_m} \varphi_j dx dy, \forall j = 1, 2, 3.$$

Here $F_j^{K_m}$ denotes the j entry of the load vector F^{K_m} . Also, $\varphi_l(l=1,2,3)$ represents a shape function at the three nodes of any triangular element K_m where $m=1,2,\ldots,32$. Using two dimensional four point rule Gauss quadrature formula, each element has the same load vector

Table 4. Load vector corresponds to each triangular element.

Local node numbers	Load vector
1	0.0104
2	0.0104
3	0.0104

The p entry of the global load vector of element K_m takes the i entry of the load vector of element K_m where $p=1,2,\ldots,25$ are global node numbers, i=1,2,3 are corresponding local node numbers for the global node

numbers and m=1,2,...,32. The sum of the 32 global load vectors gives the assembled global load vector and the assembled global load vector is

Table 5. Assembled global load vectors.

	1	0.0104		13	0.0417		
	2	0.0417		14	0.0833		
	3	0.0208		15	0.0208		
	4	0.0417		16	0.0417		
01.1.1	5	0.0104		17	0.0417		
Global	6	0.0417	Global	18	0.0833		
node numbers	7	0.0417	node	19	0.0417		
numbers	8	0.0833	numbers	20	0.0417		
	9	0.0417		21	0.0104		
	10	0.0417		22	0.0417		
	11	0.0208		23	0.0208		
	12	0.0833		24	0.0417		
				25	0.0104		
Rows thro	ugh 1 - 12		Rows through 13 - 25				

Applying the boundary conditions (2) the assembled global matrix and the load vector becomes respectively

Table 6. Reduced assembled global matrix and load vector.

		Glob	al node r	umbers									
		7	8	9	12	13	14	17	18	19			
	7	-4	1	0	1	0	0	0	0	0		7	0.0417
	8	1	-4	1	0	1	0	0	0	0		8	0.0833
	9	0	1	-4	0	0	1	0	0	0		9	0.0417
Global	12	1	0	0	-4	1	0	1	0	0	Global	12	0.0833
node	13	0	1	0	1	-4	1	0	1	0	node	13	0.0417
numbers	14	0	0	1	0	1	-4	0	0	1	numbers	14	0.0833
	17	0	0	0	1	0	0	-4	1	0		17	0.0417
	18	0	0	0	0	1	0	1	-4	1		18	0.0833
	19	0	0	0	0	0	1	0	1	-4		19	0.0417

Using Gauss Seidel method successive approximate solution for u(x, y) at the free nodes of the region R is obtained. The results are given in Table 7.

T44'	Global noc	le numbers							
Iterations	7	8	9	12	13	14	17	18	19
1	0	0	0	0	0	0	0	0	0
2	-0.0104	-0.0234	-0.0163	-0.0234	-0.0221	-0.0304	-0.0163	-0.0304	-0.0256
3	-0.0221	-0.036	-0.027	-0.036	-0.0436	-0.0449	-0.027	-0.0449	-0.0329
4	-0.0284	-0.0456	-0.033	-0.0456	-0.0557	-0.0512	-0.033	-0.0512	-0.036
5	-0.0332	-0.0513	-0.0361	-0.0513	-0.0617	-0.0543	-0.0361	-0.0543	-0.0376
6	-0.0361	-0.0543	-0.0376	-0.0543	-0.0647	-0.0558	-0.0376	-0.0558	-0.0383
7	-0.0376	-0.0558	-0.0383	-0.0558	-0.0662	-0.0565	-0.0383	-0.0565	-0.0387
8	-0.0383	-0.0565	-0.0387	-0.0565	-0.067	-0.0569	-0.0387	-0.0569	-0.0389
9	-0.0387	-0.0569	-0.0389	-0.0569	-0.0673	-0.0571	-0.0389	-0.0571	-0.039
10	-0.0389	-0.0571	-0.039	-0.0571	-0.0675	-0.0572	-0.0390	-0.0572	-0.039
11	-0.0390	-0.0572	-0.039	-0.0572	-0.0676	-0.0572	-0.0390	-0.0572	-0.039
12	-0.0390	-0.0572	-0.039	-0.0572	-0.0677	-0.0573	-0.0390	-0.0573	-0.0391
13	-0.0390	-0.0573	-0.0391	-0.0573	-0.0677	-0.0573	-0.0391	-0.0573	-0.0391
14	-0.0391	-0.0573	-0.0391	-0.0573	-0.0677	-0.0573	-0.0391	-0.0573	-0.0391
15	-0.0391	-0.0573	-0.0391	-0.0573	-0.0677	-0.0573	-0.0391	-0.0573	-0.0391

Table 7. Solution of u(x, y) at free nodes of R using Gauss Seidel method.

5. Comparison Between the Two Solution

For the differential equation (1) together with boundary condition equations (2) we have found numerical solutions using two methods, viz. Finite difference and Finite element methods. Here in this section we compare these numerical solutions with each other. The solutions of u(x, y) at the free nodes of the region obtained using the two methods are given in the Table 8.

Table 8. Finite difference and finite element solutions.

Nodes	Finite difference solution	Finite element solution
7	-0.0430	-0.0391
8	-0.0547	-0.0573
9	-0.0430	-0.0391
12	-0.0547	-0.0573
13	-0.0703	-0.0677
14	-0.0547	-0.0573
17	-0.0430	-0.0391
18	-0.0547	-0.0573
19	-0.0430	-0.0391

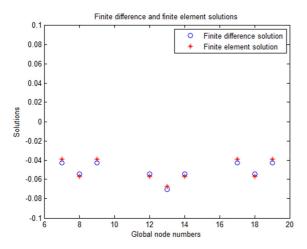


Figure 5. Comparisons between finite difference and finite element solution using two dimensional pictures.

It can be observed from Table 8 that the maximum difference of the two solutions is 0.0039 at node 7. This

difference can be minimized by taking more number of nodal points. The two solutions can be described graphically in two dimensions and three dimensions as shown in Figure 5 and Figure 6 respectively.

As it is seen in Figure 5 the finite element and finite difference solutions are close to each other at the free nodes.

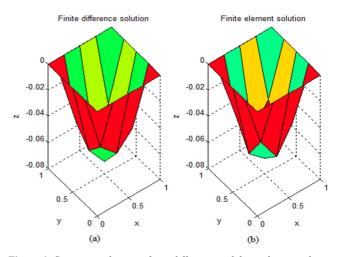


Figure 6. Comparison between finite difference and finite element solutions using three dimensional pictures.

As it is seen from Figure 6, the finite difference solution in (a) and finite element solution in (b) are somewhat the same.

6. Conclusions

In this paper we have introduced spreadsheet implementations of numerical methods for solving Poisson equation in two dimensions with Dirichlet boundary conditions. Other types of boundary conditions and right hand side function of the Poisson equation could be considered to solve the same problem using spreadsheet. Application of finite difference method is somewhat complex in comparison with that of the finite element method. Many physical problems have irregularly shaped boundaries and boundary conditions are expressed using derivatives. Boundary conditions which are expressed using differential

equations are difficult to handle using finite – difference techniques because each boundary condition involving a derivative must be approximated by a difference quotient at the grid points. Also, if the shape of the boundary is irregular then placing the grid points on the boundary is difficult. The Finite element method alleviates all difficulties appears in finite difference method. It is independent of boundary conditions of the problem.

Appendix 1 Derivation of Finite Difference Equation

To drive finite difference equations (3), we consider Figure 2. Now in Figure 2 we represent node 8, node 12, node 13, node 14 and node 18 respectively by capital letters U, P, Q, S and V. If the coordinate (x_i, y_j) corresponds to Q, then the coordinates $(x_i, y_{j-1}), (x_{i-1}, y_j), (x_{i+1}, y_j)$ and (x_i, y_{j+1}) are respectively corresponds to U, P, S and V. Expanding u_S and u_P using Taylor series expansion about the point $Q(x_i, y_j)$, we get

$$u_{S} = u_{Q} + \left(\frac{h}{1!}\right) \left(\frac{\partial u}{\partial x}\right)_{Q} + \left(\frac{h^{2}}{2!}\right) \left(\frac{\partial^{2} u}{\partial x^{2}}\right) \Big|_{Q} + \left(\frac{h^{3}}{3!}\right) \left(\frac{\partial^{3} u}{\partial x^{3}}\right) \Big|_{Q} + \left(\frac{h^{4}}{4!}\right) \left(\frac{\partial^{4} u}{\partial x^{4}}\right) \Big|_{Q} + \cdots$$
(8)

and

$$u_{P} = u_{Q} - \left(\frac{h}{1!}\right) \left(\frac{\partial u}{\partial x}\right) \Big|_{Q} + \left(\frac{h^{2}}{2!}\right) \left(\frac{\partial^{2} u}{\partial x^{2}}\right) \Big|_{Q} - \left(\frac{h^{3}}{3!}\right) \left(\frac{\partial^{3} u}{\partial x^{3}}\right) \Big|_{Q} + \left(\frac{h^{4}}{4!}\right) \left(\frac{\partial^{4} u}{\partial x^{4}}\right) \Big|_{Q} - \cdots$$
(9)

respectively. Adding (8) and (9) gives

$$u_S + u_P = 2u_Q + h^2 \left(\frac{\partial^2 u}{\partial x^2}\right)\Big|_Q + \left(\frac{h^4}{12}\right) \left(\frac{\partial^4 u}{\partial x^4}\right)\Big|_Q + \cdots$$

Hence

$$\left. \frac{\partial^2 u}{\partial x^2} \right|_Q = \frac{u_S + u_P - 2u_Q}{h^2} + O(h^2) \tag{10}$$

Similarly

$$\left. \frac{\partial^2 u}{\partial y^2} \right|_Q = \frac{u_V + u_U - 2u_Q}{h^2} + O(h^2) \tag{11}$$

Substituting equations (10) and (11) in equation (1), we obtain

$$u_Q = \frac{1}{4} \left(u_S + u_P + u_U + u_V - h^2 f_{i,j} \right) + O(h^2)$$

Hence,

$$u_{i,j} \simeq \frac{1}{4} (u_{i-1,j} + u_{i+1,j} + u_{i,j-1} + u_{i,j+1} - h^2 f_{i,j}),$$

 $i, j = 2, 3, 4$

Appendix 2 Derivation of Weak Formulation of Equation (1)

Multiply both sides of equation (1) by arbitrary test function $v = v(x, y) \in H_0^1(R)$ and integrate over the region R, we get

$$\int_{R} \Delta u v \, dR = \int_{R} f v \, dR \tag{12}$$

To reduce the order of derivatives in (12) we use Green's formula. The formula states as follows

$$\int_{R} \Delta u v \, dR = -\int_{R} \nabla u \cdot \nabla v \, dR + \int_{\partial R} \frac{\partial u}{\partial n} v \, d\gamma \qquad (13)$$

Here ∂R , $\partial \gamma$, ∇u and n denotes respectively boundary of region R, line segment on the boundary of R, gradient of u and outward unit normal on the boundary of R.

Substituting equation (13) in (12) we have

$$-\int_{R} \nabla u \cdot \nabla v \, dR + \int_{\partial R} \frac{\partial u}{\partial n} v \, d\gamma = \int_{R} f v \, dR \tag{14}$$

Since $v \in H_0^1(R)$ the condition $\int_{\partial R} \frac{\partial u}{\partial n} v \, d\gamma = 0$ is valid. Thus equation (14) becomes

$$-\int_{R} \nabla u \cdot \nabla v \, dR = \int_{R} f v \, dR$$

Appendix 3 Derivation of Equations of Stiffness Matrix and Load Vector

We now consider the m^{th} triangular element K_m . Equation (5) can be defined on triangular element K_m as

$$-\int_{K_m} [\nabla u_d \cdot \nabla v_d + f v_d] dx dy = 0 , m = 1,2,...,32. \eqno(15)$$

Substituting (6) and (7) in equation (15), we get

$$-\int_{K_m} \left[\nabla \left(\sum_{i=1}^3 u_i \varphi_i(x, y) \right) \cdot \nabla \left(\sum_{j=1}^3 v_j \varphi_j(x, y) \right) + f \left(\sum_{i=1}^3 v_i \varphi_i(x, y) \right) \right] dx dy = 0.$$

Which is equivalent to

$$\sum_{j=1}^{3} v_j \left[\sum_{i=1}^{3} u_i \int_{K_m} \nabla \varphi_i \nabla \varphi_j \ dxdy - \int_{K_m} f \varphi_j \ dxdy \right] = 0.$$

Since this equation holds true for all $v_d \in V_d$, we have

$$\sum_{i=1}^{3} u_i \int_{K_m} \nabla \varphi_i \nabla \varphi_j \ dxdy = \int_{K_m} f \varphi_j \ dxdy, j = 1,2,3.$$

Here for each $m \in \{1,2,...,32\}$ we have the stiffness matrix and the load vector respectively are $A_{i,j}^{K_m} = \int_{K_m} \nabla \varphi_i \nabla \varphi_j \ dxdy$ and $F_j^{K_m} = \int_{K_m} f \varphi_j \ dxdy$, i,j = 1,2,3.

References

[1] Alfio Quarteroni, Numerical models for differential problems (2nd edition), Springer-Verlag, Italia, 2014.

- [2] Chapra Canal, Numerical methods for engineers (4th edition), The McGraw Hill Companies, 2001.
- [3] Erwin Kreyzing, Advanced Engineering Mathematics (9thedition), 2006 John Wiley and Sons, Inc.
- [4] G. Evans, J. Blackledge and P. Yardley, Numerical methods for partial differential equations, Springer-Verlag London Limited 2000.
- [5] J. David Logan, A first course in DEs, 2006 Springer Science + Business Media. Inc.
- [6] Lichard L. Burden and J. Douglas Faires, Numerical analysis (9th edition), 2011, 2005, 2001 Brooks/Cole, Cengage Learning, 20 Channel Center Street Boston, MA02210, USA.

- [7] Mark A. Lau and Sastry P. Kuruganty, Spreadsheet implementation for solving boundary value problem in electromagnetic, spreadsheet in education (eJSiE) 4(1), 2001.
- [8] Parag V. Patil and Dr. J.S.V.R. Krishna Prasad, Numerical solution for two dimensional Laplace equation with Dirichlet boundary conditions, Volume 6, Issue 4 (May - June 2013), PP 66-75.
- [9] Ravi P. Agarwal and Donal O'Regan, Ordinary and Partial DEs, 2006 Springer Science + Business Media, LLC (2009).
- [10] Susan C. Brenner, L. Ridgway Scott, The mathematical theory of finite element methods (3rdedition), 2008 Springer Sciences + Business Media, LLC.