GATE 2008 CIVIL ENGINEERING

EE25BTECH11013 - Bhargav

O.1 - O.20 Carry one mark each.

 $\frac{d^2y}{dx^2} + y = 0$

3) A mild steel specimen is under uni-axial tensile stress. Young's modulus and yield stress for mild steel are 2×10^5 MPa and 250 MPa respectively. The maximum amount of strain energy per unit volume that can be stored in this specimen without

4) A reinforced concrete structure has to be constructed along a sea coast. The minimum

c) M 25

grade of concrete to be used as per IS:456 - 2000 is

b) M 20

c) $y = P \sin x$ d) $y = P \sin^2 x$

c) 1.56 Nmm/mm³

d) 0.156 Nmm/mm³

(GATE CE 2008)

(GATE CE 2008)

(GATE CE 2008)

(GATE CE 2008)

d) M 30

(1)

1) The product of matrices $(PQ)^{-1}P$ is

2) The general solution of

a) $y = P \cos x + Q \sin x$

b) $y = P \cos x$

permanent set is

a) 156 Nmm/mm³

b) 15.6 Nmm/mm³

(GATE CE 2008)

a) M 15

a) P⁻¹
b) Q⁻¹
c) P⁻¹Q⁻¹P
d) PQ⁻¹

is

| 5) | In the design of a reinforced concrete beam, the requirement for bond is not |
|----|--|
| | getting satisfied. The economical option to satisfy the requirement for bond is by |
| | (GATE CE 2008) |
| | a) bundling of bars |
| | b) providing smaller diameter bars more in number |
| | c) providing larger diameter bars less in number |
| | d) providing same diameter bars more in number |

6) The shape of the cross-section, which has the largest shape factor, is

d) MS and CS

(GATE CE 2008)

| a) passive earth pressureb) swelling pressure | |
|--|--|
| c) pore pressured) active earth pressure | |
| | e best method of compaction in case of |
| a) moist silty sandb) well graded dry sand | c) clay of medium compressibilityd) silt of high compressibility |
| 10) A person standing on the bank of a car notices that the disturbance on the wate because the flow in the canal is | nal drops a stone on the water surface. He or surface in not traveling upstream. This is (GATE CE 2008) |
| a) sub-criticalb) super-critical | c) steady d) uniform |
| 11) A flood wave with a known inflow hyd The outflow hydrograph will have a) attenuated peak with reduced time-base b) attenuated peak with increased time-base c) increased peak with increased time-base d) increased peak with reduced time-base | (GATE CE 2008) se ase ase |
| 12) A stable channel is to be designed for a per Lacey's method. The mean flow ve (GATE CE 2008) | a discharge of Q m ³ /s with silt factor f as elocity (m/s) in the channel is obtained by |
| a) $(Qf^2/140)^{1/6}$ b) $(Qf/140)^{1/3}$ | c) $(Q^2 f^2 / 140)^{1/6}$ d) $0.48(Q/f)^{1/3}$ |
| | the of a gravity dam of height H is b . The turn is G and uplift pressure coefficient is K . It the heel is given by: $(GATE\ CE\ 2008)$ |

c) diamond

7) Group symbols assigned to silty sand and clayey sand are respectively

8) When a retaining wall moves away from the backfill, the pressure exerted on the

a) SS and CS b) SM and SC c) SM and CS

d) solid circular

a) rectangular

(GATE CE 2008)

wall is termed as

b) I-section

(GATE CE 2008)

| a) sulphur oxide and ozone | c) sulphur oxide and hydrocarbon |
|---|---|
| b) introgen oxide and peroxyacety | l nitrated) ozone and peroxyacetyl nitrate |
| 15) Two biodegradable components of | f municipal solid waste are: (GATE CE 2008) |
| a) plastics and wood | c) leather and tin cans |
| b) cardboard and glass | d) food wastes and garden trimmings |
| 16) The specific gravity of paving (GATE CE 2008) | bitumen as per IS:73 – 1992 lies between: |
| a) 1.10 and 1.06 | c) 1.02 and 0.97 |
| b) 1.06 and 1.02 | d) 0.97 and 0.92 |
| o, 5.00 3 | 2, 3, 3, 3, 3, 3, 2 |
| | and elongation index is to be determined for a ence in which the two tests are conducted is: |
| b) flakiness index test followed byc) flakiness index test followed by | by flakiness index test on the whole sample elongation index test on the whole sample elongation index test on non-flaky aggregates by flakiness index test on non-elongated aggregates |
| sons per hour and | ne-way 1.5m wide sidewalk per- One-way 2-lane urban road cess, no standing vehicles, and very little cross traffic) (GATE CE 2008) |
| a) 1.10 and 1.06 | c) 1.02 and 0.97 |
| b) 1.06 and 1.02 | d) 0.97 and 0.92 |
| 19) The shape of the STOP sign acco | rding to IRC:67 – 2001 is: (GATE CE 2008) |
| a) circular | c) octagonal |
| b) triangular | d) rectangular |
| 20) The type of surveying in which t called: | he curvature of the earth is taken into account is (GATE CE 2008) |
| a) Geodetic surveying | c) Preliminary surveying |

d) Topographical surveying

a) $\frac{b}{H} = \frac{1}{\sqrt{G-K}}$ b) $\frac{b}{H} = \sqrt{G-K}$ c) $\frac{b}{H} = \frac{1}{G-K}$ d) $\frac{b}{H} = \frac{1}{K\sqrt{G-K}}$

14) Two primary air pollutants are:

b) Plane surveying

21) The equation

$$k_x \frac{\partial^2 h}{\partial x^2} + k_z \frac{\partial^2 h}{\partial z^2} = 0 \tag{2}$$

can be transformed to

$$\frac{\partial^2 h}{\partial x_t^2} + \frac{\partial^2 h}{\partial z^2} = 0 \tag{3}$$

by substituting:

(GATE CE 2008)

a)
$$x_t = x \frac{k_z}{k_x}$$

b) $x_t = x \frac{k_x}{k}$

c)
$$x_t = x \sqrt{\frac{k_x}{k_z}}$$

d) $x_t = x \sqrt{\frac{k_z}{k_x}}$

22) The value of

$$\int_0^3 \int_0^{3-x} (6-x-y) \, dx \, dy \quad \text{is} \tag{4}$$

(GATE CE 2008)

a) 13.5

c) 40.5

b) 27.0

- d) 54.0
- 23) Three values of x and y are to be fitted in a straight line in the form y = a + bx by the method of least squares. Given: $\sum x = 6$, $\sum y = 21$, $\sum x^2 = 14$ and $\sum xy = 46$, (GATE CE 2008) the values of a and b are respectively:
 - a) 2 and 3

c) 2 and 1

b) 1 and 2

d) 3 and 2

24) Solution of

$$\frac{dy}{dx} = \frac{-x}{y} \tag{5}$$

at x = 1 and $y = \sqrt{3}$ is:

(GATE CE 2008)

a)
$$x^2 - y^2 = -2$$

c)
$$x^2 - y^2 = -2$$

d) $x^2 + y^2 = 4$

b)
$$x^2 + y^2 = 4$$

d)
$$x^2 + y^2 = 4$$

25) If the probability density function of a random variable X is

$$f(x) = \begin{cases} x^2, & \text{for } -1 \le x \le 1\\ 0, & \text{for any other value of } x \end{cases}$$
 (6)

then, the percentage probability $P\left(-\frac{1}{3} \le x \le \frac{1}{3}\right)$ is: (GATE CE 2008)

(7)

(GATE CE 2008)

| probability of using a private c choices available are bus and n a bus is 0.55. In such a situation | A person on a trip has a choice between private car and public transport. The probability of using a private car is 0.45. While using the public transport, further choices available are bus and metro, out of which the probability of commuting by a bus is 0.55. In such a situation, the probability (<i>rounded up to two decimals</i>) of using a car, bus and metro, respectively would be: (GATE CE 2008) | |
|--|---|---|
| a) 0.45, 0.30 and 0.25 b) 0.45, 0.25 and 0.30 | c) 0.45, 0.55 ar d) 0.45, 0.35 ar | |
| 28) The following simultaneous equ | uations: | |
| | x + y + z = 3 | (8) |
| | x + 2y + 3z = 4 | (9) |
| | x + 4y + kz = 6 | (10) |
| will NOT have a unique solution | on for k equal to: | (GATE CE 2008) |
| a) 0 b) 5 | c) 6 d) 7 | |
| 0) 3 | <i>a)</i> | |
| 29) The inner (<i>dot</i>) product of two the two vectors is: | vectors P and Q is zero. The | ne angle (degrees) between (GATE CE 2008) |
| a) 0 | c) 90 | |
| b) 30 | d) 120 | |
| 30) Cross-section of a column consisting of two steel strips, each of thickness t and width b is shown in the figure below. The critical loads of the column with perfect bond and without bond between the strips are P and P_0 respectively. The ratio P/P_0 is: (GATE CE 2008) | | |

c) 24.7

d) 247

c) 3 and 4

d) 1 and 2

 $\mathbf{P} = \begin{pmatrix} 4 & -5 \\ 2 & -5 \end{pmatrix}$

a) 0.247

b) 2.47

are:

a) -7 and 8

b) -6 and 5

26) The Eigen values of the matrix

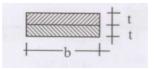


Fig. 30: Cross-section of 2 steel strips

a) 2

c) 6

b) 4

d) 8

31) A rigid bar GH of length L is supported by a hinge and a spring of stiffness K as shown in the figure below. The buckling load, $P_{C}r$, for the bar will be $(GATE\ CE\ 2008)$

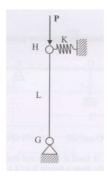


Fig. 31: Rigid Bar GH

a) 0.5*KL*

c) 1.0KL

b) 0.8*KL*

d) 1.2KL

32) The degree of static indeterminacy of the rigid frame having two internal hinges as shown in the figure below, is $(GATE\ CE\ 2008)$

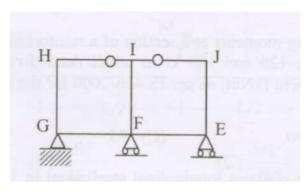


Fig. 32: Rigid Frame GHIJEF

- a) 8 c) 6 b) 7 d) 5
- 33) The members EJ and IJ of a steel truss shown in the figure below are subjected to a temperature rise of 30°C. The coefficient of thermal expansion of steel is 0.000012 per °C per unit length. The displacement (*mm*) of joint E relative to joint H along the direction HE of the truss, is

 (*GATE CE* 2008)

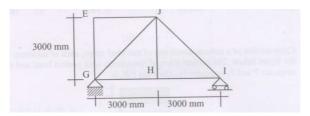


Fig. 33: Steel Truss

a) 0.255

c) 0.764

b) 0.589

- d) 1.026
- 34) The maximum shear stress in a solid shaft of circular cross-section having diameter d subjected to a torque T is τ . If the torque is increased by four times and the diameter of the shaft is increased by two times, the maximum shear stress in the shaft will be (GATE CE 2008)
 - a) 2τ

c) $\tau/2$

b) τ

- d) $\tau/4$
- 35) The span(s) to be loaded uniformly for maximum positive (upward) reaction at support P, as shown in the figure below, is(are) ($GATE\ CE\ 2008$)

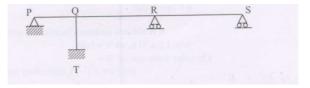


Fig. 35: Maximum reaction at P

a) PQ only

c) QR and RS

b) PQ and QR

- d) PQ and RS
- 36) A vertical rod PQ of length L is fixed at its top end P and has a flange fixed to the bottom end Q. A weight W is dropped vertically from a height h; L on to the flange. The axial stress in the rod can be reduced by

 (GATE CE 2008)

- a) increasing the length of the rod
- b) decreasing the length of the rod
- c) decreasing the area of cross-section of the rod
- d) increasing the modulus of elasticity of the material
- 37) Un-factored maximum bending moments at a section of a reinforced concrete beam resulting from a frame analysis are 50, 80, 120 and 180 kNm under dead, live, wind and earthquake loads respectively. The design moment (*kNm*) as per IS:456-2000 for the limit state of collapse (*flexure*) is (*GATE CE* 2008)
 - a) 195
 - b) 250
 - c) 345
 - d) 372
- 38) A reinforced concrete column contains longitudinal steel equal to 1 percent of net cross-sectional area of the column. Assume modular ratio as 10. The loads carried (using the elastic theory) by the longitudinal steel and the net area of concrete, are P_s , and P_c , respectively. The ratio P_s/P_c expressed as percent is (GATE CE 2008)
 - a) 0.1
 - b) 1
 - c) 1.1
 - d) 10
- 39) A pre-tensioned concrete member of section 200 mm x 250 mm contains tendons of area $500 \text{ } mm^2$ at centre of gravity of the section. The prestress in the tendons is $1000 \text{ N/}mm^2$. Assuming modular ratio as 10, the stress $\text{N/}mm^2$ in concrete is $(GATE \ CE \ 2008)$
 - a) 11
 - b) 9
 - c) 7
 - d) 5
- 40) Rivets and bolts subjected to both shear stress $\tau_{vf, cal}$ and axial tensile stress ($\sigma_{tf, cal}$) shall be so proportioned that the stresses do not exceed the respective allowable stresses τ_{vf} and σ_{tf} and the value of

$$\left(\frac{\tau_{vf, \text{ cal}}}{\tau_{vf}} + \frac{\sigma_{tf, \text{ cal}}}{\sigma_{tf}}\right) \tag{11}$$

does not exceed

(GATE CE 2008)

- a) 1.0
- b) 1.2
- c) 1.4
- d) 1.8
- 41) A continuous beam is loaded as shown in the figure below. Assuming a plastic moment capacity equal to M_p , the minimum load at which the beam would collapse is (GATE CE 2008)

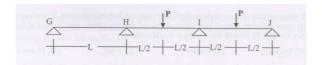


Fig. 41

- a) $\left(\frac{4M_p}{L}\right)$
- b) $\left(\frac{6M_p}{L}\right)$
- c) $\left(\frac{8M_p}{L}\right)$
- d) $\left(\frac{10M_p}{L}\right)$
- 42) The maximum tensile stress at the section X-X shown in the figure below is (GATE CE 2008)

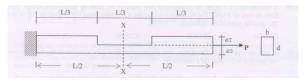


Fig. 42: Maximum tensile stress

- a) $\left(\frac{8P}{bd}\right)$
- b) $\left(\frac{6P}{bd}\right)$
- c) $\left(\frac{4P}{bd}\right)$
- d) $\left(\frac{2P}{bd}\right)$
- 43) The stepped cantilever is subjected to moments, M as shown in the figure below. The vertical deflection at the free end (neglecting the self weight) is (GATE CE 2008)

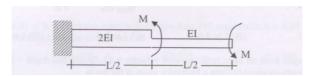


Fig. 43: Stepped cantilever

- a) $\left(\frac{ML^2}{8EI}\right)$
- b) $\left(\frac{ML^2}{4EI}\right)$
- c) $\left(\frac{ML^2}{2EI}\right)$
- d) Zero
- 44) The liquid limit (*LL*), plastic limit (*PL*) and shrinkage limit (*PL*) of a cohesive soil satisfy the relation (*GATE CE* 2008)
 - a) LL > PL < SL

c) LL < PL < SL

b) LL > PL > SL

- d) LL < PL > SL
- 45) A footing $2m \times 1m$ exerts a uniform pressure of 150 kN/ m^2 on the soil. Assuming a load dispersion of 2 vertical to 1 horizontal, the average vertical stress (kN/m) at 1.0 m below the footing is $(GATE\ CE\ 2008)$

(GATE CE 2008)

| internal friction = 0 . The skin of 0.6 , is | friction capacity (kN) of the pile for an adhesion factor $(GATE\ CE\ 2008)$ | |
|--|--|---|
| a) 671 | c) 283 | |
| b) 565 | d) 106 | |
| 48) A saturated clay stratum draining both at the top and bottom undergoes 50 per consolidation in 16 years under an applied load. If an additional drainage layer present at the middle of the clay stratum, 50 percent consolidation would occu (<i>GATE CE</i> 2008) | | |
| a) 2 years | c) 8 years | |
| b) 4 years | d) 16 years | |
| | ting on a sand deposit settles by $10mm$ under a certain $.50cm \times 200$ cm resting on the same sand deposit and sity settles by (GATE CE 2008) c) 30.2 mm d) 50.0 mm | 1 |
| 50) A volume of 3.0×10^6 m of groundwater was pumped out from an unconfined aquifer uniformly from an area of 5 km. The pumping lowered the water table from initial level of 102 m to 99 m. The specific yield of the aquifer is $(GATE\ CE\ 2008)$ | | 1 |
| a) 0.20 | c) 0.40 | |
| b) 0.30 | d) 0.50 | |
| 51) A weir on a permeable found below. The exit gradient as p | ation with downstream sheet pile is shown in the figure er Khosla's method is (GATE CE 2008) | |

c) 80

d) 100

c) 30.0

d) 32.6

46) A direct shear test was conducted on a cohesionless soil (c = 0) specimen under a normal stress of 200 kN/m?. The specimen failed at a shear stress of 100 kN/ m^2 .

47) A pile of 0.50m diameter and of length 10m is embedded in a deposit of clay. The undrained strength parameters of the clay are cohesion = 60kN/m and the angle of

The angle of internal friction of the soil (degrees) is

a) 55

b) 75

a) 26.6

b) 29.5

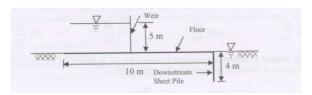


Fig. 51

a) 1 in 6.0

c) 1 in 3.4

b) 1 in 5.0

- d) 1 in 2.5
- 52) Water emerges from an ogee spillway with velocity = 13.72 m/s and depth = 0.3 m at its toe. The tail water depth required to form a hydraulic jump at the toe is ($GATE\ CE\ 2008$)
 - a) 6.48 m

c) 3.24 m

b) 5.24 m

- d) 2.24 m
- 53) The flow of water (mass density = 1000 kg/m^3 and kinematic viscosity = 10^{-6} m/s) in a commercial pipe, having equivalent roughness k_s , as 0.12 mm, yields an average shear stress at the pipe boundary = 600 N/m?. The value of k_s/δ (δ being the thickness of laminar sub layer) for this pipe is (GATE CE 2008)
 - a) 0.25 m

c) 6.0 m

b) 0.50 m

- d) 8.0 m
- 54) A river reach of 2.0 km long with maximum flood discharge of $10000 \text{ } m^3/s$ is to be physically modeled in the laboratory where maximum available discharge is 0.20 m/s. For a geometrically similar model based on equality of Froude number, the length of the river reach (m) in the model is (GATE CE 2008)
 - a) 26.4 m

c) 20.5 m

b) 25.0 m

- d) 18.0 m
- 55) An outlet irrigates an area of 20ha. The discharge (l/s) required at this outlet to meet the evapo-transpiration requirement of 20 mm occurring uniformly in 20 days neglecting other field losses is $(GATE\ CE\ 2008)$
 - a) 2.52 m

c) 2.01 m

b) 2.31 m

- d) 1.52 m
- 56) A wastewater sample contains $10^{-5.6}$ mmol/l of OH^- ions at 25 $^{\circ}$ C .The pH of this sample is (GATE CE 2008)

4. Starch

- a) 8.6 c) 5.6 b) 8.4 d) 5.4
- 57) Group *I* lists estimation methods of some of the water and wastewater quality parameters. Group *II* lists the indicators used in the estimation methods. Match the estimation method (Group *I*) with the corresponding indicator Group *II*). (*GATE CE* 2008)

Group II Group II

- P Azide modified Winkler method for dissolved oxygen 1. Eriochrome Black T O Dichromate method for chemical oxygen demand 2. Ferrion
- R EDTA titrimetric method for hardness 3. Potassium chromate
- S Mohr or Argentometric method for chlorides

Options:

- a) P-3, Q-2, R-1, S-4
- b) P-4, Q-2, R-1, S-3
- c) P-4, O-1, R-2, S-3
- d) P-4, Q-2, R-3, S-1
- 58) Determine the correctness or otherwise of the following Assertion [a] and the Reason [r]

Assertion: The crown of the outgoing larger diameter sewer is always matched with the crown of incoming smaller diameter sewer.

Reason: It eliminates backing up of sewage in the incoming smaller diameter sewer. ($GATE\ CE\ 2008$)

- a) Both [a] and [r] are true and [r] is the correct reason for [a]
- b) Both [a] and [r] are true but [r] is not the correct reason for [a]
- c) Both [a] and [r] are false
- d) Assertion [a] is true but Reason [r] is false
- 59) The 5-day BOD of a wastewater sample is obtained as 190 mg/l (with k = 0.01 h^{-1}). The ultimate oxygen demand (mg/l) of the sample will be (GATE CE 2008)
 - a) 3800
 - b) 475
 - c) 271
 - d) 190
- 60) A water treatment plant is required to process $28800 \ m^3/d$ of raw water (density = $1000 \ kg/m^3$, kinematic viscosity = $10 \ ms$). The rapid mixing tank imparts a velocity gradient of $900 \ s^{-1}$ to blend 35 mg/l of alum with the flow for a detention time of 2 minutes. The power input (W) required for rapid mixing is (GATE CE 2008)
 - a) 32.4
 - b) 36
 - c) 324
 - d) 32400
- 61) Match Group I (*Terminology*) with Group II (*Definition/BriefDescription*) for wastewater treatment systems (*GATE CE* 2008)

Group I

- P. Primary treatment
- Q. Secondary treatment
- R. Unit operation
- S. Unit process

Group II

- 1. Contaminant removal by physical forces
- 2. Involving biological and/or chemical reaction
- 3. Conversion of soluble organic matter to biomass
- 4. Removal of solid materials from incoming wastewater

Options:

- a) P-4, Q-3, R-1, S-2
- b) P-4, Q-3, R-2, S-1
- c) P-3, Q-4, R-2, S-1
- d) P-1, Q-2, R-3, S-4
- 62) A roundabout is provided with an average entry width of 8.4 m, width of weaving section as 14 m, and length of the weaving section between channelizing islands as 35 m. The crossing traffic and total traffic on the weaving section are 1000 and 2000 PCU per hour respectively. The nearest rounded capacity of the roundabout (in PCU per hour) is (GATE CE 2008)
 - a) 3800
 - b) 475

- c) 271
- d) 190
- 63) Design parameters for a signalized intersection are shown in the figure below. The green time calculated for major and minor roads are 34 s and 18 s, respectively. (*GATE CE* 2008)

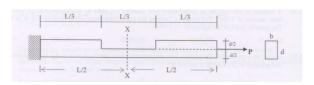


Fig. 63

The critical lane volume on the major road changes to 440 vehicles per hour per lane and the critical lane volume on the minor road remains unchanged. The green time will

- a) increase for the major road and remain same for the minor road
- b) increase for the major road and decrease for the minor road
- c) decrease for both the roads
- d) remain unchanged for both the roads
- 64) It is proposed to widen and strengthen an existing 2-lane NH section as a divided highway. The existing traffic in one direction is 2500 commercial vehicles (*CV*) per day. The construction will take 1 year. The design CBR of soil subgrade is found to

be 5 percent. Given: traffic growth rate for CV = 8 percent, vehicle damage factor = 3.5 (standard axles per CV), design life = 10 years and traffic distribution factor = 0.75. The cumulative standard axles (msa) computed are ($GATE\ CE\ 2008$)

a) 35

c) 65

b) 37

d) 70

- 65) A linear relationship is observed between speed and density on a certain section of a highway. The free flow speed is observed to be 80 km per hour and the jam density is estimated as 100 vehicles per km length. Based on the above relationship, the maximum flow expected on this section and the speed at the maximum flow will respectively be

 (GATE CE 2008)
 - a) 8000 vehicles per hour and 80 km perc) 2000 vehicles per hour and 80 km per hour
 - b) 8000 vehicles per hour and 25 km perd) 2000 vehicles per hour and 40 km per hour
- 66) The plan of a survey plotted to a scale of 10 m to 1 cm is reduced in such a way that a line originally 10 cm long now measures 9 cm. The area of the reduced plan is measured as 81 cm^2 ?. The actual area m^2 of the survey is (GATE CE 2008)

a) 10000

c) 1000

b) 6561

d) 656

67) The lengths and bearings of a closed traverse PQRSP are given below.

| Line | Length (m) | Bearing (WCB) |
|------|------------|---------------|
| PQ | 200 | 0° |
| QR | 1000 | 45° |
| RS | 907 | 180° |
| SP | ? | ? |

The missing length and bearing, respectively, of the line SP are (GATE CE 2008)

- a) $207m \text{ and } 270^{\circ}$
- b) $707m \text{ and } 270^{\circ}$
- c) $707m \text{ and } 180^{\circ}$
- d) 907m and 270°
- 68) The focal length of the object glass of a tacheometer is 200 mm, the distance between the vertical axis of the tacheometer and the optical centre of the object glass is 100 mm and the spacing between the upper and lower line of the diaphragm axis is 4 mm. With the line of collimation perfectly horizontal, the staff intercepts are 1 m (top), 2 m (middle), and 3 m (bottom). The horizontal distance (m) between the staff and the instrument station is

| , | , |
|---|---|
| and centre line radius of 250 m. the circular curve of such a leng | ntal degreeular curve having deflection angle of 55° A transition curve is to be provided at each end of the that the rate of gain of radial acceleration is 0.3 c. Length of the transition curve required at each of (GATE CE 2008) |
| a) 2.57 | c) 35.73 |
| b) 33.33 | d) 1666.67 |
| | |
| | is just visible above the horizon from a ship. The e ship and the light house considering combined action, (GATE CE 2008) |
| a) 39.098 | c) 39098 |
| b) 42.226 | d) 42226 |
| Statement for Linked Answer Questions 71 and 72: A rectangular channel 6.0 m wide carries a discharge of 16.0 m ³ /s under uniform flow condition with normal depth of 1.60 m. Manning <i>n</i> is 0.015. 71) The longitudinal slope of the channel is (GATE CE 2008) a) 0.000585 b) 0.000485 c) 0.000385 d) 0.000285 | |
| 72) A hump is to be provided on the without affecting the upstream flo | e channel bed. The maximum height of the hump ow condition is (GATE CE 2008) |
| a) 0.50 m | |
| b) 0.40 m | |
| c) 0.30 m d) 0.20 m | |
| | |

73) The channel width is to be contracted. The minimum width to which the channel can be contracted without affecting the upstream flow condition is (*GATE CE* 2008)

A reinforced concrete beam of rectangular cross section of breadth 230 mm and effective depth 400mm is subjected to a maximum factored shear force of $120 \ kN$. The grades of concrete, main steel and stirrup steel are M20, Fe415 and Fe250

Statement for Linked Answer Questions 74 and 75:

c) 150.0

d) 153.0

a) 100.3

b) 103.0

a) 3.0 mb) 3.8 mc) 4.1 md) 4.5 m

respectively. For the area of main steel provided, the design shear strength τ_c as per IS:456 – 2000 is 0.48 N/mm². The beam is designed for collapse limit state.

- 74) The spacing (mm) of 2-legged 8 mm stirrups to be provided is (GATE CE 2008)
 - a) 40
 - b) 115
 - c) 250
 - d) 400
- 75) In addition, the beam is subjected to a torque whose factored value is 10.90 kNm. The stirrups have to be provided to carry a shear kN equal to (GATE CE 2008)
 - a) 50.42
 - b) 130.56
 - c) 151.67
 - d) 200.23

Linked Answer Questions: Q.76 to Q.85 carry two marks each

Statement for Linked Answer Questions 76 and 77:

Beam GHI is supported by three pontoons as shown in the figure below. The horizontal cross-sectional area of each pontoon is $8 m^2$, the flexural rigidity of the beam is $10000 \text{ kN-}m^2$ and the unit weight of water is $10 \text{ kN/}m^3$.

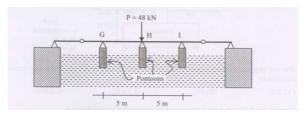


Fig. 75

- 76) When the middle pontoon is removed, the deflection at H will be (GATE CE 2008)
 - a) 0.2 m
 - b) 0.4 m
 - c) 0.6 m
 - d) 0.8 m
- 77) When the middle pontoon is brought back to its position as shown in the figure above, the reaction at H will be (GATE CE 2008)
 - a) 8.6 kN
 - b) 15.7 kN
 - c) 19.2 kN
 - d) 24.2 kN

Statement for Linked Answer Questions 78 and 79:

The ground conditions at a site are shown in the figure below.

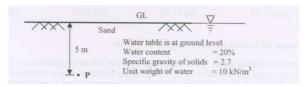


Fig. 77

78) The saturated unit weight of the sand (kN/m^3) is

(GATE CE 2008)

- a) 15
- b) 18
- c) 21
- d) 24
- 79) The total stress, pore water pressure and effective stress (kN/m^3) at the point P are, $(GATE\ CE\ 2008)$
 - a) 75, 50 and 25
 - b) 90, 50 and 40
 - c) 105, 50 and 55
 - d) 120, 50 and 70

Statement for Linked Answer Questions 80 and 81:

A column is supported on a footing as shown in the figure below. The water table is at a depth of 10 m below the base of the footing.

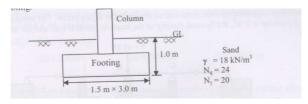


Fig. 79

- 80) The net ultimate bearing capacity (kN/m^2) of the footing based on Terzaghi's bearing capacity equation is $(GATE\ CE\ 2008)$
 - a) 216
 - b) 432
 - c) 630
 - d) 846
- 81) The safe load (kN) that the footing can carry with a factor of safety 3 is $(GATE\ CE\ 2008)$
 - a) 282
 - b) 648
 - c) 945
 - d) 1269

Statement for Linked Answer Questions 82 and 83:

An automobile with projected area 2.6 m² is running on a road with a speed of 120 km per hour. The mass density and the kinematic viscosity of air are 1.2 kg/m³ and 1.5×10^{-5} m²/s, respectively. The drag coefficient is 0.30.

82) The drag force on the automobile is

(*GATE CE* 2008)

- a) 620 N
- b) 600 N
- c) 580 N
- d) 520 N
- 83) The metric horse power required to overcome the drag force is (GATE CE 2008)
 - a) 33.23
 - b) 31.23
 - c) 23.23
 - d) 20.23

Statement for Linked Answer Questions 84 and 85:

A horizontal circular curve with a centre line radius of 200 m is provided on a 2-lane, 2-way SH section. The width of the 2-lane road is 7.0 m. Design speed for this section is 80 km per hour. The brake reaction time is 2.4 s, and the coefficients of friction in longitudinal and lateral directions are 0.355 and 0.15, respectively.

84) The safe stopping sight distance on the section is

(*GATE CE* 2008)

- a) 221 m
- b) 195 m
- c) 125 m
- d) 65 m
- 85) The set-back distance from the center line of the inner lane is (GATE CE 2008)
 - a) 7.93 m
 - b) 8.10 m
 - c) 9.60 m
 - d) 9.77 m