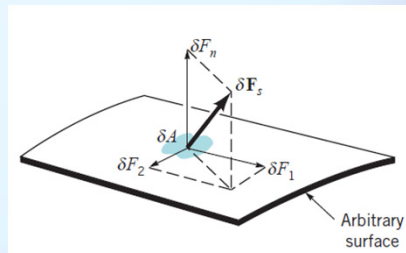




## Stress field

### Physical properties types

- **Scalar:** a magnitude, like pressure
- **Vector:** a magnitude and a direction (or 3 scalars), like velocity
- **Tensor:** a magnitude and 2 directions (or 9 scalars), like stress



$$\sigma_{nn} = \lim_{\delta A \rightarrow 0} \frac{\delta F_n}{\delta A}$$

Normal stress

$$\sigma_{n1} = \lim_{\delta A \rightarrow 0} \frac{\delta F_1}{\delta A}$$

Shear stress

$$\sigma_{n2} = \lim_{\delta A \rightarrow 0} \frac{\delta F_2}{\delta A}$$

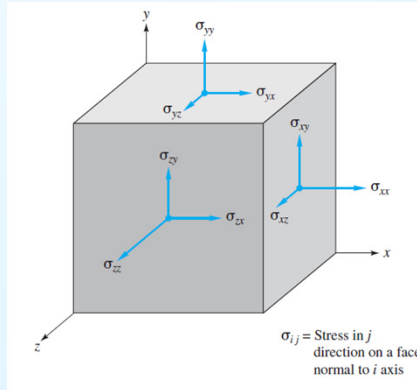
Shear stress

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## Stress field

- Choosing Cartesian coordinates
- Strength of materials I: Stress



$$\sigma_{ij} \equiv \begin{bmatrix} \sigma_{xx} & \sigma_{xy} & \sigma_{xz} \\ \sigma_{yx} & \sigma_{yy} & \sigma_{yz} \\ \sigma_{zx} & \sigma_{zy} & \sigma_{zz} \end{bmatrix}$$

**Stress tensor (symmetric)**

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## Stress field in a fluid at rest

- Fluid at rest
  - Force balance for an arbitrary wedge-shaped element
  - No motion (zero shear stresses)

$$\sum F_x = ma_x \rightarrow$$

$$-\sigma_{xx}dydz + \sigma_{nn}dsdz \sin \alpha = \rho \frac{dx dy dz}{2} a_x$$

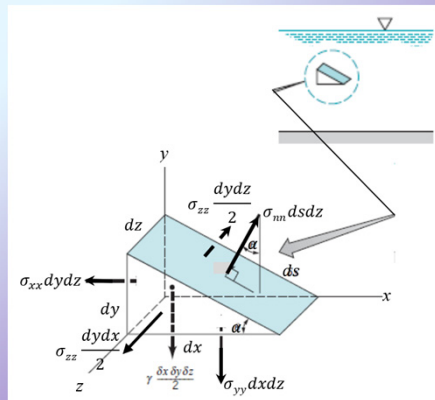
$$\rightarrow -\sigma_{xx} + \sigma_{nn} = \rho \frac{dx}{2} a_x \xrightarrow{dx, dy, dz \rightarrow 0}$$

$$\sigma_{xx} = \sigma_{nn}$$

- Similarly, it can be shown that

$$\sigma_{xx} = \sigma_{yy} = \sigma_{zz} = \sigma_{nn} \equiv -p \quad (1.3)$$

static pressure



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## Stress field in a fluid at rest

- **Fluid at rest**
  - Normal stress is **independent of direction**
  - The stress tensor is simplified to

$$\sigma_{ij} \equiv \begin{bmatrix} -p & 0 & 0 \\ 0 & -p & 0 \\ 0 & 0 & -p \end{bmatrix}$$

- **Determining the pressure field,  $p(x, y, z, t)$ , is the goal**
- **For compressible flows: The static pressure and thermodynamic pressure are identical**

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## Pressure field in a fluid at rest

- **Force balance when viscous forces are negligible**
  - Fluid with no relative motion between elements (rigid-body motion), or
  - Inviscid flow, or
  - **Fluid at rest**

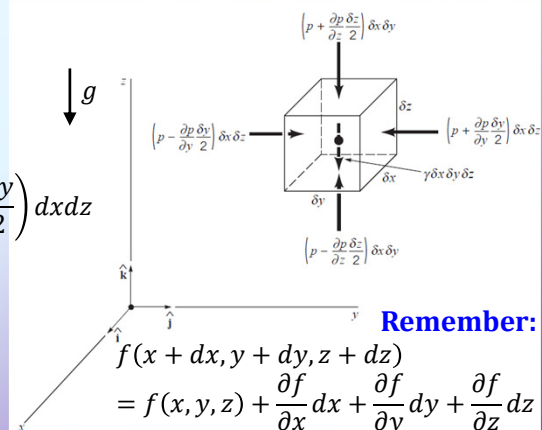
$$\begin{aligned} \sum F_y &\equiv \delta F_y = \\ &= \left( p - \frac{\partial p}{\partial y} \frac{dy}{2} \right) dx dz - \left( p + \frac{\partial p}{\partial y} \frac{dy}{2} \right) dx dz \\ &= -\frac{\partial p}{\partial y} dx dy dz \end{aligned}$$

● **Similarly,**

$$\delta F_x = -\frac{\partial p}{\partial x} dx dy dz,$$

$$\delta F_z = -\frac{\partial p}{\partial z} dx dy dz$$

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## Pressure field in a fluid at rest

- Force balance when viscous forces are negligible

$$\delta \vec{F}_s = \delta F_x \hat{i} + \delta F_y \hat{j} + \delta F_z \hat{k} = - \underbrace{\left( \frac{\partial p}{\partial x} \hat{i} + \frac{\partial p}{\partial y} \hat{j} + \frac{\partial p}{\partial z} \hat{k} \right)}_{\vec{\nabla} p \text{ or } \text{grad} p} dxdydz$$

Net pressure  
force on an  
element per  
unit volume

$$\frac{\delta \vec{F}_s}{dxdydz} = -\vec{\nabla} p = \frac{\partial p}{\partial x} \hat{i} + \frac{\partial p}{\partial y} \hat{j} + \frac{\partial p}{\partial z} \hat{k} \quad (2.3)$$

Convention: Gravity in the  
negative z-direction

$$\sum \vec{F} = m\vec{a} \rightarrow \delta \vec{F}_s - \gamma dxdydz \hat{k} = \rho dxdydz \vec{a} \rightarrow -\vec{\nabla} p - \gamma \hat{k} = \rho \vec{a} \quad (3.3)$$

- Solutions to Eq. (3.3)?

1 vectorial partial  
differential equation

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## Pressure field in a fluid at rest

- Fluid at rest ( $\vec{a} = 0$ )

$$(3.3) \rightarrow \vec{\nabla} p = -\gamma \hat{k} \rightarrow \begin{cases} \frac{\partial p}{\partial x} = 0 \rightarrow p = p(y, z) \\ \frac{\partial p}{\partial y} = 0 \rightarrow p = p(z) \\ \frac{\partial p}{\partial z} = -\gamma \rightarrow \frac{dp}{dz} = -\gamma \end{cases} \quad (4.3)$$

- Pressure is the same within a **connected** fluid at rest at all points with the **same elevation**
- For incompressible fluids ( $\rho, \gamma = cte$ )

$$\frac{dp}{dz} = -\gamma \rightarrow dp = -\gamma dz \xrightarrow{\text{integration}} p = -\gamma z + C \xrightarrow{\text{Boundary condition}} p(z = z_{ref}) = p_{ref}$$

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$$p = p_{ref} - \gamma(z - z_{ref}) \quad (5.3) \quad p = p_{ref} - \gamma h$$

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## Pressure field in a fluid at rest

- Fluid at rest ( $\vec{a} = 0$ )
- **Exercise:** For a compressible perfect gas at rest and uniform temperature, show that

$$\frac{p}{p_{ref}} = \exp\left(-\frac{g}{RT}(z - z_{ref})\right) \quad (6.3)$$

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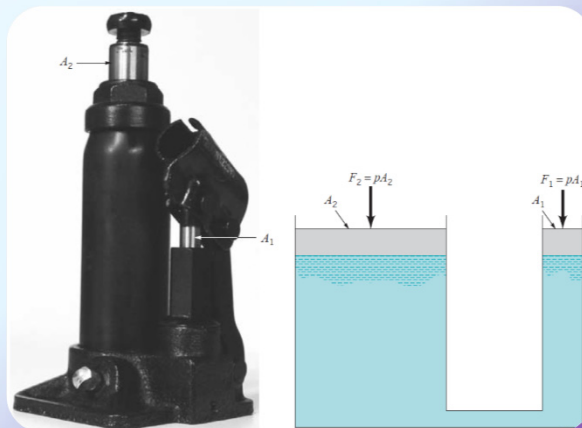
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## Example

- Hydraulic jack



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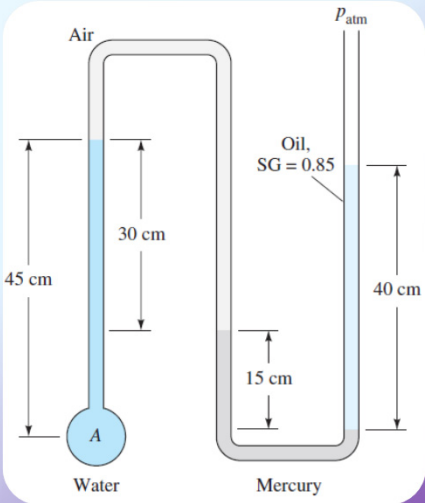
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# Example

- **Manometers**

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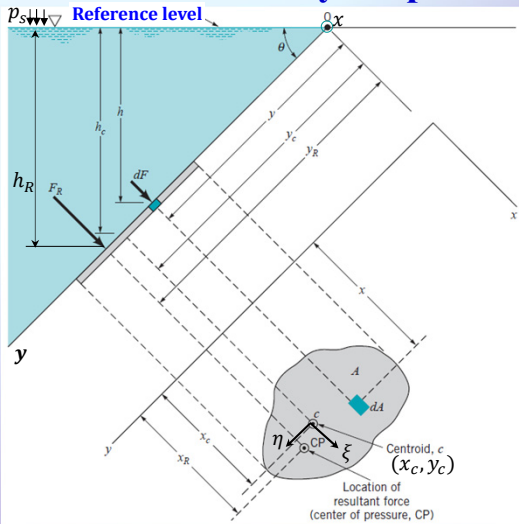
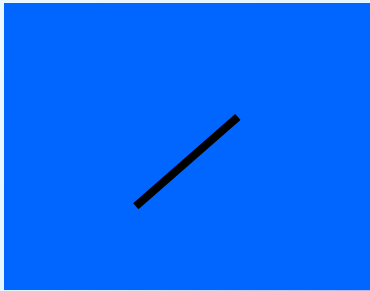


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# Hydrostatic force on a plane surface

- **An inclined plane surface of arbitrary shape**
- **The resultant hydrostatic force and its point (line) of action**



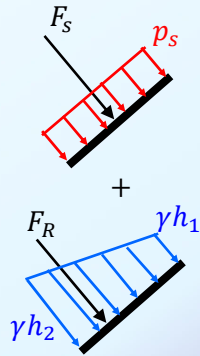
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## Hydrostatic force on a plane surface

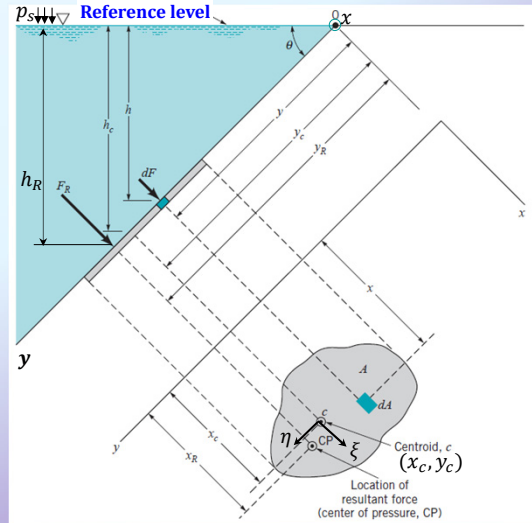
### ● Decomposition into two components

$$p = p_s + \gamma h$$



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## Hydrostatic force on a plane surface

### ● The resultant hydrostatic force and its point (line) of action

$$F_R = \gamma \sin \theta y_c A = \gamma h_c A \quad (8.3)$$

$$y_R = \frac{\int_A y^2 dA}{y_c A} = \frac{I_x}{y_c A} \quad (9.3)$$

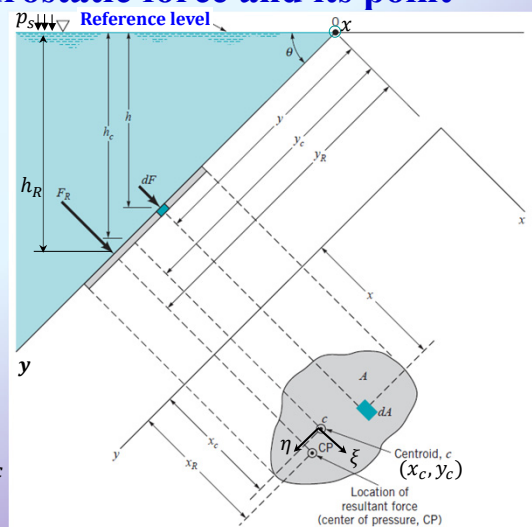
$$I_x = I_{\xi} + A y_c^2 \quad (10.3) \quad \rightarrow$$

$$y_R = \frac{I_{\xi}}{y_c A} + y_c \quad (11.3)$$

### ● Similarly

$$x_R = \frac{\int_A x y dA}{y_c A} = \frac{I_{xy}}{y_c A} = \frac{I_{\xi} \eta}{y_c A} + x_c \quad (12.3)$$

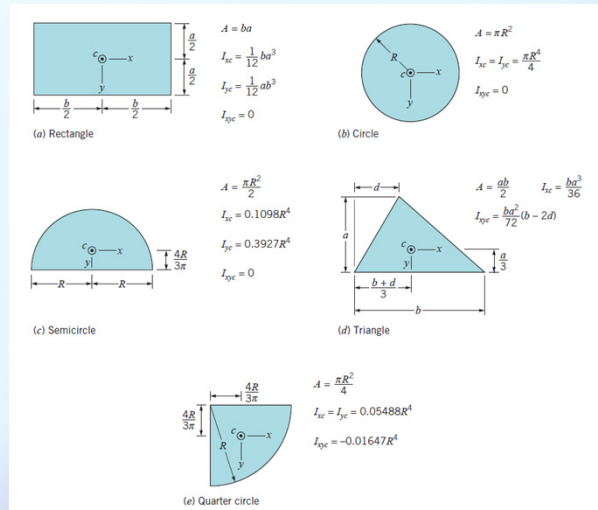
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## Hydrostatic force on a plane surface

### ● Surface moments:



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FIGURE 2.18 Geometric properties of some common shapes.

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## Hydrostatic force on a plane surface

### ● The resultant hydrostatic force and its point (line) of action

- **Corollary 1:** Since  $\frac{I_{\xi}}{y_c A} > 0$ , the point of action of the resultant force is always below the centroid

$$y_R = \frac{I_{\xi}}{y_c A} + y_c$$

- **Corollary 2:** For a symmetric surface with respect to the  $\xi$  or  $\eta$  coordinate,  $I_{\xi\eta} = 0$  and  $x_R = x_c$

$$x_R = \frac{I_{\xi\eta}}{y_c A} + x_c$$

- **Corollary 3:** The total force from the fluid in contact

$$F = F_R + F_s = \gamma h_c A + p_s A = (\gamma h_c + p_s) A = p_c A$$

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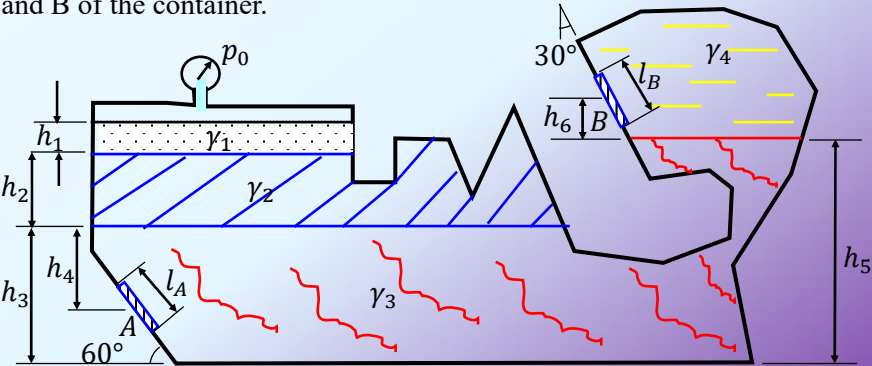
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# Hydrostatic force on a plane surface

## ● Example

Calculate the resultant hydrostatic force and its point of action on the gate A and B of the container.



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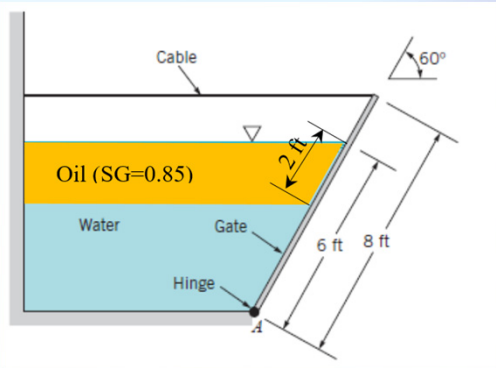
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# Hydrostatic force on a plane surface

## ● Problem

Calculate the cable tension (the gate is rectangular with the width of 4 ft and weight of 500 lb).



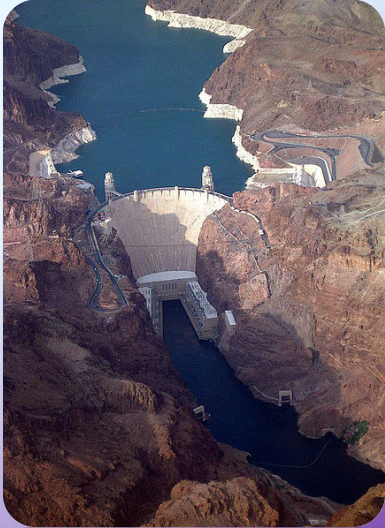
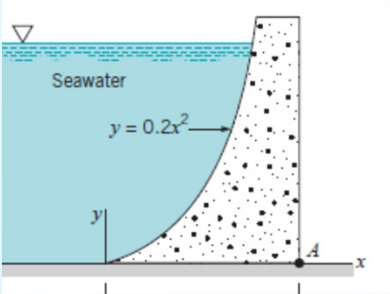
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# Hydrostatic force on a curved surface

- Dams

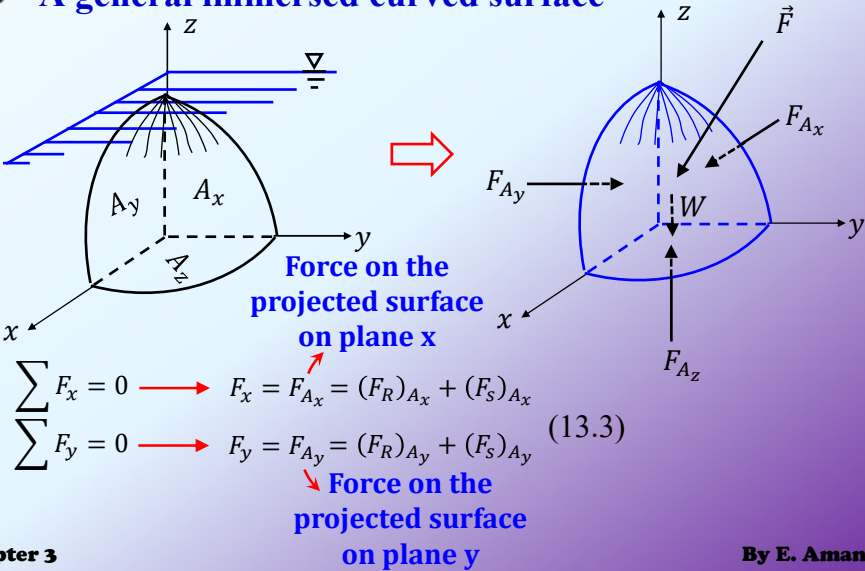


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# Hydrostatic force on a curved surface

- A general immersed curved surface



Force on the projected surface on plane x

$$\sum F_x = 0 \rightarrow F_x = F_{Ax} = (F_R)_{Ax} + (F_S)_{Ax}$$
$$\sum F_y = 0 \rightarrow F_y = F_{Ay} = (F_R)_{Ay} + (F_S)_{Ay} \quad (13.3)$$

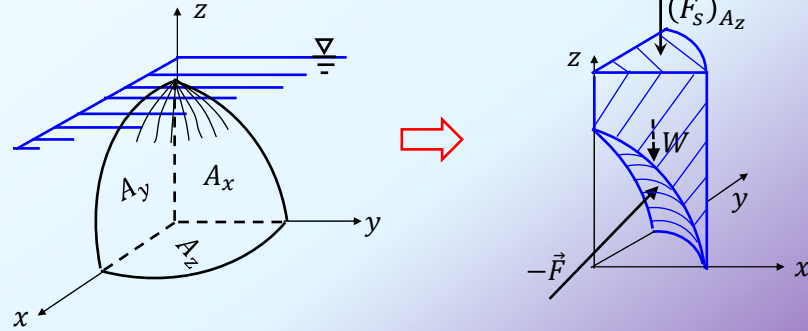
Force on the projected surface on plane y

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## Hydrostatic force on a curved surface

- The 'Column of fluid' method for  $F_z$ :



$$\sum F_z = 0 \rightarrow F_z = (F_R)_{A_z} + (F_s)_{A_z} \quad (14.3)$$

$(F_R)_{A_z} = W$  (the weight of hypothetical fluid column which can be placed above the surface, acted at the column center of gravity)

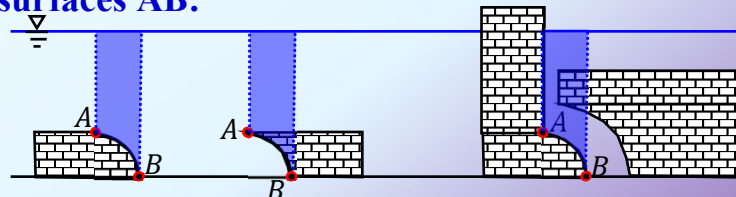
$(F_s)_{A_z} = p_s A_z$  (acted at the center of  $A_z$  surface)

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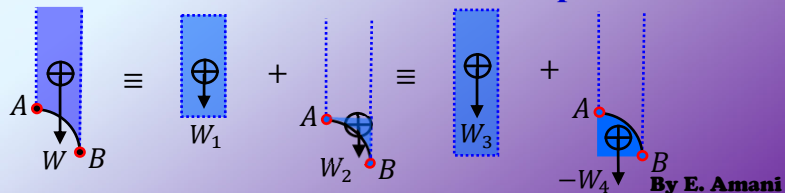
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## Hydrostatic force on a curved surface

- The pressure distribution on a surface only depends on the height of fluid free-surface above the surface
- The hydrostatic force magnitude is the same for the surfaces AB:



- Statics: Force combination and decomposition

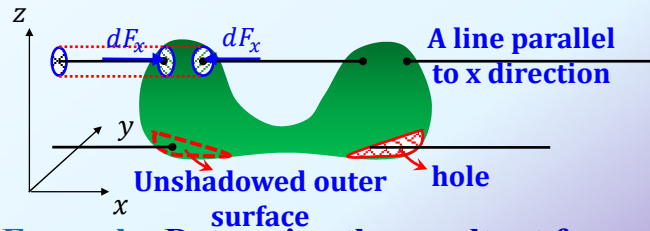


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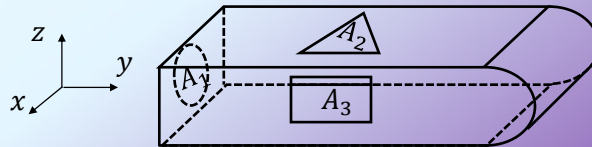
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## Hydrostatic force on a curved surface

- The 'unshadowed surface' method: For  $F_x$  and  $F_y$



- Example:** Determine the resultant force on the body and on its outer surfaces.



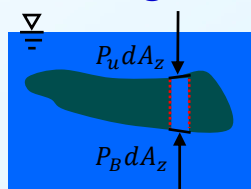
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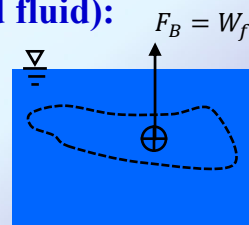
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## Buoyancy and flotation

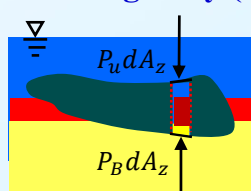
- Submerged body (in a connected fluid):



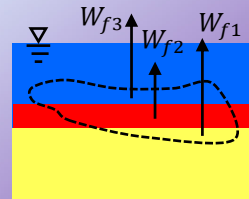
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- Floating body (between multiple fluids in free contact):



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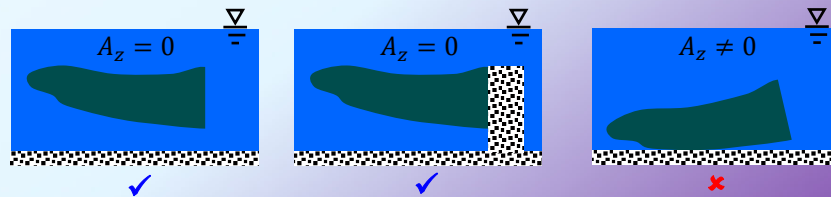
- Conclusion:** The weights of displaced fluids acting upward on their center of gravity

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## Buoyancy and flotation

- **Algorithm: Calculation of the hydrostatic force on a submerged or floating body**
  - For  $F_x$  and  $F_y$ : The ‘unshadowed surface’ method
  - For  $F_z$  :
    1. The Bouyancy force is applicable if  $A_z = 0$  and
      - a) The body is submerged in a single (connected) fluid or,

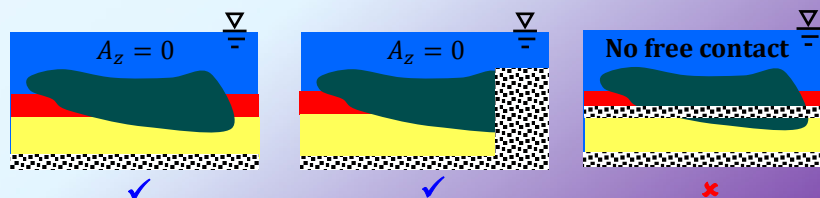


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## Buoyancy and flotation

- **Algorithm: Calculation of the hydrostatic force on a submerged or floating body**
  - For  $F_x$  and  $F_y$ : The ‘unshadowed surface’ method
  - For  $F_z$  :
    1. The Bouyancy force is applicable if  $A_z = 0$  and
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      - b) is floating between multiple fluids in free contact with each other.



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## Buoyancy and flotation

- **Algorithm: Calculation of the hydrostatic force on a submerged or floating body**
  - For  $F_x$  and  $F_y$ : The ‘unshadowed surface’ method
  - For  $F_z$  :
    1. The **Bouyancy** force is applicable if  $A_z = 0$  and
      - a) The body is submerged in a single (connected) fluid or,
      - b) is floating between multiple fluids in free contact with each other.
    2. Otherwise, the decomposition of the body into surfaces in contact with a single fluid and using the ‘**Column of fluid**’ method for each of them

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## Importance of stability analysis

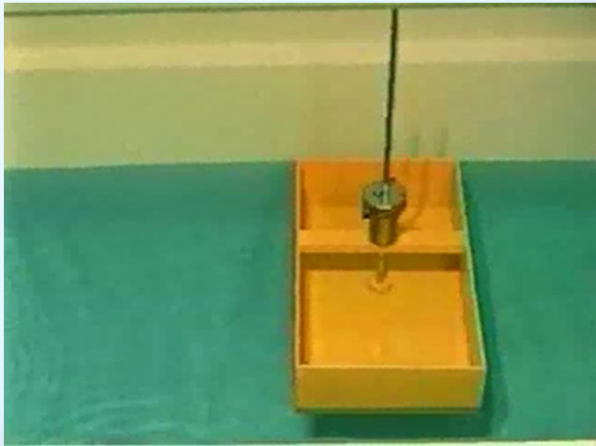
- **Costa Concordia**



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# Stability of a floating body

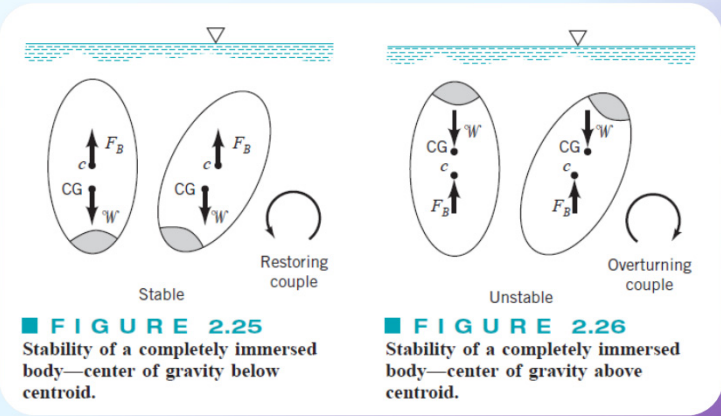


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## Stability analysis

- Submerged body

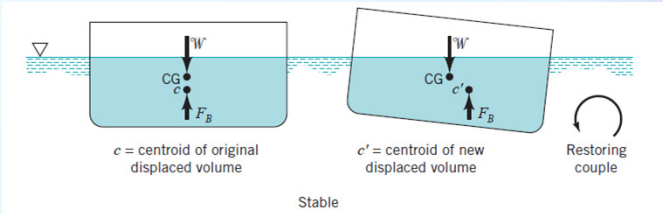


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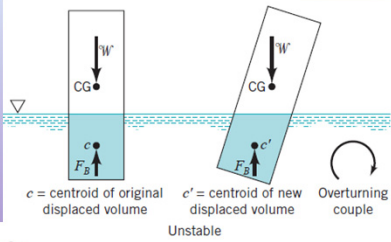
# Stability analysis

- Floating body



Stable  
■ FIGURE 2.27 Stability of a floating body—stable configuration.

- More complicated:  
The center of buoyancy moves



Unstable  
■ FIGURE 2.28 Stability of a floating body—unstable configuration.

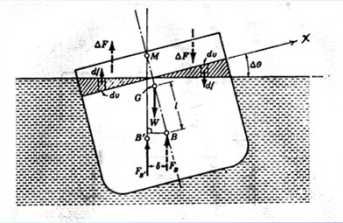
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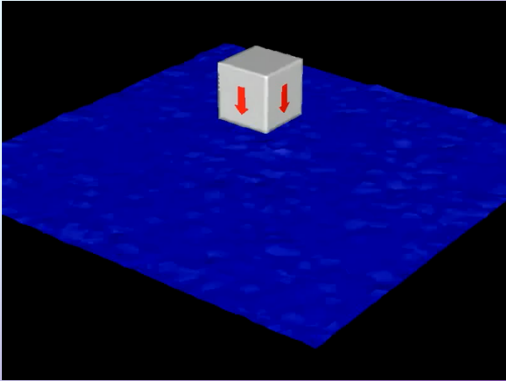
# Stability analysis

- Preliminary design

- ❑ linear stability
- ❑ smooth shapes



- Detailed design (CFD of multiphase flows)



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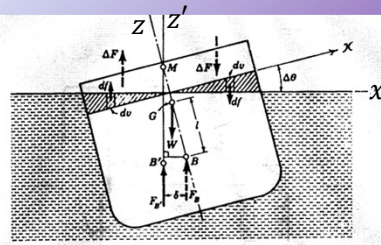
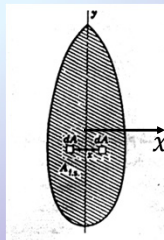
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## ● Steps

- $$\overline{MG} = \overline{MB} - \overline{GB} = \frac{\gamma I_{yy}}{W} - \overline{GB} \quad (18.3)$$

$$\overline{MG} = \begin{cases} > 0 & ; \text{stable} \\ = 0 & ; \text{neutral} \\ < 0 & ; \text{unstable} \end{cases}$$



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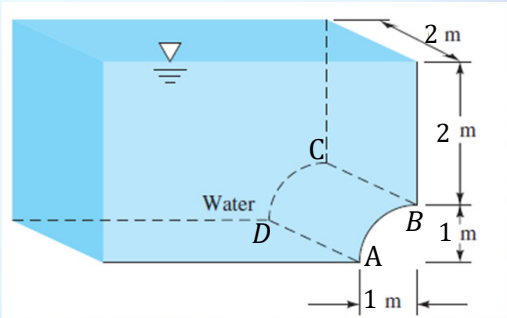
## ● Steps

- $$\overline{MG} = \overline{MB} - \overline{GB} = \frac{\gamma_{yy}^I}{W} - \overline{GB} \quad (18.3) \quad \overline{MG} = \begin{cases} > 0 & ; \text{stable} \\ = 0 & ; \text{neutral} \\ < 0 & ; \text{unstable} \end{cases}$$

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## Sample problems

Calculate the hydrostatic force and its point of action on the gate ABCD, which is a quarter-cylinder of radius 1m and height 2 m.



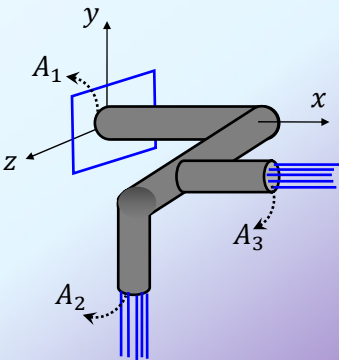
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## Sample problems

Calculate the hydrostatic force from air on the outer surface of the piping system.



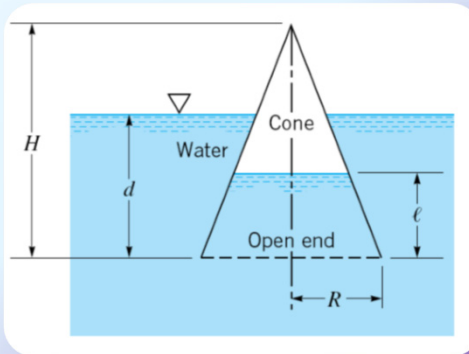
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## Sample problems

An inverted hollow cone is pushed into the water as is shown in below figure. Determine equations which are needed for calculating  $\ell$  and  $d$ . Assume the temperature of the air within the cone remains constant. Express all assumptions you make.



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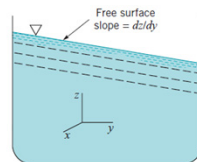
## Fluid with rigid-body motion

- Linear motion
  - Choosing coordinates system:

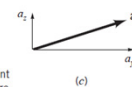
1. Attached to the container
2.  $\vec{g} = -g\hat{k}$
3.  $a_y = 0$  or  $\vec{a} = a_x\hat{i} + a_z\hat{k}$



(a)



(b)



(c)



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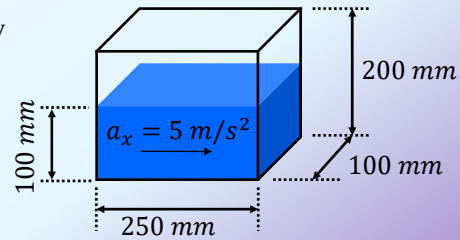
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## Sample problems

A cubic container moves horizontally with an acceleration of  $5 \text{ m/s}^2$ .

- Does the water spill out of the container?
- Determine the force on the left wall of the container from the water.



### Problem solution steps:

- Determining the free-surface location using the conservation of mass and free-surface slope

$$\left(\frac{dz}{dx}\right)_{fs} = -\frac{a_x}{g + a_z} \quad (21.3)$$



- Calculating the pressure distribution by: **Lecture Notes**

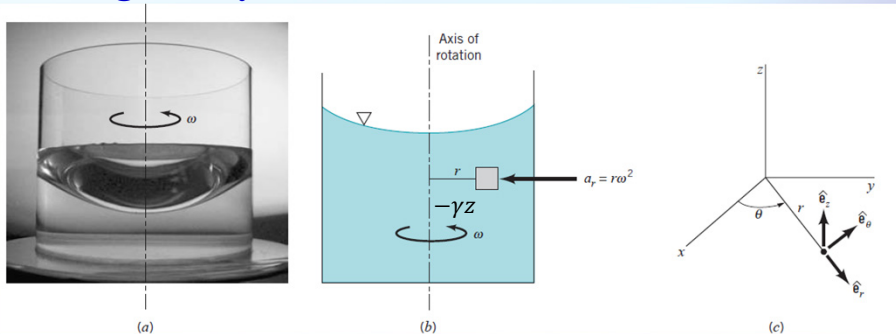
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$$p = p(x, z) = C - \gamma x \frac{a_x}{g} - \gamma z \left(1 + \frac{a_z}{g}\right) \quad (19.3)$$

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## Fluid with rigid-body motion

### Rigid-body rotation about z-axis



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$$p = p(x, r) = C - \gamma z + \frac{\rho \omega^2}{2} r^2 \quad (23.3)$$

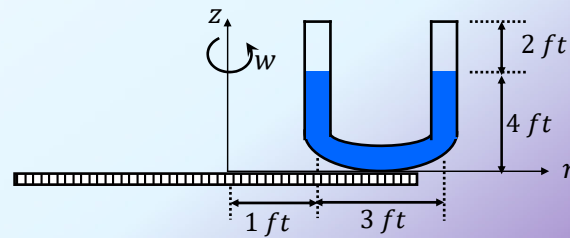
**Free-surface equation:**  $\frac{\rho \omega^2}{2} r^2 - \gamma z = -C \quad (24.3)$

**Chapter 3**

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## Sample problems

The U-tube contains water and is placed on a rotating disk. What is the minimum angular velocity of the disk to spill out water from the U-tube. The tube inner diameter is  $0.1\text{ in.}$



➡ **Lecture Notes**

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**The end of chapter 3**

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