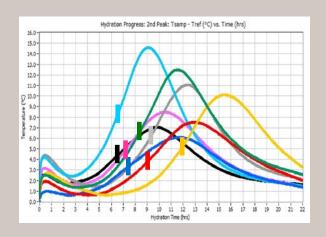
Mix Performance Evaluation using the ASTM C1753 Thermal Testing Protocol

NCC Spring 2016 Columbus, OH

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Designation: C1753 - 15

Standard Practice for Evaluating Early Hydration of Hydraulic Cementitious Mixtures Using Thermal Measurements¹

This standard is issued under the fixed designation C1753; the number immediately following the designation indicates the year of original adoption or, in the case of revision, the year of last revision. A number in parenthese indicates the year of last reapproval. A superscript epsilon (e) indicates an editorial change since the last revision or reapproval.

1. Scope

- 1.1 This practice describes the apparatus and procedure for evaluating relative differences in early hydration of hydraulic cementitious mixtures such as paste, mortar, or concrete, including those containing chemical admixtures, various supplementary cementitious materials (SCMs), and other finely divided materials, by measuring the temperature history of a specimen.
- 1.2 Calorimetry is the measurement of heat lost or gained during a chemical reaction such as cement hydration; calorimetric measurements as a function of time can be used to describe and evaluate hydration and related early-age property development. Calorimetry may be performed under isothermal conditions (as described in Practice C1679) or under adiabatic or semi-adiabatic conditions. This practice cannot be described as calorimetry because no attempt is made to measure or compute the heat evolved from test specimens due to hydration, but it can in many cases be used for similar evaluations. Variables that should be considered in the application of this practice are discussed in the Appendix.
- 1.3 Units—The values stated in either SI units or inch-

and may cause chemical burns to skin and tissue upon prolonged exposure.2

2. Referenced Documents

- 2.1 ASTM Standards:³
- C39/C39M Test Method for Compressive Strength of Cylindrical Concrete Specimens
- C125 Terminology Relating to Concrete and Concrete Ag-
- C172/C172M Practice for Sampling Freshly Mixed Concrete
- C192/C192M Practice for Making and Curing Concrete Test Specimens in the Laboratory
- C219 Terminology Relating to Hydraulic Cement
- C305 Practice for Mechanical Mixing of Hydraulic Cement Pastes and Mortars of Plastic Consistency
- C403/C403M Test Method for Time of Setting of Concrete Mixtures by Penetration Resistance
- C494/C494M Specification for Chemical Admixtures for
- C1005 Specification for Reference Masses and Devices for Determining Mass and Volume for Use in the Physical Testing of Hydraulic Cements
- C1679 Practice for Measuring Hydration Kinetics of Hy-

About the ASTM C1753 Standard Practice

- Published fall, 2015: Standard Practice for Evaluating Early Hydration of Hydraulic Cementitious Mixtures Using Thermal Measurements
- Sometimes referred to as "semi-adiabatic calorimetry"
 - True calorimetry (isothermal or adiabatic) measures and quantifies heat release from hydration (more sophisticated)
 - Thermal testing produces similar indications of evolved heat via records of temperature changes in hydrating samples
- Most applications evaluate relative behavior of similar, compared mixtures
- Samples can be concrete, mortar, paste, or slurry
- Equipment needed (simple and inexpensive) can be manufactured or devised (homemade)

About the ASTM C1753 Standard Practice

One of very few ASTM standards with extensive guidance on use & applications

∰ C1753 – 15

TABLE X1.2 Mixture Proportions for Temperature Effects Example Mixtures (Fig. X1.8)

Note 1—1 mL/100 kg = 0.0154 oz/100 lb; 1 mL = 0.0338 oz.									
Mixture description	cement, g	fly ash, g	w/cm	water, g	admix dosage rate	dose, mL	sand, g		
No ash or admix	500		0.45	225			1360		
25% ash, no admix	375	125	0.45	225			1360		
25% ash, Type A dose	375	125	0.40	200	Type A, 260 mL/100 kg	1.30	1360		
25% ash, Type D dose	375	125	0.40	200	Type D, 390 mL/100 kg	1.95	1360		

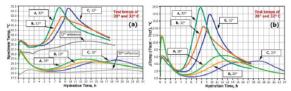
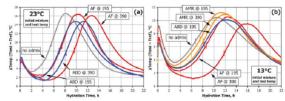


FIG. X1.9 Effects of Different Test Temperatures for Paste Specimens Mixed at 32 °C [90 °F] (a) Plotted without Subtraction of Reference Temperatures; (b) Same Data Plotted with Subtraction of Reference Temperatures



Norm 1—See Table X1.3 for descriptions of admixtures; indicated dosage rates are mL/100 kg of cement. FIG. X1.10 Effects of Admixtures and Dosages on Paste (a) at 23 °C [73 °F]; (b) at 13 °C [55 °F]

for the 13 °C [55 °C] test temperature and cooling of materials before mixing; the 23 °C [73 °C] test temperature condition was ambient room temperature. Descriptions and dosages of the admixture products are summarized in Table X1.3. Note that with this cement, the AF admixture appeared to result in more retardation at the higher dosage than even the ABD product, and also contributed more retarding influence at the lower temperature, even though it is marketed as a "set neutral" product. The AMR product included among the 13 °C [55 °C] mixtures shown in Fig. X1.10(b) showed little associated

retardation or temperature sensitivity for the same dosages, which were on the lower end of the recommended range for each product.

X1.5.4.2 Evaluation of Fly Ash Replacement Level— Thermal measurement testing can be useful in evaluating SCM effects in a proposed mixture for a given set of materials and expected field temperatures, to aid in proportioning decisions. Fig. X1.11 shows thermal profiles from a series of laboratory paste mixtures (w/cm = 0.40) made and tested at the highest expected project field temperature of 34°C [93°FI, using a

TABLE X1.3 Admixtures and Dosages as Used in Example Test Series (Fig. X1.10)

Note 1-1 mL/100 kg = 0.0154 oz/100 lb.

Product ID	Man	ufacturer's product informa	Test dosages used, mL/100 kg cement			
In graphs	water reduction categories	setting Influences	recommended dosage range, mL/100 kg cement	low dose	high dose	
AMR	Type A & mid-range	set neutral	195 to 980	195	390	
AF	Type A, F, & mid-range	set neutral	196 to 780	195	390	
ABD	Type A, B, & D	some retardation	195 to 460	195	390	

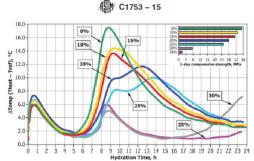


FIG. X1.11 Effects of Fly Ash Replacement Level in Paste Mixtures Made with a Type A/D Admixture at 34°C [93 °F] Initial Mixture and Test Temperature (Note: 1 MPa = 145 psi)

single sample of the Type II cement to be used and the highest expected dosage (390 mL/100 kg [6 oz/100 lb], based on total cementitious materials content) of a Type A/D admixture. Fly ash replacement level was incrementally increased from 0% to 35% using a single sample of Class C fly ash from the source planned for use in the project. Paste specimens were each about 350 g, tested in non-insulated 51 × 102 mm [2 × 4 in.] specimen containers. The fly ash replacement levels used for each mixture are indicated on the thermal profiles, and corresponding 1-day compressive strengths of the same specimens are shown in the inset bar chart. Note that sulfate imbalance begins to be indicated as fly ash replacement level is increased above 15%. Considering the possible effects of variability of other mixture components not included as test variables, it might be prudent to limit the project fly ash replacement level to around 15% with this set of materials and other proportions, if concrete temperatures in the field are to be this high.

X1.5.4.3 Evaluation of Sulfate Imbalance Effects of Admixture Dosage-The effects of possible sulfate imbalance resulting from higher admixture dosages can also be evaluated using thermal measurement testing. Fig. X1.12 shows thermal profiles of laboratory paste mixtures made and tested at 32 °C [90 °F] using a single sample of Type II cement, 25% replacement with a single sample of Class C fly ash, and dosages of a Type A/D admixture ranging from 0 to 520 mL/100 kg [0 to 8 oz/100 lb], based on total cementitious materials content. Paste specimens were each about 350 g [0.8 lb], tested in 51 × 102 mm [2] × 4 in.] non-insulated specimen containers. Admixture dosage is indicated for each thermal profile, and corresponding 1-day compressive strengths of the same specimens are shown in the inset bar chart. Note that effects of dosage increases up to 260 mL/100 kg [4 oz/100 lb] appear to result only in expected retardation, but sulfate imbalance indication is evident at the

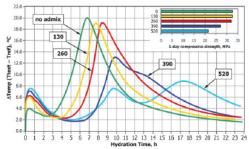


FIG. X1.12 Effects of Admixture Dosage (as Indicated, in mL/100 kg of Total Cementitious Materials) for 32°C Paste Mixtures Made with 25% Class C Fly Ash Replacement of Cement (Note: 1 mL/100 kg = 0.0154 oz/100 lb., 1 MPa = 145 psi)

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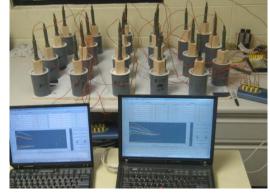
Some equipment variations, manufactured and adapted

















Holcim device: HolcimHeat™

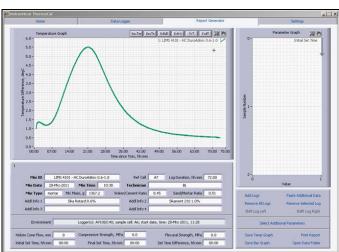
HolcimHeat[™] stands for: HolcimHeat-evolution-analysis-tool

HolcimHeat[™] – application-based testing:

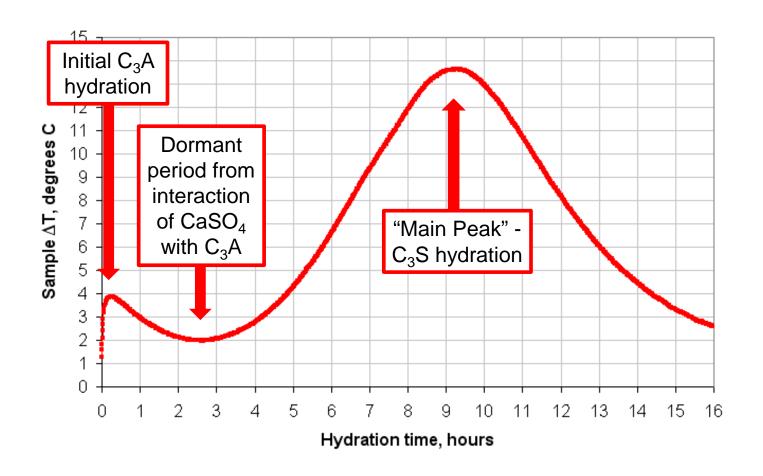
- Investigation of admixture incompatibility issues at cement and RMX plants
- Detection of cement reactivity issues (sulfate balance, reactivity, heat evolution, etc.)
- Study of workability and setting issues related to delayed or accelerated hydration reactions
- Enhanced technical services



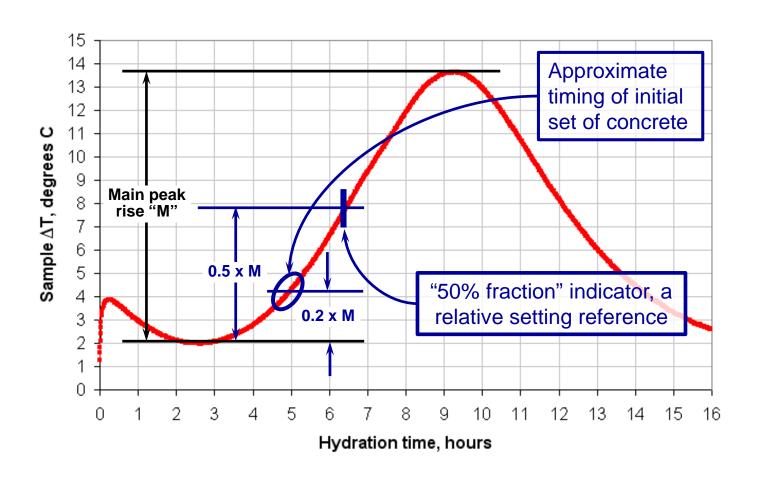




Hydration and thermal profile key features

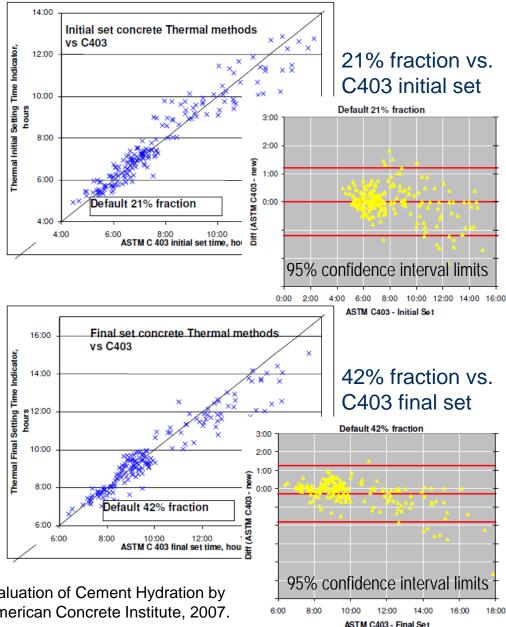


Hydration and thermal profile time of set indications



Comparisons for concrete – time of set by thermal methods vs. C403

 Fractions of 21% and 42% have found to be good default values for using thermal testing to estimate C403 times (initial and final set)



From: Sandberg and Liberman, "Monitoring and Evaluation of Cement Hydration by Semi-Adiabatic Field Calorimetry," ACI SP-241-2, American Concrete Institute, 2007.

Comparisons for concrete – time of set by thermal methods vs. C403

- New standard under development in ASTM C09.23
- Will be an application of C1573 for setting of concrete as an alternative to C403



Standard Test Method for Using Temperature as a Relative Indication of Time of

Set of Cementitious Mixtures

1. Scope

- 1.1. This method is quick method to get a relative comparison of time of set of cementitious materials under similar curing conditions by temperature measurements. This relative difference in time to achieve a fraction of the temperature rise is an indicator of relative time of setting. This method is not meant to match or replace the C403 method or any other penetration resistance method. It can be applied to compare a limited number of scenarios where the microstructural aspects remain relatively constant, namely when comparing different binder combinations and admixture dosage rates.
- 1.2. Units The values stated in SI units are to be regarded as standard. No other units of measurement are included in this standard.



Kinetics of both *normal* and *abnormal* hydration are studied for evaluation of materials & proportions, etc.

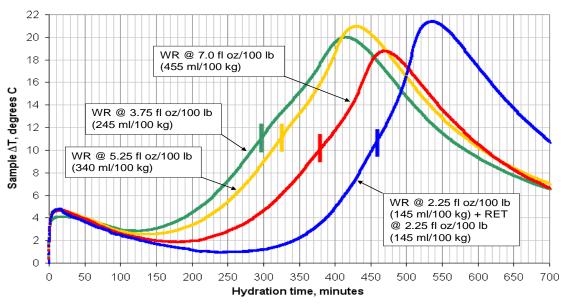
Normal hydration -

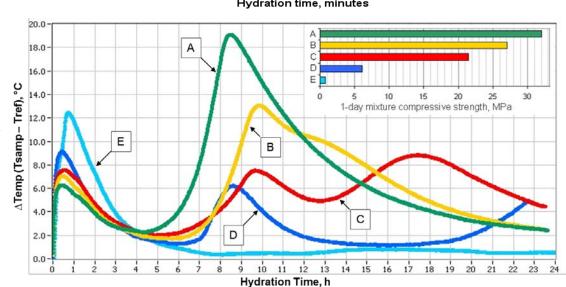
Series of otherwise identical mixtures comparing admixtures and dosages for retardation effects, robustness of hydration



Abnormal hydration -

Series of mixtures with varying admix dosage or fly ash replacement driving varying stages of incompatibility (sulfate depletion) effects, with corresponding 1-day strengths





Lab paste mixture batches – equipment and procedures



















- Paste batching with two technicians:
 - A batch every 4 to 6 minutes
 - 48 or more batches in a morning

Examples / Applications

Examples

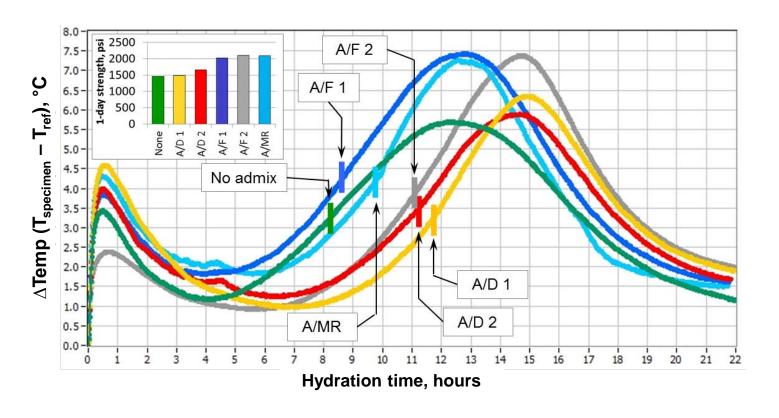
Mix plan for a paste testing series

- 3 cements
 - 1 C150 Type II
 - 2 C595 Type IL
- 3 SCMs
 - C618 Class C
 - C618 Class F
 - C989 slag cement
 - No SCM controls
 - 4 replacement rates
- 42 mixtures total
- 3 specimens each mix
 - Strengths: 1, 7, 28 days
 - Thermal set

Paste thermal profile & strength testing, lab ambient, 2" x 4" cylinders								10/9/2013				
2x4	Mix ID	total	w/cm	cement		SCM			water	Glen.	7500	data start
Channel	MIX ID	cem(g)		source / ID	wt (g)	material	%	wt (g)	(g)	oz/cwt	ml	time
A1	A1 OPC-NoAdm-NoSCM	720	0.51	TH I/II	720				367.2			
A2	A2 OPC-7500-NoSCM	850	0.43	TH I/II	850				372.7	5	2.76	
А3	A3 OPC-20%Cash	850	0.43	TH I/II	680	C ash	20%	170	372.7	5	2.76	
A4	A4 OPC-30%Cash	850	0.43	TH I/II	595	C ash	30%	255	372.7	5	2.76	
A5	A5 OPC-40%Cash	850	0.43	TH I/II	510	C ash	40%	340	372.7	5	2.76	
A6	A6 OPC-50%Cash	850	0.43	TH I/II	425	C ash	50%	425	372.7	5	2.76	
A7	A7 OPC-20%Fash	850	0.43	TH I/II	680	Fash	20%	170	372.7	5	2.76	
A8 A8 73 degrees F reference - sand + water												
B1	B1 OPC-30%Fash	850	0.43	TH I/II	595	F ash	30%	255	372.7	5	2.76	
B2	B2 OPC-40%Fash	850	0.43	TH I/II	510	F ash	40%	340	372.7	5	2.76	
B3	B3 OPC-50%Fash	850	0.43	TH I/II	425	F ash	50%	425	372.7	5	2.76	***************************************
B4	B4 OPC-40%slag	850	0.43	TH I/II	510	GranCem	40%	340	372.7	5	2.76	
B5	B5 OPC-50%slag	850	0.43	TH I/II	425	GranCem	50%	425	372.7	5	2.76	
B6	B6 bad channel											XXX
В7	B7 OPC-60%slag	850	0.43	TH I/II	340	GranCem	60%	510	372.7	5	2.76	
B8	B8 OPC-70%slag	850	0.43	TH I/II	255	GranCem	70%	595	372.7	5	2.76	
C1	C1 PLC1-NoAdm-NoSCM	720	0.51	TH GU nrm	720				367.2			
C2	C2 PLC1-7500-NoSCM	850	0.43	TH GU nrm	850				372.7	5	2.76	
C3	C3 PLC1-20%Cash	850	0.43	TH GU nrm	680	C ash	20%	170	372.7	5	2.76	
C4	C4 bad channel											XXX
C5	C5 PLC1-30%Cash	850	0.43	TH GU nrm	595	C ash	30%	255	372.7	5	2.76	
C6	C6 PLC1-40%Cash	850	0.43	TH GU nrm	510	C ash	40%	340	372.7	5	2.76	
C7	C7 PLC1-50%Cash	850	0.43	TH GU nrm	425	C ash	50%	425	372.7	5	2.76	
C8	C8 73 degrees F reference - sar	nd + wat	er									
D1	D1 PLC1-20%Fash	850	0.43	TH GU nrm	680	F ash	20%	170	372.7	5	2.76	
D2	D2 PLC1-30%Fash	850	0.43	TH GU nrm	595	F ash	30%	255	372.7	5	2.76	
D3	D3 PLC1-40%Fash	850	0.43	TH GU nrm	510	F ash	40%	340	372.7	5	2.76	
D4	D4 PLC1-50%Fash	850	0.43	TH GU nrm	425	F ash	50%	425	372.7	5	2.76	***************************************
D5	D5 PLC-40%slag	850	0.43	TH GU nrm	510	GranCem	40%	340	372.7	5	2.76	
D6	D6 PLC-50%slag	850	0.43	TH GU nrm	425	GranCem	50%	425	372.7	5	2.76	
D7	D7 PLC-60%slag	850	0.43	TH GU nrm	340	GranCem	60%	510	372.7	5	2.76	
D8	D8 PLC-70%slag	850	0.43	TH GU nrm	255	GranCem	70%	595	372.7	5	2.76	
E1	E1 PLC2-NoAdm-NoSCM	720	0.51	TH GU fine	720				367.2			
E2	E2 PLC2-7500-NoSCM	850	0.43	TH GU fine	850				372.7	5	2.76	
E3	E3 PLC2-20%Cash	850	0.43	TH GU fine	680	C ash	20%	170	372.7	5	2.76	
E4	E4 PLC2-30%Cash	850	0.43	TH GU fine	595	C ash	30%	255	372.7	5	2.76	
E5	E5 PLC2-40%Cash	850	0.43	TH GU fine	510	C ash	40%	340	372.7	5	2.76	***************************************
E6	E6 PLC2-50%Cash	850	0.43	TH GU fine	425	C ash	50%	425	372.7	5	2.76	***************************************
E7	E7 PLC2-20%Fash	850	0.43	TH GU fine	680	F ash	20%	170	372.7	5	2.76	
E8	E8 E8 73 degrees F reference - sand + water		er		•••••		***************************************	***************************************	***************************************	***************************************		***************************************
F1	F1 PLC2-30%Fash	850	0.43	TH GU fine	595	F ash	30%	255	372.7	5	2.76	
F2	F2 PLC2-40%Fash	850	0.43	TH GU fine	510	F ash	40%	340	372.7	5	2.76	
F3	F3 PLC2-50%Fash	850	0.43	TH GU fine	425	Fash	50%	425	372.7	5	2.76	
F4	F4 PLC2-40%slag	850	0.43	TH GU fine	510	GranCem	40%	340	372.7	5	2.76	
F5	F5 PLC2-50%slag	850	0.43	TH GU fine	425	GranCem	50%	425	372.7	5	2.76	
F6	F6 PLC2-60%slag	850	0.43	TH GU fine	340	GranCem	60%	510	372.7	5	2.76	
F7	F7 PLC2-70%slag	850	0.43	TH GU fine	255	GranCem	70%	595	372.7	5	2.76	
F8												
. •												

Screening of WR admixtures for a 30% Class C ash mix

5 different WR admixtures (2 Type A/D, 2 Type A/F, 1 MR) compared in paste mixtures, dosages selected for approx. 6% water reduction

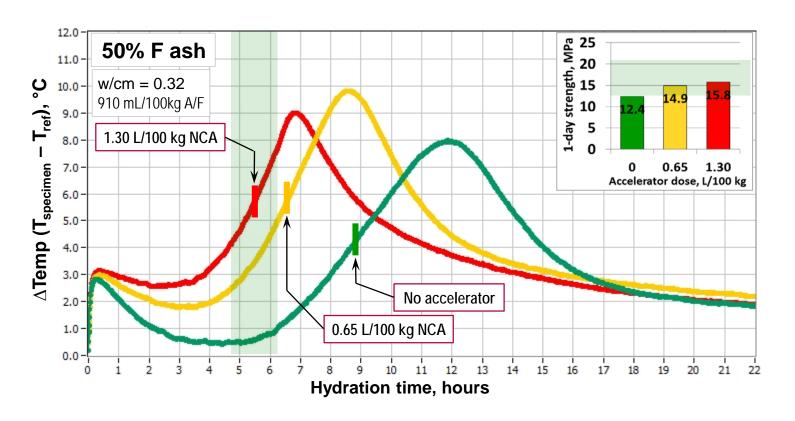


"A/F 1" causes the least retardation, with good early strength influence.

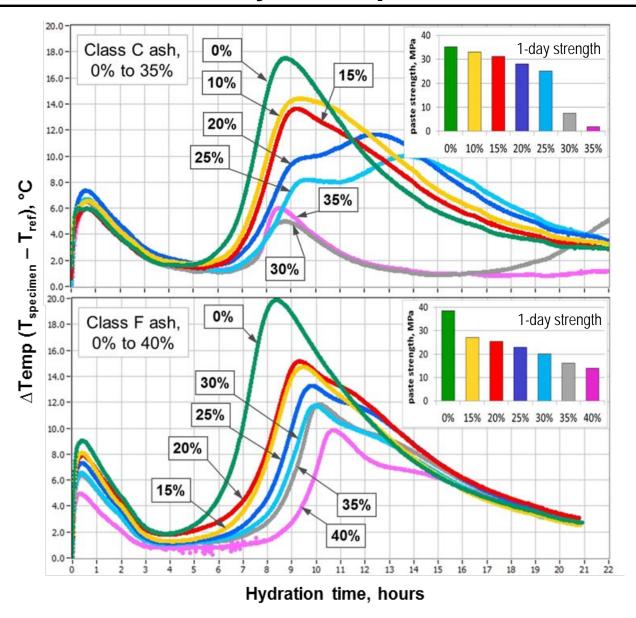


Initial NCA dosage rate for a HVFA mixture

HVFA paste mixtures with incremental NCA dosages compared against target performance ranges established for traditional mixtures



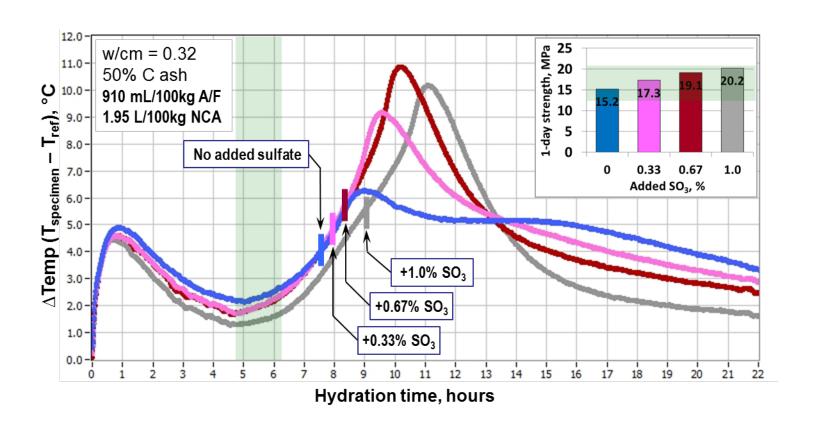
Effects of incremental fly ash replacement, C vs. F ash





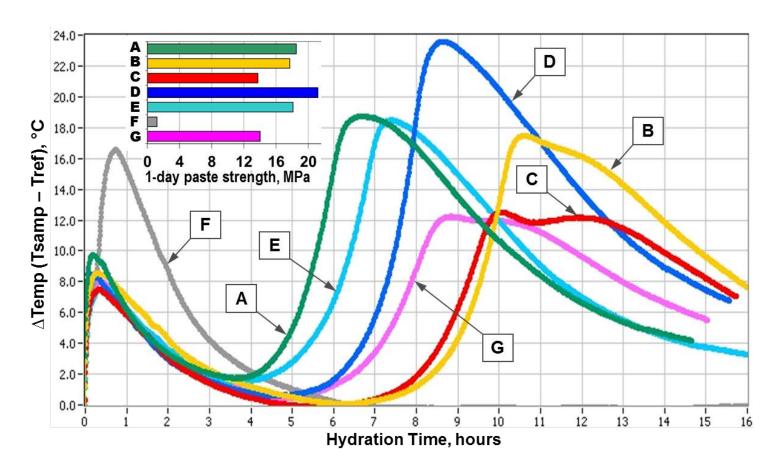
Verification of sulfate balance issue via sulfate addition

Otherwise identical paste mixtures with incremental addition of calcium sulfate



Examples

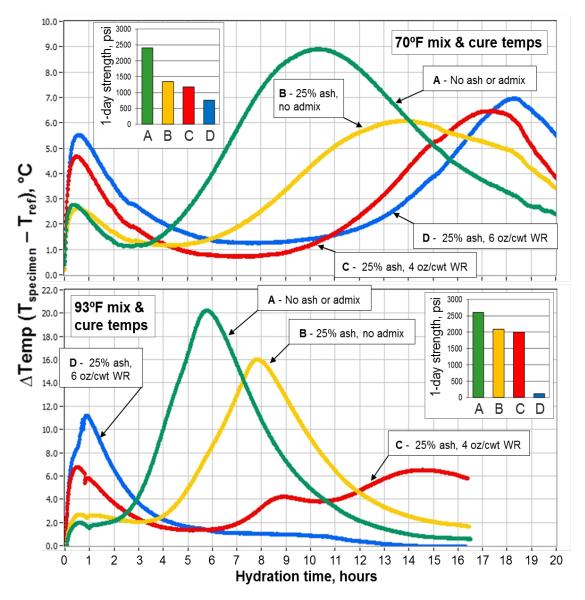
Comparison of 7 cements in an aggressive mix design



Otherwise identical paste mixtures comparing 7 cements, with 25% Class C fly ash, upper-limit dose of Type A/D WR, and 35°C (95°F) mix and cure temps



Temperature influences on incompatibility potential

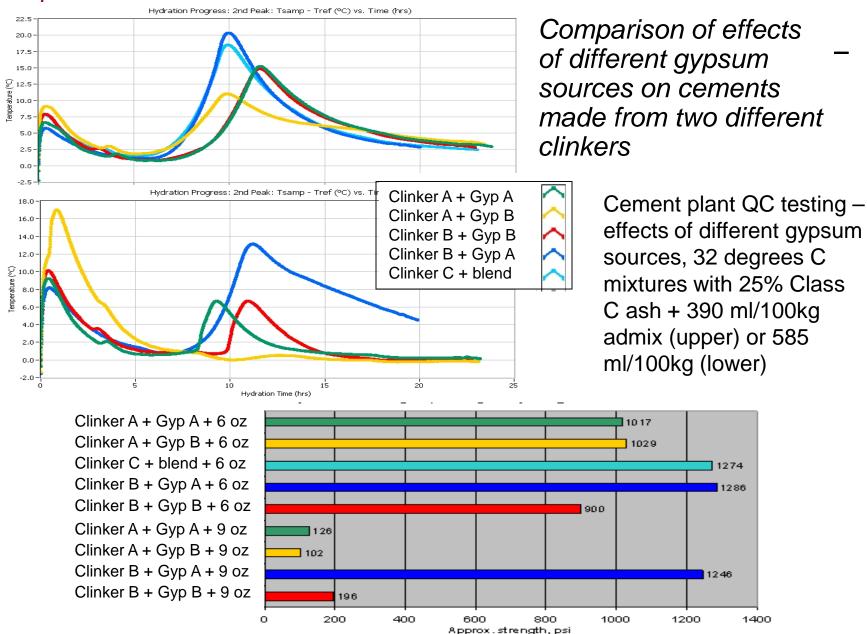


4 identical paste mixtures compared at different initial mixture and curing temps (70°F (21°C) vs. 93°F (34°C)

- A) 100% OPC, no WR, w'c = 0.45
- B) 25% C ash, no WR w/cm = 0.45
- C) 25% C ash, 4 oz/cwt WR, w/cm = 0.40
- D) 25% C ash, 6 oz/cwt WR, w/cm = 0.40

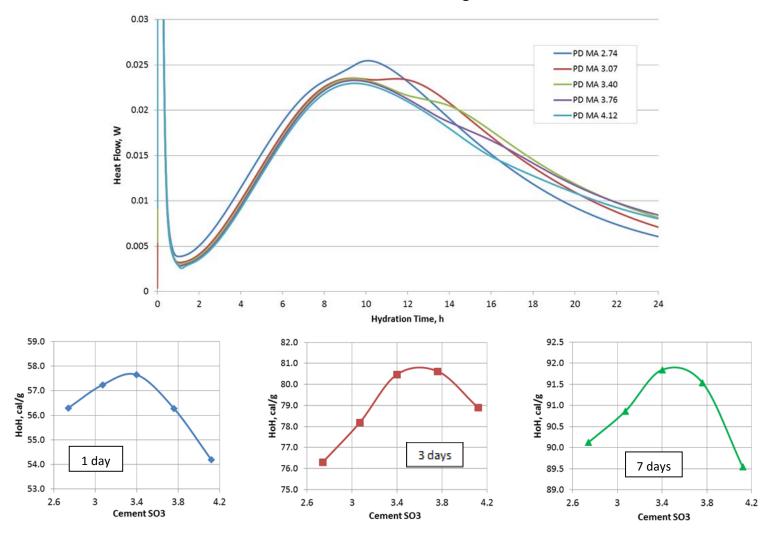
Higher temps alone drive incompatible behavior in the mixtures with WRA

Examples





Optimum SO₃ via ASTM C1702 on mill-ground cement samples cement with varying SO₃ contents



Published papers using (or about) thermal testing

- Cost, V. T., "Incompatibility of Common Concrete Materials Influential Factors, Effects, and Prevention," National Concrete Bridge Conference 2006, National Concrete Bridge Council, Skokie, IL, 2006.
- Cost, V. T., and Knight, G., "Use of Thermal Measurements to Detect Potential Incom-patibilities of Common Concrete Materials,"
 Concrete Heat Development: Monitoring, Prediction, and Management, ACI SP-241-4, Atlanta, GA, April 2007, pp 39-58.
- Cost, V. T., and Gardiner, A., "Practical Concrete Mixture Evaluation via Semi-Adiabatic Calorimetry," 2009 Concrete Technology Forum – Focus on Perfor-mance Prediction, Cincinnati, OH, May 2009, 21 pp.
- Cost, Tim, "Thermal Measurements of Hydrating Concrete Mixtures A Useful Quality Control Tool for Concrete Producers,"
 NRMCA Publication 2PE004, National Ready Mixed Concrete Association, 900 Spring Street, Silver Spring, MD, August 2009.
- Cost, V. T., "Concrete Sustainability versus Constructability Closing the Gap," 2011 International Concrete Sustainability Conference, Boston, MA, August, 2011.
- Cost, Tim, "Preliminary Optimization of Concrete Paving Mixtures for Sustainability and Performance," 10th International Conference on Concrete Pavements, Quebec City, Quebec, July 8-12, 2012, 11 pp.
- Sullivan, G., Cost, T., and Howard, I., "Measurement of Cementitiously Stabilized Soil Slurry Thermal Profiles," Geo-Congress 2012 – State of the Art and Practice in Geotechnical Engineering, American Society of Civil Engineers, Oakland, CA, March 25-29, 2012.
- Cost, V. T., and Bohme, P., "Synergies of Portland-Limestone Cements and Their Potential for Concrete Performance Enhancement," 2012 International Concrete Sustainability Conference, Seattle, WA, May 7-10, 2012, 14 pp.
- Cost, V. T., Howard, I. L., and Shannon, J., "Improving Concrete Sustainability and Performance with Use of Portland-Limestone Cement Synergies," Transportation Research Record: Journal of the Transportation Research Board, No. 2342, Washington, D.C., 2013, pp 26-34.
- Howard, I.L., Cost, T. (2014). "Curing Temperature Effects on Soils Stabilized With Portland Cement Having Different Sulfate Contents," Proc. of GeoCongress 2014 (GSP 234), Feb 23-26, Atlanta, GA, pp. 2159-2168.
- Cost, V. T., Matschei, T., Shannon, J., and Howard, I. L., "Extending the Use of Fly Ash and Slag Cement in Concrete Through the Use of Portland-Limestone Cement," 2014 International Concrete Sustainability Conference, Boston, MA, May 12-14, 2014, 15 pp.
- Howard, I.L., Sullivan, W.G., Anderson, B.K., Shannon, J., Cost, T. (2013). Design and Construction Control Guidance for Chemically Stabilized Pavement Base Layers. Report FHWA/MS-DOT-RD-13-206, Mississippi Department of Transportation, pp. 162.

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Questions?

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