IT Bombay		Created with	B Osdag
Company Name	IIT B	Project Title	Connection Designs
Group/Team Name	Osdag	Subtitle	Moment End Plate
Designer	Engineer 2	Job Number	1.2.2.1.1.2.1
Date	12 /06 /2019	Client	Somnath Mukherjee

Design Conclusion	
Beam to Column End Plate Moment Connection	Pass
Connection Properties	
Connection	
Connection Type	Moment Connection
Connection Title	Extended End Plate
End plate type	Extended both way
Connection Category	
Connectivity	Column web-Beam web
Beam to end plate Connection	Welded
Column web to end plate Connection	Bolted
Loading Details	
Bending Moment (kNm)	12.0
Shear Force (kN)	150.0
Axial Force (kN)	50.0
Components	
Beam Section	WPB 240x240x60.3
Grade of Steel	Fe 410.0
Column Section	UC 305 x 305 x 137
Grade of Steel	Fe 410.0
Plate Section	450.0 X 240.0 X 26.0
Thickness (t) (mm)	26.0
Width (mm)	240.0
Depth (mm)	450.0
Clearance holes for fasteners	Standard
Weld	
Type	Fillet Weld
Weld at Flange (mm)	10
Weld at Web (mm)	6
Bolts	
Туре	Friction Grip Bolt
Property Class	10.9
Diameter (d) (mm)	30
Hole diameter (d_0) (mm)	33.0

Number of Bolts (n)	8
End Distance (e)(mm)	60
Edge Distance (e') (mm)	70
Cross-centre gauge $(g^{'})$ (mm)	100.0
Pitch Distance (p) (mm)	
Pitch	106.0

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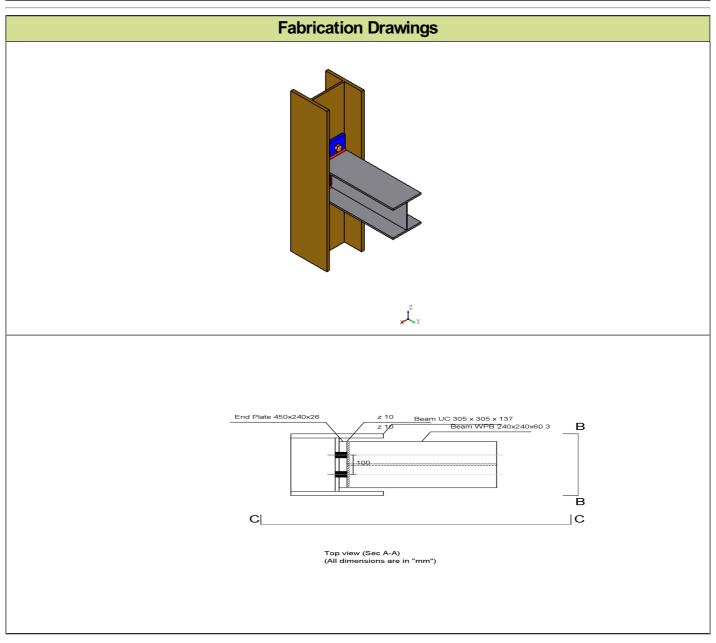
Design Preferences	
Bolt	
Hole Type	Standard
Hole Clearance (mm)	3.0
Ultimate Strength (f _u) (MPa)	1000.0
Slip factor	0.3
Beta (β)(non pre-tensioned)	2
Weld	
Type of Weld	Shop weld
Detailing	
Type of Edges	Sheared or hand flame cut
Minimum Edge-End Distance	1.7 times the hole diameter
Are members exposed to corrosive influences?	No
Design	
Design Method	Limit State Design

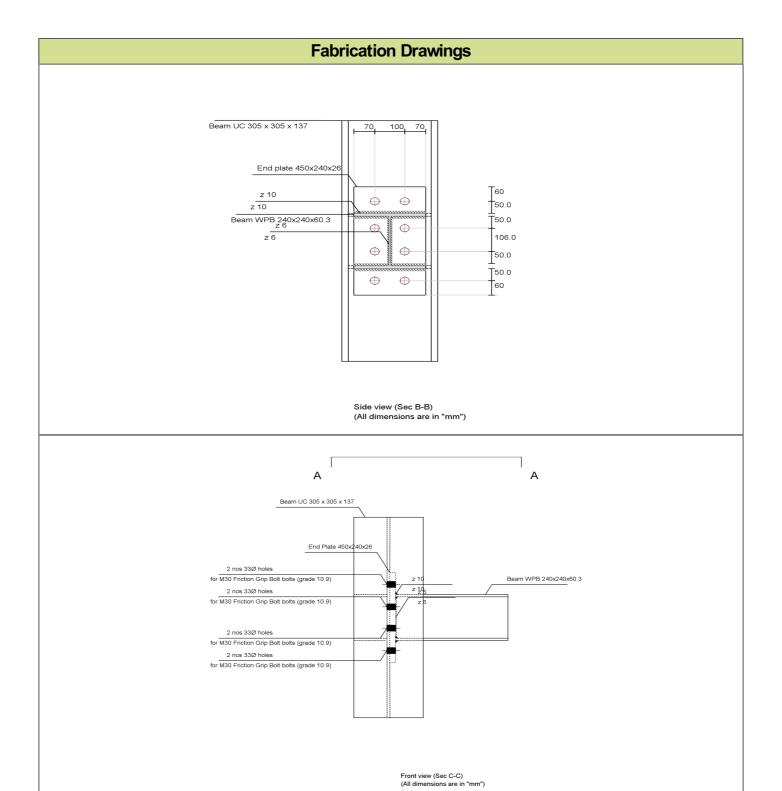
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Design Check					
Check	Required	Provided	Remark		
Bolt Checks					
Bolt slip resistance (kN)	Factored shear force / Number of bolts = 150.0 / 8 = 18.75	$V_{\text{dsf}} = (0.3*1*1.0*392.7) / 1.25 = 107.1$ [cl. 10.4.3]	Pass		
Bolt bearing capacity (kN)	N/A	NA			
Bolt capacity (kN)		Bolt slip resistance =107.1			
Tension capacity of bolt (kN)	≥ Tension in bolt due to external moment + external axial load + prying force =124.47+6.25+34.743=165.463	Tension capacity = (0.9*1000*561) / (1.25*1000) = 403.92 [cl. 10.4.5]	Pass		
Combined shear and tension capacity of bolt	≤ 1.0	$(V_{sf}/V_{df})^2 + (T_f/T_{df})^2 =$ $(18.75/107.1)^2 +$ $(165.463/403.92)^2 =$ 0.198 [cl. 10.4.6]	Pass		
No. of bolts	≥ 4 , ≤ 12	8.0			
Pitch distance (mm)	\geq 2.5 * d = 75, \leq min(32 * t, 300) = 300 [cl. 10.2.2 & cl. 10.2.3]	106	Pass		
End distance (mm)	≥ 1.7 d_0 = 56.1, ≤ 12*t* ϵ = 165.6 [cl. 10.2.4]	60	Pass		
Edge distance (mm)	\geq 1.7 d_0 = 56.1, \leq 12*t* ϵ = 165.6 [cl. 10.2.4]	60	Pass		
Distance to the centre line of bolt from face of beam flange (mm)	50mm ≤ <i>I</i> _v ≤ 62.5mm	50	Pass		
Plate Checks					
Plate thickness (mm)	$\geq \sqrt{(M * (1.1/fy) * (4/b_e))} = \geq \sqrt{(124.47* (1.1/250.0) * (4/120.0))} = 22.677$	26.0	Pass		
Plate height (mm)		450.0			
Plate width (mm)	≥ width of beam flange , ≥240.0	240.0	Pass		
Weld Checks					
	Flange				
Effective weld length					

on top flange (mm)		220.0	
Effective weld length on bottom flange (mm)		94.15	
Weld throat thickness at flange (mm)	< 12.0,> 6.0	10.0	Pass
Critical stress in weld at flange (N/mm^2)	$\geq ((M/Z_{weld,flange}) + (P/A_{weld}))$ =150.895	$(f_{\rm u} / \sqrt{3} * \square_{\rm mb}) = 189.371$	Pass
	Web		
Effective weld length at web (each side) (mm)		189.8	
Weld throat thickness at web (mm)	< 7.5,> 6.0	6.0	Pass
Critical stress in weld at web (N/mm^2)	$\geq \sqrt{((M/Z_{weld,web} + P/A_{weld})^2))} + (V/A_{weld,web})^2 = 163.104$	$(f_{\rm u} / \sqrt{3} * \square_{\rm mb}) = 189.371$	Pass
	Stiffener Checks		
	Horizontal Continuity Plate in Te	ension	,
Length (mm)		277.1	
Width (mm)		147.7	
Thickness (mm)	≥15.713	16.0	
Weld (mm)		8.0	
	Horizontal Continuity Plate in Com	pression	
Length (mm)		277.1	
Width (mm)		147.7	
Thickness (mm)	≥15.713	16.0	
Weld (mm)		8.0	
	End Plate Stiffeners		
Length (mm)		275.0	
Height (mm)		185.0	
Thickness (mm)		10.0	
Noch at top side of plate (mm)		50.0	
Noch at bottom side of plate (mm)		10.0	
Fillet weld size (mm)		8.0	

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Additional Comments	