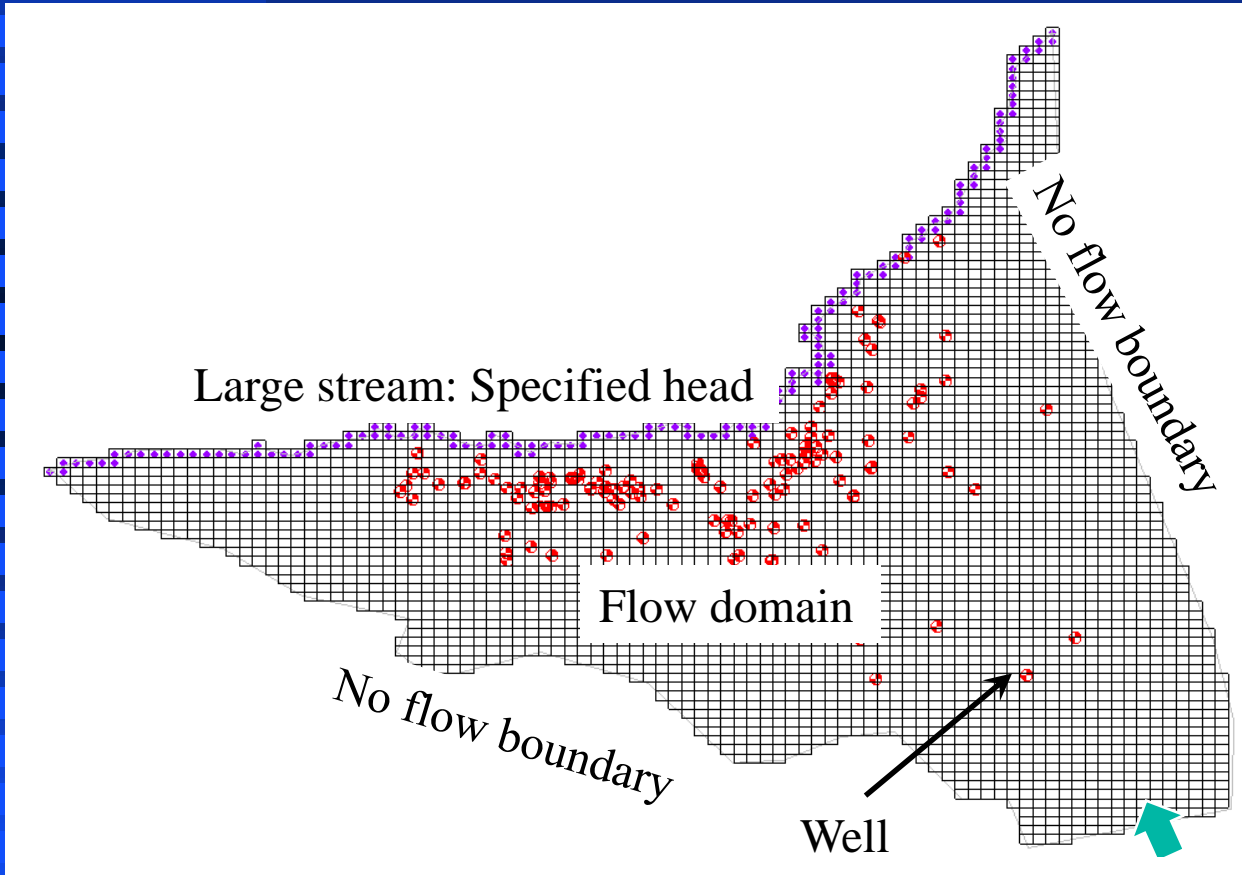


ERTH655/CEE623
Groundwater Modeling

Finite Difference Technique

Aly I. El-Kadi

Finite difference grid



Example: Saturated fresh water flow for steady state

Objective: estimate water levels

Input: well fluxes, recharge boundary conditions, aquifer data, (conductivity, storage coefficient, aquifer thickness)

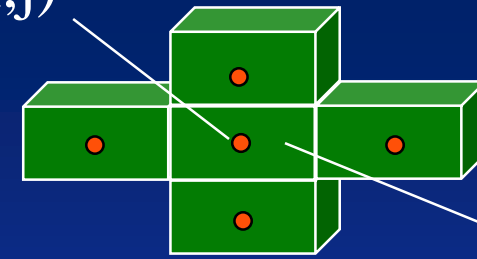
$$\frac{\partial}{\partial x} \left(K_{xx} \frac{\partial h}{\partial x} \right) + \frac{\partial}{\partial y} \left(K_{yy} \frac{\partial h}{\partial y} \right) + \frac{\partial}{\partial z} \left(K_{zz} \frac{\partial h}{\partial z} \right) = S_s \frac{\partial h}{\partial t} \pm R$$

Saturated 3-D equation

Head dependent inter-aquifer flow

Node (i,j)

Δy



Cell

Δx

i = 1 2 3 . . .

j = 1

2

3

.

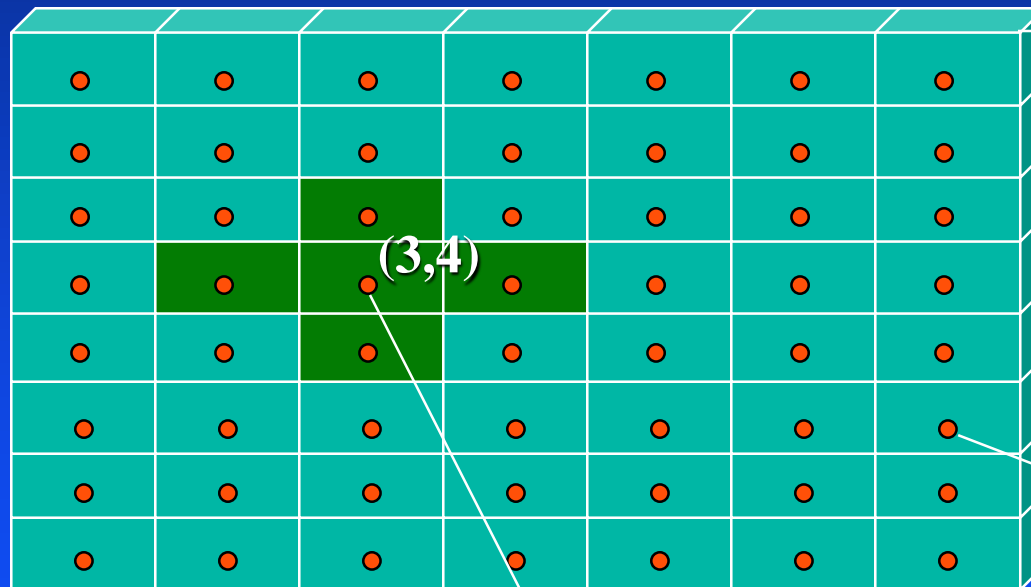
.

.

.

.

y



x

Boundary node

Interior node

Mass balance equation for a set of neighboring nodes:

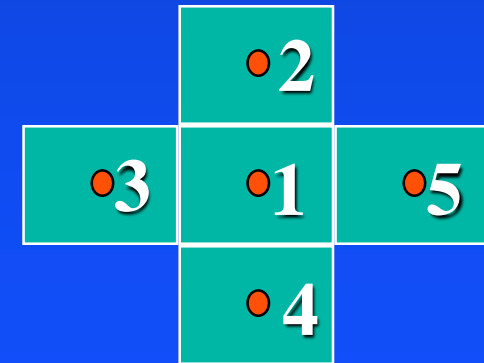
$$Q_{21} + Q_{31} + Q_{41} + Q_{51} = \Delta x_1 \Delta y_1 S_1 \frac{\partial h_1}{\partial t} \quad (1)$$

Darcy's law:

$$Q_{21} = -\Delta x_1 T_{21} \left(\frac{\partial h}{\partial y} \right)_{21} \quad (2)$$

Difference form:

$$Q_{21} \approx \Delta x_1 T_{21} \left(\frac{h_2 - h_1}{\Delta y} \right)_{21} \quad (3)$$



Definition of a partial derivative:

$$\frac{\partial h}{\partial t} \approx \frac{h^n - h^{n-1}}{\Delta t} \quad (4)$$

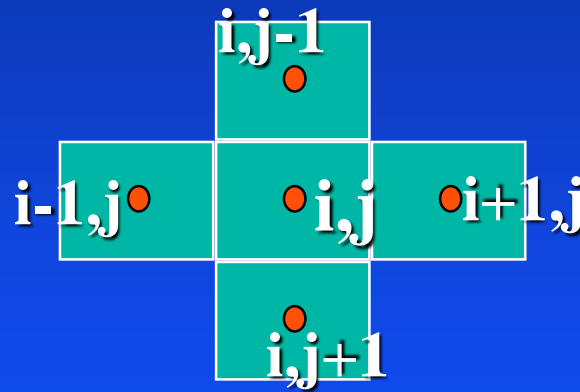
n = current time level (t+Δt), n-1 = previous time level (t).
Inserting equations (3) and (4) into (1) will provide a
relationship between head values at the neighboring nodes:

$$a_{12} h_{13}^n + b_{13} h_{11}^n + c_{11} h_{14}^n + e_{15} h_{15}^n = f_1 \quad (5)$$

in which a, b, c, d, e, and f are known coefficients, functions of Δx, Δy, Δt, T, and S, with values that depend on the type of finite-difference approximation.

Equation (5) can be generalized at node i,j to read:

$$a_i h_{i-1,j}^n + b_i h_{i,j-1}^n + c_i h_{i,j}^n + d_i h_{i+1,j}^n + e_i h_{i,j+1}^n = f_i \quad (6)$$



Types of finite-difference approximation

$$Q_{21} = -\Delta x_1 T_{21} \left(\frac{\partial h}{\partial y} \right)_{21} \quad (7)$$

$$Q_{21} \approx \Delta x_1 T_{21} \left(\frac{h_2 - h_1}{\Delta y} \right)_{21}$$

Equations above estimates the average volumetric flux between cells over a time interval Δt . Heads used in this equation can be taken as those at certain time t' in the interval between t and $t+\Delta t$:

$$h(t') = (1 - \omega)h(t) + \omega h(t + \Delta t) \quad (8)$$

in which ω is a weighting factor between 0 and 1.

$t = t'$ or $\omega = 0$ will produce an **explicit** finite difference scheme.

$$h(t') = h(t)$$

$$h_{i,j}^n = g_i h_{i,j}^{n-1} + m_i h_{i-1,j}^{n-1} + u_i h_{i+1,j}^{n-1} + v_i h_{i,j-1}^{n-1} + q_i h_{i-1,j+1}^{n-1} \quad (9)$$

The solution is obtained by starting from the initial values and estimating directly the values after a time increment Δt for all nodes in the system. The values of g_i , m_i , etc. are functions of Δx , Δy , Δt , T , and S .

The solution is easy to program but a restriction should be made on the value of Δt in order to obtain a stable solution. For stability of the solution, value of Δt should satisfy the condition at every node:

$$\frac{T}{S} \left(\frac{\Delta t}{\Delta x^2} + \frac{\Delta t}{\Delta y^2} \right) \leq \frac{1}{2} \quad (10)$$

→ Small Δt .

Plus large rounding error $O(\Delta x)$.

- For $\omega = 1 \rightarrow$ fully implicit scheme
- For $0 < \omega < 1 \rightarrow$ implicit scheme
- For $\omega = 0.5 \rightarrow$ well known Crank-Nicholson implicit scheme.

$$a_i h_{i-1,j}^n + b_i h_{i,j-1}^n + c_i h_{i,j}^n + d_i h_{i+1,j}^n + e_i h_{i,j+1}^n = f_i$$

- The exact expressions for the quantities a_i , b_i , c_i , etc., will depend on ω

For example, for fully implicit scheme, and homogenous and isotropic systems:

$$\begin{aligned}a_i &= \frac{T}{\Delta x^2} & b_i &= \frac{T}{\Delta y^2} \\c_i &= -\frac{2T}{\Delta x^2} - \frac{2T}{\Delta y^2} - \frac{S}{\Delta t} & (11) \\d_i &= \frac{T}{\Delta x^2} & e_i &= \frac{T}{\Delta y^2} \\f &= -\frac{Sh_{i,j}^{n-1}}{\Delta t}\end{aligned}$$

Implicit formulations (systems of equations) can be put in the matrix form:

$$[K]\{h\} = \{F\} \quad (12)$$

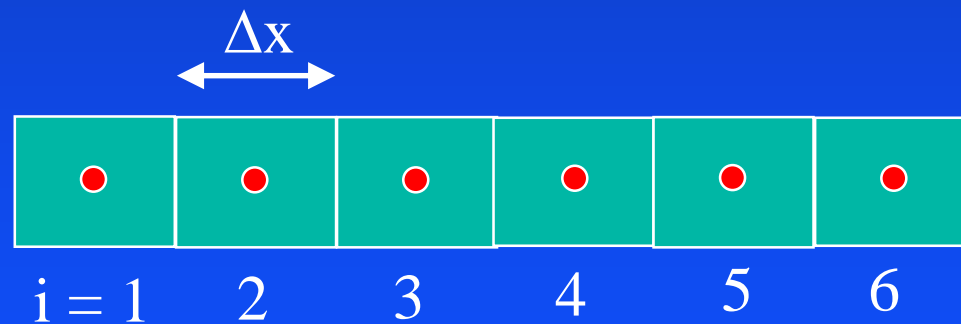
In which K is matrix of known coefficient, h is unknown vector, and F is known vector. Can be solved by matrix techniques either by direct or indirect (iterative) techniques.

Example – One dimensional problem

$$a_i h_{i-1} + b_i h_i + c_i h_{i+1} = d_i$$

$i = 1, 2, \dots, N$; N total number of nodes

$N = 6$:



Equations can be written as:

$$\begin{array}{llll} i = 1 & b_1 h_1 + c_1 h_2 & & = d_1 \\ 2 & a_2 h_1 + b_2 h_2 + c_2 h_3 & & = d_2 \\ 3 & & a_3 h_2 + b_3 h_3 + c_3 h_4 & = d_3 \\ 4 & & & a_4 h_3 + b_4 h_4 + c_4 h_5 & = d_4 \\ 5 & & & & a_5 h_4 + b_5 h_5 + c_5 h_6 & = d_5 \\ 6 & & & & & a_6 h_5 + b_6 h_6 & = d_6 \end{array}$$

Or in matrix form:

$$\begin{bmatrix}
 b_1 & c_1 & & & & \\
 a_2 & b_2 & c_2 & & & \\
 & a_3 & b_3 & c_3 & & \\
 & & a_4 & b_4 & c_4 & \\
 & & & a_5 & b_5 & c_5 \\
 & & & & a_6 & b_6
 \end{bmatrix}
 \begin{Bmatrix}
 h_1 \\ h_2 \\ h_3 \\ h_4 \\ h_5 \\ h_6
 \end{Bmatrix}
 =
 \begin{Bmatrix}
 d_1 \\ d_2 \\ d_3 \\ d_4 \\ d_5 \\ d_6
 \end{Bmatrix}$$

Matrix of known coefficients Unknown vector at time n Known vector at time n-1

Matrix direct schemes

- Matrix inversion
- Elimination – back substitution

Matrix iterative schemes

The Jacobi method

Arrange equation in the form:

$$h_{i,j}^n = F\left(h_{i-1,j}^n, h_{i+1,j}^n, h_{i,j-1}^n, h_{i,j+1}^n\right)$$

If neighboring values are known, can calculate $h_{i,j}$. But they are not, so make a guess, starting from initial condition. Then update using the new values as guesses and continue until convergence within acceptable error.

The Gauss-Seidel method

Same but use the last updated values (go row by row)

Others: Over relaxation, iterative alternating direction implicit procedure, etc.



Flow package

- ☐ BCF - Block-Centered Flow
☐ HUF - Hydrogeologic Unit Flow
☒ LPF - Layer Property Flow
☐ UPW - Upstream Weighting

Solver

- ☐ DE4 - Direct
☐ GMG - Geometric Multigrid
☐ LMG - Link-AMG
☐ NWT - Newton
☒ PCG - Pre. Conj.-Gradient
☐ PCGN - PCG with Imp. Nonlin.
☐ SIP1 - Strongly Impl. Proc.
☐ SMS - Sparse Matrix \ LMG-USG
☐ SOR1 - Slice Succ. Overrel.

Optional packages / processes

- ☐ BCT - Block Centered Transport
☐ BFH - Boundary Flow and Head
☒ CHD1 - Time-Variant Specified-Head
☐ CLN - Connected Linear Network Process
☐ DPT - Dual Porosity Transport
☒ DRN1 - Drain
☐ DRT1 - Drain Return
☐ ETS1 - Evapotranspiration Segments
☐ EVT1 - Evapotranspiration
☐ GAGE - Gage
☐ GHB1 - General-Head Boundary
☐ GNC - Ghost Node Correction
☒ HFB6 - Horizontal Flow Barrier
☐ LAK3 - Lake
☐ MNW1 - Multi-Node Well
☐ MNW2 - Multi-Node Well
☐ MNWI - Multi-Node Well Information
☐ PCB - Prescribed Concentration
☐ PEST-ASP
☒ RCH1 - Recharge
☐ RIV1 - River
☐ SFR2 - Streamflow-Routing
☐ STR1 - Stream
☐ SUB - Subsidence
☐ SWI2 - Seawater Intrusion
☐ UZF - Unsaturated Zone Flow
☒ WEL1 - Well

Help...

OK

Cancel

MODFLOW PCG Package



Maximum number of outer iterations (MXITER):

25

Number of inner iterations (ITER1):

50

Matrix preconditioning method (NPCOND):

(1) Modified Incomplete Chole ▾

Active cells surrounded by dry cells (IHCOFADD):

(0) Goes dry regardless of HC ▾

Head change criterion for convergence (HCLOSE):

0.01

(m)

Residual criterion for convergence (RCLOSE):

0.01

(m³/d)

Relaxation parameter (RELAX):

1.0

Eigenvalue upper bound (NBPOL):

(<>2) Calculate eigenvalue up ▾

Damping for all, or if < 0, only SS stress periods (DAMPPCG):

1.0

Damping for transient stress periods (DAMPPCGT):

0.0

Printout interval (IPRPCG):

0

Printing of convergence info (MUTPCG):

(2) No printing ▾

Help...

Reset

OK

Cancel