

Design Conclusion	
Beam to Column End Plate Moment Connection	Pass
Connection Properties	
Connection	
Connection Type	Moment Connection
Connection Title	Extended End Plate
End plate type	Extended one way
Connection Category	·
Connectivity	Column web-Beam web
Beam to end plate Connection	Welded
Column web to end plate Connection	Bolted
Loading Details	
Bending Moment (kNm)	12.0
Shear Force (kN)	150.0
Axial Force (kN)	50.0
Components	
Beam Section	WPB 240x240x60.3
Grade of Steel	Fe 410.0
Column Section	UC 305 x 305 x 137
Grade of Steel	Fe 410.0
Plate Section	335.0 X 240.0 X 24.0
Thickness (t) (mm)	24.0
Width (mm)	240.0
Depth (mm)	335.0
Clearance holes for fasteners	Standard
Weld	·
Туре	Fillet Weld
Weld at Flange (mm)	8
Weld at Web (mm)	6
Bolts	·
Туре	Friction Grip Bolt
Property Class	10.9
Diameter (d) (mm)	24
Hole diameter (d _o) (mm)	26.0

Number of Bolts (n)	6
End Distance (e)(mm)	45
Edge Distance (e') (mm)	70
Cross-centre gauge (g') (mm)	100.0
Pitch Distance (p) (mm)	
Pitch-1,2	112.0
Pitch-2,3	106.0

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Group/Team Name	Osdag	Subtitle	Moment End Plate
Designer	Engineer 4	Job Number	1.2.2.1.2.2.1
Date	12 /06 /2019	Client	Meera Raghunandan

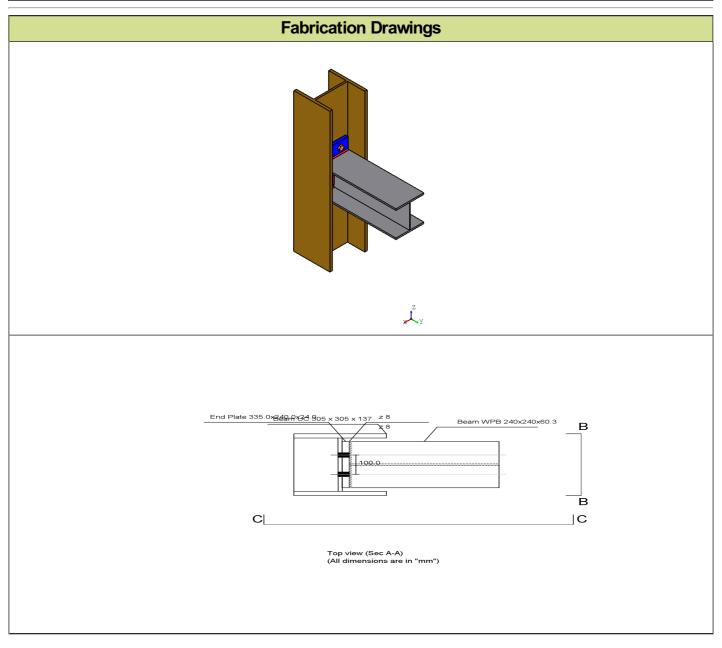
Design Preferences	
Bolt	
Hole Type	Standard
Hole Clearance (mm)	2.0
Ultimate Strength (f _u) (MPa)	1000.0
Slip factor	0.3
Beta (β)(non pre-tensioned)	2
Weld	
Type of Weld	Shop weld
Detailing	
Type of Edges	Sheared or hand flame cut
Minimum Edge-End Distance	1.7 times the hole diameter
Are members exposed to corrosive influences?	No
Design	
Design Method	Limit State Design

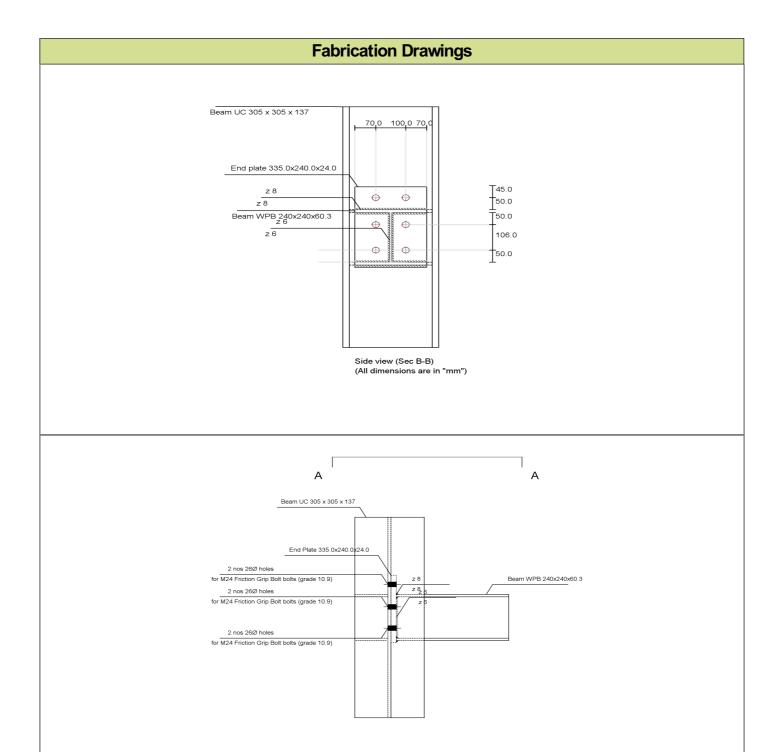
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Design Check				
Check	Required	Provided	Remark	
	Bolt Checks			
Bolt slip resistance (kN)	Factored shear force / Number of bolts = 150.0 / 6 = 25.0	V_{dsf} = (0.3*1*1.0*247.1) / 1.25 = 67.391 [cl. 10.4.3]	Pass	
Bolt bearing capacity (kN)	NA	NA		
Bolt capacity (kN)		Bolt slip resistance =67.391		
Tension capacity of bolt (kN)	≥ Tension in bolt due to external moment + external axial load + prying force =124.47+8.333+46.828=179.632	Tension capacity = (0.9*1000*353) / (1.25*1000) = 254.16 [cl. 10.4.5]	Pass	
Combined shear and tension capacity of bolt	≤ 1.0	$(V_{sf}/V_{df})^2 + (T_f/T_{df})^2 =$ (25.0/67.391)^2 + (179.632/254.16)^2 = 0.637 [cl. 10.4.6]	Pass	
No. of bolts	≥ 4 , ≤ 12	6.0		
Pitch distance (mm)	≥ 2.5 * d = 60, ≤ min(32 * t, 300) = 300 [cl. 10.2.2 & cl. 10.2.3]	60	Pass	
End distance (mm)	≥ 1.7 d_0 = 44.2, ≤ 12*t* ϵ = 165.6 [cl. 10.2.4]	45	Pass	
Edge distance (mm)	\geq 1.7 d_0 = 44.2, \leq 12*t* ϵ = 165.6 [cl. 10.2.4]	45	Pass	
Distance to the centre line of bolt from face of beam flange (mm)	25mm ≤ <i>I</i> _v ≤ 63.5mm	50	Pass	
Plate Checks				
Plate thickness (mm)	$\geq \sqrt{\text{(M *(1.1/fy) *(4/b_e))}} = \geq \sqrt{\text{(124.47*)}}$ (1.1/250.0) * (4/120.0)) = 22.603	24.0	Pass	
Plate height (mm)		335.0		
Plate width (mm)	≥ width of beam flange , ≥240.0	240.0	Pass	
	Weld Checks			
	Flange			
Effective weld length				

on top flange (mm)		224.0	
Effective weld length on bottom flange (mm)		98.15	
Weld throat thickness at flange (mm)	< 12.0,> 6.0	8.0	Pass
Critical stress in weld at flange (N/mm^2)	$\geq ((M/Z_{weld,flange}) + (P/A_{weld}))$ =180.658	$(f_{\rm u} / \sqrt{3} * \square_{\rm mb}) = 189.371$	Pass
	Web		
Effective weld length at web (each side) (mm)		189.8	
Weld throat thickness at web (mm)	< 7.5,> 6.0	6.0	Pass
Critical stress in weld at web (N/mm^2)	$\geq \sqrt{((M/Z_{weld,web} + P/A_{weld})^2))} + (V/A_{weld,web})^2 = 185.165$	$(f_{\rm u} / \sqrt{3} * \square_{\rm mb}) = 189.371$	Pass
	Stiffener Checks		
	Horizontal Continuity Plate in Te	ension	
Length (mm)		277.1	
Width (mm)		147.7	
Thickness (mm)	≥15.713	16.0	
Weld (mm)		8.0	
	Horizontal Continuity Plate in Com	pression	
Length (mm)		277.1	
Width (mm)		147.7	
Thickness (mm)	≥15.713	16.0	
Weld (mm)		8.0	
	End Plate Stiffeners		
Length (mm)		245.0	
Height (mm)		155.0	
Thickness (mm)		10.0	
Noch at top side of plate (mm)		50.0	
Noch at bottom side of plate (mm)		10.0	
Fillet weld size (mm)		8.0	

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Front view (Sec C-C)
(All dimensions are in "mm")

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Additional Comments	