

06. For the system shown in the given figure the moment of inertia of the weight W and the ball about the pivot point is I_0 . The natural frequency of the system is given by

$$f_n = \frac{1}{2\pi} \sqrt{\frac{Ka^2 - Wb}{I_0}}$$

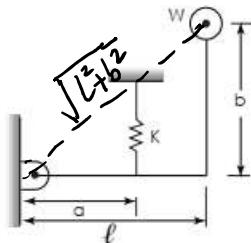
$$f_n > 0$$

$Ka^2 - Wb > 0$
only then system will oscillate

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$$f_n = (1/2\pi) \sqrt{(Ka^2 - Wb)/I_0}$$

The system will vibrate when



$$b < \frac{Ka^2}{W}$$

$$I = \frac{W}{g} (L^2 + b^2)$$

- (a) $b < (Ka^2/W)$
(b) $b = (Ka^2/W)$
(c) $b > (Ka^2/W)$
(d) $a = 0$

W is responsible for δ_{static}
given position itself can be assumed as equilibrium position.

$$\leq M_{\text{Hinge}} = 0$$

$$I\ddot{\theta} + F_s \cos\theta - WL \cos\theta - Wb \sin\theta = 0$$

$$I\ddot{\theta} + k(\delta_{\text{static}} + a\theta) \alpha - WL - Wb\theta = 0$$

$$W(L^2 + b^2)\ddot{\theta} + k\delta_{\text{static}}\alpha + ka^2\theta - WL - Wb\theta = 0$$

constant terms

$$\frac{W}{g} (L^2 + b^2)\ddot{\theta} + (Ka^2 - Wb)\theta = 0$$

$$\ddot{\theta} + \frac{Ka^2 - Wb}{\frac{W}{g} (L^2 + b^2)} \theta = 0$$

$$\omega_n = \sqrt{\frac{Ka^2 - Wb^2}{\frac{W}{g} (L^2 + b^2)}}$$

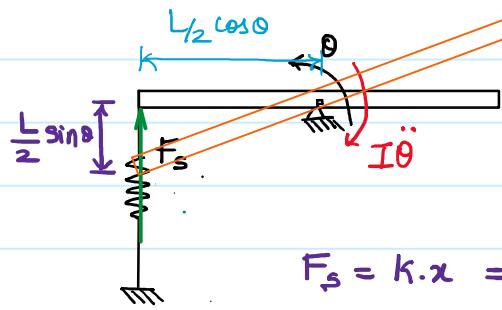
05. A uniform rigid rod of mass $m = 1\text{kg}$ and length $L = 1\text{m}$ is hinged at its centre & laterally supported at one end by a spring of spring constant $k = 300\text{N/m}$.

- The natural frequency ω_n in rad/s is (GATE-08)
(a) 10
(b) 20
(c) 30
(d) 40

Disp. variable - θ

$$DOF = 1$$

$$\sin\theta \approx \theta$$



$$F_s = kx = k \cdot \frac{L}{2} \sin\theta$$

$$= (kL/2)\theta$$

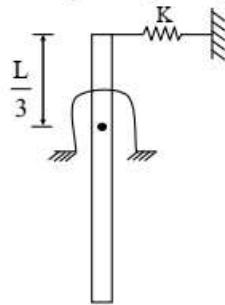
$$\leq M_{\text{Hinge}} = 0$$

$$I\ddot{\theta} + F_s \cdot L/2 \cos\theta = 0$$

$$\frac{ML^2}{12}\ddot{\theta} + (k \cdot L/2 \cdot \theta) \cdot L/2 = 0 \Rightarrow \frac{ML^2}{12}\ddot{\theta} + \frac{kL^2}{4}\theta = 0 \Rightarrow \ddot{\theta} + \frac{3k}{M}\theta = 0$$

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09. A slender link of mass 'm' and length L is pivoted at a length of $L/3$ from the top and is connected as shown in figure and it oscillates in the vertical plane. The stiffness of the spring is 'K'. The natural frequency of the system is given by

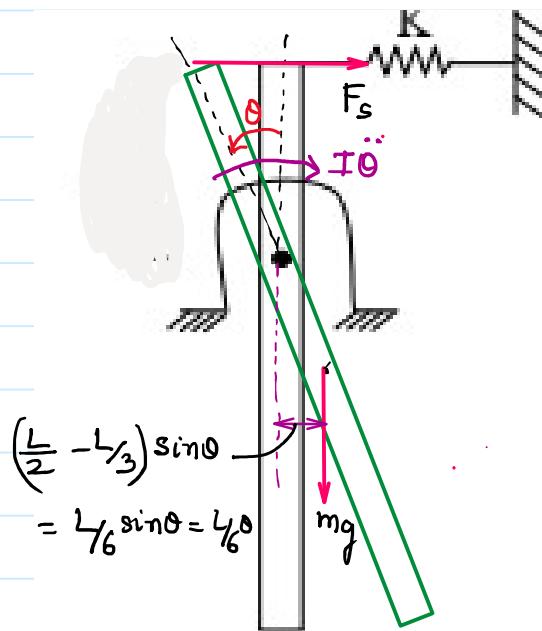


- (a) $\sqrt{\frac{3g}{2L} - \frac{K}{m}}$
 (b) $\sqrt{\frac{3g}{2L} + \frac{K}{m}}$
 (c) $\sqrt{\frac{g}{L} + \frac{K}{m}}$
 (d) $\sqrt{\frac{g}{L} - \frac{K}{m}}$

$$I = I_0 + md^2$$

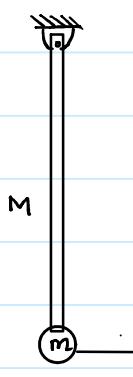
$$= \frac{ML^2}{12} + M\left(\frac{L}{6}\right)^2$$

$$= \frac{ML^2}{9}$$



$$I_0 \ddot{\theta} + (F_s \times L/3) + mg \cdot L/6 \cdot \theta = 0$$

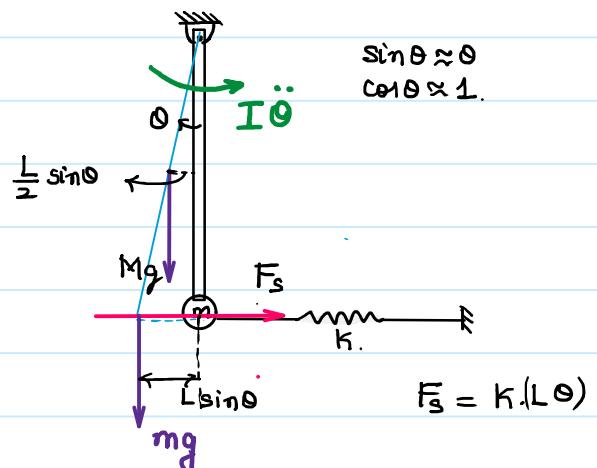
$$\frac{ML^2}{9} \ddot{\theta} + \frac{KL^2}{9} \theta + mg \cdot L/6 \cdot \theta = 0 \Rightarrow \ddot{\theta} + \left(\frac{K}{M} + \frac{3g}{2L} \right) \theta = 0$$



DOF = 1
 Disp. Variable - θ .

$$I = I_{Rod} + I_{Point mass}$$

$$= \left(\frac{ML^2}{3} + m \cdot L^2 \right)$$



$$\sum M_{Hinge} = 0$$

$$\left(\frac{ML^2}{3} + m \cdot L^2 \right) \ddot{\theta} + Mg \cdot L/2 \cdot \theta + mg \cdot L \cdot \theta + F_s \cdot L = 0$$

$$\left(\frac{ML^2}{3} + m \cdot L^2 \right) \ddot{\theta} + KL^2 \theta + \left(\frac{M}{2} + m \right) g \cdot L \cdot \theta = 0 \Rightarrow \ddot{\theta} + \frac{KL^2 + \left(\frac{M}{2} + m \right) g \cdot L \cdot \theta}{\left(\frac{ML^2}{3} + m \cdot L^2 \right)} = 0$$



Wt. of Rod will
produce Static



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$$\sin \theta = 0$$

$$\sum M_{\text{hinge}} = 0$$

$$I\ddot{\theta} + F_{S_1} \cdot L_{S_2} \cdot \cos\theta + F_{S_2} \cdot L \cos\theta = 0$$

$$\frac{ML^2}{3}\theta + (k_1 \cdot L_2 \cdot \theta) L_2 + (k_2 L \theta) \cdot L = 0$$

$$\frac{ML^2}{3}\ddot{\theta} + (k_1\frac{L^2}{4} + k_2L^2)\dot{\theta} = 0$$

$$\ddot{\theta} + \left(\frac{3k_1}{4M} + \frac{3k_2}{M} \right) \theta = 0$$

$$\sum M_{\text{Ref. point}} = 0$$

$$I_{\text{Ref-point}} = \ddot{\theta} + k(\perp^{\text{hor.}} \text{distance from ref-point})^2 \pm mg(\perp^{\text{hor.}} \text{distance from ref-point})\dot{\theta}$$

$$\frac{M L^2}{3} \ddot{\theta} + k_1 (L_2)^2 \theta + k_2 (L)^2 \theta = 0$$

10. A spring mass system with a natural frequency of 5 rad/sec is set in to free oscillations by giving an initial displacement of 10 cm. The amplitude of vibration is

$$x(t) = X_0 \cdot \sin(\omega t + \phi)$$

$$\text{At } t=0 \quad x(t=0) = 10 \text{ cm.} = X_0 \cdot \sin(0 + \phi)$$

$$10 \text{ cm} = x_0 \cdot \sin \phi$$

$$\textcircled{a} \quad t = 0 \quad \dot{x}(t=0) = 0$$

initial velocity = 0

$$\ddot{x}(t) = X_0 \cdot \omega \cdot \cos(\omega t + \phi)$$

$$O = X_0 \cdot x_5 \cos(\theta + \phi)$$

$$X_0 \cdot \cos \phi = 0$$

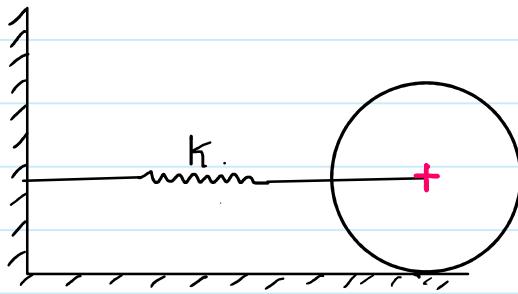
$$\cos \phi = 0$$

-Amplitude.

$$\phi = 90^\circ$$

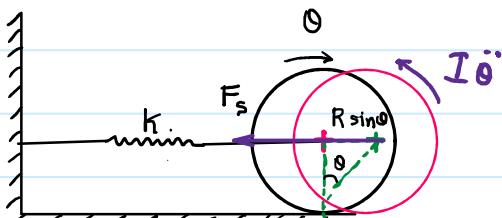
$$10 = x_0 \cdot \sin 90^\circ \Rightarrow x_0 = 10 \text{ cm}$$

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$$\sin \theta \approx \theta$$

$$\cos \theta \approx 1$$



$$\sum M_{P.O.C} = 0$$

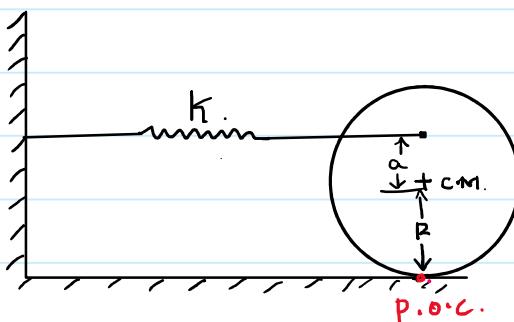
$$I_{P.O.C} \cdot \ddot{\theta} + F_s \times R \cdot \cos \theta = 0$$

$$\left(\frac{M R^2}{2} + M R^2\right) \ddot{\theta} + (K \cdot R \theta) \cdot R = 0$$

$$R \sin \theta = R \dot{\theta}$$

$$\frac{3}{2} M R^2 \ddot{\theta} + K R^2 \theta = 0$$

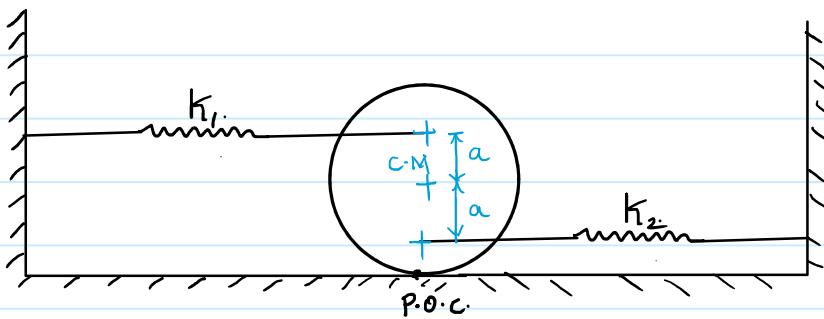
$$\ddot{\theta} + \frac{2K}{3M} \theta = 0$$



$$\sum M_{P.O.C} = 0$$

$$I_{P.O.C} \ddot{\theta} + K \cdot (\perp \text{ from P.O.C})^2 \theta = 0 \Rightarrow \frac{3}{2} M R^2 \ddot{\theta} + K \cdot (R+a)^2 \theta = 0$$

$$\ddot{\theta} + \frac{2K \cdot (R+a)^2}{3MR^2} \theta = 0$$



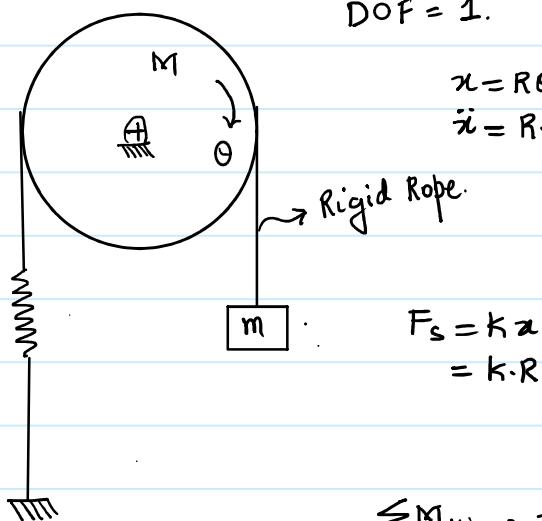
$$\leq M_{P.O.C} = 0$$

$$I_{P.O.C.} \ddot{\theta} + k_1(R+a)^2 \theta + k_2(R-a)^2 \theta = 0$$

$$\frac{3}{2}MR^2 \ddot{\theta} + [k_1(R+a)^2 + k_2(R-a)^2] \theta = 0$$

$$\ddot{\theta} + \frac{2 \cdot [k_1(R+a)^2 + k_2(R-a)^2]}{3MR^2} \theta = 0$$

DOF = 1.



$$x = R\theta$$

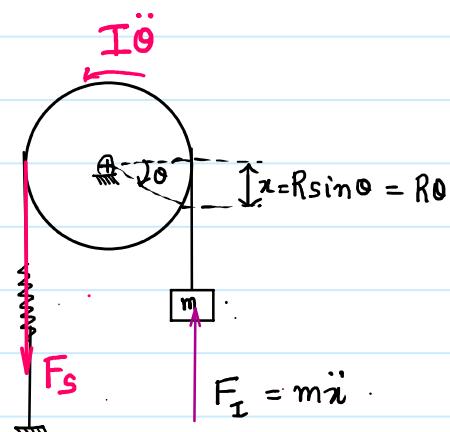
$$\dot{x} = R\dot{\theta}$$

$$m$$

Rigid Rope.

$$F_s = kx$$

$$= k \cdot R\theta$$



$$F_I = m\ddot{x}$$

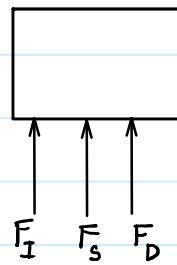
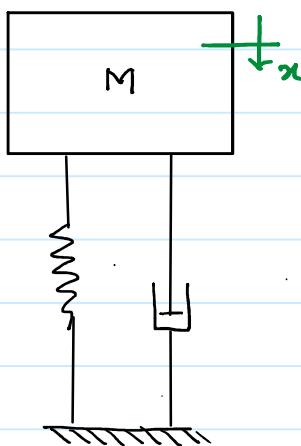
$$\leq M_{Hinge} = 0$$

$$I_{Hinge} \ddot{\theta} + F_I \times R + F_s \times R = 0$$

$$\frac{MR^2}{2} \ddot{\theta} + (mR\ddot{\theta} \times R) + k \cdot R^2 \theta = 0$$

$$\ddot{\theta} + \frac{k}{(m+\frac{M}{2})} \theta = 0$$

Free Damped Vibrations — Loss in energy after each cycle.



$$\sum F = 0$$

$$F_I + F_s + F_D = 0$$

$$M\ddot{x} + C\dot{x} + Kx = 0$$

$$\ddot{x} + \frac{C}{M}\dot{x} + \frac{K}{M}x = 0$$

$$\ddot{x} + \frac{C}{2\sqrt{MK}} \cdot 2\sqrt{K} \cdot \dot{x} + \omega_n^2 \cdot x = 0$$

$$M\ddot{x} + C\dot{x} + Kx = 0$$

$$\ddot{x} + 2\zeta\omega_n\dot{x} + \omega_n^2 x = 0$$

$$a=1 \quad b=2\zeta\omega_n \quad c=\omega_n^2$$

$$\frac{d^2}{dt^2} + 2\zeta\omega_n \cdot \frac{dx}{dt} + \omega_n^2 \cdot x = 0 \Rightarrow (D^2 + 2\zeta\omega_n D + \omega_n^2) x = 0$$

$$D = -2\zeta\omega_n \pm \sqrt{(2\zeta\omega_n)^2 - 4(1)\cdot\omega_n^2}$$

$$D = -\zeta\omega_n \pm \sqrt{(\zeta\omega_n)^2 - \omega_n^2}$$

$$D = \omega_n \left[-\zeta \pm \sqrt{\zeta^2 - 1} \right]$$

$$\zeta = 1 \quad D = -\omega_n, -\omega_n \quad \text{complimentary function}$$

$$x(t) = (A + Bt)e^{-\omega_n t}$$

$$\zeta > 1 \quad D = \underbrace{\omega_n \left[-\zeta + \sqrt{\zeta^2 - 1} \right]}_{-\alpha_1}, \quad \underbrace{\omega_n \left[-\zeta - \sqrt{\zeta^2 - 1} \right]}_{-\alpha_2}$$

$\zeta = 1, \zeta > 1$ system is coming to rest without executing oscillations.

$$\text{C.F. } x(t) = A e^{-\alpha_1 t} + B e^{-\alpha_2 t}$$

$$\zeta < 1 \quad D = \omega_n \left[-\zeta + i\sqrt{1-\zeta^2} \right], \quad \omega_n \left[-\zeta - i\sqrt{1-\zeta^2} \right]$$

$$D = -\zeta\omega_n \pm i\omega_n\sqrt{1-\zeta^2} \quad D = -\zeta\omega_n \pm \omega_d$$

$$\text{C.F. } x(t) = [A \sin(\omega_d t) + B \cos(\omega_d t)] e^{-\zeta\omega_n t}$$

$$x(t) = X_0 \cdot \sin(\omega_d t + \phi) e^{-\zeta\omega_n t}$$

$\zeta < 1$ system is coming to rest by executing decaying oscillations.

$$A = X_0 \cos \phi \quad \text{Loss in energy.}$$

$$B = X_0 \sin \phi$$

(No loss in energy)

$$\zeta = 0 \quad x(t) = X_0 \cdot \sin(\omega_n t + \phi) \quad \text{undamped system.}$$

$$\checkmark \quad \zeta < 1 \quad x(t) = X_0 e^{-\zeta \omega_n t} \sin(\omega_n t + \phi)$$

\downarrow Loss in energy \downarrow underdamped System.

$$\zeta = 1.$$

$$x(t) = (A + Bt) e^{-\omega_n t}$$

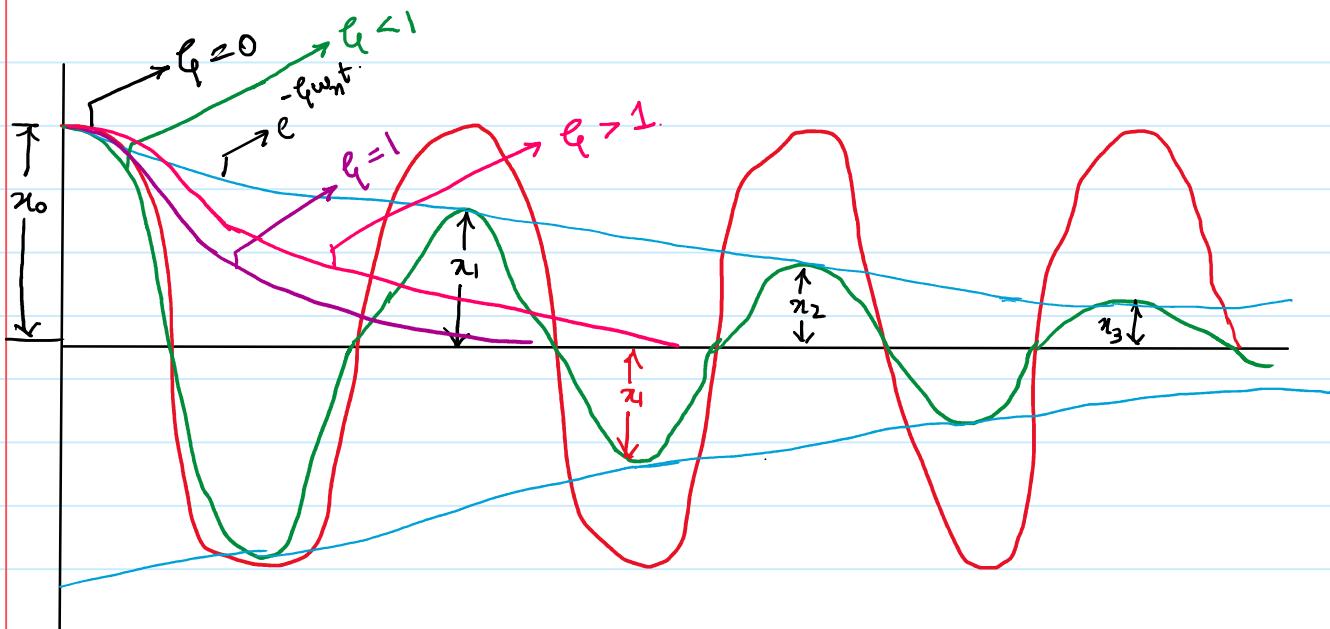
$$\zeta > 1$$

$$x(t) = A e^{-\alpha_1 t} + B e^{-\alpha_2 t}$$

critically damped system.

Overdamped system.

→ aperiodic/non-periodic response.



ζ - Damping Ratio/Damping factor.

$$\zeta = \frac{c}{2\sqrt{Mk}} = \frac{\text{Actual damping coeff.}}{\text{Critical damping coeff.}}$$

Damping factor is defined as Ratio of Actual damping to critical damping. Damping factor also represents the % of critical damping.

$$\zeta = \frac{c}{c_c} = \frac{c}{2\sqrt{Mk}}$$

$$c_c = 2\sqrt{M^2 \cdot k} = 2 \cdot M \cdot \omega_n$$

$$\zeta < 1. \quad x(t) = x_0 e^{-\zeta \omega_n t} \sin(\omega_d t + \phi)$$

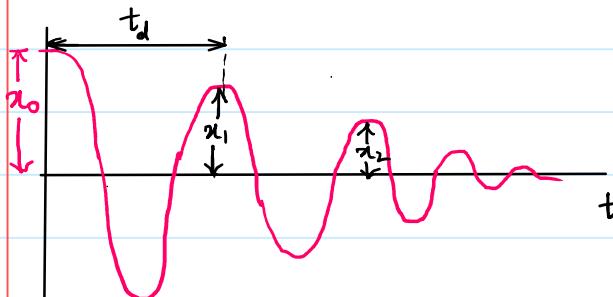
 initial condition @ $t=0$

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$$x(t=0) = x_0 e^{-\zeta \omega_n \cdot 0} \sin(\omega_d \cdot 0 + \phi)$$

 Amplitude $\rightarrow x_0$

$$x_0 = X_0$$


 ω_d - frequency of damped oscillation

 t_d - time period of damped oscillation.

$$t_d = \frac{2\pi}{\omega_d} = \frac{2\pi}{\omega_n \sqrt{1-\zeta^2}}$$

$$x_1 = x(t=t_d) = x_0 e^{-\zeta \omega_n t_d} \sin(\omega_d t_d + \phi)$$

Amplitude.

$$x_1 = x_0 e^{-\zeta \omega_n t_d}$$

Ratio of consecutive displacements.

$$\frac{x_0}{x_1} = \frac{x_0}{x_0 e^{-\zeta \omega_n t_d}} \Rightarrow \frac{x_0}{x_1} = e^{\zeta \omega_n t_d}$$

$$\frac{x_0}{x_1} = e^{\zeta \omega_n t_d}$$

$$\frac{x_0}{x_1} = \frac{x_1}{x_2} = \frac{x_2}{x_3} = \frac{x_3}{x_4} = \dots \frac{x_{n-1}}{x_n} = e^{\zeta \omega_n t_d}$$

 $x_0, x_1, x_2, \dots, x_n$ are in Geometric Progression.

$$\frac{x_0}{x_n} = \frac{x_0}{x_1} \cdot \frac{x_1}{x_2} \cdot \frac{x_2}{x_3} \cdot \frac{x_3}{x_4} \cdots \frac{x_{n-1}}{x_n}$$

$$\frac{x_0}{x_n} = e^{\zeta \omega_n t_d \cdot n}$$

n - no. of cycles.

$$\text{Logarithmic decrement} \quad \delta = \ln \left(\frac{x_0}{x_1} \right) = \zeta \omega_n t_d$$

$$\ln \left(\frac{x_0}{x_1} \right) = \zeta \cdot \omega_n \cdot \frac{2\pi}{\omega_n \sqrt{1-\zeta^2}}$$

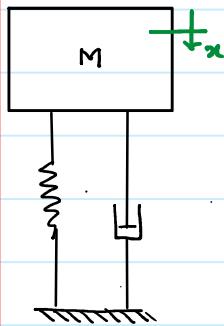
$$\delta = \ln \left(\frac{x_0}{x_1} \right) = \frac{2\pi \zeta}{\sqrt{1-\zeta^2}}$$

$$\ln \left(\frac{x_0}{x_n} \right) = \zeta \omega_n t_d \cdot n$$

$$\ln \left(\frac{x_0}{x_n} \right) = \frac{2\pi \zeta}{\sqrt{1-\zeta^2}} \cdot n$$

$$\frac{1}{n} \ln \left(\frac{x_0}{x_n} \right) = \frac{2\pi \zeta}{\sqrt{1-\zeta^2}} = \delta$$

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Equation of motion

$$M\ddot{x} + Cx + Kx = 0$$

→ Translatory motion

For Rotatory motion

$$I_{eq}\ddot{\theta} + C_{eq}\dot{\theta} + K_{eq}\theta = 0$$

Natural frequency

$$\omega_n = \sqrt{\frac{K}{M}} \rightsquigarrow \text{Translatory motion}$$

$$\omega_n = \sqrt{\frac{K_{eq}}{I_{eq}}} \rightsquigarrow \text{Rotatory motion}$$

Critical damping

$$C_c = 2\sqrt{MK} \rightsquigarrow \text{Translatory motion}$$

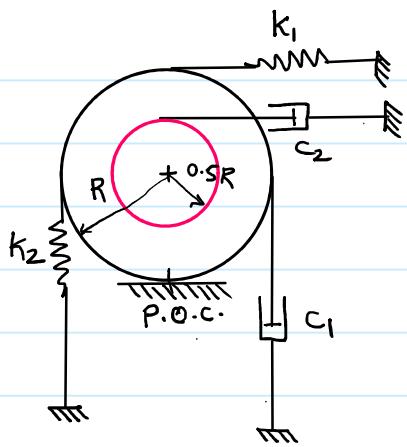
$$C_c = 2\sqrt{I_{eq}K_{eq}} \rightsquigarrow \text{Rotatory motion}$$

Damping factor

$$\zeta = \frac{c}{2\sqrt{MK}} \rightsquigarrow \text{Translatory motion}$$

$$\zeta = \frac{C_{eq}}{2\sqrt{I_{eq}K_{eq}}} \rightsquigarrow \text{Rotatory motion}$$

SNo.	System Parameter	Motion.			
		Translation.		Rotation	
		Force.	Energy.	Couple.	Energy.
1.	Inertia	$M\ddot{x}$	$\frac{1}{2}M\dot{x}^2$	$I_{eq}\ddot{\theta}$	$\frac{1}{2}I_{eq}\dot{\theta}^2$
2.	Restoration	Kx	$\frac{1}{2}Kx^2$	$K_{eq}\theta$	$\frac{1}{2}K_{eq}\dot{\theta}^2$
3.	Damping	$C\dot{x}$	$\int F_D \cdot dx = \int C\dot{x} dx$	$C_{eq}\dot{\theta}$	$\int T_D \cdot d\theta = \int C_{eq}\dot{\theta} d\theta$



$$E \cdot O \cdot M = ?$$

$$\omega_n = ?$$

$$c_c = ?$$

$$\zeta = ?$$

$$\omega_d = ?$$

$$\leq M_{P.O.C} = 0$$

$$I_{P.O.C.} \ddot{\theta} + C_1 (\text{distance from P.O.C.})^2 \dot{\theta} + C_2 (\text{distance from P.O.C.})^2 \dot{\theta} + k_1 (\text{distance from P.O.C.})^2 \theta + k_2 (\text{distance from P.O.C.})^2 \theta = 0$$

$$\frac{3}{2}MR^2\ddot{\theta} + C_1(R)^2\dot{\theta} + C_2(1.5R)^2\dot{\theta} + k_1(2R)^2\theta + k_2(R)^2\theta = 0$$

$$I_{eq} = \frac{3}{2}MR^2 \quad C_{eq} = C_1R^2 + 2.25C_2R^2 \quad K_{eq} = 4k_1R^2 + k_2R^2$$

$$\text{Natural frequency} \quad \omega_n = \sqrt{\frac{K_{eq}}{I_{eq}}} \quad \Rightarrow \quad \omega_n = \sqrt{\frac{(4k_1 + k_2)R^2}{\frac{3}{2}MR^2}} = \sqrt{\frac{8k_1 + 2k_2}{3M}}$$

$$\text{Critical Damping} \quad C_c = 2\sqrt{I_{eq} \cdot K_{eq}} \quad C_c = 2\sqrt{\frac{3}{2}MR^2 \cdot (4k_1 + k_2)R^2}$$

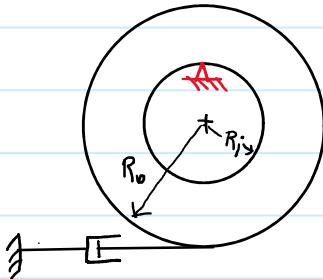
$$\text{Damping factor} \quad \zeta = \frac{C_{eq}}{2\sqrt{I_{eq} \cdot K_{eq}}} = \frac{C_1R^2 + 2.25C_2R^2}{2\sqrt{\frac{3}{2}MR^2 \cdot (4k_1 + k_2)R^2}}$$

Frequency of Damped oscillation

$$\omega_d = \omega_n \sqrt{1 - \zeta^2}$$

$$\omega_d = \sqrt{\frac{8k_1 + 2k_2}{3M}} \sqrt{1 - \left(\frac{C_1R^2 + 2.25C_2R^2}{2\sqrt{\frac{3}{2}MR^2 \cdot (4k_1 + k_2)R^2}} \right)^2}$$

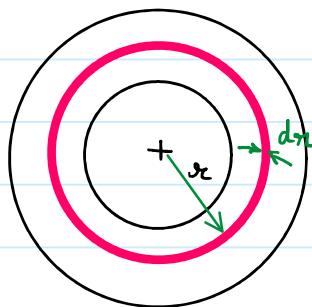
→



E.O.M.

$$R_o = R.$$

$$R_i = 0.5R.$$



$$\text{Area of elemental ring} \\ = 2\pi r \cdot dr.$$

$$\text{Volume of elemental ring} \\ = 2\pi r \cdot dr \cdot t.$$

$$\text{Mass of elemental Ring}$$

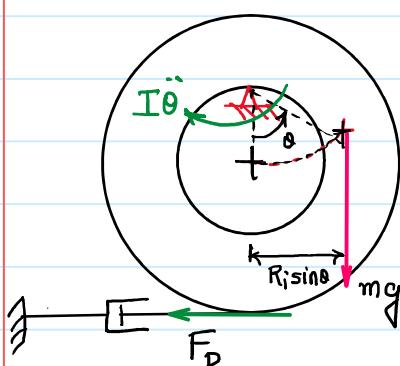
$$dm = \rho \cdot 2\pi r \cdot dr \cdot t.$$

$$\text{Moment of Inertia about Centriodal axis.} = \int r^2 \cdot dm = \int_{R_i}^{R_o} r^2 \cdot (\rho \cdot 2\pi r \cdot dr \cdot t)$$

$$I = \rho \cdot 2\pi \cdot t \int_{R_i}^{R_o} r^3 \cdot dr = \rho \cdot \frac{1}{4} \pi \cdot t \cdot \frac{(R_o^4 - R_i^4)}{4F_2}.$$

$$I = \boxed{\rho \cdot \pi \cdot (R_o^2 - R_i^2) \cdot t} \cdot \frac{(R_o^2 + R_i^2)}{2} = \frac{m \cdot (R_o^2 + R_i^2)}{2}$$

\downarrow
Mass of Annular Ring



$$\sin \theta \approx 0.$$

$$I_{\text{Hinge}} = \bar{I} + m \cdot d^2$$

$$I_{\text{Hinge}} = \frac{m \cdot (R + (0.5R))^2}{2} + m(0.5R)^2$$

$$I_{\text{Hinge}} = 0.875 m R^2$$

$$I_{\text{Hinge}} \ddot{\theta} + mg(R_i \sin \theta) + C \cdot (R + 0.5R)^2 \dot{\theta} = 0$$

$$0.875 m R^2 \ddot{\theta} + 2.25 C R^2 \dot{\theta} + 0.5 m g R \cdot \theta = 0$$

$$I_{\text{eq}} \ddot{\theta} + C_{\text{eq}} \dot{\theta} + K_{\text{eq}} \theta = 0$$

Damped free Vibration

18. Critical damping is the (GATE-14)
- largest amount of damping for which no oscillation occurs in free vibration
 - smallest amount of damping for which no oscillation occurs in free vibration
 - largest amount of damping for which the motion is simple harmonic in free vibration
 - smallest amount of damping for which the motion is simple harmonic in free vibration
19. A mass attached at the end of a spring has a natural frequency of 50 rad/sec. When the mass is increased to four times the original mass and a damper is attached, the frequency of oscillations is found to be 20 rad/sec. What is the damping ratio of the resulting system?
- 60 %
 - 40 %
 - 80 %
 - 16 %

Critical damping is least amount of damping for there will be no oscillations.

$$\omega_n = 50 \text{ rad/s.} \rightarrow \text{for mass } m.$$

$$\hookrightarrow \omega_n = \sqrt{\frac{k}{m}}$$

mass is increased to 4m. and damper is added.

$$\omega_d = 20 \text{ rad/s.}$$

$$\rightarrow \omega_n = \sqrt{\frac{k}{4m}} = \frac{1}{2} \sqrt{\frac{k}{m}} = \frac{1}{2} \times 50 = 25 \text{ rad/s}$$

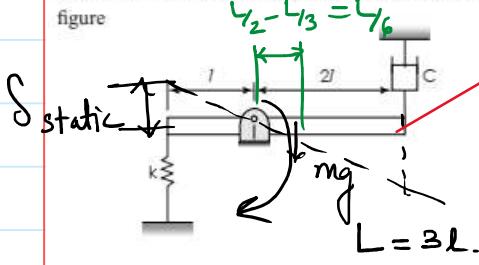
$$\omega_d = \omega_n \cdot \sqrt{1 - \zeta^2}$$

$$20 = 25 \cdot \sqrt{1 - \zeta^2}$$

$$(0.8)^2 = 1 - \zeta^2 \Rightarrow \zeta = \sqrt{1 - (0.8)^2} = 0.6 = 60\%$$

Common data for 21 & 22

A bar of mass m assumed to be distributed uniformly is constrained to undergo small oscillations as shown in figure



21. The equivalent Inertia and stiffness of the system are

- (a) (m^2) & k^2
 (b) $(3m^2)$ & k^2
 (c) $(7m^2/3)$ & k^2
 (d) $(7m^2/3)$ & $(k^2)/2$

22. The damping ratio of the system is

- (a) $\frac{2C}{\sqrt{7km}}$
 (b) $\frac{2C}{\sqrt{km}}$
 (c) $\frac{2C}{2\sqrt{km^2}}$
 (d) $\frac{2C}{\sqrt{3km}}$

Mg will cause δ_{static}

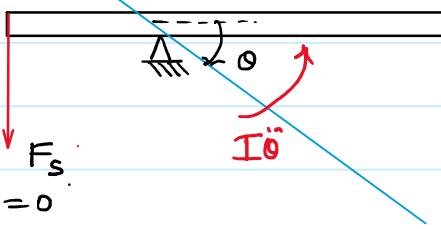
This can be assumed as equilibrium position

$$I = \bar{I} + md^2$$

$$= \frac{Ml^2}{12} + M \cdot (l_6)^2 = \frac{Ml^2}{9} = \frac{Ml^2}{3l}$$

$$L = 3l$$

$$F_D$$



$$\sum M_{\text{Hinge}} = 0$$

$$I_{\text{Hinge}} \ddot{\theta} + C(\text{Lwr distance from hinge})^2 \dot{\theta} + K(\text{Lwr distance from hinge})^2 \theta = 0$$

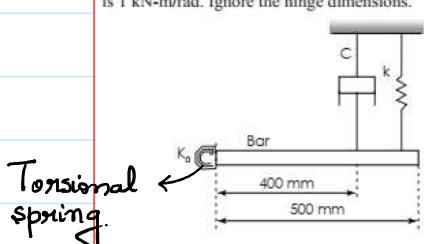
$$Ml^2 \ddot{\theta} + C(2l)^2 \dot{\theta} + K(l)^2 \theta = 0$$

$$I_{\text{eq}} = ml^2, \quad K_{\text{eq}} = kl^2$$

$$\ell_f = \frac{C_{\text{eq}}}{2\sqrt{I_{\text{eq}} \cdot K_{\text{eq}}}} = \frac{4Cl^2}{2\sqrt{Ml^2 \cdot Kl^2}} = \frac{2c}{\sqrt{MK}}$$

Common data questions 25 & 26

A uniform rigid slender bar of mass 10 kg is hinged at the left end and is suspended with the help of spring and damper arrangement as shown in the figure where $K = 2\text{kN/m}$, $C = 500 \text{ Ns/m}$ and the stiffness of the torsional spring K_T is 1 kN-m/rad . Ignore the hinge dimensions.



Torsional spring

25. The un-damped natural frequency of oscillations of the bar about the hinge point is $\omega_n = ?$ (GATE-03)

- (a) 42.43 rad/s
 (b) 30 rad/s
 (c) 17.32 rad/s
 (d) 14.14 rad/s

26. The damping coefficient in the vibration equation is given by (GATE-03)

- (a) 500 Nms/rad
 (b) 500 N/(m/s)
 (c) 80 Nms/rad
 (d) 80 N/(m/s)

Rotational system.

Translatory system.

$$L = 500\text{mm}, \quad a = 400\text{mm}.$$

$$\sum M_{\text{Hinge}} = 0$$

$$I_{\text{Hinge}} \ddot{\theta} + C(\text{Lwr distance from hinge})^2 \dot{\theta} + K(\text{Lwr distance from hinge})^2 \theta + K_T \theta = 0$$

$$\frac{ML^2}{3} \ddot{\theta} + Ca^2 \dot{\theta} + Kl^2 \theta + k_T \theta = 0$$

$$I_{\text{eq}} = ML^2/3, \quad C_{\text{eq}} = Ca^2, \quad K_{\text{eq}} = (kl^2 + k_T)$$

$$\frac{10(0.5)^2}{3} \ddot{\theta} + 500(0.4)^2 \dot{\theta} + 2000(0.5)^2 \theta + 1000 \theta = 0$$

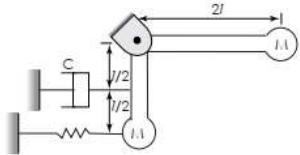
$$\frac{2.5}{3} \ddot{\theta} + 80 \dot{\theta} + 1500 \theta = 0$$

$$C_{\text{eq}}$$

$$\omega_n = \sqrt{\frac{K_{\text{eq}}}{I_{\text{eq}}}} \quad \omega_n = \sqrt{\frac{1500}{(2.5/3)}} = 42.43 \text{ rad/s.}$$

Common data for 23 & 24

In the single degree freedom system shown in figure is in free vibration $l = 1 \text{ m}$, $M = 10 \text{ kg}$ at each end $k = 400 \text{ N/m}$, $C = 400 \text{ N-sec/m}$. The L shaped bar is mass less and rigid and hinged at O.



23. The undamped natural frequency of the system is
 ✓ (a) 3.162 rad/sec (b) 6.32 rad/sec
 (c) 4.62 rad/sec (d) 2.83 rad/sec

24. The damping ratio of the system is
 ✓ (a) 0.316 (b) 0.79
 (c) 0.46 (d) 0.54

$$M = 10 \text{ kg}, L = 1 \text{ m}, K = 400 \text{ N/m}$$

$$C = \frac{400 \text{ N-sec}}{\text{m}}$$

$$\sum M_{\text{hinge}} = 0$$

$$I_{\text{hinge}} \ddot{\theta} + F_s \cdot x L \cos \theta + F_p \cdot l_2 \cos \theta + mg L \sin \theta - mg(2L) \cos \theta = 0$$

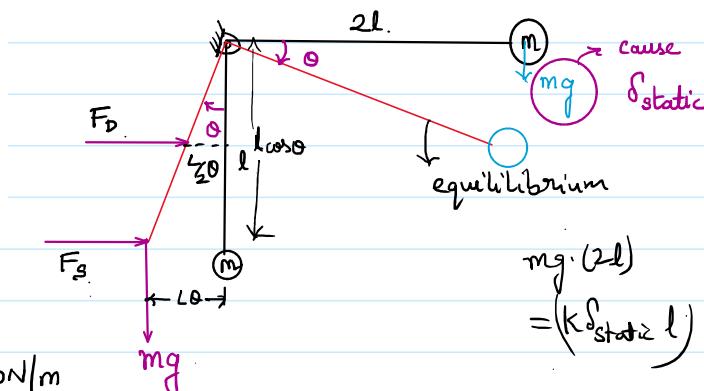
$$[m(2L)^2 + m(L)^2] \ddot{\theta} + C(l_2 \dot{\theta}) \cdot l_2 + k(\delta_{\text{static}} + L \theta) \cdot L + mgL\theta - 2mgL = 0$$

$$5ml^2 \ddot{\theta} + \frac{Cl^2}{4} \dot{\theta} + (kl^2 \theta + mgL\theta) + k\delta_{\text{static}}L - 2mgL = 0$$

$$I_{\text{eq}} \ddot{\theta} + C_{\text{eq}} \dot{\theta} + k_{\text{eq}} \theta = 0$$

$$\omega_n = \sqrt{\frac{kl^2 + mgL}{5ml^2}}$$

$$\zeta = \frac{C_{\text{eq}}}{2\sqrt{I_{\text{eq}}k_{\text{eq}}}} = \frac{\left(\frac{Cl^2}{4}\right)}{2\sqrt{5ml^2(kl^2 + mgL)}}$$



$$mg \cdot (2L) = (K \delta_{\text{static}} L)$$

28. Which of the following statements are TRUE for damped vibrations?

- P. For a system having critical damping, the value of damping ratio is unity and system does not undergo a vibratory motion.
 - Q. Logarithmic decrement method is used to determine the amount of damping in a physical system.
 - R. In case of damping due to dry friction between moving surfaces resisting force of constant magnitude acts opposite to the relative motion.
 - S. For the case of viscous damping, drag force is directly proportional to the square of relative velocity.
- (GATE-15)
- (a) P and Q only
 (b) P and S only
 (c) P,Q and R only
 (d) Q and Sonly

$$\rightarrow \zeta = 1 \text{ no oscillation}$$

$$x(t) = (A + Bt)e^{-\zeta \omega_n t}$$

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 exponential decay

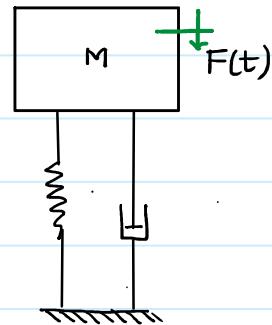
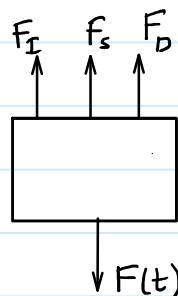
friction force $\propto -\text{velocity}$

$$f \propto -\dot{x}$$

$$F_D \propto -\dot{x}^2$$

Forced Vibrations

$$F(t) = F \sin(\omega t)$$



$$F_I + F_s + F_D = F(t)$$

$$M\ddot{x} + C\dot{x} + Kx = F(t)$$

$$x(t) = C.F + P.I.$$

$$(M\cdot D^2 + CD + K)x = F \sin(\omega t)$$

$$x = \frac{F \sin(\omega t)}{(MD^2 + CD + K)}$$

$$D = -\omega^2$$

$$D = \frac{d}{dt}$$

$$x = \frac{F \sin(\omega t)}{(CD) + (K - M\omega^2)} \times \frac{(K - M\omega^2) - (CD)}{(K - M\omega^2) - (CD)}$$

$$x = \frac{F \sin(\omega t) \cdot (K - M\omega^2) - C \cdot \left(\frac{d}{dt} (F \sin(\omega t)) \right)}{(K - M\omega^2)^2 - (CD)^2}$$

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$$x = \frac{F \sin(\omega t) \cdot (K - M\omega^2) - C \cdot \left(\frac{d}{dt} (F \sin(\omega t)) \right)}{(K - M\omega^2)^2 - (CD)^2}$$

$$D^2 = -\omega^2$$

$$x = \frac{F \sin(\omega t) \cdot (K - M\omega^2) - C \cdot F \cdot \cos(\omega t) \cdot \omega}{(K - M\omega^2)^2 - (C\omega)^2}$$

$$x = \frac{F \cdot [(K - M\omega^2) \cdot \sin(\omega t) - (C\omega) \cdot \cos(\omega t)]}{(K - M\omega^2)^2 + (C\omega)^2}$$

$$K - M\omega^2 = R \cdot \cos \phi.$$

$$C\omega = R \cdot \sin \phi$$

$$R = \sqrt{(K - M\omega^2)^2 + (C\omega)^2}$$

$$x = \frac{F \cdot [R \cos \phi \cdot \sin(\omega t) - R \sin \phi \cdot \cos(\omega t)]}{R^2}$$

$$x = \frac{F R \cdot [\sin(\omega t - \phi)]}{R^2}$$

$$x = \frac{F \sin(\omega t - \phi)}{\sqrt{(K - M\omega^2)^2 + (C\omega)^2}}$$

$$x(t) = CF + P.I.$$

$$= X_0 e^{-\zeta \omega_n t} \underbrace{\sin(\omega_n t + \phi)}_{\text{Transient Response}} + \underbrace{\frac{F \sin(\omega t - \phi)}{\sqrt{(K - M\omega^2)^2 + (C\omega)^2}}}_{\text{steady state Response.}}$$

Transient Response

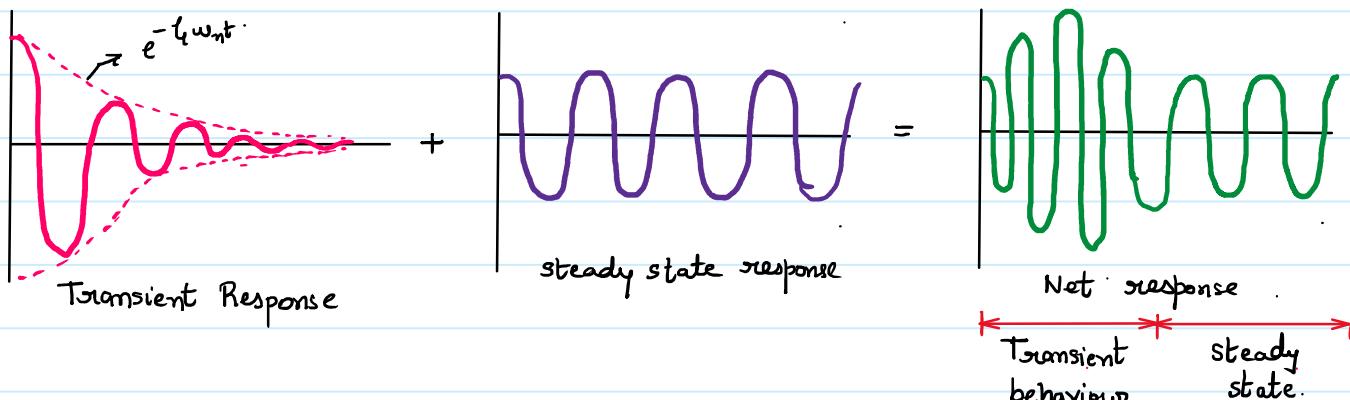
steady state
Response.

As $t \rightarrow \infty$ $e^{-\zeta \omega_n t} \rightarrow 0$ Transient Response $\rightarrow 0$

$$x(t) = \frac{F \sin(\omega t - \phi)}{\sqrt{(K - M\omega^2)^2 + (C\omega)^2}}$$

$$x(t) = C.F + P.I.$$

$$= X_0 e^{-\zeta \omega_n t} \sin(\omega_d t + \phi) + \frac{F \sin(\omega t - \phi)}{\sqrt{(k-M\omega^2)^2 + (\zeta\omega)^2}}$$



ω - frequency of excitation.

for static loading

$$\omega = 0$$

$$F(t) = F$$

$F = \text{constant}$

$k = \text{constant}$

$$x = \frac{F}{\sqrt{(k-M\omega^2)^2 + (\zeta\omega)^2}} \quad x_{\text{static}} = \frac{F}{k}$$

Response of forced vibration

$$x(t) = \frac{F \sin(\omega t - \phi)}{\sqrt{(k-M\omega^2)^2 + (\zeta\omega)^2}} \longrightarrow x(t) = A \cdot \sin(\omega t - \phi)$$

$$A = \frac{F}{\sqrt{(k-M\omega^2)^2 + (\zeta\omega)^2}}$$

A - Amplitude of forced vibration

$$x(t) = \frac{(F/k) \cdot \sin(\omega t - \phi)}{\sqrt{\left(\frac{k-M\omega^2}{k}\right)^2 + \left(\frac{\zeta\omega}{k}\right)^2}}$$

$$\frac{M}{K} = \frac{1}{\omega_n^2}$$

$$x(t) = \frac{x_{\text{static}} \cdot \sin(\omega t - \phi)}{\sqrt{\left(1 - \frac{M}{K} \cdot \omega^2\right)^2 + \left(\frac{\zeta\omega}{K}\right)^2}}$$

$$\frac{\zeta\omega}{K} = \frac{2C \cdot \omega}{2\sqrt{KM}} \cdot \frac{\sqrt{M}}{\sqrt{K}}$$

$$= 2 \cdot \frac{C}{2\sqrt{MK}} \cdot \sqrt{\frac{M}{K}} \cdot \omega$$

$$= 2 \cdot \frac{C \cdot \omega}{2\sqrt{MK}} = 2\zeta\omega$$

$$\zeta = \frac{\omega}{\omega_n}$$

$$x(t) = \frac{x_{\text{static}} \sin(\omega t - \phi)}{\sqrt{(1-\zeta^2)^2 + (2\zeta\omega)^2}}$$

ζ - ratio of frequencies - $\frac{\text{excitation frequency}}{\text{Natural frequency}}$

ζ - Damping Ratio

Dynamic Magnification factor - It is the ratio of Amplitude of forced vibration to static deflection.

$$D.M.F = \frac{A}{x_{\text{static}}} = \frac{\cancel{F}}{\sqrt{(k-M\omega^2)^2 + (c\omega)^2}} = \frac{1}{\sqrt{\left(\frac{k}{k} - \frac{M\omega^2}{k}\right)^2 + \left(\frac{c\omega}{k}\right)^2}}$$

$$D.M.F = \frac{1}{\sqrt{(1-\pi^2)^2 + (2\zeta\pi)^2}}$$

$$D.M.F = \frac{1}{\sqrt{(1-\xi^2)^2 + (2\zeta\omega)^2}}$$

$$D.M.F = f(\zeta, \omega) = f(\omega)$$

\downarrow
 $\zeta < 1$. (under damped).

$$\eta = \frac{\omega}{\omega_n} = \frac{\text{forcing frequency}}{\text{Natural frequency}}$$

So $D.M.F = \text{Max.}$ then. $\frac{d}{d\omega}(D.M.F) = 0$

$$\frac{d}{d\omega} \left(\frac{1}{\sqrt{(1-\xi^2)^2 + (2\zeta\omega)^2}} \right) = 0$$

$$-\frac{1}{2} \cdot \left[(1-\xi^2)^2 + (2\zeta\omega)^2 \right]^{-\frac{1}{2}-1} \times \left[2 \cdot (1-\xi^2) \cdot (0-2\zeta\omega) + 2 \cdot (2\zeta\omega) \cdot (2\zeta) \right] = 0$$

$$-4\xi(1-\xi^2) + 8\zeta^2\omega = 0$$

$$-1 + \xi^2 + 2\zeta^2 = 0$$

$$\xi^2 = 1 - 2\zeta^2$$

$$\xi = \sqrt{1 - 2\zeta^2}$$

$$\frac{\omega}{\omega_n} = \sqrt{1 - 2\zeta^2} \rightsquigarrow$$

$$\omega_p = \omega_n \cdot \sqrt{1 - 2\zeta^2}$$

ω_p — frequency at which peak value of D.M.F/Amplitude of forced vibration occurs.

$$D.M.F = \frac{A}{\omega_{\text{static}}} \rightsquigarrow \begin{array}{l} \text{Variable.} \\ \text{constant} \end{array}$$

D.M.F vs r Graph

$$D.M.F = \frac{1}{\sqrt{(1-\eta^2)^2 + (2\zeta\eta)^2}}$$

1. $\eta=0 \quad \zeta=0$ (undamped system) $D.M.F = 1$

$\hookrightarrow \omega=0$ static loading.

2. $\eta=0 \quad \zeta \neq 0 \quad D.M.F = 1$ ✓

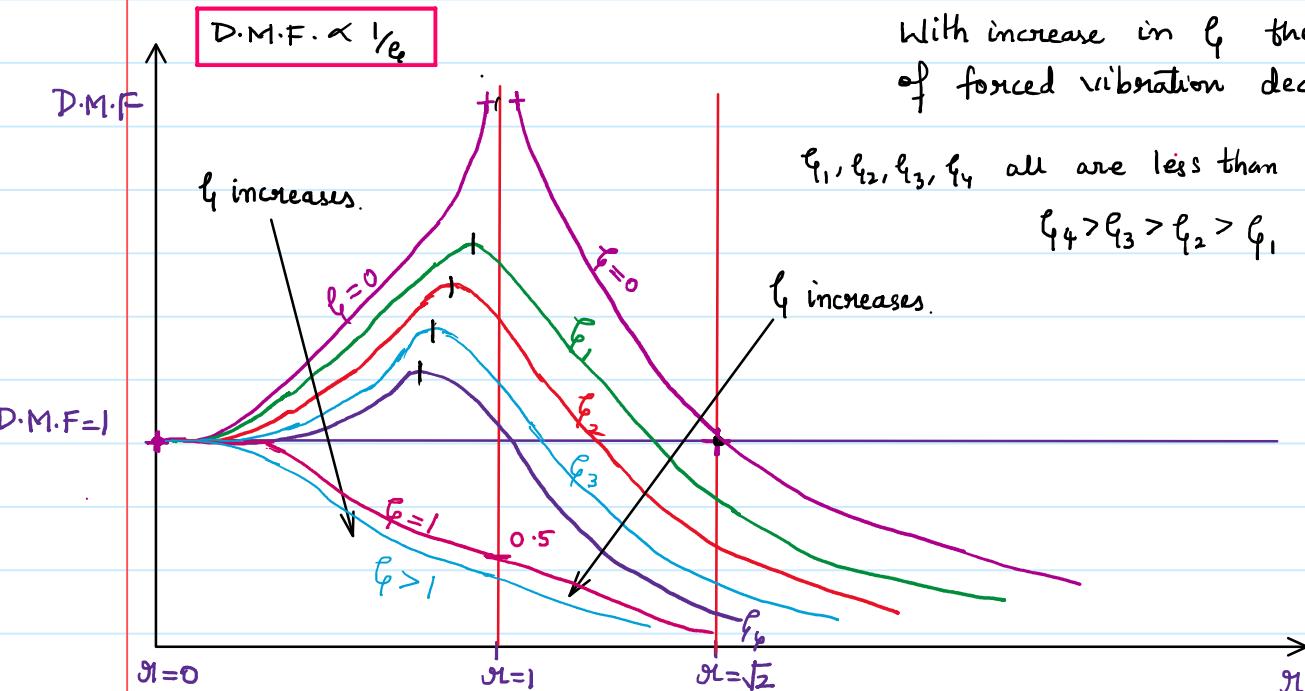
3. $\eta=1 \quad \zeta=0 \quad D.M.F = \infty$

4. $\eta=1 \quad \zeta \neq 0 \quad D.M.F = (1/2\zeta)$

5. $\eta=\sqrt{2} \quad \zeta=0 \quad D.M.F = 1$

6. $\eta=\sqrt{2} \quad \zeta \neq 0 \quad D.M.F = \frac{1}{\sqrt{1+8\zeta^2}}$

$$D.M.F \propto \frac{1}{\zeta} \quad \text{constant } \frac{A}{\zeta} \text{ static } \frac{1}{\zeta}$$



With increase in ζ the amplitude of forced vibration decreases.

$\zeta_1, \zeta_2, \zeta_3, \zeta_4$ all are less than 1.

$$\zeta_4 > \zeta_3 > \zeta_2 > \zeta_1$$

$$\eta = \sqrt{1-2\zeta^2} \rightarrow D.M.F = \text{Max.}$$

As $\zeta \uparrow \quad 2\zeta^2 \uparrow \quad 1-2\zeta^2 \downarrow \quad \eta \downarrow \quad \omega_p \downarrow$

As the value of ζ increases the peak of D.M.F vs η graph tends to shift towards Left side.

For damped systems the peak value of D.M.F occurs at η slightly less than 1.