

UCL Mechanical Engineering 2020/2021

MECH0013 Coursework 1

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1 Question 1

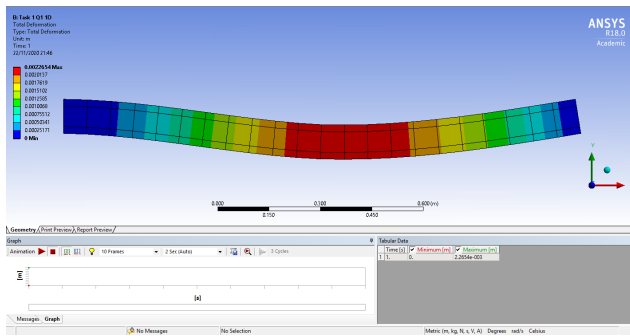


Figure 1: Total deformation in beam

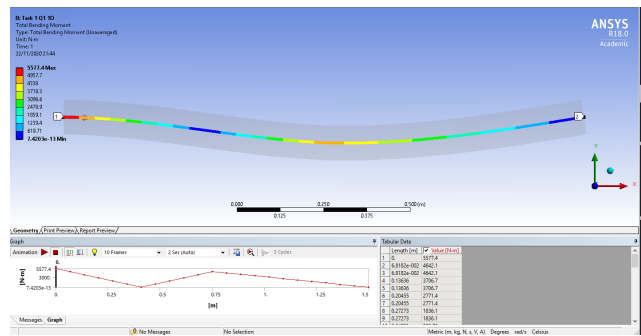


Figure 2: Bending moment in beam

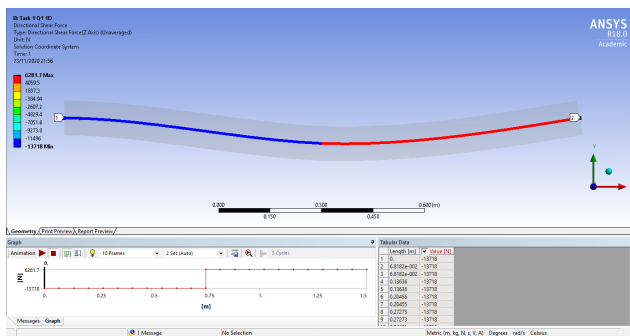


Figure 3: Directional shear in beam

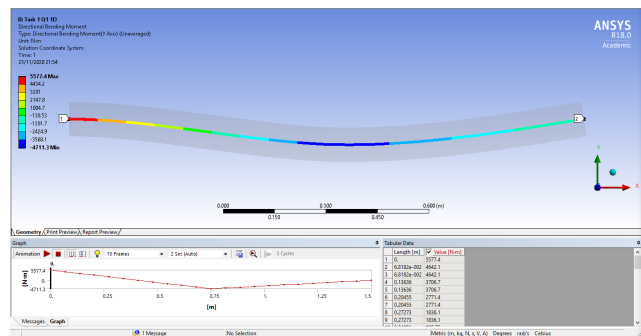
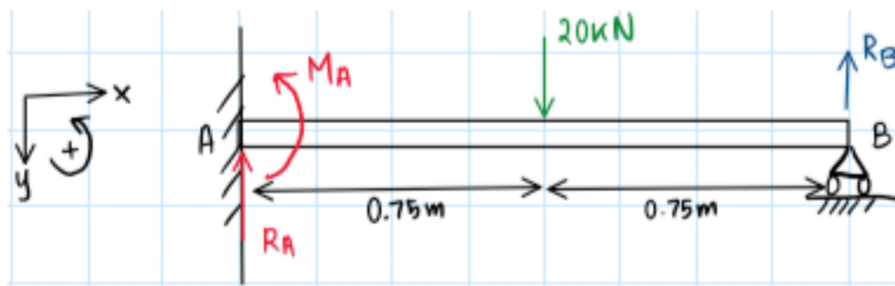


Figure 4: Directional bending in beam

FIX LINK to see numerical data of the directional deformation and the bending moment of the beam.

2 Question 2

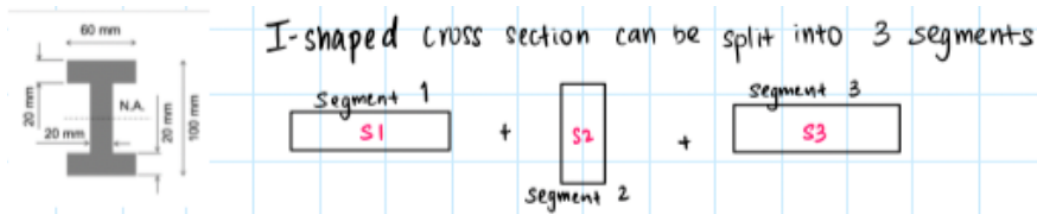


$$\sum F_y = 0 \rightarrow R_A + R_B = 20000 \quad (2.1)$$

$$\sum M_B = 0 \rightarrow M_B + 20000(0.75) - R_A(1.5) = 0 \quad (2.2)$$

$$M_A + 15000 - 1.5R_A = 0 \quad (2.3)$$

Determine second moment of area (I):



Segment 1

$$I_x = \bar{I}_x + A d y^2 = \frac{1}{12} (0.06)(0.02)^3 + (0.06)(0.02)(0.04)^2 = 1.96 \times 10^{-6} \text{ m}^4 \quad (2.4)$$

Segment 2

$$I_x = \bar{I}_x + A d y^2 = \frac{1}{12} (0.06)(0.02)^3 + (0.06)(0.02)(0)^2 = 3.6 \times 10^{-7} \text{ m}^4 \quad (2.5)$$

Segment 3

$$I_x = \bar{I}_x + A d y^2 = \frac{1}{12} (0.06)(0.02)^3 + (0.06)(0.02)(0.04)^2 = 1.96 \times 10^{-6} \text{ m}^4 \quad (2.6)$$

$$I_{\text{total}} = 4.28 \times 10^{-6} \text{ m}^4 \quad (2.7)$$

Macaulay's Method

$$M = M_A + F(x - 0.75) - R_A(x) \quad (2.8)$$

$$\theta = -\frac{1}{EI} \int M dx = -\frac{1}{EI} \left[M_A x + \frac{F(x - 0.75)^2}{2} + \frac{R_A(x)^2}{2} \right] + \theta_0 \quad (2.9)$$

$$y = \int \theta dx = -\frac{1}{EI} \left[\frac{M_A x^2}{2} + \frac{F(x - 0.75)^3}{6} + \frac{R_A(x)^3}{6} \right] + \theta_0 x + y_0 \quad (2.10)$$

Boundary conditions. At $y = 0$, $x = 0$:

$$y(0) = 0 = \theta_0 \cdot (0) + y_0 \rightarrow y_0 = 0 \quad (2.11)$$

At $\theta = 0$, $x = 0$:

$$\theta(0) = 0 = \theta_0 \rightarrow \theta_0 = 0 \quad (2.12)$$

At $y = 0$, $x = 1.5$:

$$y(1.5) = 0 = -\frac{1}{EI} \left[\frac{M_A(1.5)^2}{2} + \frac{F(1.5 - 0.75)^3}{6} + \frac{R_A(1.5)^3}{6} \right] + 0 \cdot 1.5 + 0 \quad (2.13)$$

$$0 = \frac{9}{8} M_A + 1406.25 - \frac{9}{16} R_A \quad (2.14)$$

Multiply equation (2.3) by $\frac{9}{8}$:

$$\frac{9}{8} M_A + 16875 - \frac{27}{16} R_A = 0 \quad (2.15)$$

Equations (2.15) - (2.14):

$$15468.75 = \frac{9}{8} R_A \rightarrow R_A = 13750 \text{ N} \quad (2.16)$$

$$\therefore M_A = 1.5(13750) - 15000 \rightarrow M_A = 5625 \text{ N} \quad (2.17)$$

$$\therefore R_B = 20000 - 13750 \rightarrow R_B = 6250 \text{ N} \quad (2.18)$$

We know y_{max} occurs at $\theta = 0$

$$M_A x + \frac{F(x - 0.75)^2}{2} - \frac{R_A x^2}{2} = 3125x^2 - 9325x + 5625 = 0 \quad (2.19)$$

$$x \neq 2.171 \text{ m} \rightarrow x = 0.829 \text{ m (3dp)} \quad (2.20)$$

$$y_{max} = -\frac{1}{EI} \left[\frac{M_A(0.829)^2}{2} + \frac{F(0.829 - 0.75)^3}{6} + \frac{R_A(0.829)^3}{6} \right] = -2.099 \times 10^{-3} \text{ m (3dp)} \quad (2.21)$$

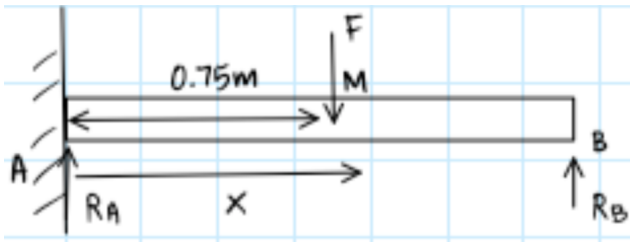


Figure 5: Shear force diagram

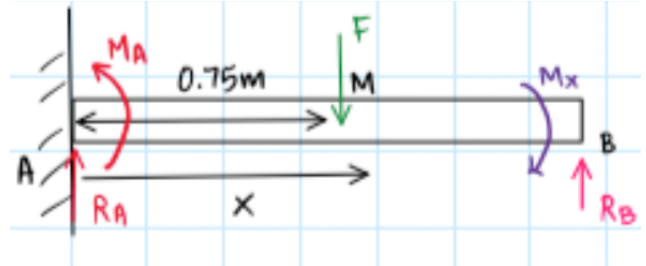


Figure 6: Bending moment diagram

Section at $0 \leq x < 0.75$.

$$Q_x = R_A = 13750 \text{ N} \quad (2.22)$$

$$M_x = M_A - R_A \cdot x = 5625 - 13750x \quad (2.23)$$

$$(2.24)$$

Section at $0 \leq x < 1.5$.

$$\text{at } x = 0, M_x = 5626 \text{ N m} \quad (2.25)$$

$$\text{at } x = 0.75, M_x = -4687.5 \text{ N m} \quad (2.26)$$

$$\text{at } x = 1.5, M_x = 0 \text{ N m} \quad (2.27)$$

$$Q_x = R_A - F = 6250 \text{ N} \quad (2.28)$$

$$M_x = M_A - R_A \cdot x + F(x - 0.75) \quad (2.29)$$

$$M_x = 5625 - 13750x + 20000x - 15000 \quad (2.30)$$

$$M_x = 6250x - 9375 \quad (2.31)$$

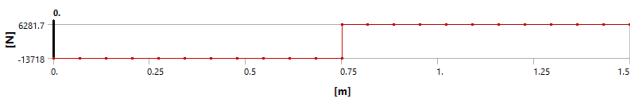


Figure 7: Ansys shear force graph

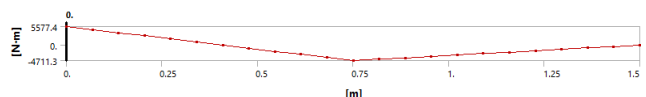


Figure 8: Ansys bending moment graph

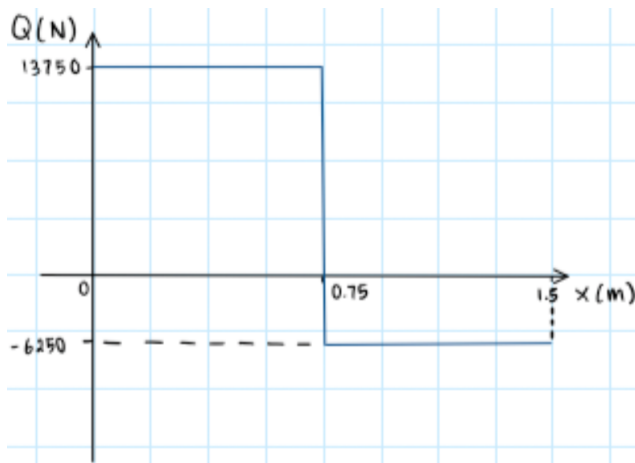


Figure 9: Shear force graph

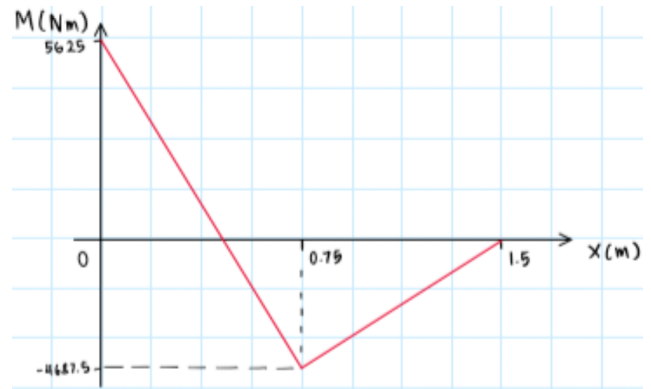


Figure 10: Bending moment graph

3 Question 3

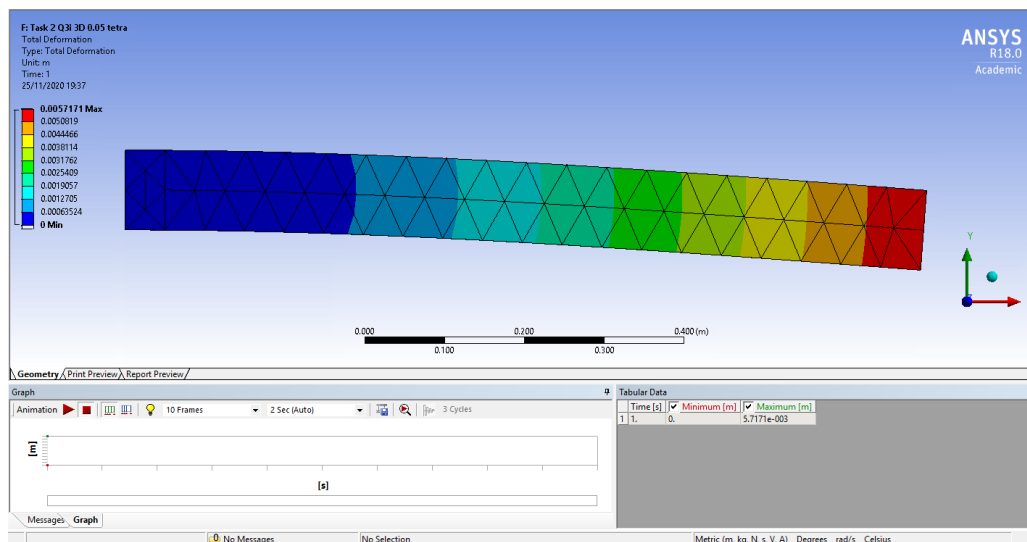


Figure 11: Tetrahedron mesh with size 0.05 m

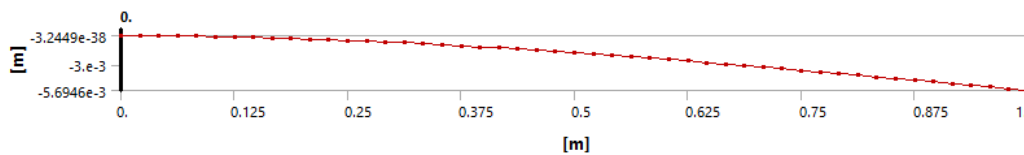


Figure 12: Tetrahedron mesh with size 0.05 m

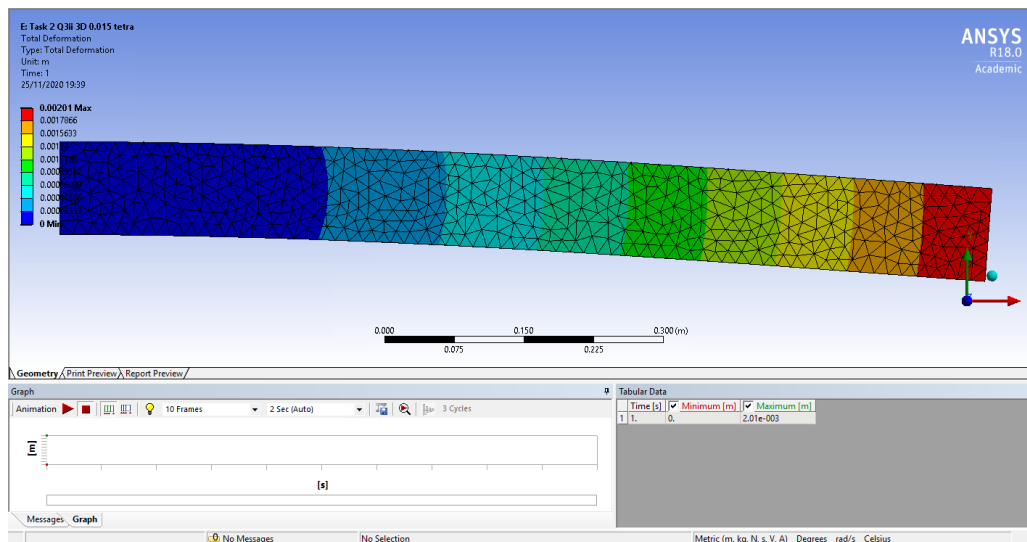


Figure 13: Tetrahedron mesh with size 0.015 m

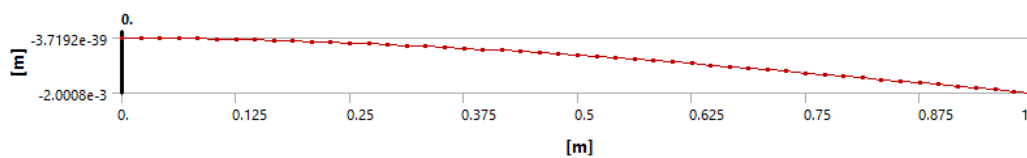


Figure 14: Tetrahedron mesh with size 0.015 m

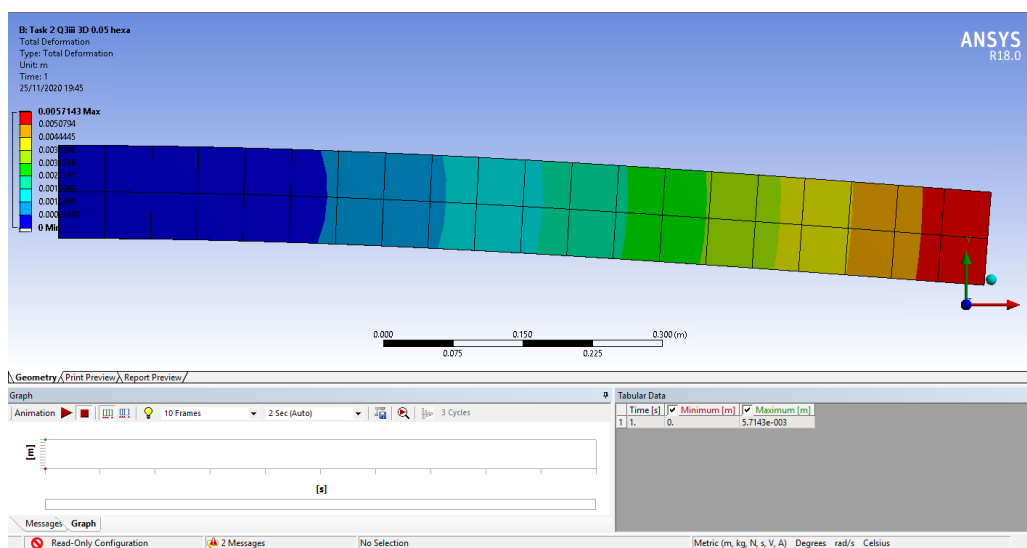


Figure 15: Hexahedron mesh with size 0.05 m

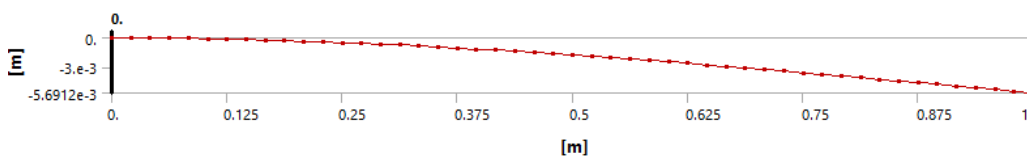


Figure 16: Hexahedron mesh with size 0.05 m

4 Question 5

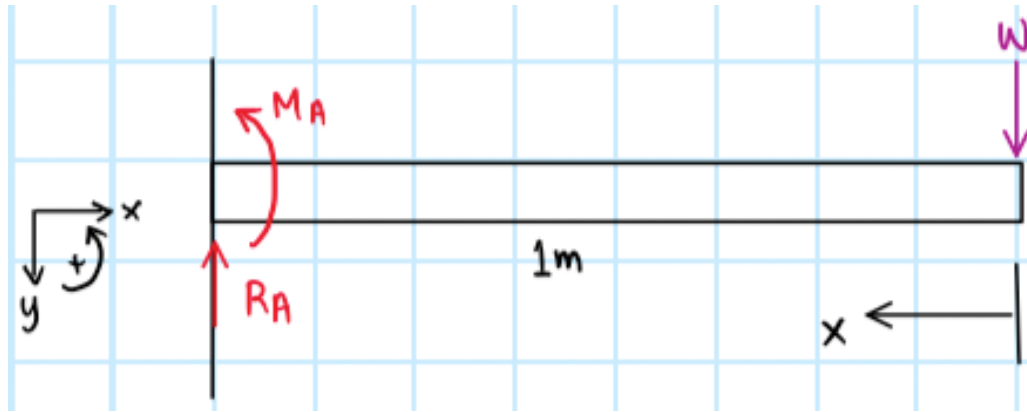


Figure 17: Square section cantilever beam

Determination of support reactions:

$$\sum F_y = 0, R_A = w, R_A = 1 \times 10^4 \text{ kN} \quad (4.1)$$

$$M_A = -w = -1 \times 10^4 \text{ kN} \quad (4.2)$$

Determine second moment of area (I). Given a square cross-section:

$$I = \frac{BH^3}{12} = \frac{(0.1)(0.1)^3}{12} = 8.33 \times 10^{-6} \text{ m}^4 \quad (4.3)$$

Determination of deflection:

$$M = wx \quad (4.4)$$

$$\theta = -\frac{1}{EI} \int M \, dx \quad (4.5)$$

$$\theta = -\frac{1}{EI} \left[\frac{wx^2}{2} \right] + \theta_0 \quad (4.6)$$

$$y = \int \theta \, dx \quad (4.7)$$

$$y = -\frac{1}{EI} \left[\frac{wx^3}{6} \right] + \theta_0 x + y_0 \quad (4.8)$$

Boundary conditions: At $x = L$, $\theta = 0$:

$$0 = -\frac{wL^2}{2EI} + \theta_0 \rightarrow \theta_0 = \frac{wL^2}{2EI} \quad (4.9)$$

At $x = L$, $y = 0$:

$$0 = -\frac{wL^3}{6EI} + \frac{wL^3}{2EI} + y_0 \rightarrow y_0 = -\frac{wL^3}{3EI} \quad (4.10)$$

Thus,

$$\theta = -\frac{1}{EI} \left[\frac{wx^2}{2} \right] + \frac{wL^2}{2EI} \quad (4.11)$$

$$y = -\frac{1}{EI} \left[\frac{wx^3}{6} \right] + \frac{wL^2}{2EI}x - \frac{wL^3}{3EI} \quad (4.12)$$

y_{max} occurs at free end, $x = 0$ m

$$y_{max} = -\frac{1}{EI} \left[\frac{w(0)^3}{6} \right] + \frac{wL^2}{2EI}(0) - \frac{wL^3}{3EI} \quad (4.13)$$

$$y_{max} = -\frac{wL^3}{3EI} \quad (4.14)$$

$$y_{max} = -\frac{10 \times 10^3 \times 1^3}{70 \times 10^9 \times 3 \times 8.33 \times 10^{-6}} \quad (4.15)$$

$$y_{max} = -5.714 \times 10^{-3} \text{ m} \quad (4.16)$$