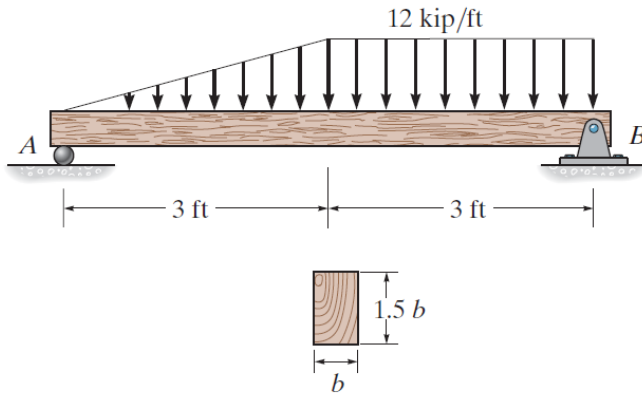


## problem 11-8

**\*11-8.** The simply supported beam is made of timber that has an allowable bending stress of  $\sigma_{\text{allow}} = 1.20$  ksi and an allowable shear stress of  $\tau_{\text{allow}} = 100$  psi. Determine its smallest dimensions to the nearest  $\frac{1}{8}$  in. if it is rectangular and has a height-to-width ratio of 1.5.



**Prob. 11-8**

### beam

```
u = symunit;
x = sym('x');
E = sym('E');

old_assum = assumptions;
clearassum;
args = {'mode' 'factor'};
wf1 = findpoly(1, 'thru', [0 0], [3*u.ft -12*u.kip/u.ft], args{:});
wf2(x) = -12*u.kip/u.ft;

b = beam; %(kip,ft)
b = b.add('reaction', 'force', 'Ra', 0);
b = b.add('reaction', 'force', 'Rb', 6*u.ft);
b = b.add('distributed', 'force', wf1, [0 3]*u.ft);
b = b.add('distributed', 'force', wf2, [3 6]*u.ft, [false true]);
b.L = 6*u.ft;
```

### section properties

```
B = sym('B');
H(B) = 1.5*B;
b.I = B*H^3/12;
```

$$A = B \cdot H;$$

## elastic curve

```
[y(x,E,B) dy(x,E,B) m v w r] = b.elastic_curve(x, 'factor'); %#ok
y
```

$$y(x, E, B) =$$

$$\begin{cases} -\frac{8x(2x^4 - 210x^2 \text{ft}^2 + 5049 \text{ft}^4)}{135B^4E} \frac{\text{kip}}{\text{ft}^2} & \text{if } x \leq 3 \text{ ft} \\ \frac{8(x - 6 \text{ft})(-10x^3 + 70x^2 \text{ft} + 240x \text{ft}^2 + 27 \text{ft}^3)}{45B^4E} \frac{\text{kip}}{\text{ft}} & \text{if } 3 \text{ ft} < x \end{cases}$$

dy

$$dy(x, E, B) =$$

$$\begin{cases} -\frac{8(10x^4 - 630x^2 \text{ft}^2 + 5049 \text{ft}^4)}{135B^4E} \frac{\text{kip}}{\text{ft}^2} & \text{if } x \leq 3 \text{ ft} \\ -\frac{8(40x^3 - 390x^2 \text{ft} + 360x \text{ft}^2 + 1413 \text{ft}^3)}{45B^4E} \frac{\text{kip}}{\text{ft}} & \text{if } 3 \text{ ft} < x \end{cases}$$

m

$$m(x) =$$

$$\begin{cases} \frac{x(63 \text{ft}^2 - 2x^2)}{3} \frac{\text{kip}}{\text{ft}^2} & \text{if } x \leq 3 \text{ ft} \\ -3(x - 6 \text{ft})(2x - 1 \text{ft}) \frac{\text{kip}}{\text{ft}} & \text{if } 3 \text{ ft} < x \end{cases}$$

v

$$v(x) =$$

$$\begin{cases} (21 \text{ft}^2 - 2x^2) \frac{\text{kip}}{\text{ft}^2} & \text{if } x \leq 3 \text{ ft} \\ -3(4x - 13 \text{ft}) \frac{\text{kip}}{\text{ft}} & \text{if } 3 \text{ ft} < x \end{cases}$$

w

$$w(x) =$$

$$\begin{cases} -4x \frac{\text{kip}}{\text{ft}^2} & \text{if } x \leq 3 \text{ ft} \\ -12 \frac{\text{kip}}{\text{ft}} & \text{if } 3 \text{ ft} < x \end{cases}$$

## reactions

```
Ra = r.Ra %#ok
```

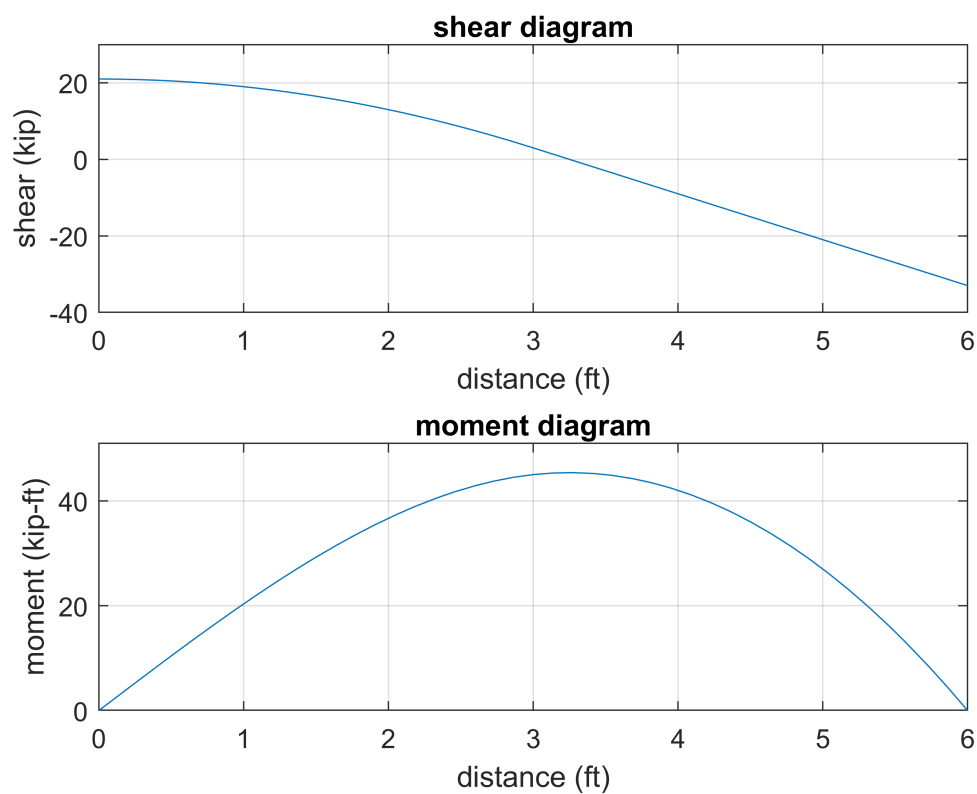
```
Ra = 21 kip
```

```
Rb = r.Rb %#ok
```

```
Rb = 33 kip
```

## shear and moment diagram

```
beam.shear_moment(m, v, [0 6], {'kip' 'ft'});  
subplot(2,1,1);  
axis([0 6 -40 30]);  
subplot(2,1,2);  
axis([0 6 0 51]);
```



## maximum loads

```
assume(0 < x & x < b.L & in(x, 'real'));  
xmax = solve(v == 0, x);  
M_val = m(xmax);
```

```
M_max = vpa(M_val, 4) %#ok
```

```
M_max = 45.38 ft kip
```

```
V_max = v(b.L)
```

```
V_max = -33 kip
```

```
M_max = M_val;
```

## maximum stresses

```
C = H/2;  
sigma_max = rewrite(M_max, u.kip*u.in)*C/b.I
```

```
sigma_max(B) =
```

$$\frac{1452}{B^3} \text{ in kip}$$

```
tau_max = 3*abs(V_max)/(2*A)
```

```
tau_max(B) =
```

$$\frac{33}{B^2} \text{ kip}$$

## minimum beam dimension

```
sigma_allow = 1.20*u.ksi;  
tau_allow = 100*u.psi;  
  
assume(B > 0 & in(B, 'real'));  
clear B_min;  
  
B_min.bend = solve(sigma_max == rewrite(sigma_allow, u.kip/u.in^2));  
B_min.bend = simplify(B_min.bend);  
B_min_bend = vpa(B_min.bend, 4) %#ok
```

```
B_min_bend = 10.66 in
```

```
B_min.shear = solve(tau_max == rewrite(tau_allow, u.kip/u.in^2));  
B_min.shear = simplify(B_min.shear);  
B_min_shear = vpa(B_min.shear, 4) %#ok
```

```
B_min_shear = 18.17 in
```

```
B_min_vals = [B_min.bend B_min.shear];  
loc = sigma_max(B_min_vals) <= sigma_allow & ...
```

```
tau_max(B_min_vals) <= tau_allow;  
B_min.limit = B_min_vals(isAlways(loc));  
B_min_limit = vpa(B_min.limit, 4) %#ok
```

```
B_min_limit = 18.17 in
```

## clean up

```
setassum(old_assum, 'clear');  
clear args old_assum Ra Rb M_val;  
clear B_min_bend B_min_shear B_min_vals loc B_min_limit;
```