			Created with
Company Name	IIT Bombay	Project Title	Moment Connection Design Example
Group/Team Name	Osdag	Subtitle Extended both way end plate moment connection	
Designer	Engineer #1	Job Number	1.2.1.2.1.2
Date	13 /06 /2019	Client	M M Ghosh

Design Conclusion		
Beam to Beam Extended End Plate Splice Connection	Pass	
Connection Properties		
Connection		
Connection Title	Beam to Beam Extended End Plate Splice	
Connection Type	Moment Connection	
Connection Category		
Connectivity	Beam - Beam	
Beam to End Plate Connection	Welded	
End Plate to End Plate Connection	Bolted	
End plate type	Extended both way	
Loading (Factored Loads)		
Bending Moment (kNm)	350.0	
Shear Force (kN)	120.0	
Axial Force (kN)	50.0	
Components		
Beam Section	UB 406 x 178 x 74	
Grade of Steel	Fe 410.0	
Plate Section	563.0 X 204.5 X 20.0	
Thickness (mm)	20.0	
Width (mm)	204.5	
Height (mm)	563.0	

Clearance Holes for Fasteners Grade of Steel	Standard Fe 410.0
Weld	
Туре	Groove Weld (CJP)
Size of Weld (mm)	20.0
Bolts	
Туре	Friction Grip Bolt
Property Class	8.8
Diameter (d) (mm)	16
Hole Diameter (d _o) (mm)	18
Number of Bolts (n)	12
End Distance (e) (mm)	31
Edge Distance (e') (mm)	57
Gauge Distance (g) (mm)	40
Cross-centre gauge (g') (mm)	90.0
Pitch Distance (p) (mm)	·
Pitch 2-3	40.0
Pitch 3-4	221.0
Pitch 4-5	40.0

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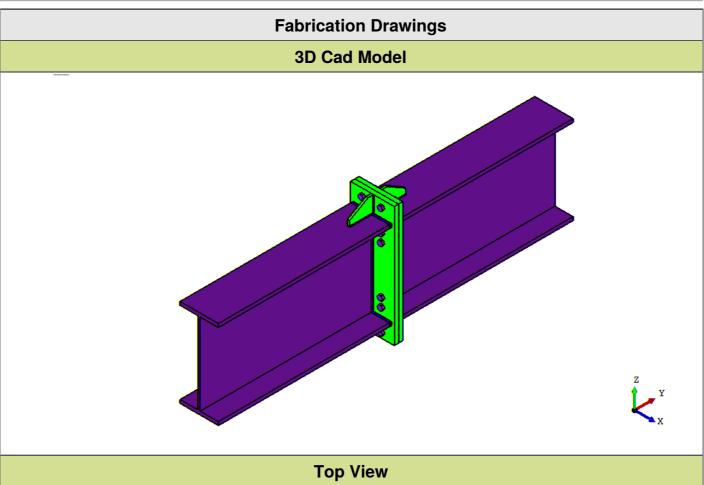
Design Preferences	
Bolt	
Hole Type	Standard
Hole Clearance (mm)	2.0
Ultimate Strength (f _u) (MPa)	800.0
Slip factor	0.48
Beta (β) (pre-tensioned bolt)	1
Weld	
Type of Weld	Shop weld
Detailing	
Type of Edges	Sheared or hand flame cut
Minimum Edge and End Distance	1.7 times the hole diameter
Are members exposed to corrosive influences?	No
Design	
Design Method	Limit State Design

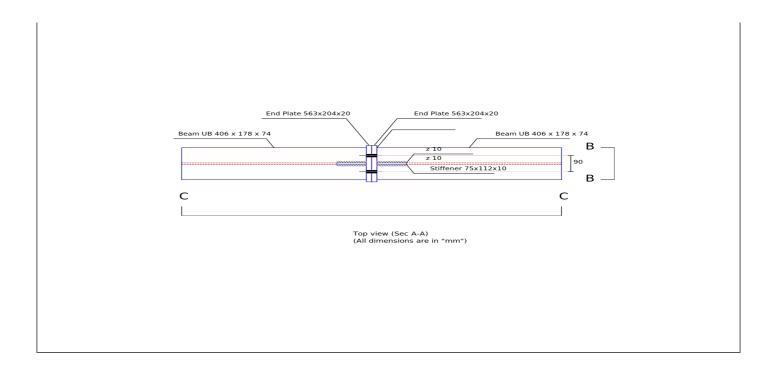
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Design Check			
Check	Required	Provided	Remark
	Bolt Checks		
Tension capacity of critical bolt (kN)	Tension in bolt due to external factored moment & external factored axial load + Prying force = 61.814+14.356 = 76.17 [cl. 10.4.7]	Tension capacity = (0.9*800*157) / (1.25*1000) = 90.432 [cl. 10.4.5]	Pass
Bolt slip resistance (kN)	Factored shear force / Number of bolts = 120.0 / 12 = 10.0	$V_{dsf} = (0.48*1*1.0*87.92) / 1.25 = 33.118$ [cl. 10.4.3]	Pass
Bolt bearing capacity (kN)	N/A	N/A	
Bolt value (kN)		Bolt Shear Capacity =33.118	
Combined shear and tension capacity of bolt	≤ 1.0	$(V_{sf}/V_{df})^2 + (T_f/T_{df})^2 =$ $(10.0/33.118)^2 +$ $(76.17/90.432)^2 = 0.801$ [cl. 10.4.6]	Pass
No. of bolts		12.0	
No. of column(s)		2	
No. of row(s)		6	
Bolt gauge (mm)	≥ 2.5 * d = 40.0, ≤ min(32 * t, 300) = 300.0 [cl. 10.2.2 & cl. 10.2.3]	40	Pass
	≥ 2.5 * d = 40.0, ≤ min(32 * t, 300) =		Pace A

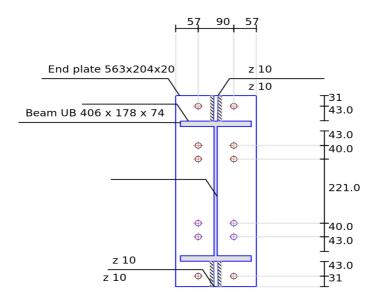
Bolt pitch (mm)	300.0	40	1 433
End distance (mm)	[cl. 10.2.2 & cl. 10.2.3] $\geq 1.7 d_0 = 30.6, \leq 12^*t^*\epsilon = 240.0$ [cl. 10.2.4]	31	Pass
Edge distance (mm)	$\geq 1.7 d_0 = 30.6, \leq 12^*t^*\epsilon = 240.0$ [cl. 10.2.4]	31	Pass
	Plate Checks		
Plate thickness (mm)	(4*1.10*1292.225*1000)/(250.0*89.75)) ^ 0.5 = 14.914 [Design of Steel Structures - N. Subramanian, 2014]	20.0	Pass
Plate height (mm)	Based on detailing requirements	563.0	
Plate width (mm)		204.5	
Plate moment capacity (kNm)	Moment demand (M_d) = $((14.914^{2*}250.0^*89.75)/(4.4)) * 10^ -3$ = 1292.225 [Design of Steel Structures - N. Subramanian, 2014]	Moment capacity (M_c) = $((20.0^{2*}250.0^*89.75)/(4.4))$ * 10^{4} - 3 = 1487.273 [Design of Steel Structures - N. Subramanian, 2014]	Pass
	Weld Checks		
Size of Butt Weld (mm)		20.0	
	Stiffener Checks		
Height (mm)		75.0	
Thickness (mm)		10.0	
WeldSize (mm)		10.0	
MomentCapacity (KN-m)	≥ 5.44	7.127	Pass

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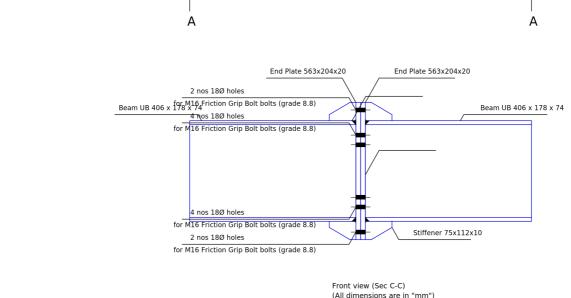


Side View



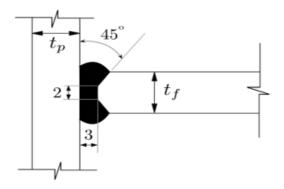
Side view (Sec B-B) (All dimensions are in "mm")

Front View

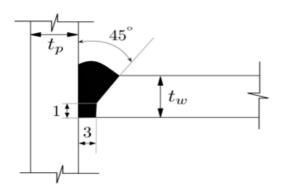


(All dimensions are in "mm")

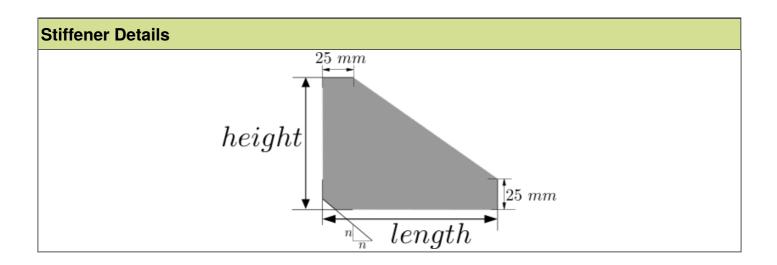
Weld Detailing



Note :- As flange thickness, $t_{\rm f}$ (16.0mm) >= 12mm, double bevel butt welding is provided [Reference: IS 9595: 1996] (All dimensions are in mm)



Note :- As web thickness, $t_{\rm W}$ (9.5mm) <= 12mm, single bevel butt welding is provided [Reference: IS 9595: 1996] (All dimensions are in mm)



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Additional Comments		