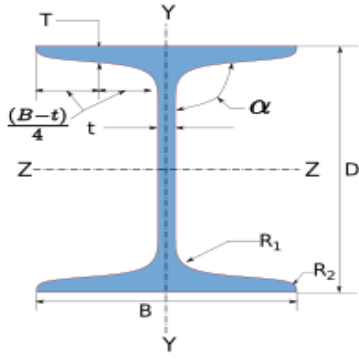
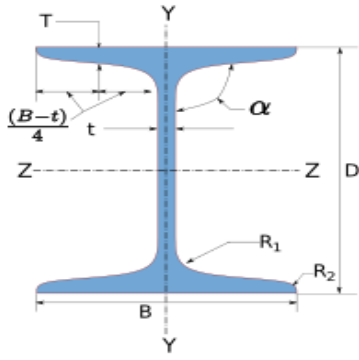


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## 1 Input Parameters

Module		Fin Plate		
MainModule		Shear Connection		
Connectivity		Column flange-Beam web		
Shear(kN)*		150.0		
Supporting Section				
	Supporting Section		HB 400	
	Material *		E 250 (Fe 410 W)A	
	Ultimate strength, fu (MPa)		410	
	Yield Strength , fy (MPa)		230	
	Mass	77.4	Iz(cm4)	281000000.0
	Area(cm2) - A	9870.0	Iy(cm4)	27300000.0
	D(mm)	400.0	rz(cm)	169.0
	B(mm)	250.0	ry(cm)	52.599999999999994
	t(mm)	9.1	Zz(cm3)	1400000.0
	T(mm)	12.7	Zy(cm3)	218000.0
	FlangeSlope	94	Zpz(cm3)	1400000.0
	R1(mm)	14.0	Zpy(cm3)	218000.0
	R2(mm)	7.0		
Supported Section				
	Supported Section		LB 400	
	Material *		E 250 (Fe 410 W)A	
	Ultimate strength, fu (MPa)		410	
	Yield Strength , fy (MPa)		230	
	Mass	56.9	Iz(cm4)	193000000.0
	Area(cm2) - A	7240.0	Iy(cm4)	7160000.0
	D(mm)	400.0	rz(cm)	163.0
	B(mm)	165.0	ry(cm)	31.5
	t(mm)	8.0	Zz(cm3)	965000.0
	T(mm)	12.5	Zy(cm3)	86800.0
	FlangeSlope	98	Zpz(cm3)	965000.0
	R1(mm)	16.0	Zpy(cm3)	86800.0
	R2(mm)	8.0		
Bolt Details				
Diameter(mm)*		[12.0, 16.0, 20.0]		
Grade *		[3.6, 4.6, 4.8, 5.6, 5.8, 6.8, 8.8, 9.8, 10.9, 12.9]		
Type *		Bearing Bolt		
Bolt hole type		Standard		
Slip factor (μ_f)		0.3		
Type of edges		a - Sheared or hand flame cut		

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Gap between beam and  support (mm)	10.0
Are the members exposed to  corrosive influences	False
<b>Weld Details</b>	
Weld Type	Fillet
Type of weld fabrication	Shop Weld
Material grade overwrite (MPa) Fu	410.0

## 2 Design Checks

### 2.1 Bolt Design Checks

Check	Required	Provided	Remarks
Shear Capacity (kN)		$V_{dsb} = \frac{f_u b n_n A_{nb}}{\sqrt{3} \gamma_{mb}}$ $= \frac{410 * 1 * 245}{\sqrt{3} * 1.25}$ $= 66.59$	
Bearing Capacity (kN)		$V_{dpb} = \frac{2.5 k_b d t f_u}{\gamma_{mb}}$ $= \frac{2.5 * 0.51 * 20.0 * 8.0 * 410}{1.25}$ $= 66.59$	
Capacity (KN)		$V_{db} = \min (V_{dsb}, V_{dpb})$ $= \min (66.59, 66.59)$ $= 66.59$	
No of Bolts	$R_u = \sqrt{V_u^2 + A_u^2}$ $n_{trial} = R_u / V_{bolt}$ $R_u = \frac{\sqrt{150.0^2 + 100.0^2}}{66.59}$ $= 3$	4	
No of Columns		1	
No of Rows		4	
Min. Pitch (mm)	$p/g_{min} = 2.5 d$ $= 2.5 * 20.0 = 50.0$	0.0	N/A
Max. Pitch (mm)	$p/g_{max} = \min(32 t, 300 mm)$ $= \min(32 * 8.0, 300 mm)$ $= 300$	0.0	N/A
Min. Gauge (mm)	$p/g_{min} = 2.5 d$ $= 2.5 * 20.0 = 50.0$	75	Pass
Max. Gauge (mm)	$p/g_{max} = \min(32 t, 300 mm)$ $= \min(32 * 8.0, 300 mm)$ $= 300$	75	Pass
Min. End Distance (mm)	$e/e'_{min} = [1.5 \text{ or } 1.7] * d_0$ $= 1.7 * 22.0 = 37.4$	40	Pass

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Check	Required	Provided	Remarks
Max. End Distance (mm)	$e/e'_{max} = 12 t \varepsilon$ $\varepsilon = \sqrt{\frac{250}{f_y}}$ $e/e'_{max} = 12 * 8.0 * \sqrt{\frac{250}{230}}$ $= 99.84$	40	Pass
Min. Edge Distance (mm)	$e/e'_{min} = [1.5 \text{ or } 1.7] * d_0$ $= 1.7 * 22.0 = 37.4$	40	Pass
Max. Edge Distance (mm)	$e/e'_{max} = 12 t \varepsilon$ $\varepsilon = \sqrt{\frac{250}{f_y}}$ $e/e'_{max} = 12 * 8.0 * \sqrt{\frac{250}{230}}$ $= 99.84$	40	Pass
Capacity (KN)	66567.63	66593.94	Pass

## 2.2 Plate Design Checks

Check	Required	Provided	Remarks
Min. Plate Height (mm)	$0.6 * d_b = 0.6 * 400.0 = 240.0$	305	Pass
Max. Plate Height (mm)	$d_b - 2(t_{bf} + r_{b1} + gap)$ $= 400.0 - 2 * (12.5 + 16.0 + 10)$ $= 343.0$	305	Pass
Min. Plate Length (mm)	$2 * e_{min} + (n c - 1) * p_{min}$ $= 2 * 37.4 + (1 - 1) * 50.0$ $= 84.8$	90.0	Pass
Min. Plate Thickness (mm)	$t_w = 8.0$	8.0	Pass
Shear yielding Capacity (V_dy) (kN)		$V_{dg} = \frac{A_v * f_y}{\sqrt{3} * \gamma_{mo}}$ $= \frac{305 * 8.0 * 230}{\sqrt{3} * 1.1}$ $= 176.73$	
Shear Rupture Capacity (V_dn) (kN)		$V_{dn} = \frac{0.75 * A_{vn} * f_u}{\sqrt{3} * \gamma_{mo}}$ $= 1 * (305 - (4 * 22.0)) * 8.0 * 410$ $= 533.82$	
Block Shear Capacity in Shear (V_db) (kN)		334.07	
Shear Capacity (V_d) (kN)	150.0	$V_d = \text{Min}(V_{dy}, V_{dn}, V_{db})$ $= \text{Min}(176.73, 533.82, 334.07)$ $= 176.73$	Pass

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Check	Required	Provided	Remarks
Tension Yielding Capacity (kN)		$T_{dg} = \frac{l * t * f_y}{\gamma_{mo}}$ $= \frac{90.0 * 8.0 * 230}{\sqrt{3} * 1.1}$ $= 150.55$	
Tension Rupture Capacity(kN)		$T_{dn} = \frac{0.9 * A_n * f_u}{\gamma_{m1}}$ $= \frac{0.9 * (90.0 - 4 * 22.0) * 8.0 * 410}{1.25}$ $= 160.59$	
Block Shear Capacity in Tension (T_db) (kN)		334.07	
Tension Capacity (kN)	100.0	$T_d = \text{Min}(T_{dg}, T_{dn}, T_{db})$ $= \text{Min}(150.55, 160.59, 334.07)$ $= 150.55$	Pass
Moment Capacity (kNm)	7.5	31.12	Pass
Interaction Ratio	$\leq 1$	$\frac{7.5}{31.12} + \frac{100.0}{150.55} = 0.91$	Pass

### 2.3 Weld Checks

Check	Required	Provided	Remarks
Min Weld Size (mm)	<i>Thickness of Thicker part</i> $= \text{Max}(12.7, 12.7) = 12.7$ <i>IS800 : 2007 cl.10.5.2.3 Table21,</i> $t_{w_{min}} = 5$	5	Pass
Max Weld Size (mm)	<i>Thickness of Thinner part</i> $= \text{Min}(12.7, 12.7) = 8.0$ $t_{w_{max}} = 8.0$	5	Pass
Weld Strength (kN/mm)	$R_w = \sqrt{(T_{wh} + A_{wh})^2 + (T_{wv} + V_{wv})^2}$ $T_{wh} = \frac{M * y_{max}}{I_{pw}} = \frac{7500000.0 * 147.5}{4278729.17}$ $T_{wv} = \frac{M * x_{max}}{I_{pw}} = \frac{7500000.0 * 0.0}{4278729.17}$ $V_{wv} = \frac{V}{l_w} = \frac{150000.0}{590}$ $A_{wh} = \frac{A}{l_w} = \frac{100000.0}{590}$ $R_w = \sqrt{(258.55 + 169.49)^2 + (0.0 + 254.24)^2}$ $= 497.85$	$f_w = \frac{t_t * f_u}{\sqrt{3} * \gamma_{mw}}$ $= \frac{3.5 * 410}{\sqrt{3}} * 1.25$ $= 662.8$	Pass

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3 3D View



Figure 1: 3D View