



Created with



Company Name	IIT B	Project Title	Connection Designs
Group/Team Name	Osdag	Subtitle	Moment End Plate
Designer	Engineer 1	Job Number	1.2.2.1.1.1.1
Date	12 /06 /2019	Client	V Kalyanaraman

Design Conclusion	
Beam to Column End Plate Moment Connection	Pass
Connection Properties	
Connection	
Connection Type	Moment Connection
Connection Title	Extended End Plate
End plate type	Extended both way
Connection Category	
Connectivity	Column flange-Beam web
Beam to end plate Connection	Welded
Column flange to end plate Connection	Bolted
Loading Details	
Bending Moment (kNm)	25.0
Shear Force (kN)	35.0
Axial Force (kN)	12.0
Components	
Beam Section	WPB 300x300x96.8
Grade of Steel	Fe 410.0
Column Section	UC 305 x 305 x 137
Grade of Steel	Fe 410.0
Plate Section	520.0 X 300.0 X 26.0
Thickness (t) (mm)	26.0
Width (mm)	300.0
Depth (mm)	520.0
Clearance holes for fasteners	Standard
Weld	
Type	Fillet Weld
Weld at Flange (mm)	10
Weld at Web (mm)	6
Bolts	
Type	Bearing Bolt
Property Class	12.9
Diameter (d) (mm)	30
Hole diameter (d_o) (mm)	33.0

Number of Bolts (n)	8
End Distance (e)(mm)	60
Edge Distance (e') (mm)	100
Cross-centre gauge (g') (mm)	100.0
Pitch Distance (p) (mm)	
Pitch	170.0



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Design Preferences

Bolt

Hole Type	Standard
Hole Clearance (mm)	3.0
Ultimate Strength (f_u) (MPa)	1200.0
Slip factor	N/A
Beta (β)(non pre-tensioned)	2

Weld

Type of Weld	Shop weld
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Detailing

Type of Edges	Sheared or hand flame cut
Minimum Edge-End Distance	1.7 times the hole diameter
Are members exposed to corrosive influences?	No

Design

Design Method	Limit State Design
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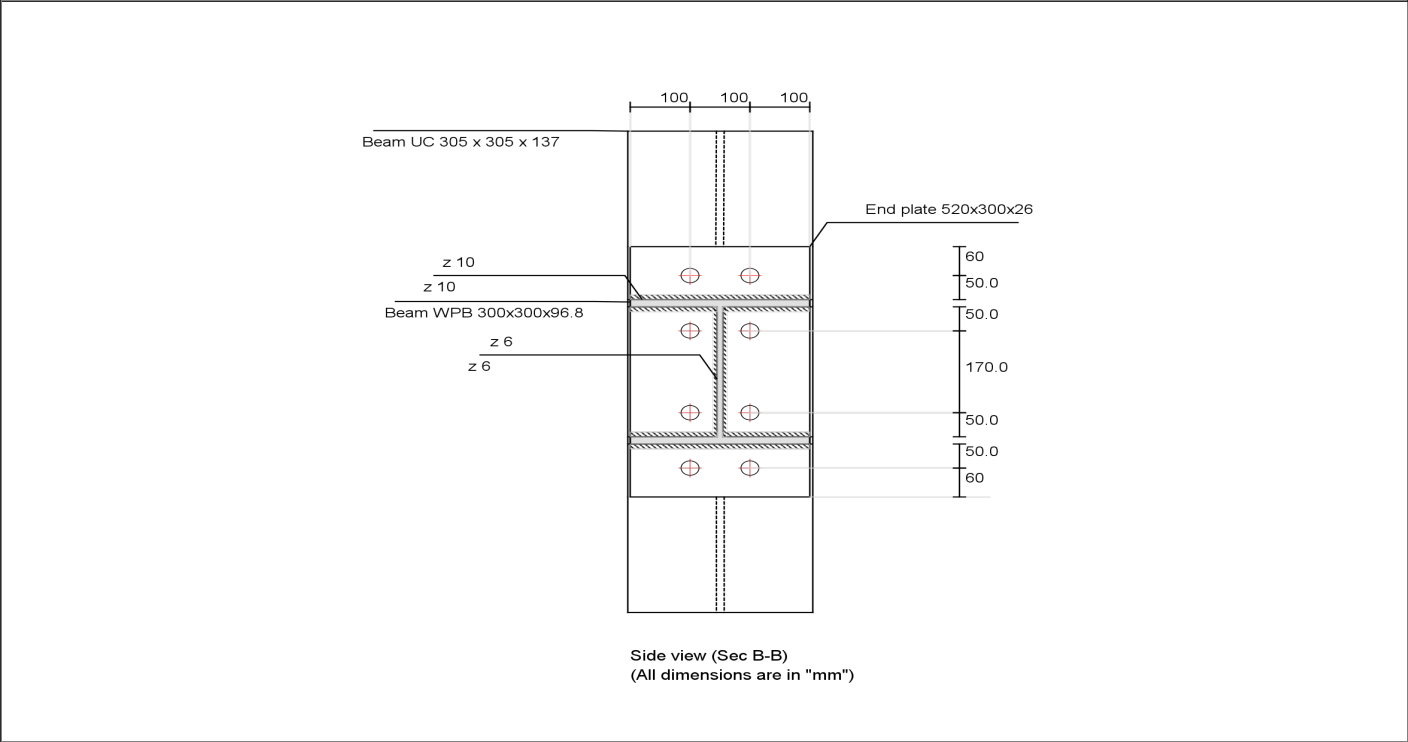
Design Check			
Check	Required	Provided	Remark
Bolt Checks			
Bolt shear capacity (kN)	Factored shear force / Number of bolts = $35.0 / 8 = 4.375$	$V_{dsb} = (1200 \times 1 \times 0.6126 \times 30 \times 30) / (\sqrt{3} \times 1.25) = 310.938$ [cl. 10.3.3]	Pass
Bolt bearing capacity (kN)		$V_{dpb} = (2.5 \times k_b \times d \times t \times f_u) = 416.92$ [cl. 10.3.4]	
Bolt capacity (kN)	min(Shear Capacity, Bearing Capacity) = min (310.938, 416.92)	310.938	
Tension capacity of bolt (kN)	\geq Tension in bolt due to external moment + external axial load + prying force $= 193.151 + 1.5 + 52.548 = 247.2$	Tension capacity = $(0.9 \times 1200 \times 561) / (1.25 \times 1000) = 484.704$ [cl. 10.4.5]	Pass
Combined shear and tension capacity of bolt	≤ 1.0	$(V_{sb}/V_{db})^2 + (T_b/T_{db})^2 = (4.375/310.938)^2 + (247.2/484.704)^2 = 0.26$ [cl. 10.3.6]	Pass
No. of bolts	$\geq 4, \leq 12$	8.0	
Pitch distance (mm)	$\geq 2.5 \times d = 75, \leq \min(32 \times t, 300) = 300$ [cl. 10.2.2 & cl. 10.2.3]	170	Pass
End distance (mm)	$\geq 1.7 d_o = 56.1, \leq 12 \times t \times \epsilon = 260.4$ [cl. 10.2.4]	60	Pass
Edge distance (mm)	$\geq 1.7 d_o = 56.1, \leq 12 \times t \times \epsilon = 260.4$ [cl. 10.2.4]	60	Pass
Distance to the centre line of bolt from face of beam flange (mm)	$50\text{mm} \leq l_v \leq 62.5\text{mm}$	50	Pass
Plate Checks			
Plate thickness (mm)	$\geq \sqrt{(M \times (1.1/f_y) \times (4/b_e))} = \geq \sqrt{(193.151 \times (1.1/250.0) \times (4/150.0))} = 24.903$	26.0	Pass
Plate height (mm)		520.0	
Plate width (mm)	\geq width of beam flange , ≥ 300.0	300.0	Pass

Weld Checks			
Flange			
Effective weld length on top flange (mm)		280.0	
Effective weld length on bottom flange (mm)		122.3	
Weld throat thickness at flange (mm)	< 15.0,> 6.0	10.0	Pass
Critical stress in weld at flange (N/mm ²)	$\geq ((M/Z_{\text{weld,flange}}) + (P/A_{\text{weld}})) = 177.945$	$(f_u / \sqrt{3} * \sigma_{mb}) = 189.371$	Pass
Web			
Effective weld length at web (each side) (mm)		252.6	
Weld throat thickness at web (mm)	< 10.0,> 6.0	6.0	Pass
Critical stress in weld at web (N/mm ²)	$\geq \sqrt{((M/Z_{\text{weld,web}} + P/A_{\text{weld}})^2) + (V/A_{\text{weld,web}})^2} = 157.961$	$(f_u / \sqrt{3} * \sigma_{mb}) = 189.371$	Pass
Stiffener Checks			
Horizontal Continuity Plate in Tension			
Length (mm)		277.1	
Width (mm)		147.7	
Thickness (mm)	≥ 15.713	16.0	
Weld (mm)		8.0	
Horizontal Continuity Plate in Compression			
Length (mm)		277.1	
Width (mm)		147.7	
Thickness (mm)	≥ 15.713	16.0	
Weld (mm)		8.0	
End Plate Stiffeners			
Length (mm)		275.0	
Height (mm)		185.0	
Thickness (mm)		10.0	
Noch at top side of plate (mm)		50.0	
Noch at bottom side of plate (mm)		10.0	
Fillet weld size		8.0	





Fabrication Drawings



Technical drawing of a beam-to-column connection. The drawing includes a front view (labeled 'Front view (Sec C-C)') and a side view (labeled 'Side view (Sec A-A)').

Front view (Sec C-C):

- Beam UC 305 x 305 x 137
- End Plate 520x300x26
- 2 nos 33Ø holes for M30 Bearing Bolt bolts (grade 12.9)
- 2 nos 33Ø holes for M30 Bearing Bolt bolts (grade 12.9)
- 2 nos 33Ø holes for M30 Bearing Bolt bolts (grade 12.9)
- 2 nos 33Ø holes for M30 Bearing Bolt bolts (grade 12.9)

Side view (Sec A-A):

- Beam WPB 300x300x96.8
- z 10
- z 10
- z 6
- z 6

(All dimensions are in "mm")



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Additional Comments	
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