

É. Savin

Kinematics

Statics

Stresses

6.1 Atomic
force
microscope

Atomic force microscope

1EL5000–Continuum Mechanics – Tutorial Class #8

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Outline

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Beam kinematics

Reference configuration

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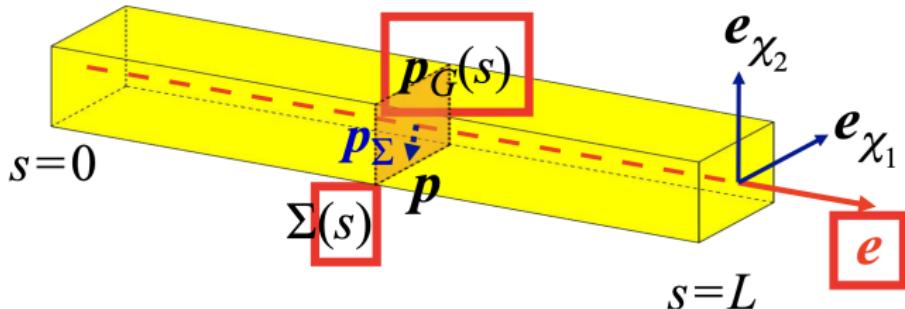
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$$\begin{aligned}\mathbf{p}(s, \chi_1, \chi_2) &= \mathbf{p}_G(s) + \mathbf{p}_\Sigma(\chi_1, \chi_2) \\ &= s\mathbf{e} + \chi_1\mathbf{e}_{\chi_1} + \chi_2\mathbf{e}_{\chi_2}\end{aligned}$$

Beam kinematics

Actual configuration

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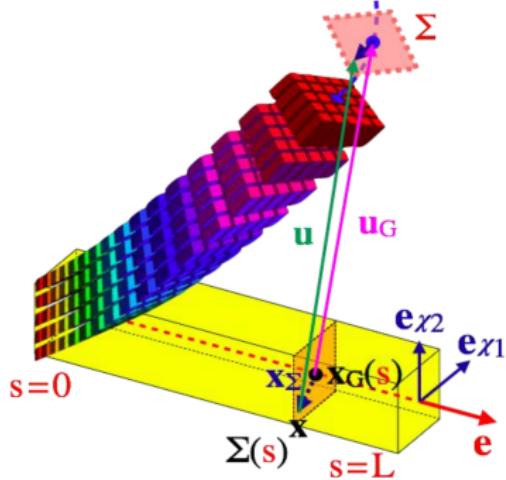
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$$\begin{aligned} \boldsymbol{x} &= \boldsymbol{x}_G(s) + \boldsymbol{x}_\Sigma \\ &= \boldsymbol{x}_G(s) + \boldsymbol{R}(s)\boldsymbol{p}_\Sigma \end{aligned}$$

Beam kinematics

Small perturbations – Timoshenko

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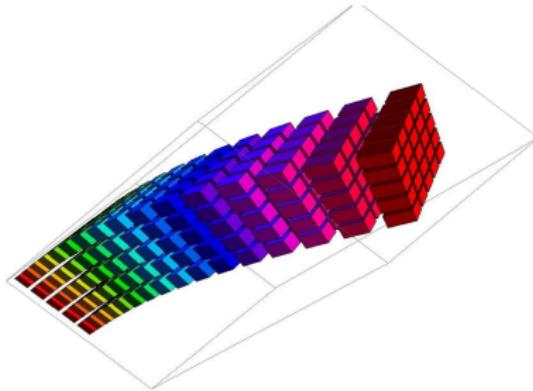
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- Small perturbations $\mathbf{R}(s) = \mathbf{I} + \boldsymbol{\Theta}(s)$, $\boldsymbol{\Theta}(s)^\top = -\boldsymbol{\Theta}(s)$:

$$\begin{aligned}\mathbf{x}_\Sigma &= \mathbf{R}(s)\mathbf{p}_\Sigma \\ &= (\mathbf{I} + \boldsymbol{\Theta}(s))\mathbf{p}_\Sigma.\end{aligned}$$

- Small displacement $\mathbf{x}_\Sigma \simeq \mathbf{p}_\Sigma$:

$$\begin{aligned}\mathbf{u} &= \mathbf{x} - \mathbf{p} \\ &= \mathbf{u}_G(s) + \boldsymbol{\theta}(s) \times \mathbf{x}_\Sigma.\end{aligned}$$

Beam kinematics

Small perturbations – Euler-Bernoulli

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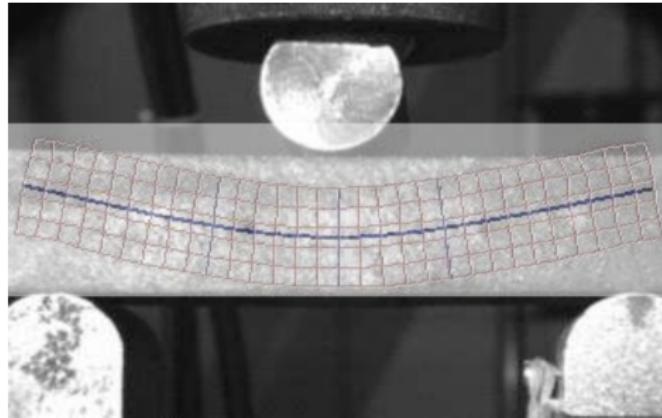
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- Cross-sections remain perpendicular to the neutral line:

$$\mathbf{R}(s)\mathbf{e} = \frac{\mathbf{x}'_G(s)}{\|\mathbf{x}'_G(s)\|}.$$

- Small perturbations $\mathbf{R}(s)\mathbf{e} = (\mathbf{I} + \boldsymbol{\Theta}(s))\mathbf{e} \simeq \mathbf{e} + \mathbf{u}'_{G\Sigma}(s)$:

$$\boldsymbol{\theta}_\Sigma(s) = \mathbf{e} \times \mathbf{u}'_{G\Sigma}(s)$$

Recap: 1.3 Large beam bending

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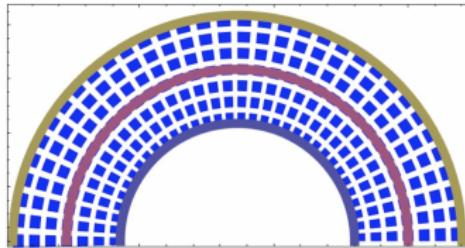
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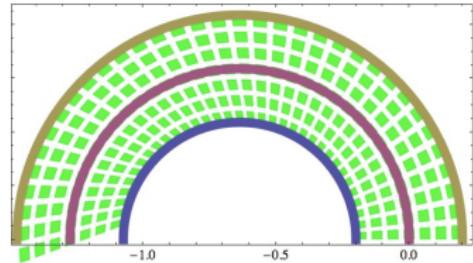
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$$\alpha = 1$$



$$\alpha = 1.1$$

Beam kinematics

Independent unknowns

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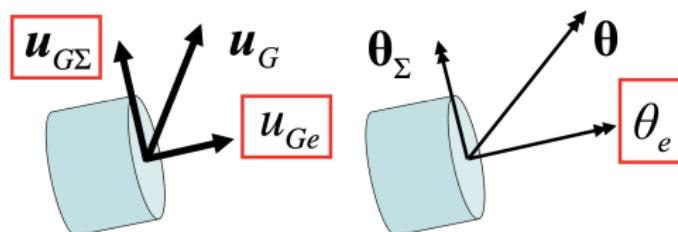
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- Elongation $u_{Ge} = \langle \mathbf{u}_G, \mathbf{e} \rangle$;
- Deflection $\mathbf{u}_{G\Sigma} = \mathbf{u}_G - u_{Ge}\mathbf{e}$;
- Torsion rotation $\theta_e = \langle \boldsymbol{\theta}, \mathbf{e} \rangle$.

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Resultant forces

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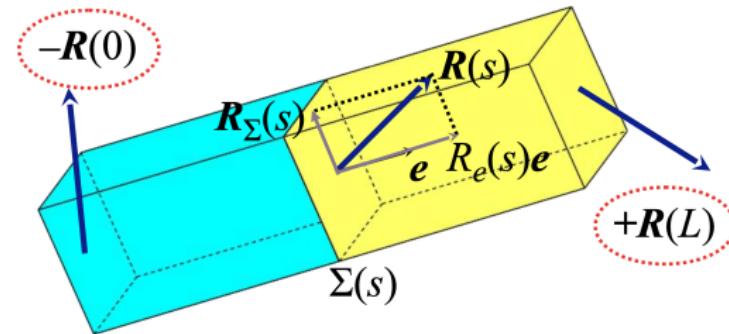
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- Resultant force:

$$\mathbf{R}(s) = \int_{\Sigma} \sigma \mathbf{e} dS;$$

- Normal force $R_e(s) = \langle \mathbf{R}(s), \mathbf{e} \rangle$;
- Shear force $\mathbf{R}_\Sigma(s) = \mathbf{R}(s) - R_e(s)\mathbf{e}$;
- $\mathbf{R}(0)$ and $\mathbf{R}(L)$ given by the boundary conditions.

Beam statics

Resultant moments

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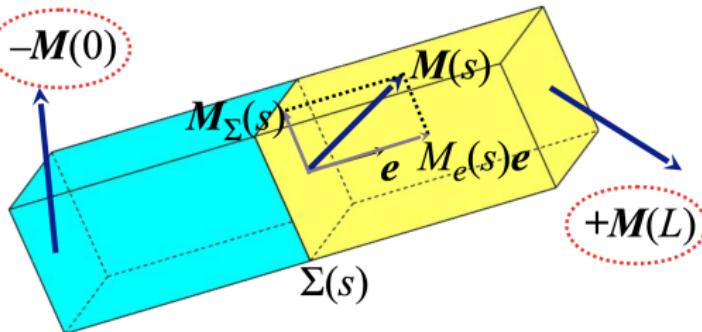
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- Resultant moment:

$$\boldsymbol{M}(s) = \int_{\Sigma} \boldsymbol{x}_{\Sigma} \times \boldsymbol{\sigma} \boldsymbol{e} dS;$$

- Torsion moment $M_e(s) = \langle \boldsymbol{M}(s), \boldsymbol{e} \rangle$;
- Bending moment $\boldsymbol{M}_\Sigma(s) = \boldsymbol{M}(s) - M_e(s)\boldsymbol{e}$;
- $\boldsymbol{M}(0)$ and $\boldsymbol{M}(L)$ given by the boundary conditions.

Beam statics

Local balance of forces

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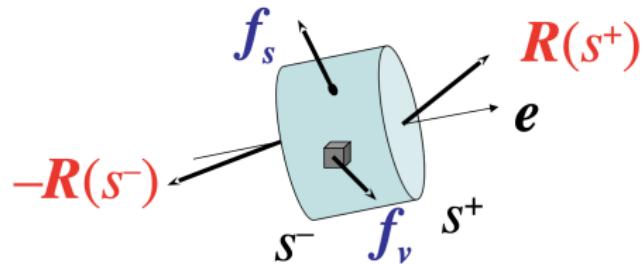
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- Linear external forces:

$$\mathbf{f}_l(s) = \int_{\Sigma} \mathbf{f}_v dS + \int_{\partial\Sigma} \mathbf{f}_s d\zeta ;$$

- Local equilibrium of the cross-section:

$$\mathbf{R}'(s) + \mathbf{f}_l(s) = \mathbf{0} .$$

Beam statics

Local balance of moments

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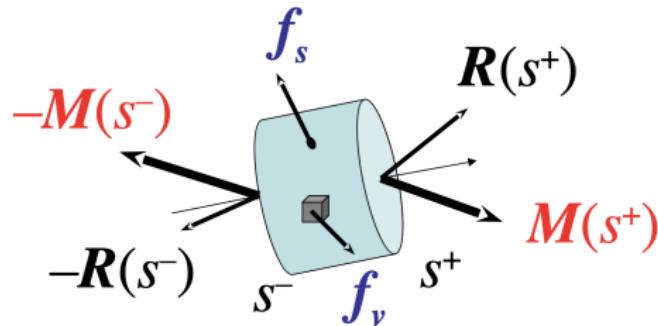
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- Linear external torques:

$$\mathbf{c}_l(s) = \int_{\Sigma} \mathbf{x}_{\Sigma} \times \mathbf{f}_v dS + \int_{\partial\Sigma} \mathbf{x}_{\Sigma} \times \mathbf{f}_s d\zeta ;$$

- Local equilibrium of the cross-section:

$$\mathbf{M}'(s) + \mathbf{e} \times \mathbf{R}(s) + \mathbf{c}_l(s) = \mathbf{0} .$$

Beam statics

Local balance of forces—alternative point of view

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- Local static equilibrium of a continuum medium:

$$\mathbf{Div}\boldsymbol{\sigma} + \mathbf{f}_v = \mathbf{0}.$$

- Then integrate over the cross-section Σ :

$$\begin{aligned}\mathbf{0} &= \int_{\Sigma} \mathbf{Div}\boldsymbol{\sigma} dS + \int_{\Sigma} \mathbf{f}_v dS \\ &= \int_{\Sigma} \frac{\partial \boldsymbol{\sigma}}{\partial s} \mathbf{e} dS + \int_{\Sigma} \mathbf{Div}_{\Sigma} \boldsymbol{\sigma} dS + \int_{\Sigma} \mathbf{f}_v dS \\ &= \frac{\partial}{\partial s} \left(\int_{\Sigma} \boldsymbol{\sigma} \mathbf{e} dS \right) + \int_{\partial\Sigma} \boldsymbol{\sigma} \mathbf{n} d\zeta + \int_{\Sigma} \mathbf{f}_v dS \\ &= \mathbf{R}'(s) + \int_{\partial\Sigma} \mathbf{f}_s d\zeta + \int_{\Sigma} \mathbf{f}_v dS \\ &= \mathbf{R}'(s) + \mathbf{f}_l(s).\end{aligned}$$

Beam statics

Local balance of moments—alternative point of view

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- Then integrate over the cross-section Σ :

$$\begin{aligned} \mathbf{0} &= \int_{\Sigma} \mathbf{x}_{\Sigma} \times \operatorname{Div} \boldsymbol{\sigma} dS + \int_{\Sigma} \mathbf{x}_{\Sigma} \times \mathbf{f}_v dS \\ &= \int_{\Sigma} \mathbf{x}_{\Sigma} \times \frac{\partial \boldsymbol{\sigma}}{\partial s} \mathbf{e} dS + \int_{\Sigma} \mathbf{x}_{\Sigma} \times \frac{\partial \boldsymbol{\sigma} \mathbf{e}_{\alpha}}{\partial x_{\alpha}} dS + \int_{\Sigma} \mathbf{x}_{\Sigma} \times \mathbf{f}_v dS \\ &= \frac{\partial}{\partial s} \left(\int_{\Sigma} \mathbf{x}_{\Sigma} \times \boldsymbol{\sigma} \mathbf{e} dS \right) + \int_{\Sigma} \frac{\partial (\mathbf{x}_{\Sigma} \times \boldsymbol{\sigma} \mathbf{e}_{\alpha})}{\partial x_{\alpha}} dS \\ &\quad - \int_{\Sigma} \mathbf{e}_{\alpha} \times \boldsymbol{\sigma} \mathbf{e}_{\alpha} dS + \int_{\Sigma} \mathbf{x}_{\Sigma} \times \mathbf{f}_v dS \\ &= \mathbf{M}'(s) + \int_{\partial \Sigma} \mathbf{x}_{\Sigma} \times \boldsymbol{\sigma} \mathbf{n} d\zeta + \int_{\Sigma} \mathbf{e} \times \boldsymbol{\sigma} \mathbf{e} dS + \int_{\Sigma} \mathbf{x}_{\Sigma} \times \mathbf{f}_v dS \\ &= \mathbf{M}'(s) + \mathbf{e} \times \mathbf{R}(s) + \mathbf{c}_l(s), \end{aligned}$$

since $\mathbf{e}_{\alpha} \times \boldsymbol{\sigma} \mathbf{e}_{\alpha} + \mathbf{e} \times \boldsymbol{\sigma} \mathbf{e} = \mathbf{0}$ from the symmetry of $\boldsymbol{\sigma}$.

Beam statics

Global balance of forces

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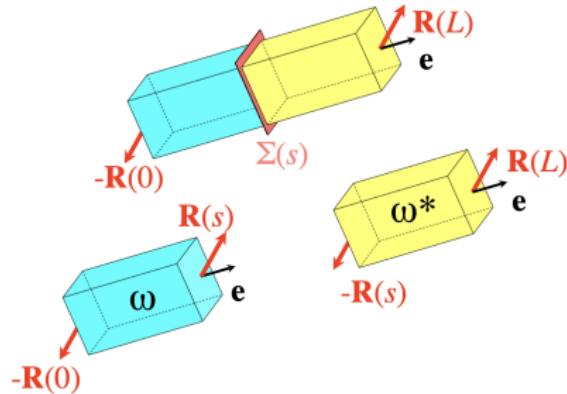
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- Global equilibrium of the **left section** $s \in [0, s]$:

$$\mathbf{R}(s) - \mathbf{R}(0) + \int_0^s \mathbf{f}_l d\zeta = \mathbf{0} ;$$

- Global equilibrium of the **right section** $s \in [s, L]$:

$$\mathbf{R}(L) - \mathbf{R}(s) + \int_s^L \mathbf{f}_l d\zeta = \mathbf{0} .$$

Beam statics

Global balance of moments

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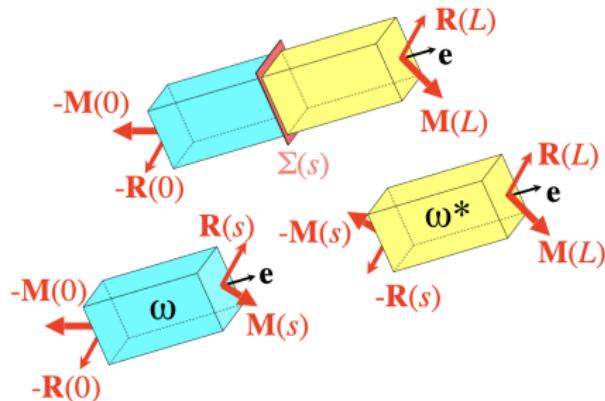
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- Global equilibrium of the **left section** $s \in [0, s]$:

$$\mathbf{M}(s) - \mathbf{M}(0) + s\mathbf{e} \times \mathbf{R}(0) + \int_0^s (\mathbf{c}_l + (\zeta - s)\mathbf{e} \times \mathbf{f}_l) d\zeta = \mathbf{0};$$

- Global equilibrium of the **right section** $s \in [s, L]$:

$$\mathbf{M}(L) + (L-s)\mathbf{e} \times \mathbf{R}(L) - \mathbf{M}(s) + \int_s^L (\mathbf{c}_l + (\zeta - s)\mathbf{e} \times \mathbf{f}_l) d\zeta = \mathbf{0}.$$

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Beam elastic law

Traction vector

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- Recap: basic kinematic assumption (Timoshenko)

$$\boldsymbol{u} = \boldsymbol{u}_G(s) + \boldsymbol{\theta}(s) \times \boldsymbol{x}_\Sigma ;$$

- Linearized strains $\boldsymbol{x}_\Sigma \simeq \boldsymbol{p}_\Sigma$:

$$\boldsymbol{\varepsilon} = (\boldsymbol{u}'_G + \boldsymbol{\theta}' \times \boldsymbol{x}_\Sigma) \otimes_s \boldsymbol{e} - (\boldsymbol{\theta}_\Sigma \times \boldsymbol{e}) \otimes_s \boldsymbol{e} ;$$

- Linear elastic, isotropic behavior:

$$\begin{aligned}\boldsymbol{\sigma} &= \lambda \operatorname{Tr}(\boldsymbol{\varepsilon}) \boldsymbol{I} + 2\mu \boldsymbol{\varepsilon} \\ &= \lambda \varepsilon_{ee} \boldsymbol{I} + 2\mu (\varepsilon_{ee} \boldsymbol{e} + \boldsymbol{\gamma}_\Sigma) \otimes_s \boldsymbol{e} + \boldsymbol{\sigma}_\Sigma ;\end{aligned}$$

- Traction vector:

$$\boldsymbol{\sigma} \boldsymbol{e} = E \left(\underbrace{\boldsymbol{u}'_{Ge} \boldsymbol{e}}_{/\!\!/ \boldsymbol{e}} + \underbrace{\boldsymbol{\theta}'_\Sigma \times \boldsymbol{x}_\Sigma}_{/\!\!/ \boldsymbol{e}, \alpha \boldsymbol{x}_\Sigma} \right) + \mu \left(\underbrace{\boldsymbol{u}'_{G\Sigma} - \boldsymbol{\theta}_\Sigma \times \boldsymbol{e}}_{\perp \boldsymbol{e}} + \underbrace{\boldsymbol{\theta}'_e \boldsymbol{e} \times \boldsymbol{x}_\Sigma}_{\perp \boldsymbol{e}, \alpha \boldsymbol{x}_\Sigma} \right).$$

Beam elastic law

Resultant force with Timoshenko's kinematical assumption

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- Recap: resultant force

$$\mathbf{R} = \int_{\Sigma} \sigma e \, dS ;$$

- Assuming $\int_{\Sigma} \mathbf{x}_{\Sigma} dS = \mathbf{0}$, $S = \int_{\Sigma} dS$:

$$\begin{aligned}\mathbf{R} &= \int_{\Sigma} E(u'_{Ge} \mathbf{e} + \underline{\theta'_{\Sigma} \times \mathbf{x}_{\Sigma}}) dS + \int_{\Sigma} \mu(u'_{G\Sigma} - \theta_{\Sigma} \times \mathbf{e} + \underline{\theta'_{e} \times \mathbf{x}_{\Sigma}}) dS \\ &= ESu'_{Ge} \mathbf{e} + \mathbf{R}_{\Sigma} ;\end{aligned}$$

- Shear force with Timoshenko's assumption:

$$\mathbf{R}_{\Sigma} = \mu S (\mathbf{u}'_{G\Sigma} - \theta_{\Sigma} \times \mathbf{e}) .$$

Beam elastic law

Resultant force with Euler-Bernoulli's kinematical assumption

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■ Recap: resultant force

$$\mathbf{R} = \int_{\Sigma} \sigma e dS ;$$

■ Assuming $\int_{\Sigma} x_{\Sigma} dS = \mathbf{0}$, $S = \int_{\Sigma} dS$:

$$\begin{aligned}\mathbf{R} &= \int_{\Sigma} E(u'_{Ge} \mathbf{e} + \underline{\theta'_{\Sigma} \times \mathbf{x}_{\Sigma}}) dS + \int_{\Sigma} \mu (\underbrace{\mathbf{u}'_{G\Sigma} - \theta_{\Sigma} \times \mathbf{e}}_{= \mathbf{0} \text{ by Euler-Bernoulli}} + \underline{\theta'_{\mathbf{e}} \mathbf{e} \times \mathbf{x}_{\Sigma}}) dS \\ &= ES u'_{Ge} \mathbf{e} + \mathbf{R}_{\Sigma} ;\end{aligned}$$

■ Shear force with Euler-Bernoulli's assumption:

$$\begin{aligned}\mathbf{M}' + \mathbf{e} \times \mathbf{R} + \mathbf{c}_l &= \mathbf{0} \\ \Rightarrow \quad \mathbf{R}_{\Sigma} &= \mathbf{e} \times (\mathbf{M}'_{\Sigma} + \mathbf{c}_l) .\end{aligned}$$

Beam elastic law

Resultant moment

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- Recap: resultant moment

$$\boldsymbol{M} = \int_{\Sigma} \boldsymbol{x}_{\Sigma} \times \boldsymbol{\sigma} \boldsymbol{e} dS ;$$

- Assuming $\int_{\Sigma} \boldsymbol{x}_{\Sigma} dS = \mathbf{0}$:

$$\begin{aligned}\boldsymbol{M} &= \int_{\Sigma} E(\underline{\boldsymbol{x}_{\Sigma} \times \boldsymbol{u}'_{Ge} \boldsymbol{e}} + \boldsymbol{x}_{\Sigma} \times (\boldsymbol{\theta}'_{\Sigma} \times \boldsymbol{x}_{\Sigma})) dS \\ &\quad + \int_{\Sigma} \mu(\underline{\boldsymbol{x}_{\Sigma} \times (\boldsymbol{u}'_{GS} - \boldsymbol{\theta}'_{\Sigma} \times \boldsymbol{e})} + \boldsymbol{x}_{\Sigma} \times (\boldsymbol{\theta}'_e \boldsymbol{e} \times \boldsymbol{x}_{\Sigma})) dS \\ &= E \mathbb{J}(\boldsymbol{\theta}'_{\Sigma}) + \mu \mathbb{J}(\boldsymbol{\theta}'_e \boldsymbol{e});\end{aligned}$$

- (Symmetric) inertia tensor:

$$\mathbb{J} = \int_{\Sigma} (\|\boldsymbol{x}_{\Sigma}\|^2 \boldsymbol{I} - \boldsymbol{x}_{\Sigma} \otimes \boldsymbol{x}_{\Sigma}) dS ;$$

- Bending moment with Timoshenko's assumption:

$$\boldsymbol{M}_{\Sigma} = E \mathbb{J}(\boldsymbol{\theta}'_{\Sigma}) .$$

Beam elastic law

Resultant moment

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- Recap: resultant moment

$$\boldsymbol{M} = \int_{\Sigma} \boldsymbol{x}_{\Sigma} \times \boldsymbol{\sigma} \boldsymbol{e} dS ;$$

- Assuming $\int_{\Sigma} \boldsymbol{x}_{\Sigma} dS = \mathbf{0}$:

$$\begin{aligned}\boldsymbol{M} &= \int_{\Sigma} E(\underline{\boldsymbol{x}_{\Sigma} \times \boldsymbol{u}'_{Ge} \boldsymbol{e}} + \boldsymbol{x}_{\Sigma} \times (\boldsymbol{\theta}'_{\Sigma} \times \boldsymbol{x}_{\Sigma})) dS \\ &\quad + \int_{\Sigma} \mu(\underline{\boldsymbol{x}_{\Sigma} \times (\boldsymbol{u}'_{GS} - \boldsymbol{\theta}'_{\Sigma} \times \boldsymbol{e})} + \boldsymbol{x}_{\Sigma} \times (\boldsymbol{\theta}'_e \boldsymbol{e} \times \boldsymbol{x}_{\Sigma})) dS \\ &= E \mathbb{J}(\boldsymbol{\theta}'_{\Sigma}) + \mu \mathbb{J}(\boldsymbol{\theta}'_e \boldsymbol{e});\end{aligned}$$

- (Symmetric) inertia tensor:

$$\mathbb{J} = \int_{\Sigma} (\|\boldsymbol{x}_{\Sigma}\|^2 \boldsymbol{I} - \boldsymbol{x}_{\Sigma} \otimes \boldsymbol{x}_{\Sigma}) dS ;$$

- Bending moment with Euler-Bernoulli's assumption:

$$\boldsymbol{M}_{\Sigma} = E \mathbb{J}(\boldsymbol{e} \times \underline{\boldsymbol{u}''_{GS}}).$$

Summary

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Constitutive equations:

$$\begin{aligned}\mathbf{R} &= ESu'_{Ge} \mathbf{e} + \mathbf{R}_\Sigma \\ \mathbf{R}_\Sigma &= \mathbf{e} \times (\mathbf{M}'_\Sigma + \mathbf{c}_l)\end{aligned}$$

$$\begin{aligned}\mathbf{M}_\Sigma &= E\mathbb{J}(\mathbf{e} \times \mathbf{u}''_{G\Sigma}) \\ M_e &= \mu J_e \theta'_e\end{aligned}$$

$$\begin{aligned}J_e &= \langle \mathbb{J}\mathbf{e}, \mathbf{e} \rangle \\ &= \int_{\Sigma} \|\mathbf{x}_\Sigma\|^2 dS\end{aligned}$$

Static equilibrium:

$$\mathbf{R}' + \mathbf{f}_l = \mathbf{0}$$

$$\begin{aligned}\mathbf{M}''_\Sigma - \mathbf{e} \times \mathbf{f}_l + \mathbf{c}'_{l\Sigma} &= \mathbf{0} \\ M'_e + c_{le} &= 0\end{aligned}$$

Differential equations

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■ Elongation:

$$ESu''_{Ge}(s) + \langle \mathbf{f}_l(s), \mathbf{e} \rangle = 0$$

with either kinematical $u_{Ge}(0), u_{Ge}(L)$ or mechanical $R_e(0), R_e(L)$ boundary conditions.

■ Torsion:

$$\mu J_e \theta''_e(s) + \langle \mathbf{c}_l(s), \mathbf{e} \rangle = 0$$

with either kinematical $\theta_e(0), \theta_e(L)$ or mechanical $M_e(0), M_e(L)$ boundary conditions.

■ Bending:

$$E\mathbb{J}(\mathbf{e} \times \mathbf{u}_{G\Sigma}^{(IV)}(s)) - \mathbf{e} \times \mathbf{f}_l(s) + \mathbf{c}'_{l\Sigma}(s) = \mathbf{0}$$

with either kinematical $\mathbf{u}_{G\Sigma}(0), \mathbf{u}_{G\Sigma}(L), \mathbf{u}'_{G\Sigma}(0), \mathbf{u}'_{G\Sigma}(L)$ or mechanical $\mathbf{M}_\Sigma(0), \mathbf{M}_\Sigma(L), \mathbf{R}_\Sigma(0), \mathbf{R}_\Sigma(L)$ boundary conditions.

Back to stresses...

■ Recap: traction vector

$$\begin{aligned}\sigma \mathbf{e} &= E(\underbrace{u'_{Ge} \mathbf{e}}_{/\!/\mathbf{e}} + \underbrace{\boldsymbol{\theta}'_\Sigma \times \mathbf{x}_\Sigma}_{/\!/\mathbf{e}, \propto \mathbf{x}_\Sigma}) + \mu(\underbrace{\mathbf{u}'_{G\Sigma} - \boldsymbol{\theta}_\Sigma \times \mathbf{e}}_{\perp \mathbf{e}} + \underbrace{\boldsymbol{\theta}'_e \mathbf{e} \times \mathbf{x}_\Sigma}_{\perp \mathbf{e}, \propto \mathbf{x}_\Sigma}) \\ &= \sigma_{ee} \mathbf{e} + \boldsymbol{\tau}_\Sigma ;\end{aligned}$$

■ Normal stress:

$$\begin{aligned}\sigma_{ee} &= E(u'_{Ge} + \langle \boldsymbol{\theta}'_\Sigma \times \mathbf{x}_\Sigma, \mathbf{e} \rangle) \\ &= \frac{R_e}{S} + \langle \mathbf{x}_\Sigma, \mathbf{e} \times \mathbb{J}^{-1}(\mathbf{M}_\Sigma) \rangle ;\end{aligned}$$

■ Shear stress:

$$\begin{aligned}\boldsymbol{\tau}_\Sigma &= \mu(\mathbf{u}'_{G\Sigma} - \boldsymbol{\theta}_\Sigma \times \mathbf{e}) + \mu \boldsymbol{\theta}'_e (\mathbf{e} \times \mathbf{x}_\Sigma) \\ &= \frac{\mathbf{R}_\Sigma}{S} + \frac{M_e}{J_e} (\mathbf{e} \times \mathbf{x}_\Sigma) .\end{aligned}$$

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Atomic force microscope

Setup

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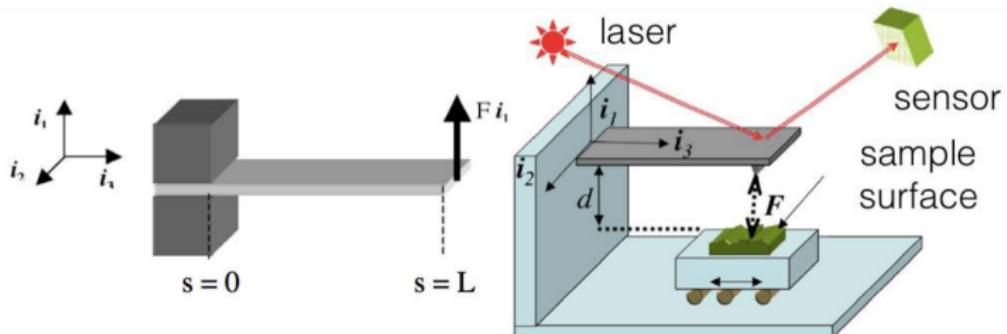
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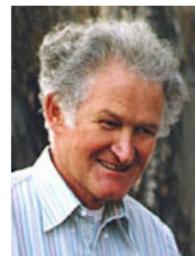
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© G. Puel



Gerd Binnig [1947–]



Calvin Quate [1923–2019]



Christoph Gerber [1942–]

Atomic force microscope

Setup

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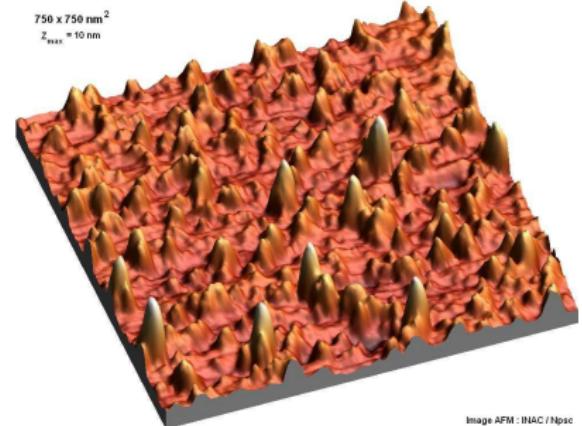
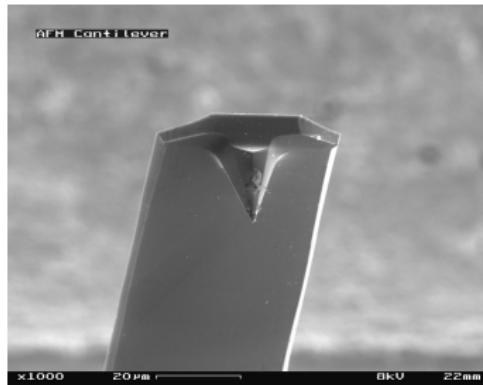
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- <http://toutestquantique.fr/afm/>
- <https://youtu.be/EUB0B5EUJK4>

Atomic force microscope

Solution

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Question #1: Resultant force \mathbf{R} ?

- Local balance of forces on the cross-section Σ at $s \in]0, L[$:

$$\mathbf{R}' + \mathbf{f}_l = \mathbf{0},$$

where $\mathbf{f}_l = \mathbf{0}$ because "the own weight of the beam can be neglected."

Atomic force microscope

Solution

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Question #1: Resultant force \mathbf{R} ?

- Local balance of forces on the cross-section Σ at $s \in]0, L[$:

$$\mathbf{R}' + \mathbf{f}_l = \mathbf{0},$$

where $\mathbf{f}_l = \mathbf{0}$ because "the own weight of the beam can be neglected."

- Therefore:

$$\mathbf{R}(s) = \mathbf{C}^{\text{st}} = \mathbf{R}(L) = \mathbf{R}_L = F(d)\mathbf{i}_1.$$

- No normal force, only a shear force.

Atomic force microscope

Solution

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Question #1: Action of the fixed support on the beam?

- Resultant force for the action of the fixed support at $s = 0$:

$$\begin{aligned}\mathbf{F}^{\text{support} \rightarrow \text{beam}} &= \int_{\Sigma(0)} \boldsymbol{\sigma}(-\mathbf{i}_3) dS \\ &\stackrel{\text{def}}{=} -\mathbf{R}(0)\end{aligned}$$

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- Therefore:

$$\mathbf{F}^{\text{support} \rightarrow \text{beam}} = -F(d)\mathbf{i}_1.$$

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Question #2: Resultant moment \mathbf{M} ?

- Local balance of moments on the cross-section Σ at $s \in]0, L[$:

$$\mathbf{M}' + \mathbf{e} \times \mathbf{R} + \mathbf{c}_l = \mathbf{0},$$

where $\mathbf{c}_l = \mathbf{0}$ and $\mathbf{e} = \mathbf{i}_3$.

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Question #2: Resultant moment \mathbf{M} ?

- Local balance of moments on the cross-section Σ at $s \in]0, L[$:

$$\mathbf{M}' + \mathbf{i}_3 \times \mathbf{R} = \mathbf{0};$$

- Therefore:

$$\mathbf{M}'(s) = -\mathbf{i}_3 \times F(d)\mathbf{i}_1 = -F(d)\mathbf{i}_2;$$

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Question #2: Resultant moment \mathbf{M} ?

- Local balance of moments on the cross-section Σ at $s \in]0, L[$:

$$\mathbf{M}' + \mathbf{i}_3 \times \mathbf{R} = \mathbf{0};$$

- Therefore:

$$\mathbf{M}'(s) = -\mathbf{i}_3 \times F(d)\mathbf{i}_1 = -F(d)\mathbf{i}_2;$$

- Consequently:

$$\begin{aligned}\mathbf{M}(s) &= -F(d)(s - L)\mathbf{i}_2 + \mathbf{M}(L) \\ &= F(d)(L - s)\mathbf{i}_2\end{aligned}$$

because $\mathbf{M}(L) = \mathbf{M}_L = \mathbf{0}$ ("an action [...] which is assumed to consist of a moment [...] equal to zero...").

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Question #2: Action of the fixed support on the beam?

- Resultant moment for the action of the fixed support at $s = 0$:

$$\begin{aligned} \mathbf{C}^{\text{support} \rightarrow \text{beam}} &= \int_{\Sigma(0)} \mathbf{x}_\Sigma \times \boldsymbol{\sigma}(-\mathbf{i}_3) dS \\ &\stackrel{\text{def}}{=} -\mathbf{M}(0) \end{aligned}$$

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- Therefore:

$$\mathbf{C}^{\text{support} \rightarrow \text{beam}} = -F(d)L\mathbf{i}_2.$$

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Question #3: Normal stress σ_{ee} ?

■ Normal stress:

$$\begin{aligned}\sigma_{ee} &= \frac{R_e}{S} + \langle \mathbf{x}_\Sigma, \mathbf{e} \times \mathbb{J}^{-1}(\mathbf{M}_\Sigma) \rangle \\ &= \left\langle \mathbf{x}_\Sigma, \mathbf{i}_3 \times \frac{F(d)(L-s)}{I_2} \mathbf{i}_2 \right\rangle \\ &= -\frac{F(d)}{I_2} (L-s) \chi_1;\end{aligned}$$

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- The inertia tensor of $\Sigma =]-\frac{h}{2}, \frac{h}{2}[\times]-\frac{b}{2}, \frac{b}{2}[$ is:

$$\begin{aligned}\mathbb{J} &= \int_\Sigma (\|\mathbf{x}_\Sigma\|^2 - \mathbf{x}_\Sigma \otimes \mathbf{x}_\Sigma) dS \\ &= I_1 \mathbf{i}_1 \otimes \mathbf{i}_1 + I_2 \mathbf{i}_2 \otimes \mathbf{i}_2 + (I_1 + I_2) \mathbf{i}_3 \otimes \mathbf{i}_3 \\ &= \frac{hb^3}{12} \mathbf{i}_1 \otimes \mathbf{i}_1 + \frac{bh^3}{12} \mathbf{i}_2 \otimes \mathbf{i}_2 + \frac{bh}{12} (b^2 + h^2) \mathbf{i}_3 \otimes \mathbf{i}_3.\end{aligned}$$

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- Its maximum is reached at $s = 0$ and $\chi_1 = -\frac{h}{2}$;

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Question #3: Normal stress σ_{ee} ?

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$$\begin{aligned}\sigma_{ee} &= \frac{R_e}{S} + \langle \mathbf{x}_\Sigma, \mathbf{e} \times \mathbb{J}^{-1}(\mathbf{M}_\Sigma) \rangle \\ &= \left\langle \mathbf{x}_\Sigma, \mathbf{i}_3 \times \frac{F(d)(L-s)}{I_2} \mathbf{i}_2 \right\rangle \\ &= -\frac{F(d)}{I_2}(L-s)\chi_1;\end{aligned}$$

- Its maximum is reached at $s = 0$ and $\chi_1 = -\frac{h}{2}$;
- The maximum force F_{\max} such that $\sigma_{ee} < \sigma_e$ is:

$$F_{\max} < \frac{2I_2}{hL}\sigma_e = \frac{bh^2}{6L}\sigma_e = 10.7 \mu\text{N}.$$

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Question #4: Transverse displacement $\mathbf{u}_{G\Sigma}$?

- Constitutive equation for the transverse displacement with Euler-Bernoulli's assumption:

$$\begin{aligned}\mathbf{M}_\Sigma(s) &= E\mathbb{J}(\mathbf{e} \times \mathbf{u}_{G\Sigma}''(s)) \\ \Rightarrow \quad \mathbf{i}_3 \times \mathbf{u}_{G\Sigma}''(s) &= \frac{F(d)(L-s)}{EI_2} \mathbf{i}_2 \\ \mathbf{u}_{G\Sigma}''(s) &= \frac{F(d)(L-s)}{EI_2} \mathbf{i}_2 \times \mathbf{i}_3;\end{aligned}$$

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Question #4: Transverse displacement $\mathbf{u}_{G\Sigma}$?

- Constitutive equation for the transverse displacement with Euler-Bernoulli's assumption:

$$\mathbf{u}_{G\Sigma}''(s) = \frac{F(d)(L-s)}{EI_2} \mathbf{i}_1;$$

- Consequently:

$$\begin{aligned}\mathbf{u}'_{G\Sigma}(s) &= \frac{F(d)}{EI_2} \left(L - \frac{s}{2}\right) s \mathbf{i}_1 + \mathbf{u}'_{G\Sigma}(0) \\ &= \frac{F(d)}{EI_2} \left(L - \frac{s}{2}\right) s \mathbf{i}_1\end{aligned}$$

since $\boldsymbol{\theta}_{\Sigma}(0) = \mathbf{i}_3 \times \mathbf{u}'_{G\Sigma}(0) = \mathbf{0}$ (the beam "is clamped at $s = 0$ on a fixed, perfectly rigid support").

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$$\mathbf{u}_{G\Sigma}''(s) = \frac{F(d)(L-s)}{EI_2} \mathbf{i}_1;$$

- Consequently:

$$\begin{aligned}\mathbf{u}_{G\Sigma}(s) &= \frac{F(d)}{2EI_2} \left(L - \frac{s}{3}\right) s^2 \mathbf{i}_1 + \mathbf{u}_{G\Sigma}(0) \\ &= \frac{F(d)}{2EI_2} \left(L - \frac{s}{3}\right) s^2 \mathbf{i}_1\end{aligned}$$

since $\mathbf{u}_{G\Sigma}(0) = \mathbf{0}$ (the beam "is clamped at $s = 0$ on a fixed, perfectly rigid support").

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Question #5: Bending stiffness k ?

■ Bending stiffness k :

$$\begin{aligned} k &= \frac{|F(d)|}{\|\boldsymbol{u}_{G\Sigma}(L)\|} \\ &= \frac{3EI_2}{L^3}. \end{aligned}$$

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Question #6: Tip end rotation $\theta_\Sigma(L)$?

- From Euler-Bernoulli's assumption:

$$\begin{aligned}\theta_\Sigma(s) &= \mathbf{i}_3 \times \mathbf{u}'_{G\Sigma}(s) \\ &= \frac{F(d)}{EI_2} \left(L - \frac{s}{2} \right) s \mathbf{i}_3 \times \mathbf{i}_1 ;\end{aligned}$$

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Question #6: Tip end rotation $\theta_\Sigma(L)$?

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$$\begin{aligned}\theta_\Sigma(s) &= \mathbf{i}_3 \times \mathbf{u}'_{G\Sigma}(s) \\ &= \frac{F(d)}{EI_2} \left(L - \frac{s}{2} \right) s \mathbf{i}_2;\end{aligned}$$

- Consequently:

$$\theta_\Sigma(L) = \frac{F(d)L^2}{2EI_2} \mathbf{i}_2;$$

- The largest rotation is at the tip end, that is why it is measured for better precision.

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Question #7: Tip end rotation with distributed tip load?

- Local balance of actions at any $s \in]0, L[$:

$$\mathbf{R}'(s) + \mathbf{f}_l(s) = \mathbf{0},$$

$$\mathbf{M}'(s) + \mathbf{i}_3 \times \mathbf{R}(s) + \mathbf{c}_l(s) = \mathbf{0},$$

where $\mathbf{c}_l(s) = \mathbf{0}$ and:

$$\mathbf{f}_l(s) = \begin{cases} \frac{F(d)}{a} \mathbf{i}_1 & \text{if } s \in]L-a, L[, \\ \mathbf{0} & \text{if } s \in]0, L-a[. \end{cases}$$

- Strategy: (i) recompute \mathbf{R} ; (ii) recompute \mathbf{M} ; (iii) deduce θ_Σ .

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Question #7: Tip end rotation with distributed tip load?

- (i) Recompute \mathbf{R} :

$$\mathbf{R}(s) = \begin{cases} \frac{F(d)}{a}(L-s)\mathbf{i}_1 & \text{if } s \in]L-a, L[, \\ F(d)\mathbf{i}_1 & \text{if } s \in]0, L-a[, \end{cases}$$

since $\mathbf{R}(L) = \mathbf{R}_L = \mathbf{0}$.

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since $\mathbf{R}(L) = \mathbf{R}_L = \mathbf{0}$.

- (ii) Recompute \mathbf{M} :

$$\mathbf{M}(s) = \begin{cases} \frac{F(d)}{2a}(L-s)^2 \mathbf{i}_2 & \text{if } s \in]L-a, L[, \\ F(d)(L-s-\frac{a}{2})\mathbf{i}_2 & \text{if } s \in]0, L-a[, \end{cases}$$

since $\mathbf{M}(L) = \mathbf{M}_L = \mathbf{0}$.

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Question #7: Tip end rotation with distributed tip load?

- (iii) Deduce $\boldsymbol{\theta}_\Sigma$:

$$\begin{aligned}\mathbf{M}_\Sigma(s) &= E\mathbb{J}(\boldsymbol{\theta}'_\Sigma(s)) \\ \Rightarrow \quad \boldsymbol{\theta}'_\Sigma(s) &= \frac{\mathbf{M}_\Sigma(s)}{EI_2}\end{aligned}$$

since $\mathbf{M}_\Sigma(s) // \mathbf{i}_2$.

- Consequently:

$$\boldsymbol{\theta}'_\Sigma(s) = \begin{cases} \frac{F(d)}{2EI_2a}(L-s)^2 \mathbf{i}_2 & \text{if } s \in]L-a, L[, \\ \frac{F(d)}{EI_2} \left(L-s-\frac{a}{2}\right) \mathbf{i}_2 & \text{if } s \in]0, L-a[, \end{cases}$$

with $\boldsymbol{\theta}_\Sigma(0) = \mathbf{0}$ (fixed support) and $[\boldsymbol{\theta}_\Sigma(L-a)] = \mathbf{0}$ (continuity).

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Question #7: Tip end rotation with distributed tip load?

- (iii) Deduce $\boldsymbol{\theta}_\Sigma$:

$$\boldsymbol{\theta}_\Sigma(s) = \begin{cases} \frac{F(d)}{6EI_2} \left[\frac{(s-L)^3}{a} + a^2 + 3L(L-a) \right] \mathbf{i}_2 & \text{if } s \in]L-a, L[, \\ \frac{F(d)}{EI_2} \left(L - \frac{s}{2} - \frac{a}{2} \right) s \mathbf{i}_2 & \text{if } s \in]0, L-a[. \end{cases}$$

- Finally:

$$\boldsymbol{\theta}_\Sigma(L) = \frac{F(d)L^2}{2EI_2} \left(1 - \frac{a}{L} + \frac{a^2}{3L^2} \right) \mathbf{i}_2$$

to be compared with the initial result $\boldsymbol{\theta}_\Sigma(L) = \frac{F(d)L^2}{2EI_2}$.

Atomic force microscope

Solution

Question #8: Tip end rotation with self-weight?

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- Local balance of actions at any $s \in]0, L[$:

$$\mathbf{R}'(s) + \mathbf{f}_l(s) = \mathbf{0},$$

$$\mathbf{M}'(s) + \mathbf{i}_3 \times \mathbf{R}(s) + \mathbf{c}_l(s) = \mathbf{0},$$

where $\mathbf{f}_l = -\varrho g S \mathbf{i}_1$ and $\mathbf{c}_l(s) = \mathbf{0}$.

- Strategy: (i) recompute \mathbf{R} ; (ii) recompute \mathbf{M} ; (iii) deduce θ_Σ .

Atomic force microscope

Solution

Question #8: Tip end rotation with self-weight?

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- (i) Recompute \mathbf{R} :

$$\mathbf{R}'(s) = \varrho g S \mathbf{i}_1$$

$$\mathbf{R}(s) = \varrho g S(s - L) \mathbf{i}_1, \quad s \in]0, L[$$

since $\mathbf{R}(L) = \mathbf{R}_L = \mathbf{0}$.

Atomic force microscope

Solution

Question #8: Tip end rotation with self-weight?

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- (i) Recompute \mathbf{R} :

$$\mathbf{R}'(s) = \varrho g S \mathbf{i}_1$$

$$\mathbf{R}(s) = \varrho g S(s - L) \mathbf{i}_1, \quad s \in]0, L[$$

since $\mathbf{R}(L) = \mathbf{R}_L = \mathbf{0}$.

- (ii) Recompute \mathbf{M} :

$$\mathbf{M}'(s) = -\varrho g S(s - L) \mathbf{i}_3 \times \mathbf{i}_1$$

$$\mathbf{M}(s) = -\frac{\varrho g S}{2} (s - L)^2 \mathbf{i}_2, \quad s \in]0, L[$$

since $\mathbf{M}(L) = \mathbf{M}_L = \mathbf{0}$.

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Question #8: Tip end rotation with self-weight?

- (iii) Deduce $\boldsymbol{\theta}_\Sigma$:

$$\begin{aligned}\mathbf{M}_\Sigma(s) &= E\mathbb{J}(\boldsymbol{\theta}'_\Sigma(s)) \\ \Rightarrow \quad \boldsymbol{\theta}'_\Sigma(s) &= \frac{\mathbf{M}_\Sigma(s)}{EI_2}.\end{aligned}$$

- Consequently:

$$\boldsymbol{\theta}'_\Sigma(s) = -\frac{\varrho g S}{2EI_2}(s-L)^2 \mathbf{i}_2$$

$$\boldsymbol{\theta}_\Sigma(s) = -\frac{\varrho g S}{6EI_2}[(s-L)^3 + L^3] \mathbf{i}_2, \quad s \in]0, L[,$$

since $\boldsymbol{\theta}_\Sigma(0) = \mathbf{0}$ (fixed support).

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Question #8: Tip end rotation with self-weight?

- (iii) Deduce $\boldsymbol{\theta}_\Sigma$:

$$\boldsymbol{\theta}_\Sigma(s) = -\frac{\varrho g S}{6EI_2} [(s-L)^3 + L^3] \mathbf{i}_2, \quad s \in]0, L[.$$

- Finally:

$$\boldsymbol{\theta}_\Sigma(L) = -\frac{\varrho g S L^3}{6EI_2} \mathbf{i}_2.$$

- This rotation is much smaller than the one induced by the tip end load, so that the influence of the self-weight can be neglected.

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Question #9: Effective rigidity k_{eff} ?

- The tip end load is expressed in two ways:

$$F(d) = ku_{\text{microlever}} = -k'u_{\text{sample}},$$

and ($d = u_{\text{microlever}} - u_{\text{sample}}$):

$$F(d) = k_{\text{eff}}(u_{\text{microlever}} - u_{\text{sample}});$$

- Thus:

$$k_{\text{eff}} = \frac{kk'}{k + k'}.$$