

Problem 5

Consider the bar loaded as shown in figure ??.

Assume $E = 200 \text{ GPa}$ and that the bar is fixed at both ends.

- Construct a 1D linear bar finite element model of the bar. *Use two elements in each section of the bar (4 elements in total). Label all nodes and elements.*
- Write the global system of equations $[K]\{d\} = \{R\}$.
- Apply the boundary conditions to this global system of equations and solve for $\{d\}$.
- Plot the displacements $u(x)$ vs. x for the entire bar.
- What are the reaction forces at the two ends?

(a) Figure 1 shows the 1D linear bar finite element model of the bar given in the problem statement. The model has 4 elements and 5 nodes, with the known tractions, forces, and boundary conditions shown in the figure.

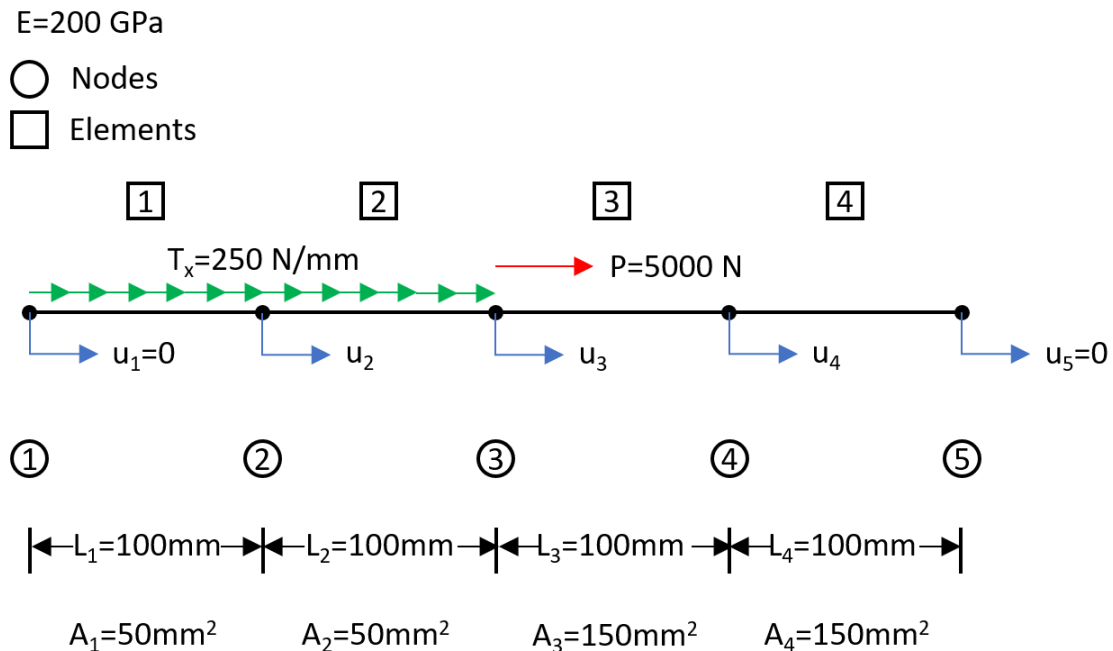


Figure 1: 1D linear bar finite element model of bar.

(b) Let $k_i = \frac{EA_i}{L_i}$. We assemble a local stiffness matrix $[K_i]$ for each element i as shown below:

$$[K_1] = \begin{bmatrix} k_1 & -k_1 \\ -k_1 & k_1 \end{bmatrix}$$

$$[K_2] = \begin{bmatrix} k_2 & -k_2 \\ -k_2 & k_2 \end{bmatrix}$$

$$[K_3] = \begin{bmatrix} k_3 & -k_3 \\ -k_3 & k_3 \end{bmatrix}$$

$$[K_4] = \begin{bmatrix} k_4 & -k_4 \\ -k_4 & k_4 \end{bmatrix}$$

Through superposition we combine the local stiffness matrices into a global stiffness matrix, $[K]$:

$$[K] = \begin{bmatrix} k_1 & -k_1 & 0 & 0 & 0 \\ -k_1 & k_1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 \end{bmatrix} + \begin{bmatrix} 0 & 0 & 0 & 0 & 0 \\ 0 & k_2 & -k_2 & 0 & 0 \\ 0 & -k_2 & k_2 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 \end{bmatrix} +$$

$$\begin{bmatrix} 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & k_3 & -k_3 & 0 \\ 0 & 0 & -k_3 & k_3 & 0 \\ 0 & 0 & 0 & 0 & 0 \end{bmatrix} + \begin{bmatrix} 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & k_4 & -k_4 \\ 0 & 0 & 0 & -k_4 & k_4 \end{bmatrix}$$

$$[K] = \begin{bmatrix} k_1 & -k_1 & 0 & 0 & 0 \\ -k_1 & k_1 + k_2 & -k_2 & 0 & 0 \\ 0 & -k_2 & k_2 + k_3 & -k_3 & 0 \\ 0 & 0 & -k_3 & k_3 + k_4 & -k_4 \\ 0 & 0 & 0 & -k_4 & k_4 \end{bmatrix}$$

Next, we assemble our displacement vector, noting that $d_1 = d_5 = 0$:

$$\{d\} = \begin{Bmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \\ d_5 \end{Bmatrix} = \begin{Bmatrix} 0 \\ d_2 \\ d_3 \\ d_4 \\ 0 \end{Bmatrix}$$

Then, the vector of applied forces:

$$\{r\} = \begin{Bmatrix} F_1 \\ F_2 \\ F_3 \\ F_4 \\ F_5 \end{Bmatrix}$$

F_1 and F_5 are unknown because we know that the displacements at both boundaries are 0. The applied forces at nodes 2-4 are outlined below:

$$F_2 = T_x * L_1$$

$$F_3 = T_x * (L_1 + L_2) + P$$

$$F_4 = P$$

$$\{r\} = \begin{Bmatrix} F_1 \\ T_x * L_1 \\ T_x * (L_1 + L_2) + P \\ P \\ F_5 \end{Bmatrix} = \begin{Bmatrix} F_1 \\ 30000 \\ 55000 \\ 5000 \\ F_5 \end{Bmatrix}$$

The global system of equations $[K]\{d\} = \{r\}$:

$$\begin{bmatrix} k_1 & -k_1 & 0 & 0 & 0 \\ -k_1 & k_1 + k_2 & -k_2 & 0 & 0 \\ 0 & -k_2 & k_2 + k_3 & -k_3 & 0 \\ 0 & 0 & -k_3 & k_3 + k_4 & -k_4 \\ 0 & 0 & 0 & -k_4 & k_4 \end{bmatrix} \begin{Bmatrix} 0 \\ d_2 \\ d_3 \\ d_4 \\ 0 \end{Bmatrix} = \begin{Bmatrix} F_1 \\ 30000 \\ 55000 \\ 5000 \\ F_5 \end{Bmatrix}$$

(c) Substituting in given values and solving in MATLAB yields the following for $\{d\}$:

$$\{d\} = \begin{Bmatrix} d_1 \\ d_2 \\ d_3 \\ d_4 \\ d_5 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0.3313 \\ 0.3625 \\ 0.1896 \\ 0 \end{Bmatrix} \text{ mm}$$

(d) some ldkjfhslkjhslkjhslkjhslkjhslk MORE HERE MORE HERE MORE HERE
dfgdf dfgdf dfgdf

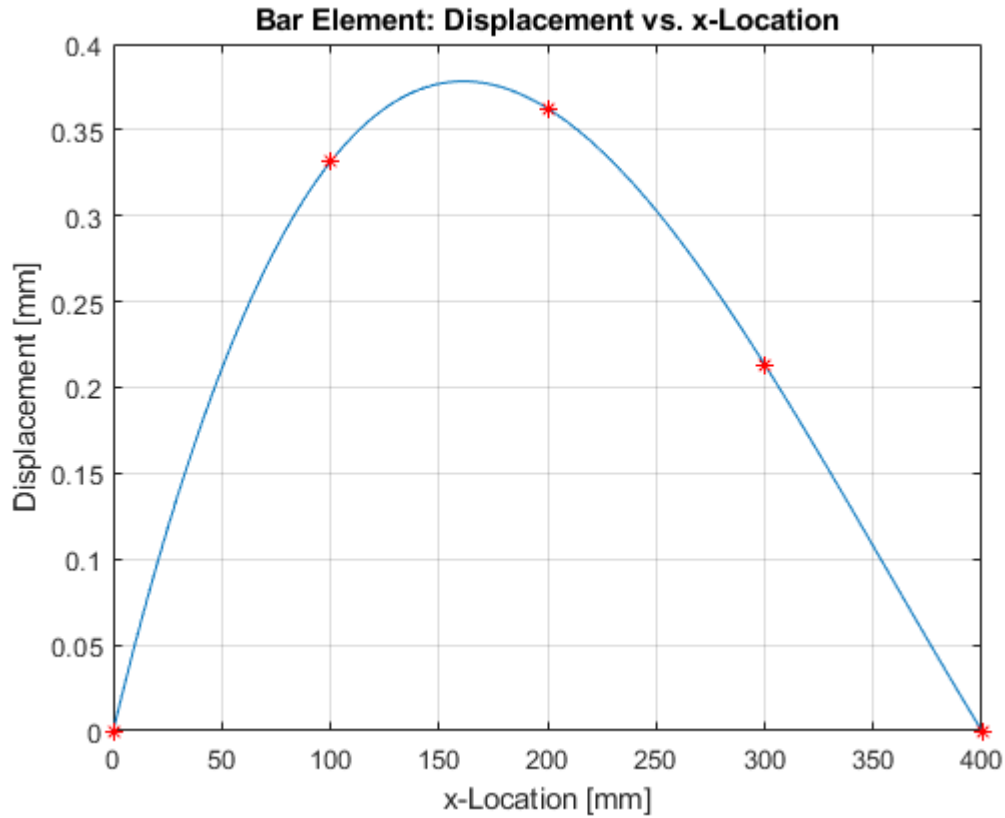


Figure 2: Displacement u vs. x location

(e) Solving the global $[K]\{d\} = \{r\}$ equation in MATLAB yields the following for $\{r\}$:

$$\{r\} = \begin{Bmatrix} F_1 \\ F_2 \\ F_3 \\ F_4 \\ F_5 \end{Bmatrix} = \begin{Bmatrix} -33125 \\ 30000 \\ 55000 \\ 5000 \\ -56875 \end{Bmatrix} \text{ N}$$

The reaction forces at the ends are given by F_1 and F_5 :

$$F_1 = -33125 \text{ N} \quad F_5 = -56875 \text{ N}$$