

# Grid based Rainfall-runoff Model User's Manual

---

Yun Seok Choi, Kyung Tak Kim

Department of Hydro Science and Engineering Research  
Korea Institute of Civil Engineering and Building Technology

**v.GRM2021.04**  
**2022.03**



KOREA INSTITUTE of CIVIL ENGINEERING  
and BUILDING TECHNOLOGY



## *History*

The Grid-based Rainfall-Runoff Model (GRM) is a grid-based and physically based distributed rainfall-runoff model developed by the Korea Institute of Civil Engineering and Building Technology (KICT). The development history of the GRM is as follows.

- ▶ The GRM was first developed in 2008. It could simulate overland flow, channel flow, subsurface flow, and controlled flow by stream facilities. The model was developed as HyGIS-GRM, an add-on for Hydro Geographic Information System (HyGIS).
- ▶ In 2010, the base flow simulation was added. Moreover, a real-time flow analysis module using real-time rainfall data was developed.
- ▶ In 2012, a multi-site calibration method for single watersheds was developed.
- ▶ In 2014, infiltration, subsurface flow, and base flow simulation methods were improved and the MW-GRM plug-in that could run on MapWindow GIS (v.4.8.8), an open source GIS, was developed.
- ▶ In 2015, the sensitivity of the calculation time step (dt) was improved. Moreover, the real-time flow analysis module was improved to allow simulations of the real-time flow control. A multi-site calibration method that allowed coupled analysis of multiple watershed systems was developed.
- ▶ In 2017, the GRM was separated from GIS and graphic user interfaces (GUIs) and was developed into an independent executable file. In addition, a parallel computation method using .NET and a method that could change the calculation time step (dt) during simulation were adopted.
- ▶ In 2018, the GUI of the GRM model was developed as QGIS plug-in (QGIS-GRM). The GRM model codes were converted from Visual Basic .NET into C#.
- ▶ In 2020, the model was improved to enable setting of multiple channel information. An option for real-time simulation was added. The GRM model using C++ was developed.
- ▶ In 2021, the model was improved to enable more detailed simulation for reservoir operation.

The GRM is continuously being developed by the KICT.

April 2021.



## Table of Contents

<b>1. Overview of the GRM .....</b>	<b>1</b>
1.1 Model structure .....	1
1.2 Surface flow .....	2
1.3 Infiltration .....	4
1.4 Subsurface flow .....	4
1.5 Base flow .....	5
1.6 Discretization of governing equations .....	7
1.7 Flow control .....	9
1.8 Calculation time step .....	11
<b>2. Model parameters .....</b>	<b>12</b>
2.1 Soil parameters .....	13
2.2 Land cover parameters .....	15
2.3 Channel base width .....	17
2.4 Initial saturation ratio .....	18
2.5 Minimum slope .....	19
2.6 Channel roughness coefficient .....	19
2.7 Dry stream order .....	20
2.8 Parameter calibration coefficients .....	21
2.9 Parameter estimation .....	21
<b>3. Multi-site calibration .....</b>	<b>22</b>
3.1 Single watershed multi-site calibration .....	23
3.2 Subwatersheds connecting multi-site calibration .....	24
<b>4. Input data .....</b>	<b>26</b>
4.1 Spatial data .....	27
4.2 Hydrological data .....	29
4.3 GRM project file .....	31

4.4 GRM Static database .....	40
<b>5. Output data .....</b>	<b>43</b>
<b>6. GRM Real Time .....</b>	<b>44</b>
<b>7. Prediction .....</b>	<b>47</b>
<b>References .....</b>	<b>49</b>
<b>Appendix .....</b>	<b>53</b>
A. Example of a GRM project file (with one watershed and one watch point) .....	53
B. Example of a discharge output file (with one watch point) .....	59
C. Example of a GRM project file (with multiple watersheds and watch points, and a flow control) ....	60
D. Example of a discharge output file (with multiple watch points) .....	69
E. Download .....	70
F. How to Run the GRM (console window) .....	72
G. How to Run the GRM (GRMAnalyzer) .....	74

# 1. Overview of the GRM

## 1.1 Model structure

The Grid-based Rainfall–Runoff Model (GRM) is a physically based distributed rainfall–runoff model used for simulating rainfall–runoff events. A kinematic wave model is used to analyze overland and channel flow and the Green–Ampt model is used to calculate infiltration. The finite volume method is used to discretize governing equations and the Newton–Raphson method is used to derive converging solutions for nonlinear terms (Choi, 2010).

Surface flow is divided into overland flow and channel flow, while direct runoff comprises overland flow and subsurface flow. Overland flow is caused by infiltration excess flow (Horton, 1933) and saturation excess flow (Dunne and Black, 1970). The infiltration process and subsurface flow are simulated in the soil water zones (Bras, 1990).

$x$  is the flow direction,  $y$  is the direction perpendicular to the flow of horizontal components, and  $h$  is the water depth. Hydrological components flowing into a control volume consist of surface inflow from the upper part of the control volume, lateral inflow, and rainfall. Hydrological components flowing out of a control volume consist of surface outflow and infiltration. Here, infiltration contributes to subsurface outflow. This subsurface outflow becomes a lateral inflow of the downstream control volume if there is a stream. If the downstream cell is a saturated overland flow control volume, subsurface outflow contributes to overland inflow (Beven and Kirkby, 1979).

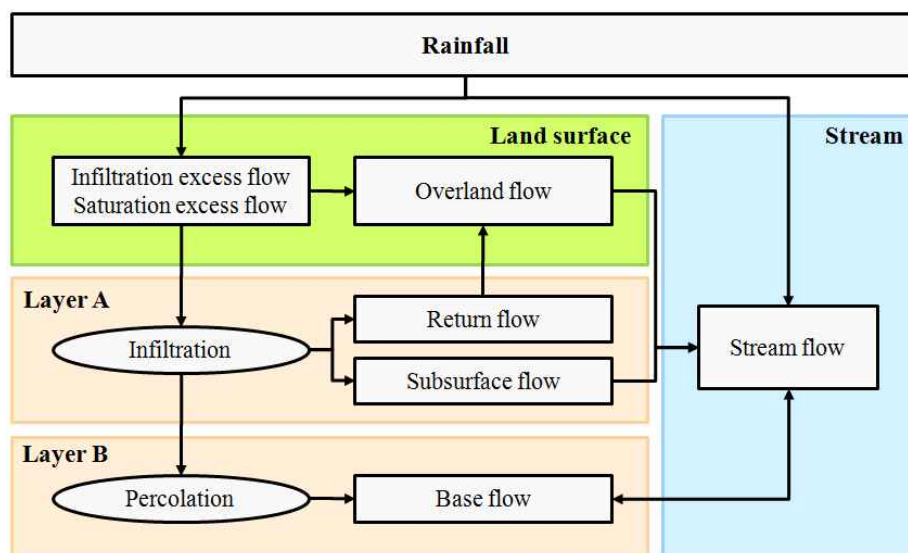


Fig. 1.1 Flow process of hydrological components

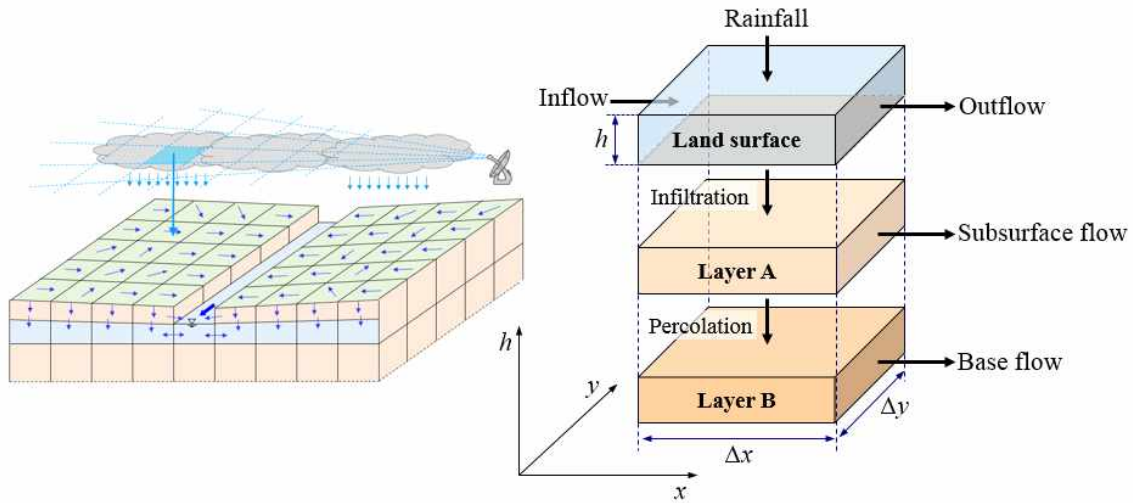


Fig. 1.2 Inflow and outflow of hydrological components within a control volume

## 1.2 Surface flow

Surface flow can be divided into overland flow and channel flow. A kinematic wave equation is used for flow analysis. The kinematic wave equation uses a combination of the continuity equation and momentum equation. Equation (1.2.1) is the continuity equation for overland flow and Eq. (1.2.2) is the equation for channel flow. In Eq. (1.2.2), effective rainfall for a channel is considered as the source term and the lateral inflow includes subsurface flow, base flow, and overland flow parts in channel flow cells (Choi, 2010). The momentum equation for the kinematic wave model is defined by Eq. (1.2.3).

$$\frac{\partial h}{\partial t} + \frac{\partial q}{\partial x} = r - f + \frac{q_r}{\Delta y} \quad (1.2.1)$$

$$\frac{\partial A}{\partial t} + \frac{\partial Q}{\partial x} = r \Delta y + q_L + q_{ss} + q_b \quad (1.2.2)$$

$$S_0 = S_f \quad (1.2.3)$$

Here,  $q$  is the flow per unit width ( $q = uh$ ),  $u$  is the flow velocity in  $x$  direction,  $r$  is the rainfall intensity,  $f$  is the infiltration rate,  $q_r$  is the return flow,  $A$  is the cross-sectional flow area perpendicular to the  $x$  direction,  $Q$  is the flow,  $h$  is the water depth,  $q_L$  is the lateral inflow,  $q_{ss}$  is the subsurface flow,  $q_b$  is the base flow, and  $t$  is the time.

From equation (1.2.3), the bed slope can be used instead of the friction slope to



calculate the flow velocity with Manning's flow velocity equation. The water depths is used to approximate the hydraulic radius of overland flow with shallow water depths. An asymmetrical trapezoidal cross section can be applied to the hydraulic radius for channel routing.

$$u = \frac{R^{2/3} S_0^{1/2}}{n} \quad (1.2.4)$$

Here,  $n$  is the roughness coefficient and  $R$  is the hydraulic radius.

$$R = \frac{b_s h}{b_s + 2h} \approx h \quad (1.2.5)$$

Here,  $b_s$  is the control volume width for overland flow ( $b_s \gg h$ )

$$R = \frac{b + \frac{h^2}{2} \left( \frac{1}{SLB} + \frac{1}{SRB} \right)}{b + h \times \sqrt{1 + \frac{1}{SLB^2}} + h \times \sqrt{1 + \frac{1}{SRB^2}}} \quad (1.2.6)$$

Here,  $SLB = h/bLB$ ,  $SRB = h/bRB$ , and  $b$  is the channel base width.

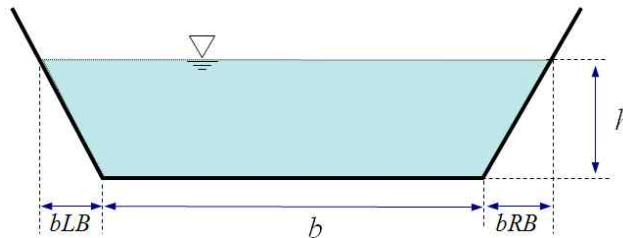


Fig. 1.3 Asymmetrical trapezoidal channel cross section

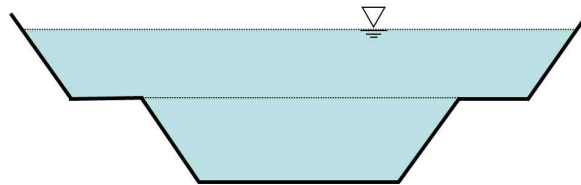


Fig. 1.4 Compound channel cross section

### 1.3 Infiltration

The GRM can simulate infiltration excess flow, which occurs when the rainfall intensity exceeds infiltration rate, and saturation excess flow, which occurs when soil is saturated. The Green-Ampt Model is used to calculate the infiltration. The Green-Ampt Model calculates cumulative infiltration, as shown in Eq. (1.3.1). The infiltration rate is then calculated with Eq. (1.3.2) using the cumulative infiltration.

$$F(t) = Kt + \Delta\theta\psi \ln\left(1 + \frac{F(t)}{\Delta\theta\psi}\right) \quad (1.3.1)$$

$$f(t) = K\left(\frac{\psi\Delta\theta}{F(t)} + 1\right) \quad (1.3.2)$$

Here,  $F(t)$  is the cumulative infiltration at time  $t$ ,  $f(t)$  is the infiltration rate at time  $t$ ,  $\Delta\theta$  is the change in the soil moisture content ( $\Delta\theta = (1 - S_e)\theta_e$ ),  $S_e$  is the effective saturation ( $S_e = (\theta - \theta_r)/(\eta - \theta_r)$ ),  $\theta$  is the moisture content ( $\theta_r \leq \theta \leq \eta$ ),  $\theta_r$  is the residual moisture content ( $\theta_r = \eta - \theta_e$ ),  $\eta$  is the porosity,  $\theta_e$  is the effective porosity,  $\psi$  is the wetting front soil suction head, and  $K$  is the hydraulic conductivity.

### 1.4 Subsurface flow

Beven (1981) applied the kinematic wave model to simulate subsurface flow at saturated conditions. The subsurface flow from the kinematic wave model is based on the assumption that the hydraulic gradient is equal to the land surface gradient within a control volume. Equation (1.4.1) can be used to calculate the subsurface flow.

$$q_{ss} = KD_s \sin(S_a) \quad (1.4.1)$$

Here,  $q_{ss}$  is the subsurface flow,  $D_s$  is the saturated soil depth, and  $S_a$  is the land surface inclination angle.

The simulation of subsurface flow from the kinematic wave model with soil depth  $D$  is shown in the figure below (Choi, 2010). The GRM assumes the hydraulic gradient of subsurface flow within a control volume to be equal to the land surface gradient and simulates the subsurface flow for saturated soil depths. When the soil becomes saturated to a certain depth due to infiltration, subsurface flow contributes as a component of lateral flow into the channel flow. When the soil is completely saturated ( $D_s = D$ ) within a control volume, local return flow and saturation excess flow

contributing to overland flow (Dunne and Black, 1970) are simulated.

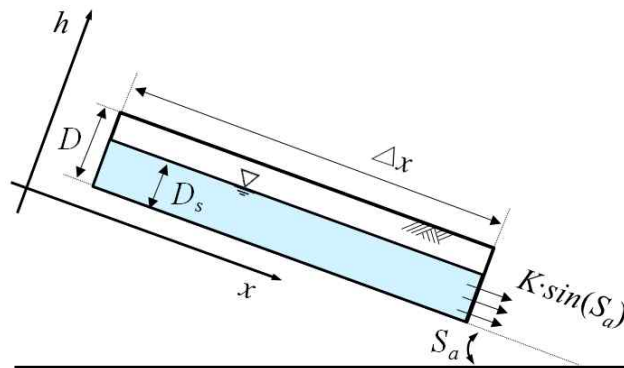


Fig. 1.5 Subsurface flow of a kinematic wave model

Sloan and Moore (1984) suggested the Simple Storage–Discharge Model based on the water budget to calculate flow in an arbitrary segment including completely saturated soil. Equation (1.4.2) shows the saturated excess flow and subsurface flow for completely saturated soil. Each component of the GRM is used to simulate the overland inflow and either lateral inflow or return flow at the downstream control volume. Particularly, when rainfall and runoff occur, the depth of the saturated soil varies. Thus the control volumes in which saturated excess flow and return flow simulated only for completely saturated soil are occurred also vary (Dunne and Black, 1970; Bras, 1990).

$$q_o = i_s L_s + q_s \quad (1.4.2)$$

Here,  $q_o$  is the flow per unit width at the downstream end of an arbitrary segment,  $L_s$  is the length of the land surface segment that is saturated up to the land surface, and  $i_s$  is the inflow perpendicular to the saturated water surface from an external area.

## 1.5 Base flow

The GRM divides soil into two layers. Simulations of the infiltration and subsurface flow are done for the upper layer A and the base flow is simulated for the lower layer B. When layer A is saturated, percolation occurs into layer B. Equation (1.5.1) is used to calculate the percolation depth.

$$p = K_{Bv} \times \Delta t \quad (1.5.1)$$

Here,  $K_{Bv}$  is the vertical hydraulic conductivity of layer B,  $p$  is the percolation during

time  $\Delta t$ .

The horizontal flow in layer B can be calculated using Eq. (1.5.2) based on Darcy's law (Freeze and Cheery, 1979). Here, the head difference of segment  $\Delta x$  is assumed to be equal to the land surface slope when applying Eq. (1.5.3).

$$q_{Bh} = K_{Bh} D_B \frac{dz_B}{dx} \quad (1.5.2)$$

$$q_{Bh} = K_{Bh} D_B \sin(S_a) \quad (1.5.3)$$

Here,  $z_B$  is the water level of layer B,  $K_{Bh}$  is the horizontal hydraulic conductivity of layer B,  $D_B$  is the water depth of layer B, and  $q_{Bh}$  is the horizontal flow per unit width of layer B.

For the flow exchange between an unconfined aquifer and stream, the base flow into the channel is calculated with Eq. (1.5.4) when the water depth of the unconfined aquifer ( $h_B$ ) is deeper than the water depth of the channel ( $h_{ch}$ ). When the water depth of the channel is deeper than the water depth of the unconfined aquifer, Eq. (1.5.5) is used to calculate the flow from the channel into soil layer B.

$$q_b = K_{Bh} \frac{h_B - h_{ch}}{h_{ch}} b \quad (\text{for } h_B > h_{ch}) \quad (1.5.4)$$

$$q_b = K_{Bh} (h_B - h_{ch}) \quad (\text{for } h_B < h_{ch}) \quad (1.5.5)$$

Here,  $h_B$  is the water depth of the unconfined aquifer,  $h_{ch}$  is the water depth of the channel,  $b$  is the channel base width, and  $q_b$  is the base flow per unit length of the control volume.

## 1.6 Discretization of governing equations

The GRM uses the finite volume method for the discretization of governing equations. The definition of a control volume for such a process is shown in the following figure. The control volume number is indicated with subscript  $i$ , the central point of the control volume with  $p$ , the control volume surface in the upstream direction ( $-x$  direction) from the inflow into the control volume with  $w$ , and the control volume surface in the downstream direction ( $x$  direction) with outflow with  $e$  (Patankar, 1980).

Equations (1.6.1) and (1.6.2) show the discretization of equations from integrating the continuity equations for overland flow and channel flow analyses with respect to  $x$  and  $t$  for  $CV_i$ . Converging solutions for nonlinear terms are found from the Newton–Raphson iteration method (Choi, 2010). When overland flow and channel flow must be simulated together from one grid, the difference between the length in direction  $y$  ( $\Delta y$ ) and the channel base width ( $b$ ) is applied to  $\Delta y_i$  which is used for overland flow analysis in Eq. (1.6.1).

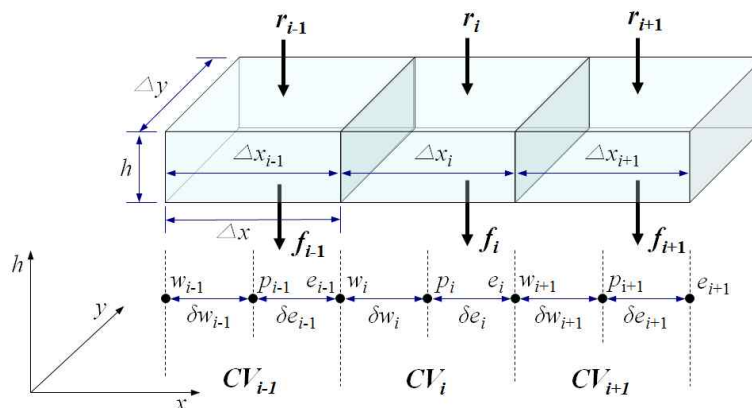


Fig. 1.6 Definition of the control volume for discretization

$$h_{ip}^{j+1} = h_{ip}^j - \alpha(\bar{u})_{ie}^{j+1} h_{ie}^{j+1} \frac{\Delta t}{\Delta x_i} + \alpha(\bar{u})_{iw}^{j+1} h_{iw}^{j+1} \frac{\Delta t}{\Delta x_i} - (1 - \alpha) \{ (\bar{u})_{ie}^j h_{ie}^j - (\bar{u})_{iw}^j h_{iw}^j \} \frac{\Delta t}{\Delta x_i} + \{ \alpha S_i^{j+1} + (1 - \alpha) S_i^j \} \Delta t \quad (1.6.1)$$

Here,  $S_i$  is the source term ( $S_i = r_i - f_i + \frac{q_{ri}}{\Delta y_i}$ ),  $\Delta y_i = \Delta y - b_i$ , and  $b_i$  is the channel base width defined for the control volume  $CV_i$ .

$$A_{ip}^{j+1} = A_{ip}^j - \alpha(\bar{u})_{ie}^{j+1} A_{ie}^{j+1} \frac{\Delta t}{\Delta x_i} + \alpha(\bar{u})_{iw}^{j+1} A_{iw}^{j+1} \frac{\Delta t}{\Delta x_i} - (1 - \alpha) \{ (\bar{u})_{ie}^j A_{ie}^j - (\bar{u})_{iw}^j A_{iw}^j \} \frac{\Delta t}{\Delta x_i} + \{ \alpha S_i^{j+1} + (1 - \alpha) S_i^j \} \Delta t \quad (1.6.2)$$

Here,  $S_i$  is the source term ( $S_i = r_i \Delta y_i + q_{Li} + q_{ssi} + q_{bi}$ ).

An arbitrary control volume which have channel properties is divided into two cases according to the channel base width and grid cell size (shown in the following figures): the case of the channel base width being smaller than the grid cell size and the case of the channel base width being equal to or larger than the grid cell size. When the channel base width is smaller than the grid cell size, all the hydrological components for overland flow and channel flow are simulated with respect to the corresponding control volume. However, when the channel base width is equal to or larger than the grid cell size, only channel flow is simulated.

Here, rainfall at the corresponding control volume either occurs within the channel base width or outside of it. The rainfall occurring within the channel base width contributes to the direct runoff of the stream and the rainfall outside the channel is needed infiltration analysis as it is done for the overland flow control volume.

Consequently,  $\Delta y_i$  in Eq. (1.6.2) for channel flow analysis must be the same as the grid cell size in direction  $y$  ( $\Delta y_i = \Delta y$ ) when the channel base width is equal to or larger than the grid cell size. And when the channel base width is smaller than the grid cell size, the channel base width should be applied ( $\Delta y_i = b_i$ ).

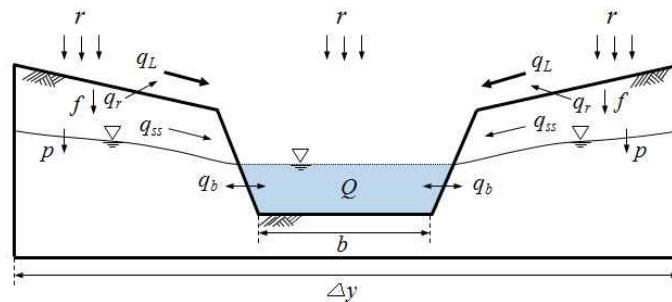


Fig. 1.7 Channel cell with channel base width is smaller than the cell size

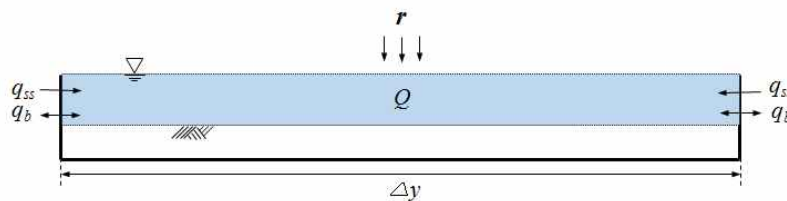


Fig. 1.8 Channel cell with channel base width is equal to or larger than the cell size

## 1.7 Flow control

The GRM can not only simulate natural flow, such as direct runoff from rainfall, but also reflects the changes of artificial flow conditions occurring within a watershed to runoff simulations using a flow control module. The flow conditions that can be simulated with the flow control module are "Reservoir outflow," "Inlet," "Reservoir operation," "Sink flow," and "Source flow."

The flow control technique can be applied to all grids with channel flow and overland flow properties. Moreover, multiple flow control conditions can be simultaneously applied to a single watershed to reflect the direct runoff from rainfall and changes of various flow conditions that occur during runoff simulations (KICT, 2011b).

### 1.7.1 Reservoir outflow

The "Reservoir outflow" function simulates runoff by dividing a watershed into upstream and downstream with respect to grids selected as the reservoir; it does not consider the reservoir operation rule. The discharge from the control volume selected as the reservoir is set using an observed hydrograph of the reservoir entered by the user (Eq. (1.7.1)). The runoff simulation of the control volume located in the immediate downstream of the reservoir is performed by including the reservoir outflow in the upstream boundary conditions. Because observed flow is applied to reservoir outflow, reservoir storage could be greater than [MaxStorage].

$$q_{ie} = q_o \quad (1.7.1)$$

Here,  $q_{ie}$  is the outflow per unit width from the control volume  $i$  selected as the reservoir and  $q_o$  is the outflow per unit width calculated from the observed discharge hydrograph of the reservoir.

### 1.7.2 Inlet

Upstream and downstream are distinguished based on the grid selected as the inlet. This is identical to the "Reservoir outflow" that simulates flow by dividing a watershed based on a particular grid. However, the "Inlet" does not simulate the upstream of inlet grid. Moreover, it does not consider the reservoir operation rule. When a runoff hydrograph for the inlet grid is given, the outflow of inlet grid is calculated with the method shown in Eq. (1.7.1). And data is applied as the upstream boundary condition for the downstream cell of the inlet.

This "Inlet" function excludes the upstream area of a dam and reservoir from

simulation area when observed runoff hydrographs of a dam or reservoir are available for large watersheds that include dams and reservoirs. It only simulates the downstream areas and thus allows the reduction of the scope of required spatial data and hydrological time series data. In grid-based distributed models, the number of grids comprising a watershed is reduced when the simulation area decreases. It can reduce the model run time. Moreover, applying equal model run times allows using small-sized grids. This can reduce uncertainties in gridded topographical and hydrological data due to issues with the scales of grid sizes.

### **1.7.3 Reservoir operation**

The "Reservoir operation" function can reflect the effects of storage in a reservoir and reservoir operation in flow simulations. The GRM can produce dynamic simulations for the initial storage, maximum storage, maximum storage ratio, constant discharge, and Reservoir Operation Method (ROM) from the relations of water level-storage and water level-discharge.

### **1.7.4 Sink flow / source flow**

"Sink flow" simulates the condition of partially omitting flow that was simulated in an arbitrary grid. "Source flow" simulates flow by reflecting flow conditions that were added to the flow simulated in an arbitrary grid. The discharge, either omitted or added due to "Sink flow" and "Source flow," is given by hydrographs and is applied as source term when simulating overland flow and channel flow. If the reservoir operation method(ROM) is additionally specified in the stream cell with "Sink flow" or "Source flow", "Sink flow" or "Source flow" can simultaneously be applied with the "Reservoir operation" function.

### **1.7.5 Reservoir specification and operation method**

When one of "Reservoir outflow", "Reservoir operation", "Sink flow", "Source flow" is applied, except "Inlet", reservoir specification (RS) and the ROM can be set as shown in the following table. When "Reservoir operation" is applied, the RS and ROM have to be entered. When "Reservoir outflow", "Sink flow", or "Source flow" are applied, ROM is not needed to be set, and if RS is not entered, the initial and maximum storages are set as "0" and the storage of the cell is not calculated.

When the "/a" option is used, the GRM model uses all the data entered by the user (e.g., the dam discharge) in the flow control simulation, and if there is no more flow control data, the AutoROM is applied using the reservoirs specification. Therefore, when applying the "/a" option, specifications of all the reservoirs included in the runoff simulation must be entered.



Table 1.1 Reservoir conditions for applying the "Reservoir operation" function (Choi, 2010)

Classification		Description
Specifications	Initial Storage	The reservoir storage at the time of starting simulation
	Maximum Storage	Maximum storage of the reservoir. Total storage from the bottom to flood water level.
	Storage for the normal high water level	The reservoir storage for the normal high water level.
	Storage for the restricted water level	The reservoir storage for the restricted water level(e.g. flood restricted water level).
	Period of the restricted water level storage	The period to apply the restricted water level storage. The simulation time except this duration the normal high water level storage is applied.
ROM	Automatic ROM	Reservoir discharge does not occur until the maximum possible storage(Maximum storage, storage for the normal high water level, storage for the restricted water level, etc.) is reached. When the maximum possible storage is reached, all flow into the reservoir is discharged.
	Rigid ROM	If the reservoir storage is less than the set discharge, all of it is discharged. If the reservoir storage is more than the set discharge, set constant flow is discharged until the storage exceeds the maximum possible storage; in that case, all inflow to the reservoir is discharged.
	Constant Discharge	If the reservoir storage is less than the set discharge, all of it is discharged. If the reservoir storage is more than the set discharge, the constant flow is discharged during specified time regardless of reservoir capacity.
	Storage-Discharge Relationship	The storage-discharge, water level-storage, or water level-discharge relationships are used to calculate the storage and discharge (only supported in source code level).

## 1.8 Calculation time step

The GRM can either use the same calculation time step ( $\Delta t$ ) for the entire simulation process or different calculation time steps according to calculated flow. The first case applies the same calculation time step set by the user repeatedly for the entire simulation process. The second case uses the Courant-Friedrichs-Lewy (CFL) condition to change the calculation time step for each calculation step. The CFL condition is defined by the equation below.

$$\Delta t \leq \frac{\Delta x}{u_{\max}} \quad (1.8.1)$$

Here,  $u_{\max}$  is the maximum flow among all grids calculated at time  $t$  and  $\Delta t$  is the calculation time step for time  $t + \Delta t$ .

## 2. Model parameters

The GRM analyzes the rainfall–runoff relationship physically and consequently physical parameters assigned to each grid are used. Theoretically, physical parameters selected based on the watershed boundaries, stream network, soil, and land cover should be omitted from the list of parameters estimated by the user. However, they can become parameters to be estimated depending on the limitations of governing equations and issues with data creation and scale.

The GRM considers the initial soil saturation ratio, minimum channel slope, channel roughness coefficient, and dry stream order as parameters to be estimated by users and not as fixed parameters reflecting physical properties. Moreover, each parameter selected according to the soil and land cover properties can be calibrated by uniform ratio using the parameter calibration coefficient.

Table 2.1 Input data and parameters for the GRM

Model input data	Selected parameters
Simulation domain (or watershed area)	Control volume number, cell size, flow analysis area
Flow direction	Upstream/downstream control volume, flow relationship, calculation order
Flow accumulation	
Slope	Slope, Minimum land surface slope, minimum channel bed slope
Stream network	Minimum channel base width, stream control volume, stream order, dry stream order, channel roughness coefficient
Land cover map	Land surface roughness coefficient, impervious ratio
Soil map	Green–Ampt parameters, effective soil depth
Rainfall	Rainfall, initial saturation ratio
Discharge	Initial discharge

## 2.1 Soil parameters

The GRM uses the Green–Ampt model to simulate the infiltration process. The Green–Ampt model parameters according to the soil properties for the simulation of infiltration excess flow and the soil depth for saturation excess flow and subsurface flow must be selected.

Calculating the infiltration using the Green–Ampt model requires the physical soil properties such as porosity, effective porosity, wetting front suction head, and hydraulic conductivity. Such soil properties can be obtained by measuring the soil moisture content (Brakensiek et al., 1981). Rawls et al. (1983) analyzed approximately 5,000 soil samples across the U.S. and suggested average values for the Green–Ampt model parameters according to the soil texture (Rawls et al., 1983; Chow et al., 1988). All parameters used to derive the effective saturation of the soil for the Green–Ampt model, except for the initial moisture content ( $\theta$ ), are defined by the values already set according to the soil texture. Moreover, each parameter can be estimated during the flow simulation process. The initial moisture content ( $\theta$ ) can be derived from measurement or parameter estimation and can vary depending on the hydrological conditions of a watershed such as antecedent rainfall.

※ Typical soil map applicable in Korea is detailed soil map (Kim, 1998). The detailed soil map is based on a detailed soil survey focused on Korea. It categorizes Korean soil into ‘soil series’ and further according to ‘soil type’ and ‘soil phase’. It also includes the physical properties of the smallest unit, the ‘soil phase’. The parameters of the Green–Ampt model are set according to the soil texture. The detailed soil map categorizes the ‘soil type’ in the same manner as soil texture. Moreover, the soil depths of Korean soil required for the calculation of the saturation excess flow are categorized with respect to the ‘soil series’ of the detailed soil map. The range of each soil depth category was defined based on the results of a soil survey project and is shown in table 2.4 (National Institute of Agricultural Science and Technology, 1992).

GRM uses unsaturated hydraulic conductivity to calculate percolation from soil layer A to B. From Fredlund et al. (1994), Averjanov (1950) suggested a power function(Eq. 2.1.1) to calculate unsaturated hydraulic conductivity ( $K_u$ ). Fredlund et al. (1994) applied 3.5 to the  $n$  value, and Noh et al. (2015) used 12 in continuous modelling for long period. The GRM suggests 6.4 as the default  $n$  value for the simulation of rainfall-runoff event.

$$K_u = K_s S_r^n \quad (2.1.1)$$

## GRM

Here,  $K_u$  is unsaturated  $K$ ,  $K_s$  is saturated  $K$ ,  $S_r$  is soil saturation ratio  $\{ = (\theta - \theta_r) / (\theta_s - \theta_r) \}$ ,  $\theta_r$  is residual moisture content,  $\theta_s$  is saturated moisture content, and  $n$  is the coefficient.

In the GRM, below equations as well as the power function can be used. In these equations, the default values of  $n$  is 0.2 and  $m$  is 0.1.

$$K_u = nK_s S_r \quad (2.1.2)$$

$$K_u = mK_s \quad (2.1.3)$$

Table 2.2 Green-Ampt model parameters according to the soil texture

Soil Texture	Porosity ( $\eta$ )	Effective porosity ( $\theta_e$ )	Residual moisture content ( $\theta_r = \eta - \theta_e$ )	Wetting front soil suction head ( $ \psi_f $ )[cm]	Hydraulic conduct. ( $K$ ) [cm/hr]
Sand	0.437 (0.374-0.5)	0.417 (0.354-0.479)	0.02	4.95 (0.97-25.35)	11.78
Loamy sand	0.437 (0.363-0.505)	0.401 (0.329-0.472)	0.036	6.13 (1.35-27.93)	2.99
Sandy loam	0.453 (0.351-0.554)	0.412 (0.283-0.54)	0.041	11.01 (2.67-45.46)	1.09
Loam	0.463 (0.375-0.55)	0.434 (0.334-0.533)	0.029	8.89 (1.33-59.37)	0.34
Silt loam	0.501 (0.42-0.581)	0.486 (0.394-0.577)	0.015	16.68 (2.92-95.38)	0.65
Sandy clay loam	0.398 (0.332-0.463)	0.33 (0.235-0.424)	0.068	21.85 (4.42-108.1)	0.15
Clay loam	0.464 (0.409-0.518)	0.309 (0.279-0.5)	0.155	20.88 (4.79-91.9)	0.1
Silty clay loam	0.471 (0.418-0.523)	0.432 (0.347-0.516)	0.039	27.3 (5.67-131.49)	0.1
Sandy clay	0.43 (0.37-0.489)	0.321 (0.207-0.434)	0.109	23.9 (4.08-140.1)	0.06
Silty clay	0.479 (0.425-0.532)	0.423 (0.334-0.511)	0.056	29.22 (6.13-139.3)	0.05
Clay	0.475 (0.427-0.522)	0.385 (0.269-0.5)	0.09	31.63 (6.39-156.4)	0.03

Table 2.3 Classification of the soil depth for soil series

Soil depth classification	Soil depth (cm)	
	USDA*	Detailed soil map
Very shallow	0 - 10	0 - 20
Shallow	10 - 30	20 - 50
Moderately deep or Moderately shallow	35 - 50	50 - 100
Deep	50 - 60	100 - 150
Very Deep	> 60	> 150

\* USDA: United States Department of Agriculture

## 2.2 Land cover parameters

The land cover of watersheds have influence on determining the roughness coefficient and impervious ratio to calculate overland flow. The GRM uses roughness coefficients suggested by Engman (1986) and Vieux (2004), as shown in the table below, to simulating overland flow from rainfall.

Table 2.4 Roughness coefficients according to land cover properties

7 Classifications of land cover (Ministry of Environment, Korea)		Roughness coefficient
Code	Attributes	
100	Urban/dry area	0.015
200	Agricultural area	0.035
300	Forest	0.1
400	Grass	0.15
500	Wetland	0.07
600	Bare	0.02
700	Water	0.03

"Impermeable areas of overland" refers to areas without infiltration through soil, even with rainfall. Sagong (2003) classified land cover as permeable or impermeable using IKONOS satellite images with a spatial resolution of 1 m. This classification system was applied to Anyang City in Korea to calculate impervious ratios for different land usage. The GRM uses impervious ratios from the study results by Sagong (2003) that

correspond to the 7 classifications of land cover attributes by the Ministry of Environment. Table below shows impervious ratios ranging from 0 – 1. When the ratio is “1,” grids with corresponding land cover properties are determined as impermeable areas. Moreover, because soil of water and wetlands is always saturated, it is assumed that infiltration from rainfall does not occur and the impervious ratio is set as “1.”

Table 2.5 Classification of land cover according to the permeability (Sagong, 2003)

Permeable area	Impermeable area
Vegetation	Paved road
Cultivated land	Concrete structure
Vinyl greenhouse	Apartment
Stream	Detached house
Bare land	Town house
Other grassland	Buildings other than houses
Railway	Factory

Table 2.6 Impervious ratios according to land cover map properties (Sagong, 2003)

Land cover map attributes	Land usage type	Impervious ratio	
		Range of values	Average
Urban/dry area	Commercial area	0.641-0.947	0.853
Agricultural area	Rice paddy	0.107-0.456	0.391
	Field	0.053-0.504	
	Vinyl greenhouse	0.422-0.842	
Forest	Greenbelt area, non-urban area, forest	0.001-0.05	0.025
Grass	Grassland	0.14-0.86	0.44
Wetland	-	-	1
Bare	Bare land	0.12-0.81	0.442
Water	-	-	1

## 2.3 Channel base width

The channel base width is a parameter to simulate the channel flow. The GRM can simulate asymmetrical trapezoidal compound cross sections. Here, the channel base width is applied to calculate cross-sectional flow areas of channels. Streams in watersheds have irregular compound cross sections and channel base widths; entering such varying channel shapes for every stream grid is very difficult. Therefore, objective parameters must be used when entering channel base widths for flood flow simulations to obtain consistent simulation results for all stream grids. In the GRM model, either the flow accumulation from grids or the design channel base width equation can be used.

### 2.3.1 Method using flow accumulation

The flow accumulation from watershed grids increases as it approaches downstream grids. The channel base width is typically larger in the downstream than upstream. A method using flow accumulation reflects these trends to set the channel base width proportional to flow accumulation. Here, the channel base width for an arbitrary control volume ( $CV_i$ ) can be calculated using Eq. (2.3.1). The grid at the most downstream of a watershed shows maximum flow accumulation. The channel base width calculated with Eq. (2.3.1) takes a maximum value for the most downstream grid. This value decreases as it approaches grids upstream. When the channel base width is defined based on Eq. (2.3.1), the measured channel base width from the most downstream location can be applied and used to define the channel base width for upstream grids with the consistent method (Choi, 2010).

$$b_i = \frac{FA_i \times b_{max}}{FA_{max}} \quad (2.3.1)$$

Here,  $b_i$  is the channel base width for  $CV_i$ ,  $FA_{max}$  is the flow accumulation for the most downstream control volume,  $FA_i$  is the flow accumulation for  $CV_i$ , and  $b_{max}$  is the channel base width for the most downstream control volume.

### 2.3.2 Method using the design channel width equation

The Korea Ministry of Construction and Transportation (2005) recommends an empirical equation that adopts topographical properties, such as slope and watershed area, to select the design channel base widths according to design floods when designing channels. Equation (2.3.2) can be used for southern regions (Honam and Youngnam regions, Korea) and Eq. (2.3.3) can be used for central regions (Gyeonggi,

Gangwon, Chungnam, and Chungbuk provinces, Korea) of Korea.

$$B = 1.698 \frac{A_w^{0.318}}{S_0^{0.5}} \quad (2.3.2)$$

$$B = 1.303 \frac{A_w^{0.318}}{S_0^{0.5}} \quad (2.3.3)$$

Here,  $B$  is channel width,  $A_w$  is the watershed area ( $km^2$ ), and  $S_0$  is the channel bed slope.

The watershed area of each grid is used as the watershed area ( $A_w$ ) to calculate the channel base width for each grid applying the design channel base width equation. The watershed area of each grid is calculated using the equation below, by multiplying the flow accumulation of the corresponding grid and grid area. The slope of each grid selected in the slope layer (input data for the model) can be used as the channel bed slope ( $S_0$ ).

$$A_{wi} = (FA_i + 1) \times (\Delta y)^2 \quad (2.3.4)$$

Here,  $A_{wi}$  is the watershed area of an arbitrary control volume.

## 2.4 Initial saturation ratio

Initial saturation ratio is a parameter used to calculate infiltration and maximum possible infiltration and shows the soil saturation ratio at the time of runoff simulation. The initial saturation ratio is estimated within the range of "0 – 1" during the calibration process of the model and can approach 1 with increasing antecedent rainfall. If there is a soil saturation ratio raster data, it can be applied to flow simulation instead of estimating initial soil saturation ratio parameter.



## 2.5 Minimum slope

The kinematic wave model uses the land surface slope as friction slope to calculate the flow velocity. The flat area calibration process of a DEM entails a very small elevation modification to minimize changes in the original DEM. Therefore, the surface slope of areas that went through flat area processing can take very small values. When such miniscule surface slopes are applied to the kinematic wave model, the calculated flow velocity and discharge can be close to "0". Various studies were performed to determine minimum values that can be applied to the kinematic wave model as surface slopes (Ponce et al., 1978; Woolhiser and Liggett, 1967); the values mostly ranged from 0.0001–0.01. Moreover, Henderson (1966) and ASCE (1996) suggested that the kinematic wave model was appropriate for streams with a bed slope of 0.002 (10 *ft/mi*) or more when analyzing flood waves.

Minimum slopes of the channel and overland flow are parameters estimated by users to apply the slope layer including grids with very small slope values that resulted from flat area calibration of a DEM to the kinematic wave model. The minimum slope is part of the flow calculations for flood routing and thus affects discharge and flood wave arrival times. Consequently, it must be estimated by considering the watershed and stream properties and DEM properties applied during flow simulation.

## 2.6 Channel roughness coefficient

The roughness coefficient of a channel can vary depending on the channel shape, ground composition materials, vegetation, and degree of management (Chow, 1959). Chow (1959) combined existing studies on the roughness coefficient selection for channels to suggest roughness coefficients for various channel conditions. Among these roughness coefficients, those for natural streams are shown in Table below. Chaudhry (1993) suggested roughness coefficients for "clean, straight, full stage, no rifts, or deep pools," "bottom: gravel, cobbles, and few boulders," and "bottom: cobbles with large boulders" for natural streams. However, for actual flow simulations, it is appropriate to estimate roughness coefficients considering the channel conditions that can reflect the properties of a watershed within the range of roughness coefficient for each channel condition.

Table 2.7 Roughness coefficients for natural rivers (Chow, 1959)

Classification		Roughness Coefficient		
		Minimum	Typical	Maximum
Streams on plain	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.033
	Same as above, but more stones and weeds	0.030	0.035	0.040
	Clean, winding, some pools and shoals	0.033	0.040	0.045
	Same as above, but some weeds and stones(Ⓐ)	0.035	0.045	0.050
	Same as above, lower stages, more ineffective slopes and sections	0.040	0.048	0.055
	Same as Ⓐ, but more stones	0.045	0.050	0.060
	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080
	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150
Mountain streams, no vegetation in channel, banks usually steep, trees and brush along banks submerged at high stage	Bottom: gravels, cobbles, and few boulders	0.030	0.040	0.050
	Bottom: cobbles with large boulders	0.040	0.050	0.070

## 2.7 Dry stream order

The GRM simulates under the condition that all rainfall occurring within a channel base width contributes to direct flow. Therefore, when a stream network entered into the river raster data is dense, large direct runoff can be simulated, even from small amounts of rainfall. However, when the antecedent rainfall is small in natural streams, their upstream tributaries can be dry streams and infiltration may occur first.

To take into account early infiltration at dry streams during flow analysis, the GRM uses the dry stream order. When the entered stream network is dense, the dry stream order can be entered by considering antecedent rainfall and stream flow. The direct runoff of dry streams is calculated with the same method used for overland flow. The dry stream order is estimated within the range of "0 to maximum stream order." To apply the dry stream order during flow analysis, stream order data must be entered into the stream network raster data. Hydrological GIS S/W, such as HyGIS, uses the

stream order as grid cell value when creating a stream network raster data. The maximum stream order can also be identified based on the entered stream network raster data.

## 2.8 Parameter calibration coefficients

In the case of calibrating roughness coefficients and parameters of the Green–Ampt model set from land cover and soil properties with a certain ratio, a calibration coefficient corresponding to each parameter can be applied. The parameter calibration coefficient is a value applied to each parameter of the entire grid to be simulated. When 1 is entered, the corresponding parameter takes the value initially set in the flow simulations.

## 2.9 Parameter estimation

A physically based model assumes that parameters set according to spatial data from field surveys represent true values. Therefore, parameters which are difficult to set observed values for every cell and have relatively high uncertainties and high sensitivities such as initial saturation, channel minimum slope, channel roughness coefficient, permeability, and soil depth, are estimated by users.

The GRM simulates rainfall-runoff events. Consequently, the model calibration mainly reproduces observed hydrographs for peak discharge, peak time, and total discharge. Moreover, the overall trend of the calculated hydrograph can be considered to determine how well it reproduces the trend of the observed hydrograph. The typical trial-and-error method and any other optimization techniques can be used for model calibration.

Goodness-of-fit evaluation of a model can be performed using various objective functions such as relative errors of peak discharge, peak time, and the total discharge and root-mean-square error (RMSE), normalized RMSE (nRMSE), mean absolute percentage error (MAPE), correlation coefficient (CC), and Nash–Sutcliffe model efficiency (ME) of time series data.

### 3. Multi-site calibration

Most distributed models are set up with uniform grid sizes for single watersheds. Moreover, flow simulations are carried out using one parameter group that was set for each grid. Here, when one parameter group is used for flow simulations of a watershed that is comprised of many subwatersheds with varying runoff properties (Ajami et al., 2004), the physical and hydrological properties of the subwatersheds cannot be reflected properly in the model.

Distributed models have the advantage of easily obtaining flow simulation results from arbitrary sites within a watershed divided by grids (Beven and O'Connell, 1982). To improve the reliability of the flow analysis results for arbitrary cells which are not calibration sites, the model should be set up securing physical and hydrological similarities between the watershed of a cell to get simulation results and the calibrated watershed (Pilgrim, 1983; Dawson et al., 2006). The most intuitive method to secure physical and hydrological similarities between the calibrated watershed and the watershed outputting flow analysis results is to establish the model such that model calibration is possible at a point close to the point of the flow analysis output (Ajami et al., 2004; Merz, R. and Blöschl, 2004; Young, 2006). This requires the calibration of models for multiple sites (multi-site calibration) within a watershed.

The multi-site calibration method can be divided into a method for a single watershed system and a method that connects multiple subwatersheds. If verifiable stream gauges exist within a watershed, the watershed can be divided into subwatersheds according to the gauges. The single watershed multi-site calibration method calibrates a model collectively using the corresponding observed flow for subwatershed areas that were divided according to stream gauges within a watershed. All subwatersheds have a uniform grid size, which restricts applications of varying resolutions for each subwatershed. However, the advantage is the simple model calibration for multiple sites within one watershed.

The multi-site calibration method of connecting subwatersheds connects the flow analysis results from each watershed using the flow control module of the GRM. This method can improve the flow analysis results for an entire area consisting of multiple watersheds. Here, each subwatershed is an independent watershed system and thus different resolutions can be applied and data suitable for the properties of each subwatershed can be applied separately. Therefore, the properties of each subwatershed are reflected as much as possible. Moreover, flow analysis for each subwatershed is

carried out by an independent process, which allows parallel computation through separated processes and consequently reduces the calculation time required for an entire watershed. In a typical hydrological event simulation, the multi-site calibration method of connecting subwatersheds takes the entire time series of flow analysis results and applies it to the flow control module of a different watershed. However, a real-time flow analysis system using real-time analysis modules of the GRM (GRM Real Time) requires real-time input of rainfall and flow control data and flow analysis is performed through dynamic connection of each subwatershed.

### 3.1 Single watershed multi-site calibration

The multi-site calibration method for single watersheds is used to set up a model with a single watershed system with uniform grid size. Moreover, it is used to collectively calibrate a model with respect to numerous stream gauges existing within a watershed. Parameters are set for each subwatershed using watershed raster data divided into subwatersheds and the model is calibrated according to each subwatershed. The basic principle of the multi-site calibration module is the application of the parameter set of an arbitrary watershed to the entire upstream area. If there is no subwatershed upstream with parameters set by user, a single parameter group is applied (Choi et al., 2012).

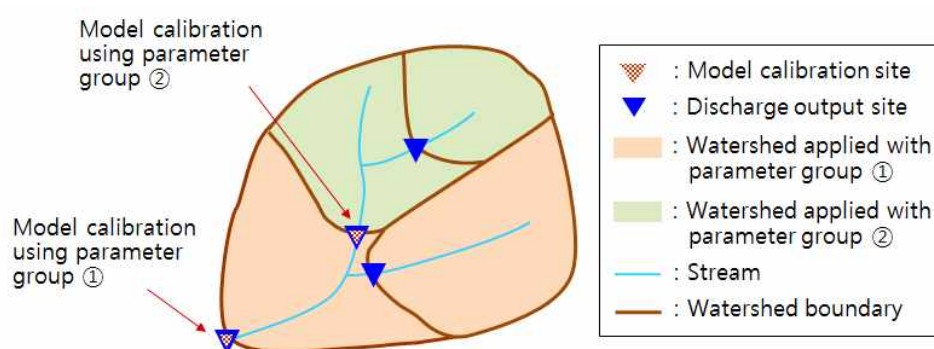


Fig. 3.1 Application method of the single watershed multi-site calibration

Table 3.1 Parameters of the GRM subject to multi-site calibration

Classification	Parameter subject to multi-site calibration	Abbreviation
Topographic	Minimum land surface slope	MinSlopeOF
	Minimum channel bed slope	MinSlopeChBed
	Minimum channel base width	MinChBaseWidth
	Channel roughness coefficient	ChRoughness
	Dry stream order	DryStreamOrder
Land Cover and Soil	Land surface roughness coefficient calibration coefficient	CalCoefLCRoughness
	Porosity calibration coefficient	CalCoefPorosity
	Wetting front soil suction head calibration coefficient	CalCoefWFSuctionHead
	Hydraulic conductivity calibration Coefficient	CalCoefHydraulicK
	Soil depth calibration coefficient	CalCoefSoilDepth
Hydrological conditions	Initial saturation ratio	IniSaturation
	Initial flow	IniFlow

The GRM uses project files in xml format to save the environment and parameters of a modelling project. Multi-site calibration data are saved as a subwatershed parameter table ("SubWatershedSettings") in an xml project file. A subwatershed parameter table takes the subwatershed number as the key code, has parameters entered according to subwatersheds, and can reuse previously saved parameters of each subwatershed through the open project command.

### 3.2 Subwatersheds connecting multi-site calibration

To obtain reliable flow analysis using a physical model for watersheds with various hydrological properties and watershed areas, models suitable for each watershed property must be established. Runoff analysis using a distributed model usually builds a model for one watershed with uniform grid size and uses the one parameter group. However, when single-sized grids are used to build a model and a single parameter group is used to calibrate a model for multiple subwatersheds with various hydrological properties, the physical and hydrological properties of subwatersheds are not properly reflected. Therefore, different watershed systems must be established according to different watershed properties and a modeling technique that can integrate each watershed is required.

The subwatersheds connecting multi-site calibration method builds models for each subwatershed using the flow control module of the GRM. Moreover, the flow analysis results for each subwatershed can be connected to perform flow analysis of the entire watershed. Here, flow analysis results from the upstream of a watershed are transferred to a subwatershed downstream using the inlet function of the flow control module (Korea Institute of Civil Engineering and Building Technology, 2015). To execute flow analysis dynamically for each watershed using the subwatersheds connecting multi-site calibration method, the real-time analysis module of the GRM (GRM RT) is used. The GRM RT not only enters rainfall data but also input data of the flow control module, such as dam discharge, in real-time for flow analysis. Therefore, the downstream of a watershed receives simulation results from the upstream in real-time and dynamic simulation can be obtained for the entire watershed. The description of GRM RT is included in the GRM RT section in this user manual.

Models with different grid sizes can be created using the subwatersheds connecting multi-site calibration and reflecting properties of different subwatersheds. Therefore, the model calibration for each subwatershed can be optimized and the flow analysis results for arbitrary sites within watersheds can be improved compared with the flow analysis results from calibrating only the site at the lowest end of a stream in a single watershed system. However, it is inconvenient because the input data for each watershed must be established separately and model calibration must be performed with individual processes according to the watersheds. Such model calibration is very complex and requires great efforts to maintain a dependable operation of the modeling system and stable analyses of flow preservation between subwatersheds and of flood routing problems.

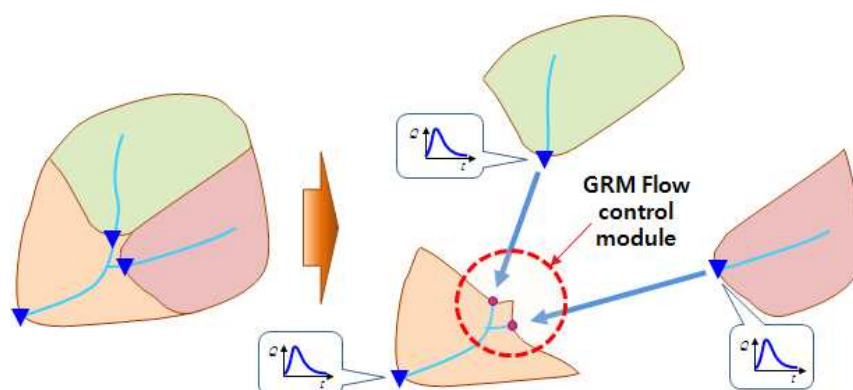


Fig. 3.2 Conceptual diagram of the subwatersheds connecting multi-site calibration

## 4. Input data

The GRM is executed in project units. Project file of the GRM have .gmp extension and is saved in xml format. The input data required to run the GRM, simulation environment, and parameters are saved in GRM project file. The GRM uses topographical and spatial data established based on a DEM, soil and land cover maps, and rainfall raster file as input data. The watershed, slope, flow direction, flow accumulation, stream network files generated in DEM analyses can be created using the GIS tool, which can also produce hydrological spatial data. The general S/W that includes hydrological spatial data-creating tools based on DEM analyses, such as HyGIS, TauDEM, QGIS Drainage plug-in, and ArcGIS, can be used for the GIS tool, which creates input data. The GRM uses a ASCII raster format. Thus, data of various formats created using the GIS tool can be converted to ASCII format and applied to the model.

Table 4.1 Input data for the GRM

Classification	Data	Format	Applicable Original Data	Note
Topography	Simulation domain (or watershed area)	ASCII raster	DEM(digital topography map, Remote Sensing (RS) images)	Stream network and channel base width data are optional
	Slope			
	Flow direction			
	Flow accumulation			
	Stream			
	Channel width			
Land Cover	Land cover map	ASCII raster	Land cover map RS image	The GRM parameters of 7 classifications of land cover are given as reference values
Soil	Soil texture		Detailed soil map Global soil data	Parameters for Green-Ampt equation
	Soil depth			
Hydrological	Rainfall	ASCII raster	Observed Estimated	Raster format time-series and time series of mean rainfall are optional
		Text		
	Discharge	Text	Observed Estimated	Text time series data is for flow control simulation. A flow value is for initial stream flow at a model calibration grid. ASCII format data is the initial flow values for all the stream grids.
		ASCII raster		
	Soil saturation ratio	ASCII raster	Observed Estimated	Instead of initial saturation ratio parameter text value, ASCII data for all grids in a watershed can be used. (optional)



The flow direction data used by the GRM is unidirectional, created with the D8 method. The flow direction is determined based on the value in the flow direction raster data (flow direction index). The selections of the 1 o'clock position as 1 (northeast, NE), 12 o'clock position as 1 (north, N), 3 o'clock position as 1 (east, E), or TauDEM flow direction index are applicable in the GRM. When the flow direction data is created from DEM data, the index corresponding the flow direction can be different according to the GIS S/W and thus caution must be needed when applying the flow direction data.

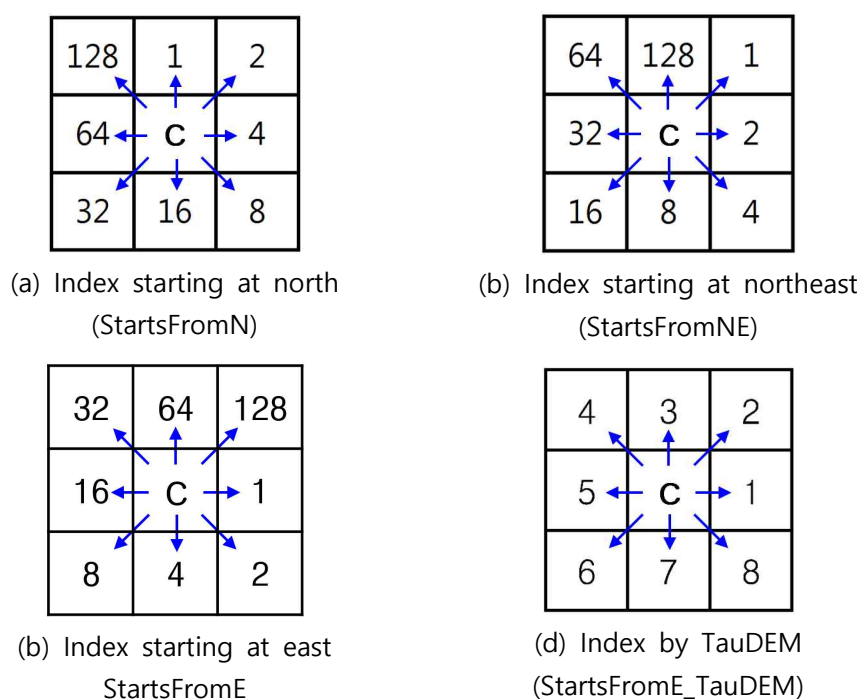


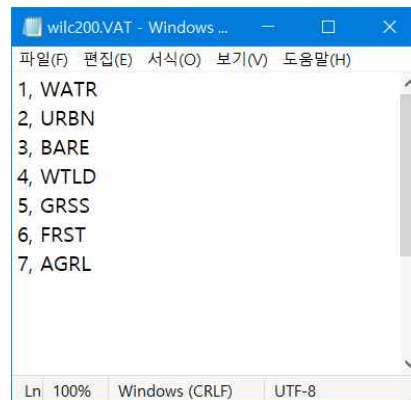
Fig 4.1 Flow direction index

## 4.1 Spatial data

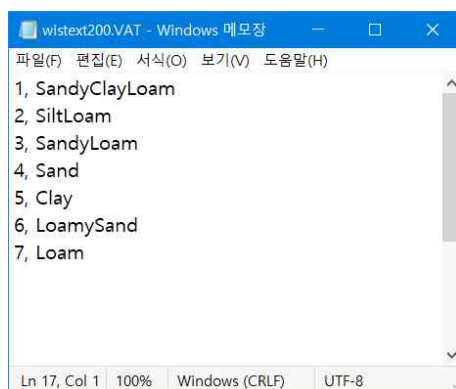
The GRM uses raster data in ASCII format as input data; the raster data required to run the GRM is shown in the table below. The values of raster files for land cover, soil texture, and soil depth are entered in numbers. Therefore, a response between the numbers in each raster file and the properties to be used for actual flow analysis is required. The QGIS-GRM (GRM model GUI) uses a Value Attribute Table (VAT) text file for this response. The VAT files can be manually created by users with a text editor or automatically created using GIS software. The model parameters related to land cover and soil are saved in the GRM static database (i.e., C:\GRM\GRMStaticDB.xml), which can be used to set the parameter default values for each property.

Table 4.2 Spatial input data for the GRM

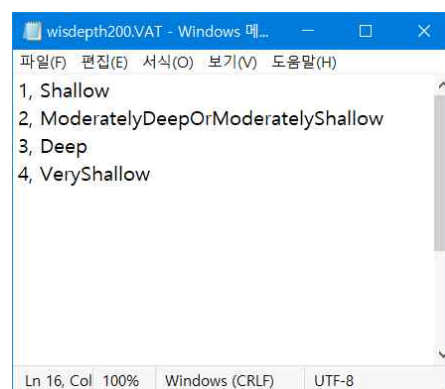
Data	Definition	Data type
Simulation domain (or watershed area)	Raster file with distinguished watershed area or simulation domain	Integer
Slope	Steepest slope data assigned to each grid	Double
Flow direction	Unidirectional flow direction according to the D8 method	Integer
Flow accumulation	Flow accumulation	Integer
Stream	Stream network	Integer
Channel width	Channel base width data for the same location as the stream network grid	Single
Soil texture	Soil texture	Integer
Soil depth	Soil depth	Integer
Land cover	Land cover	Integer



(a) Land Cover VAT File



(b) Soil Texture VAT File



(c) Soil Depth VAT File

Fig. 4.2 Examples of VAT files

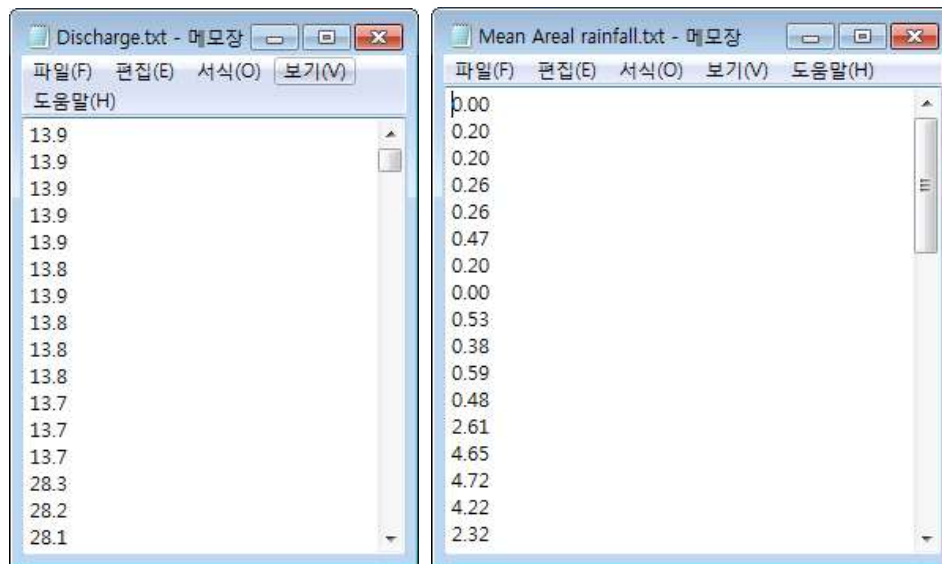
## 4.2 Hydrological data

The mean areal rainfall of a watershed and spatially distributed rainfall raster files can be selected and applied for the GRM. The distributed rainfall raster file can be either created from spatial interpolation of point rainfall observed at a rainfall gauge or spatially distributed rainfall data such as radar or numerical prediction data. Rainfall and flow data are saved as text files. Here, the mean areal rainfall and discharge are saved as time series (text). When the grid-based distributed rainfall ASCII raster file is used, a list of rainfall time series files (ASCII files) is saved.

When there is a soil saturation ratio raster data from observation or estimation, the data save as ASCII format can be applied. If this data is used, initial soil saturation ratio parameter is not estimated during runoff simulation.

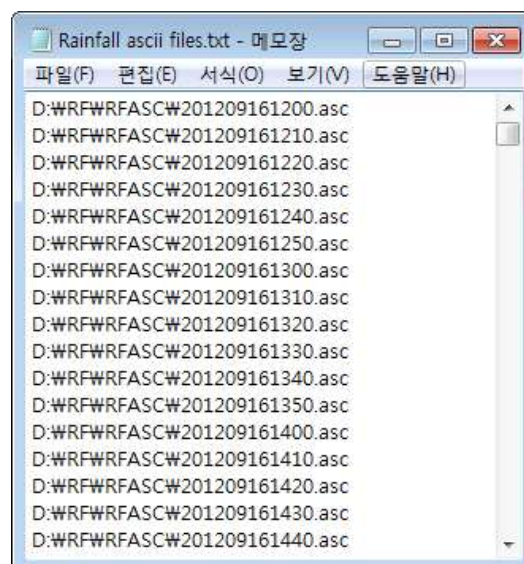
Table 4.3 Hydrological input data for the GRM

Data	Definition	Data type
Rainfall	Mean areal rainfall of a watershed (mm), text file	Single
	Grid-based distributed rainfall (mm), ASCII file	
Discharge	Time series data is applied to flow control simulation. A flow value is applied to initial stream flow at a model calibration grid. Observed or simulated flow (CMS)	Single
Soil saturation ratio	Soil saturation ratio saved as ASCII format for all grids in a watershed (optional)	Single



(a) Discharge input file

(b) Mean areal rainfall input file



(c) ASCII raster rainfall input file

Fig. 4.3 Examples of hydrologic input data files

### 4.3 GRM project file

The GRM runs using project file (.gmp). Running the GRM on a console window requires setting the gmp file as the switch.

(i.e., D:\wgrm.exe "projectFilePathAndName.gmp")

The gmp file is saved in xml format. The tables, contents of tables, field names and description for each table, and settings for each field included in the gmp files are shown in the following table. Users can create gmp file with a text editor; however, it is difficult to intuitively determine certain parameters (grid location, etc.). Thus, it is convenient to automatically create gmp files using GUI S/W (QGIS plug-in QGIS-GRM, etc.) of the GRM.

If gmp file includes Korean, it must be saved as UTF-8 format. If gmp file does not include Korean, it can be saved as ANSI or UTF-8 format.

Table 4.4 Descriptions of the tables in the GRM project xml file

Table name	Description	Required
ProjectSettings	Environment settings, input files, and global parameters to run the model	Required
SubWatershedSettings	Parameters set for each subwatershed	Required
WatchPoints	Grid information subjected to output selected by user	Required
FlowControlGrid	Grids subjected to flow control selected by user, flow control type, data properties, reservoir specifications, and ROM data	Required
ChannelSettings	Channel shape parameters for each watershed	Required
GreenAmptParameter	Soil texture data properties and Green-Ampt parameters values applied to flow simulations	Required
SoilDepth	Soil depth data properties and soil depth values applied to flow simulations	Required
LandCover	Land cover data properties and roughness coefficient and impervious ratio applied to flow simulations	Required

Table 4.5 ProjectSettings table

Field name	Description	Data type	Required
GRMSimulationType	Modeling type (SingleEvent or RealTme)	String	Required
DomainFile	Simulation domain(or watershed) ASCII file path and name	String	Required
SlopeFile	Slope ASCII file path and name	String	Required
FlowDirectionFile	Flow direction ASCII file path and name	String	Required
FlowAccumFile	Flow accumulation ASCII file path and name	String	Required
StreamFile	Stream ASCII file path and name	String	Required
ChannelWidthFile	Channel base width ASCII file path and name	String	Optional
LandCoverDataType	Land cover data type (File or Constant)	String	Required
LandCoverFile	Land cover ASCII file path and name, only used when 'File' is selected for LandCoverDataType	String	Optional
LandCoverVATFile	Land cover ASCII file VAT file path and name, only used if 'File' is selected for LandCoverDataType (If Korean is included, it must be saved as UTF-8 format)	String	Optional
ConstantRoughness-Coeff	Land cover roughness coefficient, only used if 'Constant' is selected for LandCoverDataType	Single	Optional
ConstantImpervious-Ratio	Impervious ratio, only used if 'Constant' is selected for LandCoverDataType	Single	Optional
SoilTextureDataType	Soil texture data type (File or Constant)	String	Required
SoilTextureFile	Soil texture ASCII file path and name, only used if 'File' is selected for SoilTextureDataType	String	Optional
SoilTextureVATFile	Soil texture ASCII file VAT file path and name, only used if 'File' is selected for SoilTextureDataType (If Korean is included, it must be saved as UTF-8 format)	String	Optional
ConstantSoilPorosity	Soil porosity, only used if 'Constant' is selected for SoilTextureDataType	Single	Optional
ConstantSoilEffPorosity	Effective soil porosity, only used if 'Constant' is selected for SoilTextureDataType	Single	Optional
ConstantSoilWetting-FrontSuctionHead	Wetting front soil suction head, only used if 'Constant' is selected for SoilTextureDataType	Single	Optional
ConstantSoilHydraulic-Conductivity	Hydraulic conductivity, only used if 'Constant' is selected for SoilTextureDataType	Single	Optional

&lt;ProjectSettings table (continued)&gt;

Field name	Description	Data type	Required
SoilDepthDataType	Soil depth data type (File or Constant)	String	Required
SoilDepthFile	Soil depth ASCII file path and name, only used when 'File' is selected for SoilDepthDataType	String	Optional
SoilDepthVATFile	Soil depth ASCII file VAT file path and name, only used when 'File' is selected for SoilDepthDataType (If Korean is included, it must be saved as UTF-8 format)	String	Optional
ConstantSoilDepth	Soil depth, only used when 'Constant' is selected for SoilDepthDataType (cm)	Single	Optional
InitialSoilSaturation-RatioFile	Initial soil saturation ratio ASCII file path and name	Single	Optional
InitialChannelFlowFile	Initial stream flow ASCII file path and name. Values are set only for stream cell.	Single	Optional
RainfallDataType	Rainfall data type (TextFileMAP or TextFileASCgrid)	String	Required
RainfallInterval	Rainfall data time interval (min)	Integer	Required
RainfallDataFile	Rainfall data file path and name	String	Required
FlowDirectionType	Flow direction data type StartsFromN, StartsFromNE, StartsFromE, or StartsFromE_TauDEM	String	Required
MaxDegreeOfParallelism	Limit of Parallelism using CPU, maximum value is applied when '-1' or no value are set	Integer	Optional
SimulStartingTime	Simulation start time. Set date time format string when time format is selected (i.e., 2012-09-16 12:00 LST). If date time format is not selected, set to '0'.	String	Required
SimulationDuration	Simulation duration (h)	Integer	Required
ComputationalTimeStep	Computational time step (min)	Integer	Required
IsFixedTimeStep	Fixed calculation time step used (true or false), 'true' is applied when nothing is selected	String	Optional
OutputTimeStep	Output time step (min)	Integer	Required
SimulateInfiltration	Infiltration simulation (true or false)	String	Required
SimulateSubsurface-Flow	Subsurface flow simulation (true or false)	String	Required
SimulateBaseFlow	Base flow simulation (true or false)	String	Required
SimulateFlowControl	Flow control simulation (true or false)	String	Required

&lt;ProjectSettings Table (continued)&gt;

Field name	Description	Data type	Required
MakeIMGFile	Raster image file creation (true or false)	String	Required
MakeASCFile	ASCII raster file creation (true or false)	String	Required
MakeSoilSaturationDistFile	Write soil saturation distribution file (true or false) (either MakeIMGFile or MakeASCFile must be true for it to be applied)	String	Required
MakeRfDistFile	Write rainfall distribution file (true or false) (either MakeIMGFile or MakeASCFile must be true for it to be applied)	String	Required
MakeRFaccDistFile	Write flow accumulation distribution file (true or false) (either MakeIMGFile or MakeASCFile must be true for it to be applied)	String	Required
MakeFlowDistFile	Write flow distribution file (true or false) (either MakeIMGFile or MakeASCFile must be true for it to be applied)	String	Required
PrintOption	Print out option (All, DischargeFile, DischargeAndFcFile, DischargeQ, or AllQ) - All : Print all simulation results - DischargeFile : Write just *Discharge.out file and do not make other output files - DischargeAndFcFile : Write just *Discharge.out file and flow control simulation results. Do not make other output files - DischargeQ : Write just *Discharge.out file with discharge value only - AllQ : Write all output files with discharge value only	String	Required
AboutThisProject	Project description entered by user	String	Optional
AboutWatershed	Watershed description entered by user	String	Optional
AboutLandCoverMap	Land cover map description entered by user	String	Optional
AboutSoilMap	Soil texture map description entered by user	String	Optional
AboutSoilDepthMap	Soil depth map description entered by user	String	Optional
AboutRainfall	Rainfall data description entered by user	String	Optional



Table 4.6 SubWatershedSettings table

Field name	Description	Data type	Required
ID	Watershed number Integer greater than 0 is entered as a watershed identifier	Integer	Required
IniSaturation	Initial saturation parameter, if soil saturation ratio ASCII file is applied, this parameter is not used	Single	Required
MinSlopeOF	Parameter of minimum bed slope condition for overland flow	Single	Required
UnsaturatedKType	Set unsaturated hydraulic conductivity calculation method (Constant, Linear or Exponential) If not set, 'Linear' is applied If 'Constant' is set, the fixed hydraulic conductivity value of Green-Ampt parameter multiplied by a coefficient is applied.	Single	Required
CoefUnsaturatedK	The coefficient for calculating unsaturated hydraulic conductivity If UnsaturatedKType is 'Linear', 0.2 is applied as default value, and if 'Exponential', 6.4 is applied as default value. When it is 'Constant', 0.1 is used as default value.	Single	Required
MinSlopeChBed	Parameter of minimum bed slope condition for channel flow	Single	Required
MinChBaseWidth	Minimum channel base width parameter	Single	Required
ChRoughness	Channel roughness coefficient parameter	Single	Required
DryStreamOrder	Dry stream order condition parameter; the stream order is entered; in case of entering 0, the dry stream order is not applied	Integer	Required
IniFlow	Initial flow parameter (m <sup>3</sup> /s) The flow observed at the simulation start time at the lowest stream end of a watershed is entered; if initial stream flow ASCII file is applied, this parameter is not used	Single	Required
CalCoefLCRoughness	Roughness coefficient calibration parameter selected according to land cover	Single	Required
CalCoefPorosity	Soil porosity calibration parameter	Single	Required
CalCoefWFSuction-Head	Wetting front soil suction head calibration parameter	Single	Required
CalCoefHydraulicK	Soil hydraulic conductivity calibration parameter	Single	Required
CalCoefSoilDepth	Soil depth calibration parameter	Single	Required
UserSet	If parameters of the current watershed were selected by the user or not (true or false)	Boolean	Required

Table 4.7 ChannelSettings table

Field name	Description	Data type	Required
WSID	Most downstream sub-watershed ID(raster value). Integer value greater than 0	Integer	Required
CrossSectionType	Channel cross section type (CSSingle or CSCCompound)	Single	Required
SingleCSChannel-WidthType	Channel base width calculation method (CWGeneration or CWEquation)	String	Optional
ChannelWidthEQc	Coefficient of the channel base width equation, only used if 'CWEquation' is selected for SingleCSChannelWidthType	Single	Optional
ChannelWidthEQd	Coefficient of the channel base width equation, only used if 'CWEquation' is selected for SingleCSChannelWidthType	Single	Optional
ChannelWidthEQe	Coefficient of the channel base width equation, only used if 'CWEquation' is selected for SingleCSChannelWidthType	Single	Optional
ChannelWidthMost-DownStream	Channel base width at the lowest downstream end of stream (m). Only used if 'CSCCompound' is selected for CrossSectionType	Single	Optional
LowerRegionHeight	Low-water area height of compound cross section. Only used if 'CSCCompound' is selected for CrossSectionType	Single	Optional
LowerRegionBaseWidth	Channel base width of low-water area of compound cross section. Only used if 'CSCCompound' is selected for CrossSectionType	Single	Optional
UpperRegionBaseWidth	Channel base width of high-water area of compound cross section. Only used if 'CSCCompound' is selected for CrossSectionType	Single	Optional
CompoundCSChannel-WidthLimit	Range of the channel base width limit applicable to channel compound cross section (Single cross section is applied for stream sections with a channel base width smaller than this limit). Only used if 'CSCCompound' is selected for CrossSectionType	Single	Optional
BankSideSlopeRight	Right bank slope	Single	Required
BankSideSlopeLeft	Left bank slope	Single	Required

Table 4.8 WatchPoints table

Field name	Description	Data type	Required
Name	Watch point name	String	Required
ColX	Watch point grid column number, numbering starts from top left corner (0,0). Max. value is 'column count - 1'	Integer	Required
RowY	Watch point grid row number, numbering starts from top left corner (0,0). Max. value is 'row count - 1'	Integer	Required

Table 4.9 FlowControlGrid table

Field name	Description	Data type	Required
Name	Name of FlowControlGrid	String	Required
ColX	Column number for FlowControlGrid, entered with the same method as for ColX in the WatchPoints table	Integer	Required
RowY	Row number for FlowControlGrid, entered with the same method as Row Y in the WatchPoints table	Integer	Required
ControlType	Flow control type (1 chosen from ReservoirOutflow, Inlet, SinkFlow, SourceFlow, and ReservoirOperation)	String	Required
DT	Flow data time interval (min)	Integer	Required
FlowDataFile	Flow data file path and name. Not used when ControlType is ReservoirOperation.	String	Optional
IniStorage	Reservoir initial storage ( $m^3$ ), only used when ControlType is not 'Inlet'	Single	Optional
MaxStorage	Reservoir maximum storage ( $m^3$ ), only used when ControlType is not 'Inlet'. If "0" or not set, storage is not calculated.	Single	Optional
NormalHighStorage	Reservoir storage for the normal high water level ( $m^3$ ).	Single	Optional
RestrictedStorage	Reservoir storage for the restricted water level (e.g. flood restricted water level) ( $m^3$ ).	Single	Optional
RestrictedPeriod_Start	The starting time of applying restricted water level storage. If the simulation uses 'DateTime' format, set this value as 'mmMddD'(06, 06M21D) format, if not use 'DateTime' format, set this value as the time (e.g. 120) after starting simulation(00) by hours unit	String	Optional
RestrictedPeriod_End	The ending time of applying restricted water level storage. Applying restricted water level storage includes this time. If the simulation uses 'DateTime' format, set this value as 'mmMddD'(06, 06M21D) format, if not use 'DateTime' format, set this value as the time (e.g. 120) after starting simulation(00) by hours unit	String	Optional
ROType	Reservoir operation type, only used when ControlType is 'ReservoirOperation' (1 chosen from AutoROM, RigidROM, and ConstantQ) ** Storage and discharge relation equation can be applied by 'SDEquation' option and source code writing is required.	String	Optional
ROConstQ	Constant discharge value (CMS), only used when ControlType is not 'Inlet' and also when ROType is ConstantQ	Single	Optional
ROConstQDuration	Constant discharge duration (h) from simulation starting. Only used when ControlType is not 'Inlet' and also when ROType is ConstantQ or RigidROM	Integer	Optional

Table 4.10 GreenAmptParameter table

Field name	Description	Data type	Required
GridValue	Grid value in soil texture raster file	Integer	Required
USERSoil	Name of soil texture attribute selected by user	String	Optional
GRMCode	Soil texture code (refer to "GRMCode" field value in GreenAmptSoilParameter table of the static DB)	String	Required
GRMTextureE	Soil texture English name (refer to "SoilTextureE" field value in GreenAmptSoilParameter table of the static DB)	String	Optional
GRMTextureK	Soil texture Korean name (refer to "SoilTextureK" field value in GreenAmptSoilParameter table of the static DB)	String	Optional
Porosity	Porosity	Single	Required
EffectivePorosity	Effective porosity	Single	Required
WFSoilSuctionHead	Wetting front soil suction head	Single	Required
HydraulicConductivity	Hydraulic conductivity	Single	Required

Table 4.11 SoilDepth table

Field name	Description	Data type	Required
GridValue	Grid value in the soil depth raster file	Integer	Required
UserDepthClass	Soil depth attribute name selected by user	String	Optional
GRMCode	Soil depth code (refer to "GRMCode" field value in the SoilDepthParameter table of the static DB)	String	Required
SoilDepthClassE	Soil depth English name (refer to "SoilDepthClassE" field value in the SoilDepthParameter table of the static DB)	String	Optional
SoilDepthClassK	Soil depth Korean name (refer to "SoilDepthClassK" field value in the SoilDepthParameter table of the static DB)	String	Optional
SoilDepth_cm	Soil depth value (cm) ** Under GRM v2020.1, "SoilDepth" was used. GRM v2020.5 and after, "SoilDepth_cm" is used	Single	Required

Table 4.12 LandCover table

Field name	Description	Data type	Required
GridValue	Grid value in land cover raster file	Integer	Required
UserLandCover	Land cover attribute name selected by user	String	Optional
GRMCode	Land cover code (refer to "GRMCode" field value in the LandCoverParameter table of the static DB)	String	Required
GRMLandCoverE	Land cover English name (refer to "LandCoverE" field value in the LandCoverParameter table of the static DB)	String	Optional
GRMLandCoverK	Land cover Korean name (refer to "LandCoverK" field value in the LandCoverParameter table of the static DB)	String	Optional
RoughnessCoefficient	Roughness coefficient	Single	Required
ImperviousRatio	Impervious ratio	Single	Required

#### 4.4 GRM Static database

The GRM static database saves the default reference values for soil and land cover parameters used in the GRM model in xml format(GRMStaticDB.xml). Default values for soil and land cover parameters can be selected using the GRM Static database when a GRM project file (.gmp) is created. Table descriptions and specifications for each table of the GRM static database are shown below.

Table 4.13 Table Definitions for a GRM Static xml file

Table name	Description
GreenAmptSoilParameter	Green-Ampt parameter values according to soil texture
SoilDepthParameter	Soil depth according to soil depth classification
LandCoverParameter	Roughness coefficient and impervious ratio according to land cover properties, land cover properties categorized into 7 are set as default values

Table 4.14 GreenAmptSoilParameter table

Field name	Description	Data type
SoilTextureE	Soil texture, English name	String
SoilTextureK	Soil texture, Korean name	String
GRMCode	Soil texture code	String
PorosityMin	Minimum porosity	Single
PorosityMax	Maximum porosity	Single
PorosityDefault	Default porosity	Single
EffectivePorosityMin	Minimum effective porosity	Single
EffectivePorosityMax	Maximum effective porosity	Single
EffectivePorosityDefault	Default effective porosity	Single
ResidualMoistureContent	Residual moisture content	Single
WFSoilSuctionHeadMin	Minimum wetting front soil suction head	Single
WFSoilSuctionHeadMax	Maximum wetting front soil suction head	Single
WFSoilSuctionHeadDefault	Default wetting front soil suction head	Single
HydraulicConductivity	Hydraulic conductivity	Single

Table 4.15 SoilDepthParameter table

Field name	Description	Data type
GRMCode	Soil depth code	String
SoilDepthClassE	Soil depth classification, English name	String
SoilDepthClassK	Soil depth classification, Korean name	String
SoilDepthMin	Minimum soil depth	Single
SoilDepthMax	Maximum soil depth	Single
SoilDepthDefault	Default soil depth	Single

Table 4.16 LandCoverParameter table

Field name	Description	Data type
LandCoverE	Land cover, English name	String
LandCoverK	Land cover, Korean name	String
GRMCode	Land cover code	String
RoughnessCoefficient	Roughness coefficient	Single
ImperviousRatio	Impervious ratio	Single

Attributes codes used in the GRM are shown below. Parameters values for each attributes can referred to GRMStaticDB.xml file.

Table 4.17 Soil texture code used in the GRM and soil texture name

GRMCode	SoilTextureE
C	Clay
CL	ClayLoam
L	Loam
LS	LoamySand
S	Sand
SC	SandyClay
SCL	SandyClayLoam
SiC	SiltyClay
SiCL	SiltyClayLoam
SiL	SiltLoam
SL	SandyLoam
USER	User defined attribute

Table 4.18 Soil depth code used in the GRM and soil depth name

GRMCode	SoilDepthClassE
D	Deep
M	ModeratelyDeepOrModeratelyShallow
S	Shallow
VD	VeryDeep
VS	VeryShallow
USER	User defined attribute

Table 4.19 Land cover code used in the GRM and land cover name

GRMCode	LandCoverE
AGRL	Agricultural Area
BARE	Bare
FRST	Forest
GRSS	Grass
URBN	Urban
WATR	Water
WTLD	Wetland
USER	User defined attribute



## 5. Output data

All hydrologic components of the GRM are calculated from every grid existing within a watershed and the user selects a grid cell for the calculation output, which becomes the 'watch point'. Calculations from a grid cell set as the watch point are saved as a text file if GRMSSimulationType is set as SingleEvent in ProjectSettings of the project file (gmp). If GRMSimulationType is set as RealTime, they are saved as a text file and database (SQL, etc.). And simulation results for all grids within a watershed can be saved as ASCII raster file and image (png) file (by using MakeIMGFile and MakeASCFile options in gmp file).

The calculation results mainly include discharge data. The water depth should not be calculated from a hydrologic model, such as the GRM, but from a hydraulic stream model or the water level–discharge relationship equation. The calculation result files created for each watch point include various results such as flow and saturation.

Table 5.1 GRM simulation output file

Simulation Type	Output File	Content
SingleEvent	[Project name]Discharge.out	Flow calculation results, mean rainfall, etc. for the watershed, and used calculation time for every watch point
	[Project name]FCData.out	Flow control discharge data for every flow control grid (only printed if the flow control is simulated)
	[Project name]FCStorage.out	Reservoir storage for every flow control grid (only printed if the flow control is simulated)
	[Project name]FCinflow.out	Reservoir inflow for every flow control grid (only printed if the flow control is simulated)
	[Project name]WP_ [watch point name].out	All calculation results for corresponding watch points (the files are created for every watch points)
RealTme	[Project name]RealTime_ [watch point name].out	Discharge, upstream watershed mean rainfall, etc. from corresponding watch points
	Saved in DB	Discharge, upstream watershed mean rainfall, etc. from corresponding watch points
ASCII file		Distributions of discharge, soil saturation ratio, incremental rainfall, and cumulative rainfall are saved in ASCII files
Image file		Distributions of discharge, soil saturation ratio, incremental rainfall, and cumulative rainfall are saved in png files

## 6. GRM Real Time

Real-time simulation using GRM can be performed using the "/r" option. However, it is recommended to modify and optimize at the source code level of the GRM model in order to build an optimized real-time system that reflects whether DBMS is used, the file and folder structure of the inputs, and the characteristics of the catchment system. (Under GRM v2020.1 developed by C#, the "/r" option is not provided, and the system must be developed using the real-time analysis module in GRMCore.dll or source code)

Real-time flow analysis uses distributed rainfall data created from real-time radar or watershed mean rainfall data collected in real time. Using distributed rainfall data requires clipping and resampling areas that correspond to topographical data of the targeted watershed with the same grid size and region.

Environmental parameters for real-time flow analysis are saved in xml text file (.REF). The REF file includes data on watershed system components and initial environmental conditions for flow simulations such as data on flow analysis project file (gmp), subwatersheds connected to the downstream (when multi-site calibration of connecting subwatersheds is used), real time flow control data, etc.

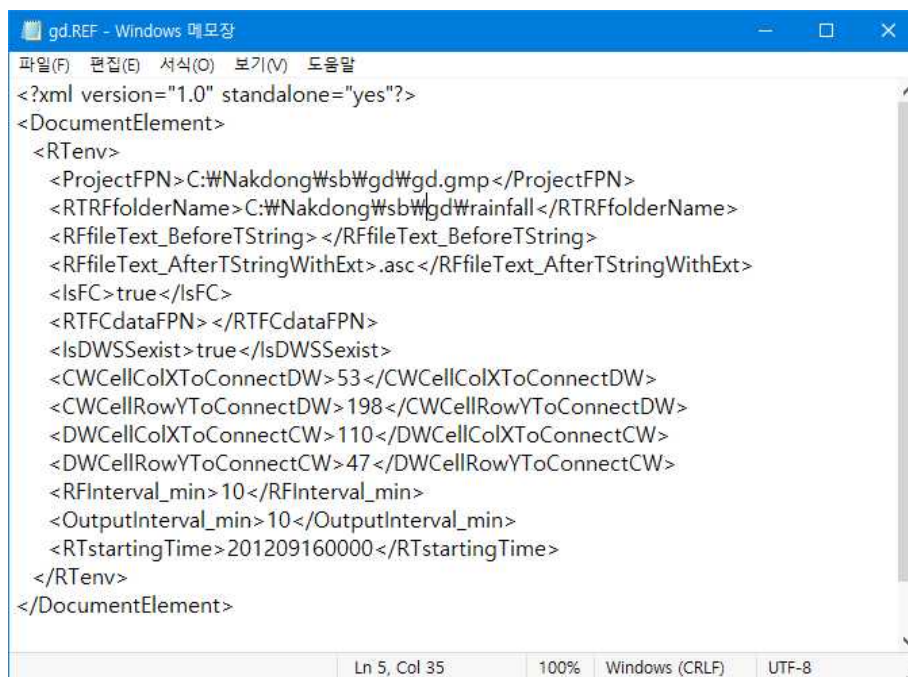


Fig. 6.1 Example of a REF file

Table 6.1 File (.REF) structure for real-time flow analysis settings

File name	Field name	Description
<i>[Project name].REF</i>	ProjectFPN	Name and path of the GRM project file (gmp) of the current watershed
	RTRFolderName	Real-time rainfall data receiving folder path
	RFileText_BeforeTString	Characters before the time identification text in the rainfall data file (For example, if the rainfall file name is RDR_201609021600_RKDP.asc, "RDR_")
	RFileText_AfterTStringWithExt	Characters after the time identification text in the rainfall data file (For example, if the rainfall file name is RDR_201609021600_RKDP.asc, "_RKDP.asc")
	IsFC	Whether FlowControlGrid is included or not
	RTFCdataFPN	The full path and name of the file where the real-time flow control data will be stored. **This term is not needed if real-time data from the DBMS will be used.
	IsDWSSexist	Whether subwatershed connected to the downstream exists or not
	CWCellColXToConnectDW	Column number of the current watershed grid that is to be connected with a downstream watershed
	CWCellRowYToConnectDW	Row number of the current watershed grid that is to be connected with a downstream watershed
	DWCellColXToConnectCW	Column number of a downstream watershed grid that is to be connected with the current watershed
	DWCellRowYToConnectCW	Row number of a downstream watershed grid that is to be connected with the current watershed
	RfInterval_min	Rainfall data time interval (min)
	OutputInterval_min	Output time interval (min)
	RTstartingTime	Real-time modeling start time (yyyymmddhhmm)

For real-time simulation using the "/r" option, a real-time simulation environment file (.REF) have to be specified to run the model (see Appendix 'How to Run').

In case of real-time simulation using the "/r" option, the real-time rainfall data shall be in a folder separated by month. That is, the rainfall files for September 2016 (for example, the "201609021600.asc" file) should all be in the "RTRFolderName#201609" folder. If you wish to change this rule, you must modify the source code of the GRM

model.

If real-time flow control information stored in a text file (.ref file's *RTFCdataFPN* field value) is used, "flow control point name", "date and time" and "flow" shall be stored separated by commas (,) as shown below. If there are multiple flow control points in one basin, store the real-time data of all points in the same file. However, if there are multiple real-time simulation processes (i.e., when multiple grm.exe are running), access to the data in one text file causes an error in relation to the file lock issue. Therefore, it is recommended to use DBMS when multiple real-time simulation processes are implemented in connection with each other.

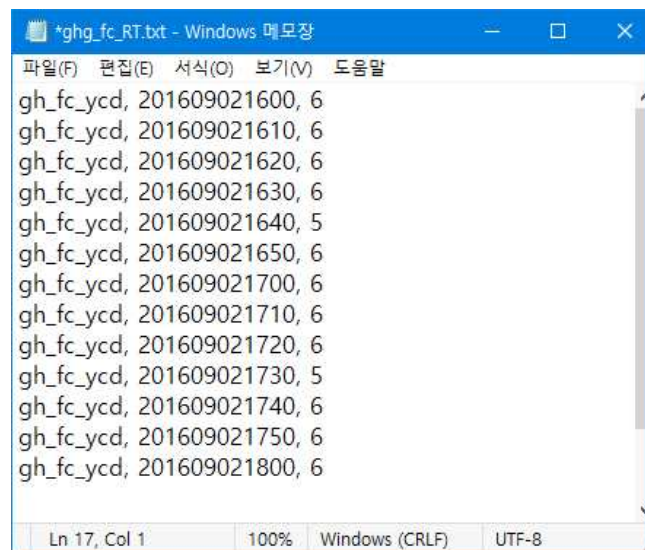


Fig. 6.2 An example of real-time flow control data file

## 7. Prediction

In order to predict future floods, all input time series data used in the flood simulation must be forecasts. If all the input time series data used in the GRM could be constructed using predictive data, this case can be simulated in the same way as in simulating common flood events (i.e., flood simulation of past events). In general, however, forecasted rainfall data are available (e.g., numerical forecasting data), but dam discharge data are difficult to predict.

In the GRM model (v.2020.05 or later), if rainfall data are available, but dam discharge data are not available. In the GRM model (v.2020.05 or later version), when forecasted rainfall data are available, but dam discharge data are not, the "/a" option can be used. Using the "/a" option, the GRM model uses all the data entered by the user (e.g., the dam discharge) in the flow control simulation, and if there is no more flow control data, the AutoROM is applied using the reservoirs specification. Therefore, when applying the "/a" option, specifications of all the reservoirs included in the runoff simulation must be entered.

If *ControlType* of *FlowControlGrid* is 'Inlet', the upper region of the reservoir is not be simulated. Therefore, if *ControlType* is 'Inlet', "/a" option can not be applied.



## References

- Ajami, N.K., Gupta, H., Wagener, T., Sorooshian, S. 2004. Calibration of a semi-distributed hydrologic model for streamflow estimation along a river system. *Journal of Hydrology*, 298, pp. 112-135.
- ASCE. 1996. River hydraulics. Technical engineering and design guides as adapted from the US Army Corps of Engineers, no. 18, ASCE Press, New York, pp. 58-61.
- Averjanov, S.F. 1950. About Permeability of Subsurface Soils in Case of Incomplete Saturation. *Eng. Collect.* 7, as Quoted by P. Ya. Polubarinova Kochina, *The Theory of Ground Water Movement*, English Translation by J.M. Roger De Wiest, 1962.
- Bras, R.L. 1990. *Hydrology : an introduction to hydrologic science*. Addison-Wesley publishing company, pp. 283-388.
- Beven, K. 1981. Kinematic subsurface stormflow. *Water Resources Research*, 17(5), pp. 1419-1424.
- Beven K.J., O'Connell P.E. 1982. On the role of a physically-based distributed modeling in hydrology. *Institute of Hydrology Report No.81*, Wallingford, UK, pp. 7-10.
- Beven, K.J., Kirkby, M.J. 1979. A physically based, variable contributing area model of basin hydrology. *Hydrological Sciences*, 24(1), pp. 43-69.
- Bouwer, H. 1966. Rapid field measurement of air entry value and hydraulic conductivity of soil as significant parameters in flow system analysis. *Water Resources Research* 2(4), pp. 729-738.
- Brakensiek, D.L., Engleman, R.L., Rawls, W.J. 1981. Variation within texture classes of soil water parameters. *Transactions of the American Society of Agricultural Engineers*, 24(2), pp. 335-339.
- Chaudhry, M.H. 1993. *Open-channel Flow*. Prentice-Hall, pp. 82-86.
- Choi, Y.S. 2010. Development and evaluation of a physically based distributed rainfall-runoff model embedded in GIS. *Doctoral dissertation*, Inha University, Incheon, Korea. pp. 19-26. (in Korean)
- Choi, Y.S., Choi, C.K., Kim, K.T. 2012. Development of a multi-site calibration module of distributed model -the case of GRM-. *Journal of Korean Association of Geographic Information Studies*, Korean Association of Geographic Information Studies, 15(3),

- pp.103-118. (in Korean)
- Choi, Y.S., Choi, C.K., Kim, H.S., Kim, K.T., Kim, S.J. 2015. Multi-site calibration using a grid-based event rainfall-runoff model: a case study of the upstream areas of the Nakdong River basin in Korea. *Hydrological Processes*, 29, pp. 2089-2099.
- Choi, Y.S., Je, Y.H., Kim, K.T., Kim, J.H. 2015. MapWindow Plug-in of GRM Model Using Open Source Software. *Proceedings of FOSS4G SEOUL 2015*.
- Chow, V.T. 1959. *Open-channel hydraulics*. McGraw-Hill, pp. 101-123.
- Chow, V.T., Maidment, D.R., Mays, L.W. 1988. *Applied hydrology*. McGraw-Hill, pp. 110-147.
- Dawson, C.W., Abrahart, R.J., Shamseldin, A.Y., Wilby, R.L. 2006. Flood estimation at ungauged sites using artificial neural networks. *Journal of Hydrology*, 319, pp. 391-409.
- Doherty, J. 2010. *PEST: Model-Independent Parameter Estimation User Manual : 5<sup>th</sup> Edition*. Watermark Numerical Computing, Australia.
- Dunne, T., Black R.D. 1970. An experimental investigation of runoff production in permeable soils. *Water Resources Research*, 6(2), pp. 478-490.
- Engman, E.T. 1986. Roughness coefficients for routing surface runoff. *Journal of Irrigation and Drainage Engineering*, 112(1), pp. 39-53.
- Fredlund, D.G., Xing, A., Huang, S. 1994. Predicting the permeability function for unsaturated soils using the soil-water characteristic curve. *Canadian Geotechnical Journal*, 31, pp. 533-546.
- Freeze, R.A., Cherry, J.A. 1979. *Groundwater*. Prentice Hall, Inc., New Jersey, pp. 15-236.
- Henderson, F.M. 1966. *Open channel flow*. Macmillan Publishing Co., Inc., New York, pp. 355-383.
- Horton, R.E. 1933. The role of infiltration of hydrologic cycle. *Transactions: American Geophysical Union*, 14, pp. 446-460.
- KICT. 2011a. *Development and Application of HyGIS*. Ministry of Education, Science and Technology. Korea Institute of Construction Technology (in Korean)
- KICT. 2011b. *HyGIS-GRM User's Manual*. Korea Institute of Construction Technology. (in Korean)
- Kim, K.T. 1998. *Analysis of Runoff Response Using GIS*. Doctoral dissertation, Inha



- University, Incheon, Korea. pp. 94-98. (in Korean)
- Merz, R., Blöschl, G. 2004. Regionalisation of catchment model parameters. *Journal of Hydrology*, 287, pp. 95-123.
- Ministry of Construction and Transportation. 2005. Stream design manual and. pp. 262-265. (in Korean)
- National Institute of Agricultural Science and Technology, 1992. An Introduction to Korean Soils-Supplement, Soil Survey Material 13, Rural Development Administration, pp. 283-290. (in Korean)
- Noh, S.J., An, H., Kim, S.H., Kim, H.J. 2015. Simulation of soil moisture on a hillslope using multiple hydrologic models in comparison to field measurements. *Journal of Hydrology*, 523, pp. 342-355.
- Noh, S.J., Choi, S.W., Choi, Y.S., Kim, K.T. 2014. Impact assessment of spatial resolution of radar rainfall and a distributed hydrologic model on parameter estimation. *Journal of the Korean Society of Civil Engineers*, 34(5), pp 1443-1454. (in Korean)
- Pilgrim, D.H. 1983. Some problems in transferring hydrological relationships between small and large drainage basins and between regions. *Journal of Hydrology*, 65, pp. 49-72.
- Ponce, V.C., Li, R.M., Simons, D.B. 1978. Applicability of kinematic and diffusion models. *Journal of the Hydraulics Division, ASCE*, 104(HY3), pp. 353-360.
- Rawls, W.J., Brakensiek, D.L., Miller, N. 1983. Green-Ampt infiltration parameters from soils data. *Journal of Hydarulic Engineering*, 109(1), pp. 62-70.
- Sakong, H.S. 2003. An empirical study on analysis method of impervious surface using IKONOS image. *The Journal of GIS Association of Korea*, 11, pp.509-518. (In Korean)
- Sloan, P.G., Moore, I.D. 1984. Modeling subsurface stormflow on steeply sloping forested watersheds. *Water Resources Research*, 20(12), pp. 1815-1822.
- Vieux, B.E. 2004. *Distributed Hydrologic Modeling Using GIS*. Kluwer Academic Publishers.
- Woolhiser, D.A., Liggett, J.A. 1967. Unsteady, one-dimensional flow over a plane - the rising hydrograph. *Water Resources Research*, 3(3), pp. 753-771.
- Young, A. 2006. Stream flow simulation within UK ungauged catchments using a daily rainfall-runoff model. *Journal of Hydrology*, 320, pp. 155-172.



## Appendix

### A. Example of a GRM project file (with one watershed and one watch point)

```
<?xml version="1.0" standalone="yes"?>
<GRMProject xmlns="http://tempuri.org/GRMProject.xsd">
  <ProjectSettings>
    <GRMSimulationType>SingleEvent</GRMSimulationType>
    <DomainFile>C:/GRM/SampleWC/Data/WiWatershed.asc</DomainFile>
    <SlopeFile>C:/GRM/SampleWC/Data/Wi_Slope_ST.asc</SlopeFile>
    <FlowDirectionFile>C:/GRM/SampleWC/Data/WiFDir.asc</FlowDirectionFile>
    <FlowAccumFile>C:/GRM/SampleWC/Data/WiFAc.asc</FlowAccumFile>
    <StreamFile>C:/GRM/SampleWC/Data/WiStream6.asc</StreamFile>
    <ChannelWidthFile />
    <LandCoverDataType>File</LandCoverDataType>
    <LandCoverFile>C:/GRM/SampleWC/Data/wilc200.asc</LandCoverFile>
    <LandCoverVATFile>C:/GRM/SampleWC/Data/wilc200.vat</LandCoverVATFile>
    <ConstantRoughnessCoeff />
    <ConstantImperviousRatio />
    <SoilTextureDataType>File</SoilTextureDataType>
    <SoilTextureFile>C:/GRM/SampleWC/Data/wistext200.asc</SoilTextureFile>
    <SoilTextureVATFile>C:/GRM/SampleWC/Data/wistext200.vat</SoilTextureVATFile>
    <ConstantSoilPorosity />
    <ConstantSoilEffPorosity />
    <ConstantSoilWettingFrontSuctionHead />
    <ConstantSoilHydraulicConductivity />
    <SoilDepthDataType>File</SoilDepthDataType>
    <SoilDepthFile>C:/GRM/SampleWC/Data/wisdepth200.asc</SoilDepthFile>
    <SoilDepthVATFile>C:/GRM/SampleWC/Data/wisdepth200.vat</SoilDepthVATFile>
    <ConstantSoilDepth />
    <InitialSoilSaturationRatioFile />
    <InitialChannelFlowFile />
    <RainfallDataType>TextFileMAP</RainfallDataType>
    <RainfallInterval>60</RainfallInterval>
    <RainfallDataFile>C:/GRM/SampleWC/Data/RF_MAP.txt</RainfallDataFile>
    <FlowDirectionType>StartsFromE_TauDEM</FlowDirectionType>
    <MaxDegreeOfParallelism>14</MaxDegreeOfParallelism>
    <SimulStartingTime>0</SimulStartingTime>
    <SimulationDuration>80</SimulationDuration>
    <ComputationalTimeStep>5</ComputationalTimeStep>
    <IsFixedTimeStep>true</IsFixedTimeStep>
    <OutputTimeStep>60</OutputTimeStep>
    <SimulateInfiltration>true</SimulateInfiltration>
```

```

<SimulateSubsurfaceFlow>true</SimulateSubsurfaceFlow>
<SimulateBaseFlow>true</SimulateBaseFlow>
<SimulateFlowControl>false</SimulateFlowControl>
<MakeIMGFile>false</MakeIMGFile>
<MakeASCFile>false</MakeASCFile>
<MakeSoilSaturationDistFile>true</MakeSoilSaturationDistFile>
<MakeRfDistFile>true</MakeRfDistFile>
<MakeRFaccDistFile>true</MakeRFaccDistFile>
<MakeFlowDistFile>true</MakeFlowDistFile>
<PrintOption>All</PrintOption>
<WriteLog>false</WriteLog>
<AboutThisProject />
<AboutWatershed />
<AboutLandCoverMap />
<AboutSoilMap />
<AboutSoilDepthMap />
<AboutRainfall />
<ProjectSavedTime>2018-2-14 16:34</ProjectSavedTime>
<ComputerName>CYS-PC</ComputerName>
<ComputerUserName>CYS</ComputerUserName>
<GRMVersion>2018.02</GRMVersion>
</ProjectSettings>
<SubWatershedSettings>
  <ID>1</ID>
  <IniSaturation>0.95</IniSaturation>
  <MinSlopeOF>0.0001</MinSlopeOF>
  <UnsaturatedKType>Linear</UnsaturatedKType>
  <CoefUnsaturatedK>0.2</CoefUnsaturatedK>
  <MinSlopeChBed>0.0005</MinSlopeChBed>
  <MinChBaseWidth>20</MinChBaseWidth>
  <ChRoughness>0.045</ChRoughness>
  <DryStreamOrder>0</DryStreamOrder>
  <IniFlow>94.8</IniFlow>
  <CalCoefLCRoughness>1</CalCoefLCRoughness>
  <CalCoefPorosity>1</CalCoefPorosity>
  <CalCoefWFSuctionHead>1</CalCoefWFSuctionHead>
  <CalCoefHydraulicK>1.2</CalCoefHydraulicK>
  <CalCoefSoilDepth>1</CalCoefSoilDepth>
  <UserSet>True</UserSet>
</SubWatershedSettings>
<ChannelSettings>
  <WSID>1</WSID>
  <CrossSectionType>CSSingle</CrossSectionType>
  <SingleCSChannelWidthType>CWEquation</SingleCSChannelWidthType>
  <ChannelWidthEQc>1.698</ChannelWidthEQc>
  <ChannelWidthEQd>0.318</ChannelWidthEQd>

```

```

<ChannelWidthEQe>0.5</ChannelWidthEQe>
<ChannelWidthMostDownStream>260</ChannelWidthMostDownStream>
<LowerRegionHeight>0</LowerRegionHeight>
<LowerRegionBaseWidth>0</LowerRegionBaseWidth>
<UpperRegionBaseWidth>0</UpperRegionBaseWidth>
<CompoundCSChannelWidthLimit>0</CompoundCSChannelWidthLimit>
<BankSideSlopeRight>1.5</BankSideSlopeRight>
<BankSideSlopeLeft>1.5</BankSideSlopeLeft>
</ChannelSettings>
<WatchPoints>
  <Name>MD</Name>
  <ColX>21</ColX>
  <RowY>39</RowY>
</WatchPoints>
<GreenAmptParameter>
  <GridValue>1</GridValue>
  <USERSoil>미사질식양토</USERSoil>
  <GRMCode>SiCL</GRMCode>
  <Porosity>0.471</Porosity>
  <EffectivePorosity>0.432</EffectivePorosity>
  <WFSoilSuctionHead>27.3</WFSoilSuctionHead>
  <HydraulicConductivity>0.1</HydraulicConductivity>
</GreenAmptParameter>
<GreenAmptParameter>
  <GridValue>2</GridValue>
  <USERSoil>미사질양토</USERSoil>
  <GRMCode>SiL</GRMCode>
  <Porosity>0.501</Porosity>
  <EffectivePorosity>0.486</EffectivePorosity>
  <WFSoilSuctionHead>16.68</WFSoilSuctionHead>
  <HydraulicConductivity>0.65</HydraulicConductivity>
</GreenAmptParameter>
<GreenAmptParameter>
  <GridValue>3</GridValue>
  <USERSoil>사양토</USERSoil>
  <GRMCode>SL</GRMCode>
  <Porosity>0.453</Porosity>
  <EffectivePorosity>0.412</EffectivePorosity>
  <WFSoilSuctionHead>11.01</WFSoilSuctionHead>
  <HydraulicConductivity>1.09</HydraulicConductivity>
</GreenAmptParameter>
<GreenAmptParameter>
  <GridValue>4</GridValue>
  <USERSoil>사토</USERSoil>
  <GRMCode>S</GRMCode>
  <Porosity>0.437</Porosity>

```

```

    <EffectivePorosity>0.417</EffectivePorosity>
    <WFSoilSuctionHead>4.95</WFSoilSuctionHead>
    <HydraulicConductivity>11.78</HydraulicConductivity>
  </GreenAmptParameter>
  <GreenAmptParameter>
    <GridValue>5</GridValue>
    <USERSoil>식토</USERSoil>
    <GRMCode>C</GRMCode>
    <Porosity>0.475</Porosity>
    <EffectivePorosity>0.385</EffectivePorosity>
    <WFSoilSuctionHead>31.63</WFSoilSuctionHead>
    <HydraulicConductivity>0.03</HydraulicConductivity>
  </GreenAmptParameter>
  <GreenAmptParameter>
    <GridValue>6</GridValue>
    <USERSoil>양질사토</USERSoil>
    <GRMCode>LS</GRMCode>
    <Porosity>0.437</Porosity>
    <EffectivePorosity>0.401</EffectivePorosity>
    <WFSoilSuctionHead>6.13</WFSoilSuctionHead>
    <HydraulicConductivity>2.99</HydraulicConductivity>
  </GreenAmptParameter>
  <GreenAmptParameter>
    <GridValue>7</GridValue>
    <USERSoil>양토</USERSoil>
    <GRMCode>L</GRMCode>
    <Porosity>0.463</Porosity>
    <EffectivePorosity>0.434</EffectivePorosity>
    <WFSoilSuctionHead>8.89</WFSoilSuctionHead>
    <HydraulicConductivity>0.34</HydraulicConductivity>
  </GreenAmptParameter>
  <SoilDepth>
    <GridValue>1</GridValue>
    <UserDepthClass>얕음</UserDepthClass>
    <GRMCode>S</GRMCode>
    <SoilDepth_cm>25</SoilDepth_cm>
  </SoilDepth>
  <SoilDepth>
    <GridValue>2</GridValue>
    <UserDepthClass>보통</UserDepthClass>
    <GRMCode>M</GRMCode>
    <SoilDepth_cm>75</SoilDepth_cm>
  </SoilDepth>
  <SoilDepth>
    <GridValue>3</GridValue>
    <UserDepthClass>깊음</UserDepthClass>

```

```

    <GRMCode>D</GRMCode>
    <SoilDepth_cm>125</SoilDepth_cm>
</SoilDepth>
<SoilDepth>
    <GridValue>4</GridValue>
    <UserDepthClass>매우얕음</UserDepthClass>
    <GRMCode>VS</GRMCode>
    <SoilDepth_cm>10</SoilDepth_cm>
</SoilDepth>
<LandCover>
    <GridValue>1</GridValue>
    <UserLandCover>수역</UserLandCover>
    <GRMCode>WATR</GRMCode>
    <RoughnessCoefficient>0.03</RoughnessCoefficient>
    <ImperviousRatio>1</ImperviousRatio>
</LandCover>
<LandCover>
    <GridValue>2</GridValue>
    <UserLandCover>시가화/건조지역</UserLandCover>
    <GRMCode>URBN</GRMCode>
    <RoughnessCoefficient>0.015</RoughnessCoefficient>
    <ImperviousRatio>0.853</ImperviousRatio>
</LandCover>
<LandCover>
    <GridValue>3</GridValue>
    <UserLandCover>나지</UserLandCover>
    <GRMCode>BARE</GRMCode>
    <RoughnessCoefficient>0.02</RoughnessCoefficient>
    <ImperviousRatio>0.442</ImperviousRatio>
</LandCover>
<LandCover>
    <GridValue>4</GridValue>
    <UserLandCover>습지</UserLandCover>
    <GRMCode>WTLD</GRMCode>
    <RoughnessCoefficient>0.07</RoughnessCoefficient>
    <ImperviousRatio>1</ImperviousRatio>
</LandCover>
<LandCover>
    <GridValue>5</GridValue>
    <UserLandCover>초지</UserLandCover>
    <GRMCode>GRSS</GRMCode>
    <RoughnessCoefficient>0.15</RoughnessCoefficient>
    <ImperviousRatio>0.44</ImperviousRatio>
</LandCover>
<LandCover>
    <GridValue>6</GridValue>

```

```
<UserLandCover>산림지역</UserLandCover>
<GRMCode>FRST</GRMCode>
<RoughnessCoefficient>0.1</RoughnessCoefficient>
<ImperviousRatio>0.05</ImperviousRatio>
</LandCover>
<LandCover>
  <GridValue>7</GridValue>
  <UserLandCover>농업지역</UserLandCover>
  <GRMCode>AGRL</GRMCode>
  <RoughnessCoefficient>0.035</RoughnessCoefficient>
  <ImperviousRatio>0.391</ImperviousRatio>
</LandCover>
</GRMProject>
```



## B. Example of a discharge output file (with one watch point)

Project name : SampleProject.gmp 2020-05-13 15:42 by GRM v.2020.5.12 Built in 2020-05-13 15:42

Output data : Discharge[CMS]

DataTime	[MD]	Rainfall_Mean	FromStarting[sec]
0	94.63	0	0
1	94.7	0.2	0
2	95.2	0.2	0
3	96.04	0.26	0
4	96.72	0.26	0
5	96.96	0.47	0
6	96.47	0.2	0
7	95.34	0	0
8	94.14	0.53	0
9	93.22	0.38	0
10	93.01	0.59	0
11	93.26	0.48	0
12	94.2	2.61	0
13	95.82	4.65	0
14	97.81	4.72	0
15	100.31	4.22	0
16	103.2	2.32	0
17	107.98	3.58	0
18	116.48	3.32	1
19	131.13	2.05	1
20	154.69	7.77	1
21	186.52	10.4	1
22	242.99	8.92	1
23	369.72	9.7	1
24	601.74	8.65	1
25	814.62	7.32	1
26	952.43	4.41	1
27	977.69	1.38	1
28	933.03	2.03	1
29	849.61	2.7	1
30	749.42	1.54	1
31	652.72	0.16	1
32	568.8	0.46	1
33	498.67	0.12	1
34	438.98	0.21	1
35	385.72	0.15	1
36	339.49	0.33	1
37	301.03	0.33	2
38	269.71	0.08	2

....

....

....

....

## C. Example of a GRM project file (with multiple watersheds and watch points, and a flow control)

```
<?xml version="1.0" standalone="yes"?>
<GRMProject xmlns="http://tempuri.org/GRMProject.xsd">
  <ProjectSettings>
    <GRMSimulationType>SingleEvent</GRMSimulationType>
    <DomainFile>C:/GRM/SampleGHG/watershed/GHG_Watershed.asc</DomainFile>
    <SlopeFile>C:/GRM/SampleGHG/watershed/GHG_Slope_ST.asc</SlopeFile>
    <FlowDirectionFile>C:/GRM/SampleGHG/watershed/GHG_FDir.asc</FlowDirectionFile>
    <FlowAccumFile>C:/GRM/SampleGHG/watershed/GHG_FAc.asc</FlowAccumFile>
    <StreamFile>C:/GRM/SampleGHG/watershed/GHG_Stream.asc</StreamFile>
    <ChannelWidthFile />
    <LandCoverDataType>File</LandCoverDataType>
    <LandCoverFile>C:/GRM/SampleGHG/watershed/GHG_Lc.asc</LandCoverFile>
    <LandCoverVATFile>C:/GRM/SampleGHG/watershed/GHG_Lc.vat</LandCoverVATFile>
    <ConstantRoughnessCoeff />
    <ConstantImperviousRatio />
    <SoilTextureDataType>File</SoilTextureDataType>
    <SoilTextureFile>C:/GRM/SampleGHG/watershed/GHG_SoilTexture.asc</SoilTextureFile>
    <SoilTextureVATFile>C:/GRM/SampleGHG/watershed/GHG_SoilTexture.vat</SoilTextureVATFile>
    <ConstantSoilPorosity />
    <ConstantSoilEffPorosity />
    <ConstantSoilWettingFrontSuctionHead />
    <ConstantSoilHydraulicConductivity />
    <SoilDepthDataType>File</SoilDepthDataType>
    <SoilDepthFile>C:/GRM/SampleGHG/watershed/GHG_SoilDepth.asc</SoilDepthFile>
    <SoilDepthVATFile>C:/GRM/SampleGHG/watershed/GHG_SoilDepth.vat</SoilDepthVATFile>
    <ConstantSoilDepth />
    <InitialSoilSaturationRatioFile />
    <InitialChannelFlowFile />
    <RainfallDataType>TextFileASCgrid</RainfallDataType>
    <RainfallInterval>10</RainfallInterval>
    <RainfallDataFile>C:/GRM/SampleGHG/ghg_rf_201609021600.txt</RainfallDataFile>
    <FlowDirectionType>StartsFromE_TauDEM</FlowDirectionType>
    <MaxDegreeOfParallelism>15</MaxDegreeOfParallelism>
    <SimulStartingTime>2016-09-02 16:00</SimulStartingTime>
    <SimulationDuration>56</SimulationDuration>
    <ComputationalTimeStep>5</ComputationalTimeStep>
    <IsFixedTimeStep>>false</IsFixedTimeStep>
    <OutputTimeStep>10</OutputTimeStep>
    <SimulateInfiltration>true</SimulateInfiltration>
    <SimulateSubsurfaceFlow>true</SimulateSubsurfaceFlow>
    <SimulateBaseFlow>true</SimulateBaseFlow>
    <SimulateFlowControl>true</SimulateFlowControl>
```

```

<MakeMGFile>true</MakeMGFile>
<MakeASCFile>true</MakeASCFile>
<MakeSoilSaturationDistFile>true</MakeSoilSaturationDistFile>
<MakeRfDistFile>true</MakeRfDistFile>
<MakeRFaccDistFile>true</MakeRFaccDistFile>
<MakeFlowDistFile>true</MakeFlowDistFile>
<PrintOption>All</PrintOption>
<WriteLog>>false</WriteLog>
<AboutThisProject />
<AboutWatershed />
<AboutLandCoverMap />
<AboutSoilMap />
<AboutSoilDepthMap />
<AboutRainfall />
<ProjectSavedTime />
<ComputerName>CYS-PC</ComputerName>
<ComputerUserName>CYS</ComputerUserName>
<GRMVersion>2018.03</GRMVersion>
</ProjectSettings>
<SubWatershedSettings>
  <ID>1</ID>
  <IniSaturation>0.93</IniSaturation>
  <MinSlopeOF>0.0001</MinSlopeOF>
  <UnsaturatedKType>Linear</UnsaturatedKType>
  <CoefUnsaturatedK>0.2</CoefUnsaturatedK>
  <MinSlopeChBed>0.0002</MinSlopeChBed>
  <MinChBaseWidth>50</MinChBaseWidth>
  <ChRoughness>0.045</ChRoughness>
  <DryStreamOrder>0</DryStreamOrder>
  <IniFlow>19</IniFlow>
  <CalCoefLCRoughness>1</CalCoefLCRoughness>
  <CalCoefPorosity>1</CalCoefPorosity>
  <CalCoefWFSuctionHead>1</CalCoefWFSuctionHead>
  <CalCoefHydraulicK>1.8</CalCoefHydraulicK>
  <CalCoefSoilDepth>1</CalCoefSoilDepth>
  <UserSet>True</UserSet>
</SubWatershedSettings>
<SubWatershedSettings>
  <ID>2</ID>
  <IniSaturation>0.93</IniSaturation>
  <MinSlopeOF>0.0001</MinSlopeOF>
  <UnsaturatedKType>Linear</UnsaturatedKType>
  <CoefUnsaturatedK>0.2</CoefUnsaturatedK>
  <MinSlopeChBed>0.0002</MinSlopeChBed>

```

```

<MinChBaseWidth>50</MinChBaseWidth>
<ChRoughness>0.045</ChRoughness>
<DryStreamOrder>0</DryStreamOrder>
<IniFlow>0</IniFlow>
<CalCoefLCRoughness>1</CalCoefLCRoughness>
<CalCoefPorosity>1</CalCoefPorosity>
<CalCoefWFSuctionHead>1</CalCoefWFSuctionHead>
<CalCoefHydraulicK>1.8</CalCoefHydraulicK>
<CalCoefSoilDepth>1</CalCoefSoilDepth>
<UserSet>False</UserSet>
</SubWatershedSettings>
<SubWatershedSettings>
  <ID>3</ID>
  <IniSaturation>0.93</IniSaturation>
  <MinSlopeOF>0.0001</MinSlopeOF>
  <UnsaturatedKType>Linear</UnsaturatedKType>
  <CoefUnsaturatedK>0.2</CoefUnsaturatedK>
  <MinSlopeChBed>0.0002</MinSlopeChBed>
  <MinChBaseWidth>50</MinChBaseWidth>
  <ChRoughness>0.045</ChRoughness>
  <DryStreamOrder>0</DryStreamOrder>
  <IniFlow>0</IniFlow>
  <CalCoefLCRoughness>1</CalCoefLCRoughness>
  <CalCoefPorosity>1</CalCoefPorosity>
  <CalCoefWFSuctionHead>1</CalCoefWFSuctionHead>
  <CalCoefHydraulicK>1.8</CalCoefHydraulicK>
  <CalCoefSoilDepth>1</CalCoefSoilDepth>
  <UserSet>False</UserSet>
</SubWatershedSettings>
<SubWatershedSettings>
  <ID>4</ID>
  <IniSaturation>0.93</IniSaturation>
  <MinSlopeOF>0.0001</MinSlopeOF>
  <UnsaturatedKType>Linear</UnsaturatedKType>
  <CoefUnsaturatedK>0.2</CoefUnsaturatedK>
  <MinSlopeChBed>0.0002</MinSlopeChBed>
  <MinChBaseWidth>50</MinChBaseWidth>
  <ChRoughness>0.045</ChRoughness>
  <DryStreamOrder>0</DryStreamOrder>
  <IniFlow>0</IniFlow>
  <CalCoefLCRoughness>1</CalCoefLCRoughness>
  <CalCoefPorosity>1</CalCoefPorosity>
  <CalCoefWFSuctionHead>1</CalCoefWFSuctionHead>
  <CalCoefHydraulicK>1.8</CalCoefHydraulicK>

```

```

    <CalCoefSoilDepth>1</CalCoefSoilDepth>
    <UserSet>False</UserSet>
  </SubWatershedSettings>
  <SubWatershedSettings>
    <ID>5</ID>
    <IniSaturation>0.93</IniSaturation>
    <MinSlopeOF>0.0001</MinSlopeOF>
    <UnsaturatedKType>Linear</UnsaturatedKType>
    <CoefUnsaturatedK>0.2</CoefUnsaturatedK>
    <MinSlopeChBed>0.0002</MinSlopeChBed>
    <MinChBaseWidth>50</MinChBaseWidth>
    <ChRoughness>0.045</ChRoughness>
    <DryStreamOrder>0</DryStreamOrder>
    <IniFlow>8.4</IniFlow>
    <CalCoefLCRoughness>1</CalCoefLCRoughness>
    <CalCoefPorosity>1</CalCoefPorosity>
    <CalCoefWFSuctionHead>1</CalCoefWFSuctionHead>
    <CalCoefHydraulicK>1.8</CalCoefHydraulicK>
    <CalCoefSoilDepth>1</CalCoefSoilDepth>
    <UserSet>True</UserSet>
  </SubWatershedSettings>
  <SubWatershedSettings>
    <ID>6</ID>
    <IniSaturation>0.93</IniSaturation>
    <MinSlopeOF>0.0001</MinSlopeOF>
    <UnsaturatedKType>Linear</UnsaturatedKType>
    <CoefUnsaturatedK>0.2</CoefUnsaturatedK>
    <MinSlopeChBed>0.0002</MinSlopeChBed>
    <MinChBaseWidth>50</MinChBaseWidth>
    <ChRoughness>0.045</ChRoughness>
    <DryStreamOrder>0</DryStreamOrder>
    <IniFlow>0</IniFlow>
    <CalCoefLCRoughness>1</CalCoefLCRoughness>
    <CalCoefPorosity>1</CalCoefPorosity>
    <CalCoefWFSuctionHead>1</CalCoefWFSuctionHead>
    <CalCoefHydraulicK>1.8</CalCoefHydraulicK>
    <CalCoefSoilDepth>1</CalCoefSoilDepth>
    <UserSet>False</UserSet>
  </SubWatershedSettings>
  <SubWatershedSettings>
    <ID>7</ID>
    <IniSaturation>0.93</IniSaturation>
    <MinSlopeOF>0.0001</MinSlopeOF>
    <UnsaturatedKType>Linear</UnsaturatedKType>

```

```

<CoefUnsaturatedK>0.2</CoefUnsaturatedK>
<MinSlopeChBed>0.0002</MinSlopeChBed>
<MinChBaseWidth>50</MinChBaseWidth>
<ChRoughness>0.045</ChRoughness>
<DryStreamOrder>0</DryStreamOrder>
<IniFlow>0</IniFlow>
<CalCoefLCRoughness>1</CalCoefLCRoughness>
<CalCoefPorosity>1</CalCoefPorosity>
<CalCoefWFSuctionHead>1</CalCoefWFSuctionHead>
<CalCoefHydraulicK>1.8</CalCoefHydraulicK>
<CalCoefSoilDepth>1</CalCoefSoilDepth>
<UserSet>False</UserSet>
</SubWatershedSettings>
<ChannelSettings>
  <WSID>1</WSID>
  <CrossSectionType>CSSingle</CrossSectionType>
  <SingleCSChannelWidthType>CWGeneration</SingleCSChannelWidthType>
  <ChannelWidthEQc>1.698</ChannelWidthEQc>
  <ChannelWidthEQd>0.318</ChannelWidthEQd>
  <ChannelWidthEQe>0.5</ChannelWidthEQe>
  <ChannelWidthMostDownStream>400</ChannelWidthMostDownStream>
  <LowerRegionHeight>0</LowerRegionHeight>
  <LowerRegionBaseWidth>0</LowerRegionBaseWidth>
  <UpperRegionBaseWidth>0</UpperRegionBaseWidth>
  <CompoundCSChannelWidthLimit>0</CompoundCSChannelWidthLimit>
  <BankSideSlopeRight>1.5</BankSideSlopeRight>
  <BankSideSlopeLeft>1.5</BankSideSlopeLeft>
</ChannelSettings>
<WatchPoints>
  <Name>GHG_md</Name>
  <ColX>9</ColX>
  <RowY>93</RowY>
</WatchPoints>
<WatchPoints>
  <Name>GHG_gh</Name>
  <ColX>82</ColX>
  <RowY>72</RowY>
</WatchPoints>
<WatchPoints>
  <Name>GHG_YCD</Name>
  <ColX>107</ColX>
  <RowY>42</RowY>
</WatchPoints>
<FlowControlGrid>

```

```

<Name>gh_fc_ycd</Name>
<ColX>107</ColX>
<RowY>42</RowY>
<ControlType>Inlet</ControlType>
<DT>10</DT>
<FlowDataFile>C:/GRM/SampleGHG/ghg_fc_ycd_201609021600.txt</FlowDataFile>
<IniStorage />
<MaxStorage />
<NormalHighStorage />
<RestrictedStorage/>
<RestrictedPeriod_Start />
<RestrictedPeriod_End />
<ROType />
<ROConstQ />
<ROConstQDuration />
</FlowControlGrid>
<GreenAmptParameter>
  <GridValue>1</GridValue>
  <USERSoil>미사질식양토</USERSoil>
  <GRMCode>SiCL</GRMCode>
  <Porosity>0.471</Porosity>
  <EffectivePorosity>0.432</EffectivePorosity>
  <WFSoilSuctionHead>27.3</WFSoilSuctionHead>
  <HydraulicConductivity>0.1</HydraulicConductivity>
</GreenAmptParameter>
<GreenAmptParameter>
  <GridValue>2</GridValue>
  <USERSoil>미사질양토</USERSoil>
  <GRMCode>SiL</GRMCode>
  <Porosity>0.501</Porosity>
  <EffectivePorosity>0.486</EffectivePorosity>
  <WFSoilSuctionHead>16.68</WFSoilSuctionHead>
  <HydraulicConductivity>0.65</HydraulicConductivity>
</GreenAmptParameter>
<GreenAmptParameter>
  <GridValue>3</GridValue>
  <USERSoil>사양토</USERSoil>
  <GRMCode>SL</GRMCode>
  <Porosity>0.453</Porosity>
  <EffectivePorosity>0.412</EffectivePorosity>
  <WFSoilSuctionHead>11.01</WFSoilSuctionHead>
  <HydraulicConductivity>1.09</HydraulicConductivity>
</GreenAmptParameter>
<GreenAmptParameter>

```

```

<GridValue>4</GridValue>
<USERSoil>사토</USERSoil>
<GRMCode>S</GRMCode>
<Porosity>0.437</Porosity>
<EffectivePorosity>0.417</EffectivePorosity>
<WFSoilSuctionHead>4.95</WFSoilSuctionHead>
<HydraulicConductivity>11.78</HydraulicConductivity>
</GreenAmptParameter>
<GreenAmptParameter>
  <GridValue>5</GridValue>
  <USERSoil>식양토</USERSoil>
  <GRMCode>CL</GRMCode>
  <Porosity>0.464</Porosity>
  <EffectivePorosity>0.309</EffectivePorosity>
  <WFSoilSuctionHead>20.88</WFSoilSuctionHead>
  <HydraulicConductivity>0.1</HydraulicConductivity>
</GreenAmptParameter>
<GreenAmptParameter>
  <GridValue>6</GridValue>
  <USERSoil>식토</USERSoil>
  <GRMCode>C</GRMCode>
  <Porosity>0.475</Porosity>
  <EffectivePorosity>0.385</EffectivePorosity>
  <WFSoilSuctionHead>31.63</WFSoilSuctionHead>
  <HydraulicConductivity>0.03</HydraulicConductivity>
</GreenAmptParameter>
<GreenAmptParameter>
  <GridValue>7</GridValue>
  <USERSoil>양질사토</USERSoil>
  <GRMCode>LS</GRMCode>
  <Porosity>0.437</Porosity>
  <EffectivePorosity>0.401</EffectivePorosity>
  <WFSoilSuctionHead>6.13</WFSoilSuctionHead>
  <HydraulicConductivity>2.99</HydraulicConductivity>
</GreenAmptParameter>
<GreenAmptParameter>
  <GridValue>8</GridValue>
  <USERSoil>양토</USERSoil>
  <GRMCode>L</GRMCode>
  <Porosity>0.463</Porosity>
  <EffectivePorosity>0.434</EffectivePorosity>
  <WFSoilSuctionHead>8.89</WFSoilSuctionHead>
  <HydraulicConductivity>0.34</HydraulicConductivity>
</GreenAmptParameter>

```



```

<SoilDepth>
  <GridValue>1</GridValue>
  <UserDepthClass>깊음</UserDepthClass>
  <GRMCode>D</GRMCode>
  <SoilDepth_cm>125</SoilDepth_cm>
</SoilDepth>
<SoilDepth>
  <GridValue>2</GridValue>
  <UserDepthClass>매우얕음</UserDepthClass>
  <GRMCode>VS</GRMCode>
  <SoilDepth_cm>10</SoilDepth_cm>
</SoilDepth>
<SoilDepth>
  <GridValue>3</GridValue>
  <UserDepthClass>보통</UserDepthClass>
  <GRMCode>M</GRMCode>
  <SoilDepth_cm>75</SoilDepth_cm>
</SoilDepth>
<SoilDepth>
  <GridValue>4</GridValue>
  <UserDepthClass>얕음</UserDepthClass>
  <GRMCode>S</GRMCode>
  <SoilDepth_cm>25</SoilDepth_cm>
</SoilDepth>
<LandCover>
  <GridValue>100</GridValue>
  <UserLandCover>시가화/건조지역</UserLandCover>
  <GRMCode>URBN</GRMCode>
  <RoughnessCoefficient>0.015</RoughnessCoefficient>
  <ImperviousRatio>0.853</ImperviousRatio>
</LandCover>
<LandCover>
  <GridValue>200</GridValue>
  <UserLandCover>농업지역</UserLandCover>
  <GRMCode>AGRL</GRMCode>
  <RoughnessCoefficient>0.035</RoughnessCoefficient>
  <ImperviousRatio>0.391</ImperviousRatio>
</LandCover>
<LandCover>
  <GridValue>300</GridValue>
  <UserLandCover>산림지역</UserLandCover>
  <GRMCode>FRST</GRMCode>
  <RoughnessCoefficient>0.1</RoughnessCoefficient>
  <ImperviousRatio>0.05</ImperviousRatio>

```

```
</LandCover>
<LandCover>
  <GridValue>400</GridValue>
  <UserLandCover>초지</UserLandCover>
  <GRMCode>GRSS</GRMCode>
  <RoughnessCoefficient>0.15</RoughnessCoefficient>
  <ImperviousRatio>0.44</ImperviousRatio>
</LandCover>
<LandCover>
  <GridValue>500</GridValue>
  <UserLandCover>습지</UserLandCover>
  <GRMCode>WTLD</GRMCode>
  <RoughnessCoefficient>0.07</RoughnessCoefficient>
  <ImperviousRatio>1</ImperviousRatio>
</LandCover>
<LandCover>
  <GridValue>600</GridValue>
  <UserLandCover>나지</UserLandCover>
  <GRMCode>BARE</GRMCode>
  <RoughnessCoefficient>0.02</RoughnessCoefficient>
  <ImperviousRatio>0.442</ImperviousRatio>
</LandCover>
<LandCover>
  <GridValue>700</GridValue>
  <UserLandCover>수역</UserLandCover>
  <GRMCode>WATR</GRMCode>
  <RoughnessCoefficient>0.03</RoughnessCoefficient>
  <ImperviousRatio>1</ImperviousRatio>
</LandCover>
</GRMProject>
```

## D. Example of a discharge output file (with multiple watch points)

Project name : GHG500.gmp 2020-05-13 15:43 by GRM v.2020.5.12 Built in 2020-05-13 15:43

Output data : Discharge[CMS]

DateTime	[GHG_md]	[GHG_gh]	[GHG_YCD]	Rainfall_Mean	FromStarting[sec]
2016-09-02 16:00	19	8.45	6	0	0
2016-09-02 16:10	19	8.55	6	0.03	0
2016-09-02 16:20	19	8.69	6	0.07	0
2016-09-02 16:30	19.06	8.85	6	0.19	0
2016-09-02 16:40	19.06	9.03	5	0.22	0
2016-09-02 16:50	19.06	9.22	6	0.2	0
2016-09-02 17:00	19.09	9.41	6	0.16	0
2016-09-02 17:10	19.08	9.6	6	0.2	0
2016-09-02 17:20	19.07	9.81	6	0.15	0
2016-09-02 17:30	19.13	10.01	5	0.2	0
2016-09-02 17:40	19.18	10.22	6	0.3	0
2016-09-02 17:50	19.2	10.41	6	0.22	0
2016-09-02 18:00	19.19	10.61	6	0.25	0
2016-09-02 18:10	19.21	10.8	6	0.26	0
2016-09-02 18:20	19.23	10.99	6	0.17	0
2016-09-02 18:30	19.27	11.19	6	0.26	0
2016-09-02 18:40	19.34	11.38	6	0.45	0
2016-09-02 18:50	19.45	11.58	6	0.62	0
2016-09-02 19:00	19.48	11.79	6	0.39	0
2016-09-02 19:10	19.54	12.01	6	0.44	0
2016-09-02 19:20	19.62	12.24	6	0.45	0
2016-09-02 19:30	19.91	12.46	6	0.44	0
2016-09-02 19:40	20.07	12.71	6	0.41	0
2016-09-02 19:50	20.04	12.96	6	0.39	0
2016-09-02 20:00	20.19	13.23	6	0.41	0
2016-09-02 20:10	20.23	13.5	6	0.44	0
2016-09-02 20:20	20.3	13.76	6	0.36	0
2016-09-02 20:30	20.42	14.04	5	0.44	0
2016-09-02 20:40	20.49	14.32	6	0.46	0
2016-09-02 20:50	20.54	14.61	6	0.49	0
2016-09-02 21:00	20.56	14.91	6	0.55	0
2016-09-02 21:10	20.6	15.23	6	0.35	0
2016-09-02 21:20	20.65	15.57	6	0.3	0
2016-09-02 21:30	20.72	15.96	6	0.22	0
2016-09-02 21:40	20.78	16.31	6	0.26	0
2016-09-02 21:50	20.84	16.66	6	0.29	0
2016-09-02 22:00	20.93	17.02	6	0.31	0
2016-09-02 22:10	21.08	17.38	6	0.29	0
2016-09-02 22:20	21.13	17.74	6	0.28	0
2016-09-02 22:30	21.19	18.11	6	0.32	0
2016-09-02 22:40	21.27	18.49	6	0.38	0
2016-09-02 22:50	21.34	18.87	6	0.36	0
2016-09-02 23:00	21.39	19.25	6	0.63	0
2016-09-02 23:10	21.44	19.72	6	0.59	0
2016-09-02 23:20	21.49	20.25	6	0.57	0
2016-09-02 23:30	21.54	20.7	6	0.42	0
2016-09-02 23:40	21.6	21.18	6	0.42	1
2016-09-02 23:50	21.67	21.69	6	0.35	1
2016-09-03 0:00	21.72	22.23	6	0.3	1

....

....

....

....

....

## E. Download

### 1. Download executable files (GRM v2020.5 or later)

(1) The GRM does not require a software installation process but it simply copies the dll and exe files to be used.

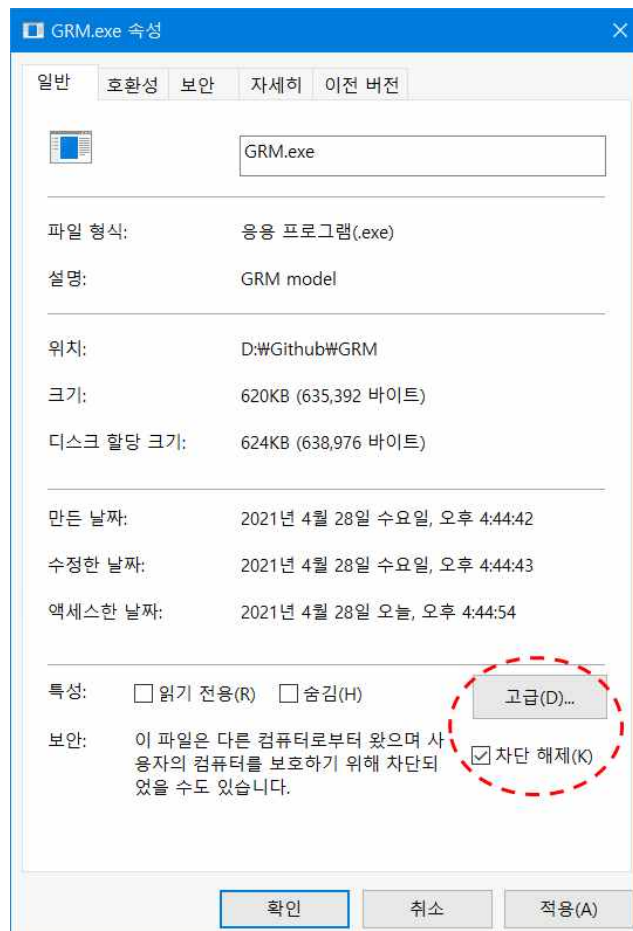
- Download the GRM executable file (GRM.exe) from

<https://github.com/floodmodel/GRM/tree/master/DownloadStableVersion>

- If you want to develop an GRM application using Python, you can use GRM.dll, and you can refer to the code below for how to use the APIs.

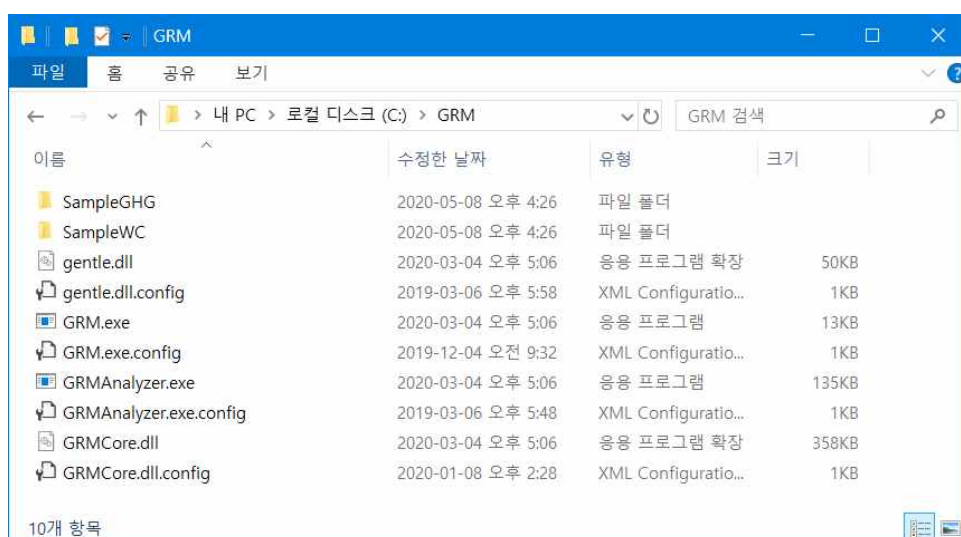
[https://github.com/floodmodel/GRM/tree/master/GRM\\_cpp/pyGRM/pyGRMdll.py](https://github.com/floodmodel/GRM/tree/master/GRM_cpp/pyGRM/pyGRMdll.py)

(2) Users have to check on 'Unblock' in file property window the downloaded executable files (exe, dll, zip, etc) from web.

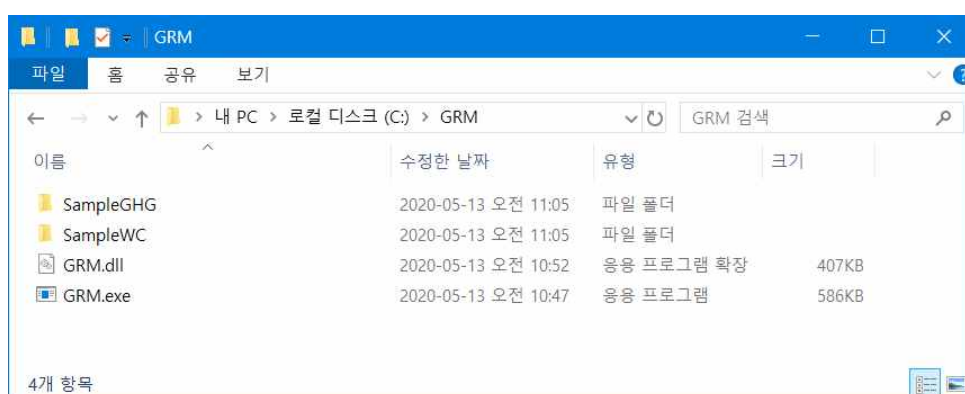


## 2. Download sample data

- (1) Download sample data (SampleGHG.zip, SampleWC.zip) from  
<https://github.com/floodmodel/GRM/tree/master/DownloadStableVersion>
- (2) Unzip the downloaded files to "C:\GRM". When the sample files are in another folder, the file paths in the gmp files (C:\GRM\Sample\SampleProject.gmp, C:\GRM\SampleGHG\GHG500.gmp) and rainfall file (C:\GRM\SampleGHG\ghg\_rf\_201609021600.txt) have to be changed.
- (3) The following is a picture of all executable files and sample data placed in the "C:\GRM" folder.



<Files and folders – under GRM v2020.1>



<Files and folders - GRM v2020.5 or later>

## **F. How to Run the GRM (console window)**

To run the GRM, either the menu in a modeling softwares (QGIS-GRM, GRMAnalyzer, etc.) can be used or the user can manually run it in a console window. The methods to run the model in console window are described below.

1. Input of spatial data, VAT file, and hydrological time-series data is required.
2. Make a gmp file by using a text editor or the QGIS-GRM.  
  
(GRM v2020.1 and v2020.5 and later, gmp files are compatible with each other.  
The gmp file is not compatible with the versions prior to GRM v2020.1)
3. In the console window, the gmp file is entered as the argument to run GRM.exe.

For example,

The execute statement when the GRM.exe file is in the 'C:\WGRM' folder and the SampleProject.gmp file is in the 'C:\WGRM\Sample' folder is as follows:

```
C:\WGRM>GRM.exe C:\WGRM\Sample\SampleProject.gmp
```

If there are spaces in the project file name or path, quotation marks "" are used to enclose it for input.

```
C:\WGRM>GRM.exe "C:\WGRM\Sample\Sample Project.gmp"
```

When the GRM.exe and gmp files are in the same folder, the project file path does not have to be entered. Thus, the following example shows how to run it when the GRM.exe and gmp files are in the 'C:\WGRM' folder.

```
C:\WGRM>GRM.exe SampleProject.gmp
```

When running it by entering "/f folder path," the GRM can run at once for all gmp files in the corresponding folder.

```
C:\WGRM>GRM.exe /f C:\WGRM\Sample
```

When "/fd folder path" is entered, the GRM can run at once for all gmp files in the corresponding folder and all files, except for the discharge file (\*discharge.out), are deleted. (So, you have to backup the gmp files before using this option.)

```
C:\WGRM>GRM.exe /fd C:\WGRM\Sample
```

"/?" can be entered for help.

```
C:\GRM>GRM.exe /?
```

Real-time simulation can be executed using the "/r" option. When applying the "/r" option, you should specify a real-time simulation environment file (.REF).

```
C:\GRM>GRM.exe /r C:\GRM\SampleWC\SampleProject_RT.REF
```

If "/a" option is applied, the cells set as "ReservoirOutflow", "SinkFlow", or "SourceFlow" ("Inlet" is exclusive) are converted to "ReservoirOperation" and "AutoROM" when there is no (more) flow data to apply.

```
C:\GRM>GRM.exe /a C:\GRM\SampleGHG\GHG500.gmp
```

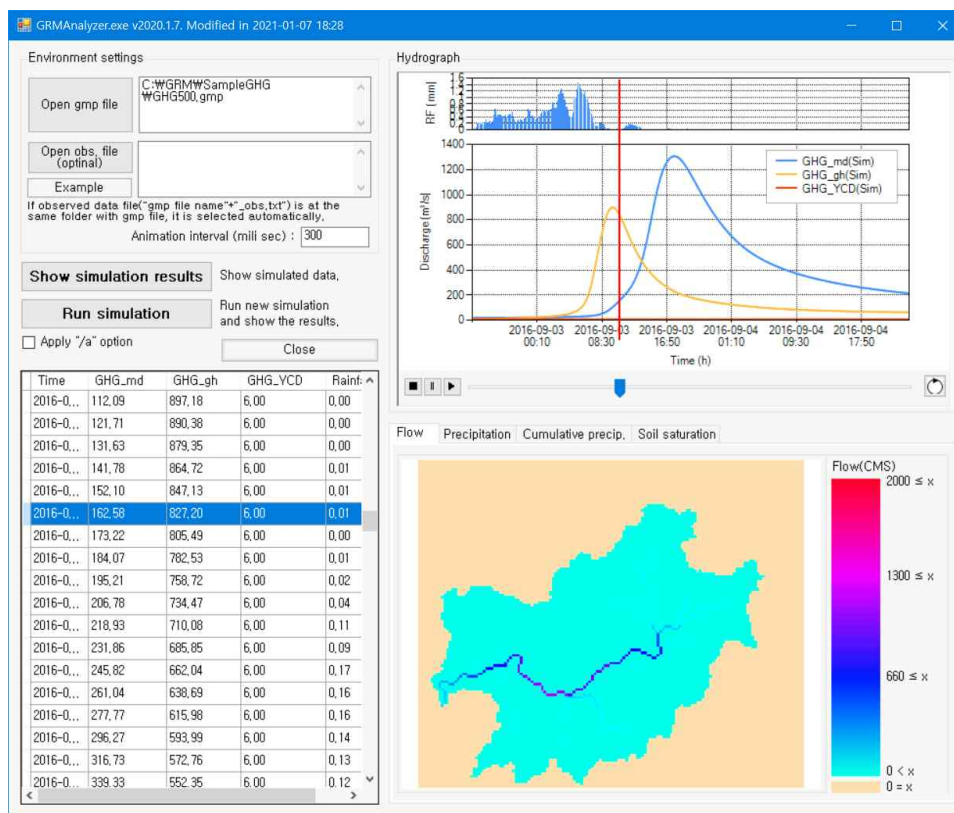
```
C:\GRM>GRM.exe /r /a C:\GRM\SampleWC\SampleProject_RT.REF
```

## G. How to Run the GRM (GRMAnalyzer)

GRMAnalyzer provides a convenient tool to compare the simulated results with observed data. (Only available when PrintOption is set as one of All, DischargeFile, or DischargeAndFcFile in the GRM project file(.gmp))

Download executable files(gentle.dll, GRM.exe, GRMAnalyzer.exe) from <https://github.com/floodmodel/GRM/tree/master/DownloadStableVersion>

1. Input of spatial data, VAT file, and hydrological time-series data is required.
2. Make a gmp file by using a text editor or the QGIS-GRM.
3. Run GRMAnalyzer.exe.
4. Click [Open gmp file] button and select a gmp file.
5. (Optional) Click [Open obs. file] button and select observed data file.
6. Start simulation using [Start simulation] button.
7. In the case of reviewing the simulated results, not of new simulation, click [Show simulation results] button.



<Running GRMAnalyzer>