

Design Conclusion	
Beam to Beam Extended One Way	End Plate Pass
Splice Connection	
Connection Properties	
Connection	
Connection Title	Beam to Beam Extended One Way End Plate Splice
Connection Type	Moment Connection
Connection Category	
Connectivity	Beam - Beam
Beam to End Plate Connection	Welded
End Plate to End Plate Connection	Bolted
End plate type	Extended one way
Loading (Factored Loads)	
Bending Moment (kNm)	75.0
Shear Force (kN)	12.0
Axial Force (kN)	0.0
Components	
Beam Section	MB 400
Grade of Steel	Fe 410.0
Plate Section	535.0 X 268.9 X 16.0
Thickness (mm)	16.0
Width (mm)	268.9
Height (mm)	535.0
Clearance Holes for Fasteners	Over-sized
Grade of Steel	Fe 410.0
Weld	
Туре	Groove Weld (CJP)
Size of Weld (mm)	16.0
Bolts	

Туре	Bearing Bolt
Property Class	3.6
Diameter (d) (mm)	30
Hole Diameter (d ₀) (mm)	38
Number of Bolts (n)	6
End Distance (e) (mm)	65
Edge Distance (e') (mm)	64
Gauge Distance (g) (mm)	75
Cross-centre gauge (g') (mm)	138.9
Pitch Distance (p) (mm)	
Pitch 2-3	248.0

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Group/Team Name	Osdag	Subtitle	End Plate Moment Connetion
Designer	Enginner 2	Job Number	1.2.1.2.2.1
Date	13 /06 /2019	Client	Yogesh D Pisal

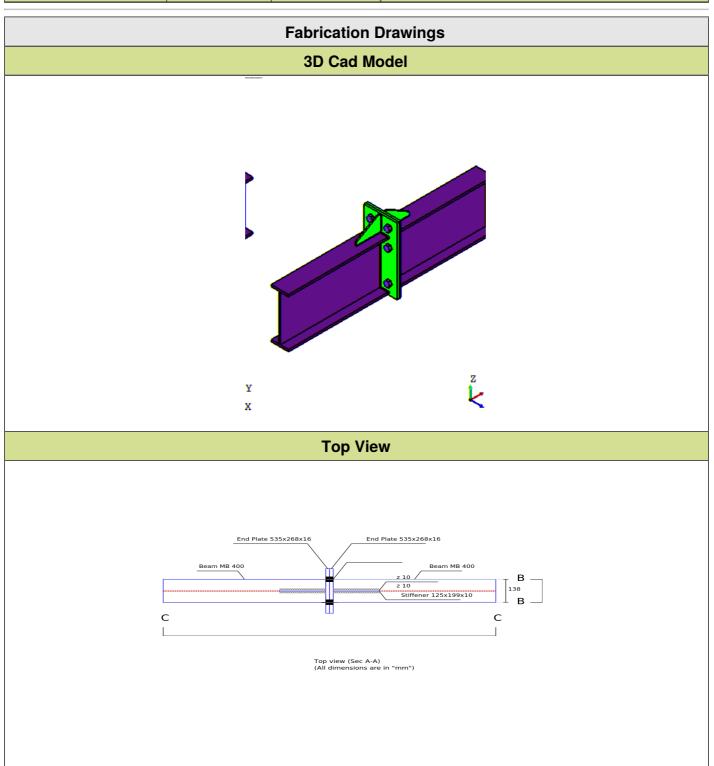
Design Preferences	
Bolt	
Hole Type	Over-sized
Hole Clearance (mm)	8.0
Ultimate Strength (f _u) (MPa)	300.0
Slip factor	N/A
Beta (β)(non pre-tensioned)	2
Weld	
Type of Weld	Shop weld
Detailing	
Type of Edges	Sheared or hand flame cut
Minimum Edge and End Distance	1.7 times the hole diameter
Are members exposed to corrosive influences?	No
Design	
Design Method	Limit State Design

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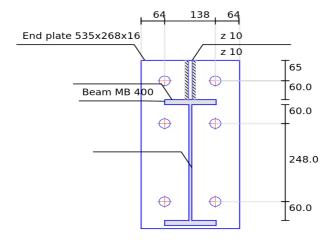
Design Check			
Check	Required	Provided	Remark
	Bolt Che	cks	
Tension capacity of critical bolt (kN)	Tension in bolt due to external factored moment & external factored axial load + Prying force = 61.17+58.347 = 119.517 [cl. 10.4.7]	Tension capacity = (0.9*300*561) / (1.25*1000) = 121.176 [cl. 10.4.5]	Pass
Bolt shear capacity (kN)	Factored shear force / Number of bolts = 12.0 / 6 = 2.0	V_{dsb} = (300*1*0.6126*30*30)/($\sqrt{3}$ *1.25*1000) = 77.7 [cl. 10.3.3]	Pass
Bolt bearing capacity (kN)		V_{dpb} = (2.5*0.408*30*32.0*410.0) / (1.25*1000) = 321.1 [cl. 10.3.4]	
Bolt capacity (kN)	min (77.7, 321.1)	77.7	
Combined shear and tension capacity of bolt	≤ 1.0	$(V_{\rm sb}/V_{\rm db})^2 + (T_{\rm b}/T_{\rm db})^2 =$ $(2.0/77.7)^2 + (119.517/121.176)^2 =$ 0.973 [cl. 10.3.6]	Pass
No. of bolts		6.0	
No. of column(s)		2	
No. of row(s)		3	
Bolt gauge (mm)	≥ 2.5 * d = 75.0, ≤ min(32 * t, 300) = 300.0 [cl. 10.2.2 & cl. 10.2.3]	75	Pass
Bolt pitch (mm)	≥ 2.5 * d = 75.0, ≤ min(32 * t, 300) = 300.0 [cl. 10.2.2 & cl. 10.2.3]	75	Pass
End distance (mm)	≥ 1.7 d_0 = 64.6,≤ 12*t* ϵ = 195.0 [cl. 10.2.4]	65	Pass
(mm)	[cl. 10.2.4]		

Edge distance	$\geq 1.7 d_0 = 64.6, \leq 12 \text{ t} \approx 195.0$	65	Pass	
(mm)	[cl. 10.2.4] Plate Che	cks	<u>'</u>	
Plate thickness (mm)	((4*1.10*1696.1*1000)/(250.0*70.0)) ^ 0.5 = 14.901 [Design of Steel Structures - N. Subramanian, 2014]	16.0	Pass	
Plate height (mm)	Based on detailing requirements	535.0		
Plate width (mm)		268.9		
Plate moment capacity (kNm)	Moment demand (M_d) = $((14.901^{2*}250.0*70.0)/(4.4))*10^{-3}$ = 1696.1 [Design of Steel Structures - N. Subramanian, 2014]	Moment capacity (M_c) = $((16.0^2*250.0*70.0)/(4.4))*10^-3 = 1955.636$ [Design of Steel Structures - N. Subramanian, 2014]	Pass	
	Weld Che	cks	,	
Size of Butt Weld (mm)		16.0		
	Stiffener Ch	necks		
Height (mm)		125.0		
Thickness (mm)		10.0		
WeldSize (mm)		10.0		
MomentCapacity (KN-m)	≥ 7.34	22.501	Pass	

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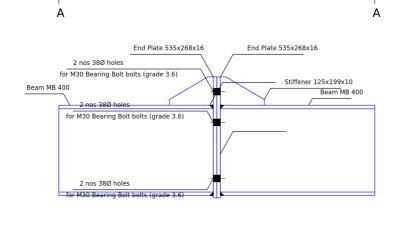


Side View



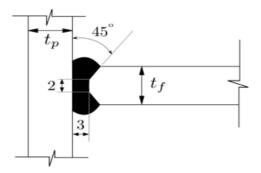
Side view (Sec B-B) (All dimensions are in "mm")

Front View

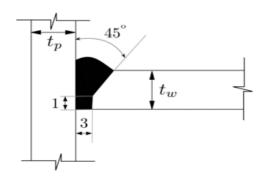


Front view (Sec C-C)
(All dimensions are in "mm")

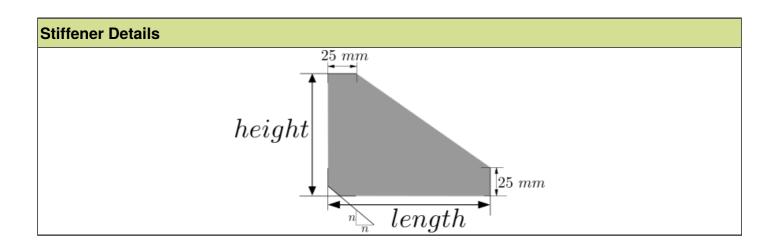
Weld Detailing



Note :- As flange thickness, $t_{\rm f}$ (16.0mm) >= 12mm, double bevel butt welding is provided [Reference: IS 9595: 1996] (All dimensions are in mm)



Note :- As web thickness, $t_{\rm W}$ (8.9mm) <= 12mm, single bevel butt welding is provided [Reference: IS 9595: 1996] (All dimensions are in mm)



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Additional Comments	
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