

Virtual Element method for Timoshenko beam

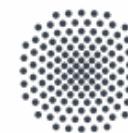


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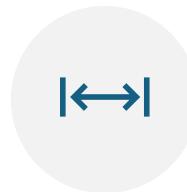


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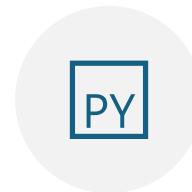
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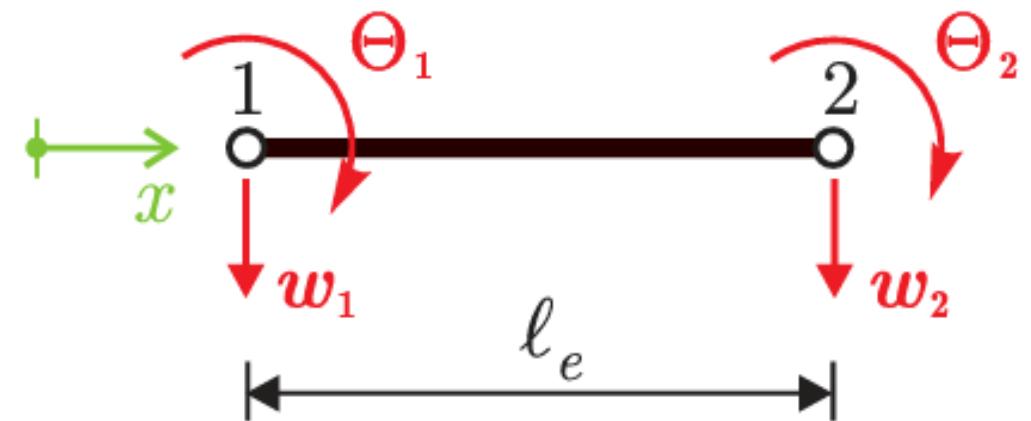
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Introduction



- **Virtual Element Method (VEM)**
- The Virtual Element Method is based on an **ansatz space** where:
 - Displacements are known **only at nodal points**
 - Displacements on **element edges** are assumed (edges do not exist in 1D problems)
- Unlike the **Finite Element Method (FEM)**:
 - FEM defines the displacement field over the **entire domain**
 - VEM does **not** require explicit knowledge of the displacement field inside the element

Derivation of Virtual element Method



How can we derive an ansatz for the displacement field u_h ?

- The idea is to use a projection $\Pi[u_h]$ of the displacement onto a polynomial space.
So, approximations displacement u_h are written as

$$u_h = \Pi[u_h] + (u_h - \Pi[u_h])$$

Where **projected** part or **consistency terms** $\Pi[u_h]$
And **remainder** $(u_h - \Pi[u_h])$

- Also, for short notation

$$u_\pi = \Pi[u_h]$$

Also

$$u_h \approx u_\pi$$

- Note

Contrary to virtual elements for two- and three-dimensional solids, the consistency term, as we will see in the following, provides element matrices that have full rank and thus the **remainder can be neglected**. This means that the virtual Timoshenko beam elements do not have to be stabilized.

How can we derive projected $\Pi[u_h]$ of ansatz space are derived ?

- Since only **first order derivatives appear in the weak form and potential** it is sufficient to compute e.g. the projection of the displacement by using **two conditions** for the
- **first derivative (gradient)**

$$\int_0^{l_e} p'(u'_\pi - u'_h) \, dx = 0$$

and the **average of the displacements** related to u_h and $\Pi[u_h]$

$$\int_0^{l_e} (u_\pi - u_h) \, dx = 0$$

- Note for **Euler–Bernoulli beams** we have to use this condition:

$$\int_0^{l_e} p'' (w''_h - w''_\pi) \, dx = 0 \quad \int_0^{l_e} w_\pi(x) \, dx = \int_0^{l_e} w_h(x) \, dx \quad \int_0^{l_e} w'_\pi(x) \, dx = \int_0^{l_e} w'_h(x) \, dx .$$

VEM formulation for second order ansatz

We take gradient equation

$$\int_0^{l_e} p' (u'_h - u'_\pi) \, dx = 0 \rightarrow \int_0^{l_e} p' u'_\pi \, dx = \int_0^{l_e} p' u'_h \, dx$$

We apply integration by part on projected part

$$\int_0^{l_e} p' u'_\pi \, dx = (p' u_h) \Big|_0^{l_e} - \int_0^{l_e} p'' u_h \, dx .$$

Projected ansatz and its derivative is given by is given by

$$u_\pi = a_1 + a_2 x + a_3 x^2$$

$$u'_\pi = a_2 + 2a_3 x = \langle 1 \ 2x \rangle \begin{Bmatrix} a_2 \\ a_3 \end{Bmatrix}$$

$$p' = \langle 1 \ 2x \rangle^T \quad p'' = \langle 0 \ 2 \rangle^T$$

- Left hand of equation:

$$\int_0^{l_e} p' u'_\pi \, dx = \int_0^{l_e} \begin{Bmatrix} 1 \\ 2x \end{Bmatrix} \langle 1 \ 2x \rangle \, dx \begin{Bmatrix} a_2 \\ a_3 \end{Bmatrix} = \int_0^{l_e} \begin{bmatrix} 1 & 2x \\ 2x & 4x^2 \end{bmatrix} \, dx \begin{Bmatrix} a_2 \\ a_3 \end{Bmatrix}$$

$$\int_0^{l_e} p' u'_\pi \, dx = \begin{bmatrix} l_e & l_e^2 \\ l_e^2 & \frac{4}{3}l_e^3 \end{bmatrix} \begin{Bmatrix} a_2 \\ a_3 \end{Bmatrix} = \mathbf{G} \mathbf{a}$$

- Right hand side of equation:

$$(p' u_h) \Big|_0^{l_e} - \int_0^{l_e} p'' u_h \, dx = \left[\begin{Bmatrix} 1 \\ 2x \end{Bmatrix} u_h \right]_0^{l_e} - \begin{Bmatrix} 0 \\ 2 \end{Bmatrix} \int_0^{l_e} u_h \, dx$$

- Since we don't use u_h inside element, so we introduced new internal variable that is not associated with any node, is called **moment** in the virtual element literature

$$m_0 = \frac{1}{l_e} \int_0^{l_e} u_h \, dx$$

- Note:

it only appear higher order ansatz $n \geq 2$. It has **same unit** as displacement

$$(p' u_h)|_0^{l_e} - \int_0^{l_e} p'' u_h \, dx = \left[\begin{Bmatrix} 1 \\ 2x \end{Bmatrix} u_h \right]_0^{l_e} - \begin{Bmatrix} 0 \\ 2 \end{Bmatrix} \int_0^{l_e} u_h \, dx \quad m_0 = \frac{1}{l_e} \int_0^{l_e} u_h \, dx$$

- Since $\mathbf{u}_h(0) = \mathbf{u}_1$ and $\mathbf{u}(l_e) = \mathbf{u}_2$:

$$(p' u_h)|_0^{l_e} - \int_0^{l_e} p'' u_h \, dx = \begin{Bmatrix} u_2 - u_1 \\ 2 l_e u_2 \end{Bmatrix} - \begin{Bmatrix} 0 \\ 2 l_e m_0 \end{Bmatrix} = \mathbf{r}(u_i, m_0)$$

$$\mathbf{G} \mathbf{a} = \mathbf{r}(u_i, m_0) \rightarrow \begin{Bmatrix} a_2 \\ a_3 \end{Bmatrix} = \begin{Bmatrix} -\frac{2}{l_e}(2u_1 + u_2 - 3m_0) \\ \frac{3}{l_e^2}(u_1 + u_2 - 2m_0) \end{Bmatrix}$$

- So **derivative of projected ansatz** is given by:

$$u'_\pi = \langle 1 \ 2x \rangle \begin{Bmatrix} -\frac{2}{l_e}(2u_1 + u_2 - 3m_0) \\ \frac{3}{l_e^2}(u_1 + u_2 - 2m_0) \end{Bmatrix}$$

- Now how we computed a_1 ?

- Now **average of displacement** is given by :

$$\int_0^{l_e} u_\pi dx = \int_0^{l_e} u_h dx .$$

$$u_\pi = a_1 + a_2 x + a_3 x^2$$

$$m_0 = \frac{1}{l_e} \int_0^{l_e} u_h dx \quad \begin{Bmatrix} a_2 \\ a_3 \end{Bmatrix} = \begin{Bmatrix} -\frac{2}{l_e}(2u_1 + u_2 - 3m_0) \\ \frac{3}{l_e^2}(u_1 + u_2 - 2m_0) \end{Bmatrix}$$

$$a_1 l_e + \frac{1}{2} a_2 l_e^2 + \frac{1}{3} a_3 l_e^3 = l_e m_0 \quad \rightarrow a_1 = u_1$$

- Also **projection ansatz** is given by

$$u_\pi = \langle 1 \ x \ x^2 \rangle \begin{Bmatrix} u_1 \\ -\frac{2}{l_e}(2u_1 + u_2 - 3m_0) \\ \frac{3}{l_e^2}(u_1 + u_2 - 2m_0) \end{Bmatrix}$$

Projected ansatz and derivative in term of nodal displacements and internal variable

$$u_\pi = \langle 1 \ x \ x^2 \rangle \begin{Bmatrix} u_1 \\ -\frac{2}{l_e}(2u_1 + u_2 - 3m_0) \\ \frac{3}{l_e^2}(u_1 + u_2 - 2m_0) \end{Bmatrix}$$

$$u'_\pi = \langle 1 \ 2x \rangle \begin{Bmatrix} -\frac{2}{l_e}(2u_1 + u_2 - 3m_0) \\ \frac{3}{l_e^2}(u_1 + u_2 - 2m_0) \end{Bmatrix}$$

$$\mathbf{N}_\pi^{(2)}(x) = \langle 1 \ x \ x^2 \rangle$$

$$\nabla \mathbf{N}_\pi^{(2)}(x) = \langle 1 \ 2x \rangle$$

$$\mathbb{P}^{(2)} = \frac{1}{l_e^2} \begin{bmatrix} l_e^2 & 0 & 0 \\ -4l_e & -2l_e & 6l_e \\ 3 & 3 & -6 \end{bmatrix}$$

$$\mathbb{B}^{(2)} = \frac{1}{l_e^2} \begin{bmatrix} -4l_e & -2l_e & 6l_e \\ 3 & 3 & -6 \end{bmatrix}$$

$$u_\pi = \mathbf{N}_\pi^{(2)}(x) \mathbb{P}^{(2)} \hat{\mathbf{u}}_e$$

$$u'_\pi = \nabla \mathbf{N}_\pi^{(2)}(x) \mathbb{B}^{(2)} \hat{\mathbf{u}}_e$$

Generalization for higher-order ansatz functions

$$w_\pi(x) = \sum_{k=0}^n a_{k+1} x^k = \mathbf{N}_\pi^{(n)}(x) \mathbf{a} \quad \mathbf{N}_\pi^{(n)}(x) = \langle 1 \quad x \quad x^2 \quad \dots \quad x^n \rangle \quad \mathbf{a}^T = \langle a_1 \quad a_2 \quad a_3 \quad \dots \quad a_{n+1} \rangle^T$$

$$w'_\pi = \mathbf{B}_\pi^{(n)}(x) \hat{\mathbf{a}} \quad \mathbf{B}_\pi^{(n)}(x) = \langle 1 \quad 2x \quad \dots \quad n x^{n-1} \rangle \quad \hat{\mathbf{a}}^T = \langle a_2 \quad a_3 \quad \dots \quad a_{n+1} \rangle^T$$

- **Gradient equation**

$$\int_0^{l_e} p' \left\{ (\Pi[w_h])' - w'_h \right\} dx = 0$$

$$\int_0^{l_e} p' w'_h dx = (p' w_h) \Big|_0^{l_e} - \int_0^{l_e} p'' w_h dx$$

$$p' = [\mathbf{B}_\pi^{(n)}(x)]^T = \langle 1 \quad 2x \quad \dots \quad n x^{n-1} \rangle^T$$

$$p'' = \langle 0 \quad 2 \quad \dots \quad n(n-1)x^{n-2} \rangle^T$$

- **Here n is order of ansatz**

$$\int_0^{l_e} p' w'_\pi \, dx = \int_0^{l_e} [\mathbf{B}_\pi^{(n)}(x)]^T [\mathbf{B}_\pi^{(n)}(x)] \, dx \hat{\mathbf{a}}$$

$$= \int_0^{l_e} \begin{bmatrix} 1 & 2x & \dots & nx^{n-1} \\ 2x & 4x^2 & \dots & 2nx^n \\ \vdots & \vdots & \ddots & \vdots \\ nx^{n-1} & 2nx^n & \dots & n^2 x^{2n-2} \end{bmatrix} \, dx \begin{Bmatrix} a_2 \\ a_3 \\ \vdots \\ a_{n+1} \end{Bmatrix}$$

$$\int_0^{l_e} p' w'_\pi \, dx = \begin{bmatrix} l_e & l_e^2 & \dots & l_e^n \\ l_e^2 & \frac{4}{3}l_e^3 & \dots & \frac{2n}{n+1}l_e^{n+1} \\ \vdots & \vdots & \ddots & \vdots \\ l_e^n & \frac{2n}{n+1}l_e^{n+1} & \dots & \frac{n^2}{2n-1}l_e^{2n-1} \end{bmatrix} \begin{Bmatrix} a_2 \\ a_3 \\ \vdots \\ a_{k+1} \end{Bmatrix} = \mathbf{G} \hat{\mathbf{a}}$$

$$\mathbf{r}(w_h) = (p' w_h)|_0^{l_e} - \int_0^{l_e} p'' w_h \, dx = \left[\begin{Bmatrix} 1 \\ 2x \\ \vdots \\ n x^{n-1} \end{Bmatrix} w'_h \right]_0^{l_e} - \begin{Bmatrix} 0 \\ 2l_e m_0 \\ \vdots \\ n(n-1) l_e^{n-1} m_{n-2} \end{Bmatrix}$$

$$p' = [\mathbf{B}_\pi^{(n)}(x)]^T = \langle 1 \quad 2x \quad \dots \quad n x^{n-1} \rangle^T \quad p'' = \langle 0 \quad 2 \quad \dots \quad n(n-1)x^{n-2} \rangle^T \quad m_k = \frac{1}{l_e^{k+1}} \int_0^{l_e} x^k w_h \, dx \quad \text{with } k = 0, 1, \dots$$

Here m_k is internal variable called **moment** same unit as displacement. Also, $\mathbf{k} = \mathbf{n}-2$.

$$\mathbf{r}(w_h) = (p' w_h)|_0^{l_e} - \int_0^{l_e} p'' w_h \, dx = \left[\begin{Bmatrix} 1 \\ 2x \\ \vdots \\ n x^{n-1} \end{Bmatrix} w'_h \right]_0^{l_e} - \begin{Bmatrix} 0 \\ 2l_e m_0 \\ \vdots \\ n(n-1) l_e^{n-1} m_{n-2} \end{Bmatrix}$$

$$\mathbf{r}(\mathbf{w}_e, \mathbf{m}_e) = \begin{Bmatrix} (w_2 - w_1) \\ 2l_e w_2 \\ \vdots \\ nl_e^{n-1} w_2 \end{Bmatrix} - \begin{Bmatrix} 0 \\ 2l_e m_0 \\ \vdots \\ n(n-1) l_e^{n-1} m_{n-2} \end{Bmatrix}$$

$w_h(0) = w_1$
 $w_h(l_e) = w_2$

$$\mathbf{G} \hat{\mathbf{a}} = \mathbf{r}(\mathbf{w}_e, \mathbf{m})$$

$$\hat{\mathbf{a}} = \mathbf{G}^{-1} \mathbf{r}(\mathbf{w}_e, \mathbf{m}) = \mathbb{B}^{(n)} \hat{\mathbf{w}}_e^{(n)}$$

$$\hat{\mathbf{a}}^T = \langle a_2 \quad a_3 \quad \dots \quad a_{n+1} \rangle^T \qquad \qquad \hat{\mathbf{w}}_e^{(n)} = \langle w_1 \quad w_2 \quad m_0 \quad \dots \quad m_{n-2} \rangle^T$$

- **So gradient of projected ansatz in term of nodal and internal variable is given by**

$$w'_\pi = \mathbf{B}_\pi^{(n)}(x) \mathbb{B}^{(n)} \hat{\mathbf{w}}_e^{(n)}.$$

- Average displacement equation is given by

$$\int_0^{l_e} (\Pi[w_h] - w_h) dx = 0.$$

$$w_\pi(0) = w_1$$

$$a_1 = w_1$$

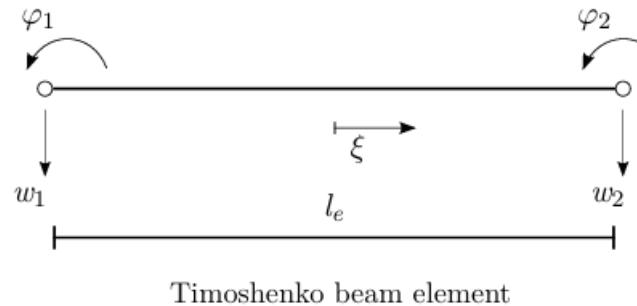
- So projected ansatz is given by

$$w_\pi(x) = \mathbf{N}_\pi^{(n)}(x) \mathbf{a} = \mathbf{N}_\pi^{(n)}(x) \mathbb{P}^{(n)} \hat{\mathbf{w}}_e^{(n)}$$

$$\mathbf{a}^T = \langle a_1 \quad a_2 \quad a_3 \quad \dots \quad a_{n+1} \rangle^T \qquad \qquad \mathbb{P}^{(n)} = \begin{bmatrix} 1 & \mathbf{o}^T \\ \mathbb{B}^{(n)} & \end{bmatrix} \qquad \qquad \mathbf{o}^T = \langle 0 \quad 0 \quad \dots \quad 0 \rangle$$

- Here o vector has **n-1 zeros**

Implementation of a Timoshenko beam element based on VEM



- **Strong form**

$$V(x) = G A_s [w'(x) - \psi(x)] \quad M(x) = -EI\psi'(x)$$

- **Weak form**

$$\int_0^l \{ \phi'(x) EI \psi'(x) + [v'(x) - \phi(x)] G A_s [w'(x) - \psi(x)] \} dx - \int_0^l f(x) v(x) dx = 0$$

- **Implementation of VEM is same as fem . We need to change the FEM ansatz to VEM(projected) ansatz in weak form**

Python Implementation & Live Demo



- Demonstrate the **implementation of FEM and VEM in Python**
- Present the **stiffness matrix formulations** for FEM and VEM:
 - Linear formulation**
 - Quadratic formulation**
 - Reduced integration formulation**
- Analyze **cantilever beams and simply supported beams** under **uniformly distributed loading**
- Investigate locking phenomena
- Study **locking-free schemes**, including:
 - Reduced integration methods**
 - Mixed formulations**
- Evaluate relative L2 error for **h-convergence**
- Evaluate relative L2 error with respect to **slenderness ratio** for a fixed number of elements

Conclusion

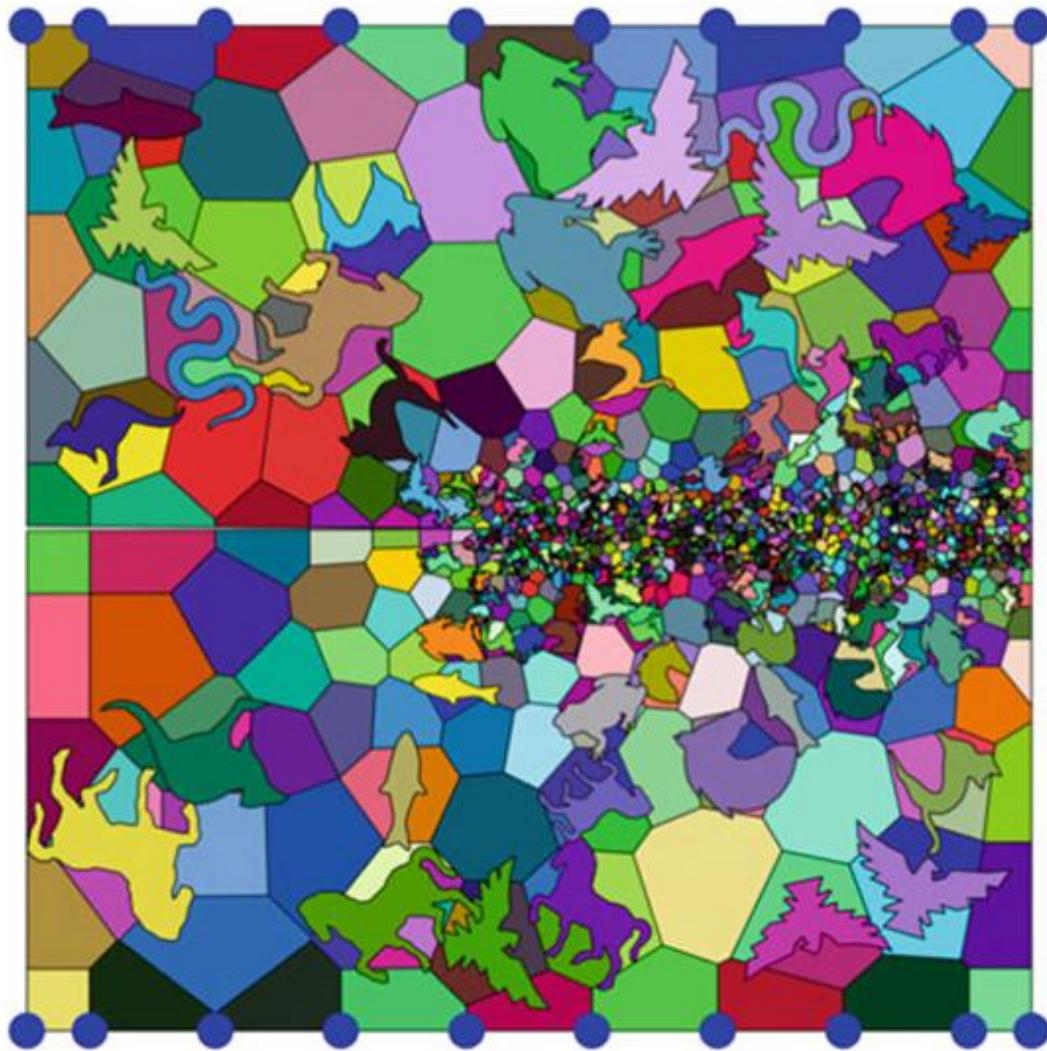
- For the Timoshenko beam, **FEM and VEM give identical stiffness matrix for linear ansatz**; differences only appear for higher-order formulations.
- In **higher-order VEM**, stiffness matrix differ from FEM unless **internal moment variables are condensed out**. After condensation, VEM becomes equivalent to **FEM static condensation**.
- **FEM and VEM produce nearly identical results and L2 errors** for both **linear and quadratic approximations**, indicating no significant advantage of one over the other in these cases.
- In VEM, a **numerical quadratic scheme can be constructed using linear mapping**, due to the presence of **two nodal degrees of freedom and internal moment variables**.
- In contrast, FEM requires **higher-order (isoparametric) mapping** to achieve the same accuracy for higher-order approximations.
- However, for **1D problems**, linear mapping are sufficient for higher order FEM ansatz, so **VEM provides no clear benefit over FEM** in mapping.
- Both FEM and VEM can be implemented in **commercial solvers**, but for linear and quadratic Timoshenko beam problems, **VEM offers no practical advantage**.
- Differences between results of FEM and VEM may appear for **higher-order formulations**, but most commercial solvers **do not use polynomial orders higher than two**.

- The **VEM formulation does not inherently eliminate shear locking**; locking must be addressed using **special techniques** such as mixed formulations or reduced integration, similar to FEM.
- The source of locking lies in the **Timoshenko beam equations themselves**, particularly the shear term with **one variable and one derivative**, not in the numerical formulation.
- FEM and VEM show **similar h-convergence behavior and L2 errors**, even for varying beam slenderness ratios.
- VEM can be viewed as an **alternative formulation to FEM**, based on exact geometry and projection operators.
- In VEM, the displacement field u_h is defined through a **projection operator** $u_h = \Pi_h u$, and is **not explicitly known inside the element**, whereas in FEM the variation of u_h within the element is explicitly defined.

Literature review



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Any
Question?