

**Finite Element Analysis of Rock Deformation in Deep Twin Tunnels**

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**Abstract.** Relying upon a three-dimensional finite element analysis, this contribution investigates the instantaneous irreversible response induced by the constitutive behavior of the rock mass in the convergence profile of twin tunnels with transverse gallery. At the rock material level, elastoplastic state equations based on a Drucker-Prager yield surface with an associated flow rule are adopted in the modeling. As regards the tunnel support, the formulation accounts for the presence of an elastic shotcrete-like lining. From a computational point of view, the deactivation-activation method is used to simulate the excavation process and the installation of the lining. The accuracy of the finite element predictions is assessed through comparisons with the available analytical solutions formulated in a simplified scenario for the twin tunnel configuration. A parametric study investigates the mutual interaction induced by the proximity of the tunnels and the influence of the lining stiffness.

**Keywords:** Twin tunnels, Transverse gallery, Elastoplasticity, Finite element modeling

# Introduction

Many design methods often focus on single tunnels, however twin tunnels are a common occurrence. The interaction between tunnels can be significant, especially when the spacing between them is minimal. Additionally, many twin tunnels incorporate transverse galleries, introducing a localized effect on displacements and stresses. While the simulation of tunnel convergence in single tunnels has been widely investigated and reported in published literature, few works have addressed the computational evaluation of deformation in twin tunnels. Some studies on deep twin tunnels can be found at , , , , , , , , . However, less attention has been dedicated to assessing the mutual mechanical interaction induced by the excavation of the transverse gallery connecting the twin tunnels.

The constitutive state equations of the rock mass are developed using a plasticity framework, which is suitable for clayey rocks. For the mechanical behavior of the concrete lining, the traditional linear elastic model is employed. The constitutive models for the rock mass and the associated numerical integration schemes are implemented into the UPF/USERMAT customization tool [] of the ANSYS standard software. The main contributions of this paper can be summarized at the structural analysis level. The deformation of the highly interactive components of the material system (i.e., rock mass and lining) resulting from the excavation of twin tunnels and transverse gallery is simulated using three-dimensional finite element simulations. The excavation and lining placement processes are simulated through the activation/deactivation technique. This three-dimensional finite element analysis is specifically designed to address the interactions induced by the construction process, the proximity of twin tunnels, and the presence of the transverse gallery. For the constitutive parameters and tunneling conditions adopted, the stiffness of the lining had a significant impact on the convergence profile. In addition, the interaction between the longitudinal tunnels became significant at a distance of four radii between the tunnel axis, however, this distance had minimal impact on the extent of the disturbance zone caused by the gallery over the longitudinal tunnel.

# Constitutive Models

The constitutive model for the rock mass corresponds to the associated Drucker-Prager elastoplastic model. The local strain rate is split into two contributions , so that the constitutive relationships relating the Cauchy stress rate and strain rate components can be written as:

In the above relationship, and , represent respectively the elastic and plastic strain rate, and denote the fourth-order isotropic elastic linear constitutive tensor defined by the rock mass elastic Young modulus and Poisson ratio . The plastic strain rate is given by the flow rule:

$$\label{eq\_plastic\_flow}
\dot \boldsymbol{\varepsilon}^{p}= \left\{
\begin{array}{ll}
\dot \lambda \dfrac{\partial g}{\partial \boldsymbol{\sigma}} & \text{for } f > 0 \\
\boldsymbol 0, & \text{for } f \le 0 \\
\end{array}\right.$$

where is the plastic flow surface, is the plasticity multiplier (obtained through the consistency condition ) and is a potential flow function analogous to used to simulate the volume dilatation during the evolution of plastic deformations. However, for this analysis, associated plasticity was adopted, i.e., . In this model, the Drucker-Prager plastic flow surface is employed and given by

which is the first invariant of the stress tensor, is the second invariant of the deviator tensor and and are strength parameters related to the friction angle and cohesion , respectively. The Drucker-Prager plasticity surface inscribed in the Mohr-Coulomb surface is considered, i.e. []:

where . The internal variable is the equivalent plastic strain used to simulate strain hardening/softening phenomena. However, for this study, we adopt perfect plasticity, meaning that c is a constant.

A linear elastic constitutive model is used for the concrete lining, which can be expressed, within the framework of infinitesimal analysis, as , where, and are respectively the elastic strain rate and the fourth-order isotropic elastic constitutive tensor defined by the concrete lining Poisson ratio and elastic Young modulus . In the analyses, for comparisons, the tunnel lining stiffness will be given by the following expression []:

where is the tunnel radius and is the tunnel wall thickness.

# Spatial discretization of the domain

The material domain for finite element simulations is defined as a parallelepiped with dimensions (see [1](#Mesh1)). Due to symmetry, only the material portion in the region is discretized for F.E. analysis. In [1](#Mesh1), represents the distance between the axes of longitudinal tunnels, is the total excavated length along longitudinal direction , is the domain thickness along vertical direction , is the length of the unexcavated region after tunneling, is the domain length along transversal direction , and indicates the position of the transverse gallery axis that intersects the longitudinal tunnel at .

The mesh used consists of either or total elements (hexahedra and tetrahedra), depending on the longitudinal tunnel spacing . To enhance model accuracy in the intersection zone, 10-node quadratic tetrahedral elements are used around the transverse gallery, while 8-node trilinear hexahedral elements are employed in the rest of the domain. Regions significantly influenced by tunneling are highlighted in light gray in [1](#Mesh1). Two values of are considered in the parametric simulations: and .

The concrete lining along the gallery wall, shown in red in [1](#Mesh1), has a thickness . The gallery radius is fixed at for simplicity, with the same lining system (same concrete material and layer thickness) applied to both longitudinal tunnels. Parameters and define the size of the transition region involving tetrahedral elements in the plane around the gallery.

The initial stress state prevailing in the rock mass prior to the tunnel excavation process is defined by constant vertical and horizontal geostatic stress and taking the following form:

![](data:application/pdf;base64,)

Geometry, mesh and boundary conditions of domain and details of a) longitudinal tunnel cross-section for configuration and gallery cross-section for configurations b) and c) .

As mentioned previously, the tunneling process, including the excavation steps and lining installation, is simulated by resorting to the activation-deactivation method shown in the schematic representation in [2](#Diagram of excavations). Each excavation step is modeled by deactivation of the corresponding elements (the elements stiffness is reduced by a factor ), whereas installation of elements of lining at a distance from the excavation face (unlined length) is achieved through activation of the corresponding elements by assigning them concrete properties. In this figure, is the total number of excavation steps and represents the number of longitudinal tunnel excavation steps before gallery excavation. After achievement of the excavation steps, the excavation of the gallery is initiated starting from the longitudinal tunnel wall. Referring to the notation of [2](#Diagram of excavations), is the considered step length for the gallery excavation, and is the unlined length of the gallery. After the gallery excavation is completed, we proceed to further excavation steps of the longitudinal tunnel. The main parameters defining the geometry domain as well as the excavation process and lining installation are summarized in Table [1](#table1).

![](data:application/pdf;base64,)

Schematic representation of the excavation process.

Parameters related to the geometry of the domain, excavation, and installation of the lining.

| PARAMETERS | SYMBOL | UNIT | VALUES |
| --- | --- | --- | --- |
| Longitudinal tunnels | | | |
| Radius of the longitudinal tunnel |  | m |  |
| Thickness of the lining |  | m | , |
| Length of the excavation step |  | m |  |
| Unlined length |  | m |  |
| Gallery | | | |
| Radius of the gallery |  | m |  |
| Thickness of the lining |  | m |  |
| Length of the excavation step |  | m |  |
| Unlined length |  | m |  |
| Number of steps that starts gallery excavation |  | un |  |
| Rest of domain | | | |
| Distance between longitudinal tunnel axes |  | m |  |
| Total length along vertical direction |  | m |  |
| Length of unexcavated region |  | m |  |
| Total excavated length |  | m |  |
| Total length along transversal direction |  | m |  |

# Verification with unlined twin tunnel in elastoplastic medium

In the context of plane strain conditions, developed an approximate analytical solution for the stresses and the plastic zone boundary around deep twin circular tunnels excavated in a homogeneous elastoplastic medium. For the constitutive model, the authors considered perfectly plastic Mohr-Coulomb criterion with associated plastic flow rule. The stress solution for twin tunnels was formulated on the premise that the plastic zone around each tunnel fully encloses the tunnel edge, with the two plastic zones remaining separate and unconnected.

[3](#MA_FIG1) shows the comparison between the 3D F.E. Solution (from a far behind the excavation face) and the analytical solution for plastic zone boundary provided in []. For these analysis, m, , rock Young’s modulus GPa, Poisson’s ratio and, friction angle . This analysis shows that finite element modeling produces predictions very similar with those shown in [3](#MA_FIG1). In addition, the results show that lower values of cohesion result in larger plastic zones.

![](data:application/pdf;base64,)

The plastic zone extent obtained from the present F.E. simulations and from the stress solution provided in .

Further comparisons are illustrated in Fig. [4](#MA_stresspaths), which shows the radial and orthoradial stress components along three radial paths defined in polar coordinates by , , and . It is important to note that although the finite element simulations use the Drucker-Prager yield surface inscribed within the Mohr-Coulomb surface (as used in the solution by , the numerical predictions closely match the analytical stress solution.

|  |  |
| --- | --- |
|  | . |

Distribution of radial and orthoradial stress components along different radial directions: comparison between numerical and analytical predictions.

# Numerical Results and Discussion

To develop the analysis, we employed Young’s modulus MPa, Poisson ratio , , and, isotropic initial stresses MPa, which correspond to the constitutive parameters and tunneling conditions (450 m depth) in the clay rock mass in the Paris basin (in Aisne, France), as detailed in , and, . For the lining, two stiffness values will be considered: MPa and MPa. Assuming a Young’s modulus MPa and Poisson’s ratio , these values corresponds to lining thicknesses of and .

Denoting by the displacement component following the -axis, [5](#EP_d1_16Ri) and [6](#EP_d1_4Ri) displays the convergence curves that characterize the inward movement at the tunnel roof as a function of normalized longitudinal distance to the facing for different conditions: without lining (NL), with elastic lining (EL), with (WG) and without gallery (NG) for and . In these figures, represents convergence at (far from the effect of the excavation face and gallery), and is highlighted at the position where the gallery axis intersects the longitudinal tunnel axis.

For the single tunnel, the higher stiffness lining (black solid line) reduced convergence by approximately 35% compared to the unlined scenario (black dashed line). Conversely, the moderately stiff lining (black dotted line) increased convergence by 12% compared to the higher stiffness lining.

When (blue and yellow lines), the results of are similar to the isolated tunnel (black line). However, with a distance reduced to , the interaction between the tunnels becomes significant. A smaller , the higher stiffness lining (yellow and blue solid lines) can restrict convergence by up to 46% of the unlined (yellow and blue dashed lines) convergence. A moderate stiffness lining (dotted lines) leads to an increase of up to 16% in convergence compared to the higher stiffness lining (solid lines).

|  |  |
| --- | --- |
|  | . |

Convergence Profiles at the tunnel roof (point B) - for .

|  |  |
| --- | --- |
|  | . |

Convergence Profiles at the tunnel roof (point B) - for .

When comparing between twin tunnels with spacings of and , differences of 6% with higher stiffness lining (yellow and blue solid lines), 10% with moderate stiffness lining (yellow and blue dotted lines), and 30% without lining (yellow and blue dashed lines) are observed. These results show the direct impact of lining stiffness and the distance between twin tunnels on convergence.

When analyzing the convergence at the point where the gallery axis meets the longitudinal tunnel axis, there is an increase of 16% when using an moderate stiffness lining (blue dotted line) compared to a higher stiffness lining (blue solid line), for both distances . However, when analyzing the difference between the and , there is a difference of up to 12% for the higher stiffness lining (blue solid line to and ) and up to 13% for the moderate stiffness lining (blue dotted line to and ) for . In both figures it can be seen that the increase in stiffness reduces the extent of the disturbed region caused by the gallery in the longitudinal tunnel convergence profile. The range decreases from (without lining) to and (with lining). Additionally, the proximity of the tunnel has a minimal impact on the length of this disturbed zone.

# Conclusions

Considering the constitutive parameters and tunneling conditions adopted, the analyses show that the lining has a profound impact on the convergence profile of the twin tunnels. It reduces overall convergence by up to 35% and diminishes the localized convergence of the gallery by approximately a third compared to unlined scenario. In addition, a less rigid lining, approximately 3.5 times less stiff, increases convergence by 12% and expands the localized effect by 40% compared to the stiffer lining. Tunnel interaction becomes significant at however has minimal impact in the range of gallery’s localized effect along the longitudinal tunnel.

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