

UCL Mechanical Engineering 2020/2021

MECH0013 Final Assessment

NCWT3

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1 Question 1

1.1 i

Straight section analysis:

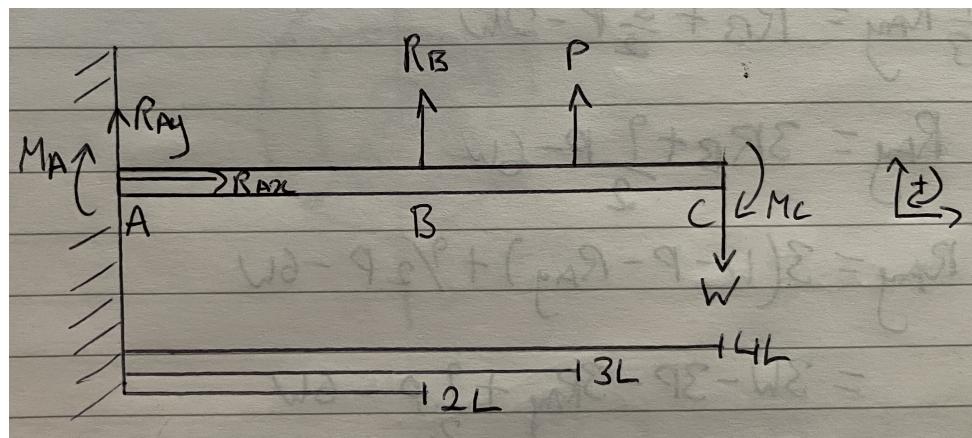


Figure 1: Diagram to show horizontal beam arrangement.

Equilibrium conditions:

$$\sum F_x : R_{Ax} = 0 \quad (1.1)$$

$$\sum F_y : R_{Ay} + R_B + P = W \quad (1.2)$$

$$\sum M_A : -M_A + M_C + 4WL = 2R_BL + 3PL \quad (1.3)$$

Using Macaulay's method:

$$M = -M_A + R_{Ay}x + R_B <x - 2L> + P <x - 3L> \quad (1.4)$$

Slope:

$$\theta = \frac{1}{EI} \int (M) dx \quad (1.5)$$

$$\theta = \frac{1}{EI} \int (-M_A + R_{Ay}x + R_B <x - 2L> + P <x - 3L>) dx \quad (1.6)$$

$$\theta = \frac{1}{EI} \left[-M_Ax + \frac{R_{Ay}x^2}{2} + \frac{R_b <x - 2L>^2}{2} + \frac{P <x - 3L>^2}{2} \right] + \theta_0 \quad (1.7)$$

Deflection:

$$y = \int (\theta) dx \quad (1.8)$$

$$y = \int \left(\frac{1}{EI} \left[-M_Ax + \frac{R_{Ay}x^2}{2} + \frac{R_b <x - 2L>^2}{2} + \frac{P <x - 3L>^2}{2} \right] + \theta_0 \right) dx \quad (1.9)$$

$$y = \frac{1}{EI} \left[-\frac{M_Ax^2}{2} + \frac{R_{Ay}x^3}{6} + \frac{R_b <x - 2L>^3}{6} + \frac{P <x - 3L>^3}{6} \right] + \theta_0 x + y_0 \quad (1.10)$$

Curved section analysis:

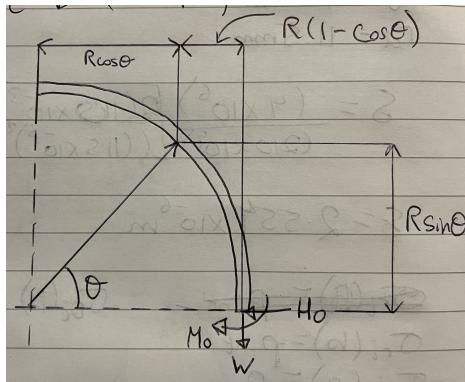


Figure 2: Diagram to show curved beam arrangement.

$$M(\theta) = WR(1 - \cos \theta) + H_0R \sin \theta + M_0 \quad (1.11)$$

$$\frac{\partial M}{\partial M_0} = 1 \quad (1.12)$$

$$\varphi_A = \int_0^L \left(\frac{M}{EI} \frac{\partial M}{\partial M_0} \right) dx = 0 \quad (1.13)$$

Converting to polar ($dx = R d\theta$):

$$\varphi_A = \frac{1}{EI} \int_0^{\frac{\pi}{2}} \left((WR(1 - \cos \theta) + H_0 R \sin \theta + M_0) (1)(R) \right) d\theta \quad (1.14)$$

M_0 and H_0 represent dummy loads and can be neglected:

$$\varphi_A = \frac{1}{EI} \int_0^{\frac{\pi}{2}} \left(WR^2 (1 - \cos \theta) \right) d\theta \quad (1.15)$$

$$\varphi_A = \frac{1}{EI} \left[WR^2 (\theta - \sin \theta) \right]_0^{\frac{\pi}{2}} \quad (1.16)$$

$$\varphi_A = \frac{WR^2 \left(\frac{\pi}{2} - 1 \right)}{EI} \quad (1.17)$$

Boundary conditions:

$$x = 0, y = 0 \therefore y_0 = 0 \quad (1.18)$$

$$x = 0, \theta = 0 \therefore \theta_0 = 0 \quad (1.19)$$

$$x = 2L, y = 0 \quad (1.20)$$

$$x = 4l, \theta = \frac{WR^2 \left(\frac{\pi}{2} - 1 \right)}{EI} \quad (1.21)$$

From 1.20:

$$0 = \frac{1}{EI} \left[-\frac{M_A (2L)^2}{2} + \frac{R_{Ay} (2L)^3}{6} + \frac{R_B < 2L - 2L >^3}{6} \right] \quad (1.22)$$

$$0 = \frac{1}{EI} \left[-2M_A L^2 + \frac{4R_{Ay} L^3}{3} + 0 \right] \quad (1.23)$$

$$0 = -2M_A L^2 + \frac{4R_{Ay} L^3}{3} \quad (1.24)$$

$$M_A = \frac{2R_{Ay} L}{3} \quad (1.25)$$

From 1.21:

$$\frac{WR^2 \left(\frac{\pi}{2} - 1 \right)}{EI} = \frac{1}{EI} \left[-M_A (4L) + \frac{R_{Ay} (4L)^2}{2} + \frac{R_B < 4L - 2L >^2}{2} + \frac{P < 4L - 3L >^2}{2} \right] \quad (1.26)$$

$$WR^2 \left(\frac{\pi}{2} - 1 \right) = -4M_A L + 8R_{Ay} L^2 + 2R_B L^2 + \frac{PL^2}{2} \quad (1.27)$$

Substituting 1.25:

$$WR^2 \left(\frac{\pi}{2} - 1 \right) = -\frac{8R_{Ay} L^2}{3} + 8R_{Ay} L^2 + 2R_B L^2 + \frac{PL^2}{2} \quad (1.28)$$

$$WR^2 \left(\frac{\pi}{2} - 1 \right) = \frac{16R_{Ay} L^2}{3} + 2R_B L^2 + \frac{PL^2}{2} \quad (1.29)$$

From 1.2:

$$R_B = W - P - R_{Ay} \quad (1.30)$$

Substituting 1.30:

$$WR^2 \left(\frac{\pi}{2} - 1 \right) = \frac{16R_{Ay} L^2}{3} + 2(W - P - R_A) L^2 + \frac{PL^2}{2} \quad (1.31)$$

$$WR^2 \left(\frac{\pi}{2} - 1 \right) = \frac{16R_{Ay} L^2}{3} + 2WL^2 - 2PL^2 - 2R_A L^2 + \frac{PL^2}{2} \quad (1.32)$$

$$WR^2 \left(\frac{\pi}{2} - 1 \right) = \frac{10R_{Ay} L^2}{3} + 2WL^2 - \frac{3PL^2}{2} \quad (1.33)$$

From 1.3, 1.2, 1.30, 1.25:

$$-\frac{2R_{Ay}L}{3} + WR + 4WL = 2(W - P - R_{Ay}) + 3PL \quad (1.34)$$

$$-\frac{2R_{Ay}L}{3} + 2R_{Ay}L = 2WL - 2PL + 3PL - WR - 4WL \quad (1.35)$$

$$\frac{4R_{Ay}L}{3} = PL - WR - 2WL \quad (1.36)$$

$$R_{Ay} = \frac{3}{4} \left(P - \frac{WR}{L} - 2W \right) \quad (1.37)$$

Substituting 1.37:

$$WR^2 \left(\frac{\pi}{2} - 1 \right) = \frac{10L^2}{3} \left(\frac{3}{4} \left(P - \frac{WR}{L} - 2W \right) \right) + 2WL^2 - \frac{3PL^2}{2} \quad (1.38)$$

$$WR^2 \left(\frac{\pi}{2} - 1 \right) = \frac{5PL^2}{2} - \frac{5WRL}{2} - 5WL^2 + 2WL^2 - \frac{3PL^2}{2} \quad (1.39)$$

$$WR^2 \left(\frac{\pi}{2} - 1 \right) = PL^2 - 3WL^2 - \frac{5WRL}{2} \quad (1.40)$$

Rearranging for P :

$$PL^2 = 3WL^2 + \frac{5WRL}{2} + WR^2 \left(\frac{\pi}{2} - 1 \right) \quad (1.41)$$

$$P = 3W + \frac{5WR}{2L} + \frac{WR^2}{L^2} \left(\frac{\pi}{2} - 1 \right) \quad (1.42)$$

$$P = 3(42.67) + \frac{5(42.67)(0.3)}{2(0.35)} + \frac{(42.67)(0.3)^2}{(0.35)^2} \left(\frac{\pi}{2} - 1 \right) \quad (1.43)$$

$$P = 237.34 \text{ N} \quad (1.44)$$

1.2 ii

From Figure 2, the bending moment is:

$$M = W(R(1 - \cos \theta)) + H_0(R \sin \theta) + M_0 \quad (1.45)$$

The horizontal displacement of the curved beam can be found using 1.46:

$$\delta_H = \int_0^\theta \left(R \frac{M}{EI} \frac{\partial M}{\partial H_0} \right) d\theta \quad (1.46)$$

From 1.45

$$\frac{\partial M}{\partial H_0} = R \sin \theta \quad (1.47)$$

Substituting 1.47 and 1.45 into 1.46

$$\delta_H = \frac{1}{EI} \int_0^{\frac{\pi}{2}} \left(R \left(W(R(1 - \cos \theta)) + H_0(R \sin \theta) + M_0 \right) R \sin \theta \right) d\theta \quad (1.48)$$

H_0 and M_0 are dummy forces and can be neglected:

$$\delta_H = \frac{1}{EI} \int_0^{\frac{\pi}{2}} \left(R \left(W(R(1 - \cos \theta)) \right) R \sin \theta \right) d\theta \quad (1.49)$$

$$\delta_H = \frac{R^3 W}{EI} \int_0^{\frac{\pi}{2}} ((1 - \cos \theta) (\sin \theta)) d\theta \quad (1.50)$$

$$\delta_H = \frac{R^3 W}{EI} \int_0^{\frac{\pi}{2}} (\sin \theta - \sin \theta \cos \theta) d\theta \quad (1.51)$$

$$\delta_H = \frac{R^3 W}{EI} \left[-\cos \theta - \frac{\sin^2 \theta}{2} \right]_0^{\frac{\pi}{2}} \quad (1.52)$$

$$\delta_H = \frac{R^3 W}{EI} \left[-0 - \frac{1}{2} + 1 + 0 \right] \quad (1.53)$$

$$\delta_H = \frac{R^3 W}{2EI} \quad (1.54)$$

Substituting the values of R , W , E , $I = \frac{\pi}{4} \left(\frac{d_0^4}{2^4} - \frac{d_i^4}{2^4} \right)$:

$$\delta_H = \frac{0.3^3 \times 42.67}{2 \times 210 \times 10^9 \times \left(\frac{\pi}{4} (0.01^4 - 0.0085^4) \right)} \quad (1.55)$$

$$\delta_H = 7.307 \times 10^{-4} \text{ m} = 0.731 \text{ mm (3sf)} \quad (1.56)$$

The vertical displacement of the curved beam can be found using 1.57:

$$\delta_V = \int_0^\theta \left(R \frac{M}{EI} \frac{\partial M}{\partial W} \right) d\theta \quad (1.57)$$

From 1.45:

$$\frac{\partial M}{\partial W} = R(1 - \cos \theta) \quad (1.58)$$

Substituting 1.58 and 1.45 into 1.57 and neglecting dummy loads:

$$\delta_V = \frac{1}{EI} \int_0^{\frac{\pi}{2}} \left(R \left(W(R(1 - \cos \theta)) \right) R(1 - \cos \theta) \right) d\theta \quad (1.59)$$

$$\delta_V = \frac{R^3 W}{EI} \int_0^{\frac{\pi}{2}} (1 - \cos \theta)^2 d\theta \quad (1.60)$$

$$\delta_V = \frac{R^3 W}{EI} \int_0^{\frac{\pi}{2}} (1 - 2 \cos \theta + \cos^2 \theta) d\theta \quad (1.61)$$

$$\delta_V = \frac{R^3 W}{EI} \int_0^{\frac{\pi}{2}} \left(1 - 2 \cos \theta + \frac{1}{2} + \frac{\cos 2\theta}{2} \right) d\theta \quad (1.62)$$

$$\delta_V = \frac{R^3 W}{EI} \int_0^{\frac{\pi}{2}} \left(\frac{3}{2} - 2 \cos \theta + \frac{\cos 2\theta}{2} \right) d\theta \quad (1.63)$$

$$\delta_V = \frac{R^3 W}{EI} \left[\frac{3\theta}{2} - 2 \sin \theta + \frac{\sin 2\theta}{4} \right]_0^{\frac{\pi}{2}} \quad (1.64)$$

$$\delta_V = \frac{R^3 W}{EI} \left[\frac{3\pi}{4} - 2 + 0 - 0 + 2 - 0 \right] \quad (1.65)$$

$$\delta_V = \frac{R^3 W}{EI} \left(\frac{3\pi}{4} - 2 \right) \quad (1.66)$$

Substituting the values of R , W , E , $I = \frac{\pi}{4} \left(\frac{d_0^4}{2^4} - \frac{d_i^4}{2^4} \right)$:

$$\delta_V = \frac{0.3^3 \times 42.67}{210 \times 10^9 \times \left(\frac{\pi}{4} (0.01^4 - 0.0085^4) \right)} \left(\frac{3\pi}{4} - 2 \right) \quad (1.67)$$

$$\delta_V = 5.205 \times 10^{-4} \text{ m} = 0.521 \text{ mm (3sf)} \quad (1.68)$$

From Figure 1 and 1.10:

$$y = \frac{1}{EI} \left[-\frac{M_A x^2}{2} + \frac{R_{Ay} x^3}{6} + \frac{R_B < x - 2L >^3}{6} + \frac{P < x - 3L >^3}{6} \right] + \theta_0 x + y_0 \quad (1.69)$$

Applying boundary conditions and finding slope at location $x = 4L$:

$$y = \frac{1}{EI} \left[-\frac{M_A(4L)^2}{2} + \frac{R_{Ay}(4L)^3}{6} + \frac{R_B(2L)^3}{6} + \frac{PL^3}{6} \right] \quad (1.70)$$

Substituting values of M_A , R_{Ay} , R_B , P , E , I and L :

$$y = \frac{1}{\left(\frac{\pi}{4} (0.01^4 - 0.0085^4) \right)} \left[-\frac{20.1995(4(0.35))^2}{2} + \frac{86.5694(4(0.35))^3}{6} + \frac{-281.2393(2(0.35))^3}{6} + \frac{237(0.35)^3}{6} \right] \quad (1.71)$$

$$y = 6.864 \times 10^{-3} \text{ m} = 6.86 \text{ mm (3sf)} \quad (1.72)$$

1.3 iii

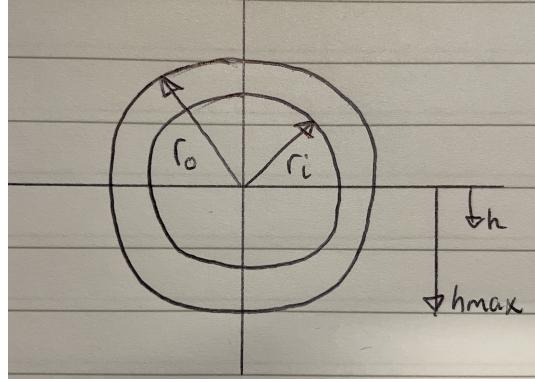


Figure 3: Diagram to show cross-section of horizontal beam.

Where $r_i = 8.5 \text{ mm}$, $r_o = 10 \text{ mm}$. The yield moment can be found using 1.73:

$$M_Y = \frac{\sigma_Y I}{h_{max}} \quad (1.73)$$

Where $I = \frac{\pi}{4} (r_o^4 - r_i^4)$ and $h_{max} = r_o$. Substituting:

$$M_Y = \frac{480 \times 10^6 \times \frac{\pi}{4} (0.01^4 - 0.0085^4)}{0.01} \quad (1.74)$$

$$M_Y = 180.199 \text{ N m} = 180 \text{ N m (3sf)} \quad (1.75)$$

We now want to plot the bending moment as a function of x . MATLAB was used to plot Figure 4.

```

1 clc
2 clear
3 close all
4
5 %define vars
6 P = 0; %value of corrective load
7 W = 42.67; %camera weight
8 R = 0.3; %radius of curvature of curved section
9 L = 0.35; %length of a quarter of the beam
10
11 %calc vars
12 R_Ay = 0.75*(P-((W*R)/L) - 2*W); %support reaction at A
13 R_B = W - P - R_Ay; %support reaction at B
14 M_A = (2/3)*R_Ay*L; %moment at A
15
16 %bending moment equation
17 %x = 0
18 M0 = -M_A;
19 %x = L
20 M1 = -M_A + R_Ay*L;
21 %x = 2L
22 M2 = -M_A + 2*R_Ay*L;
23 %x = 3L
24 M3 = -M_A + 3*R_Ay*L + R_B*L;
25 %x = 4L
26 M4 = -M_A + 4*R_Ay*L + 2*R_B*L + P*L;
27
28 %concatenate
29 x = [0 L 2*L 3*L 4*L];
30 M = [M0 M1 M2 M3 M4];
31
32 %plot
33 plot(x,M, 'color', 'red')
34 xlim([0 4*L])
35 ylim([-30 45])
36 grid on
37 hline = refline(0,0);
38 hline.Color = 'b';
39 title('Graph to show bending moment of beam')
40 xlabel('Length along beam/m')
41 ylabel('Magnitude of bending moment/Nm')

```

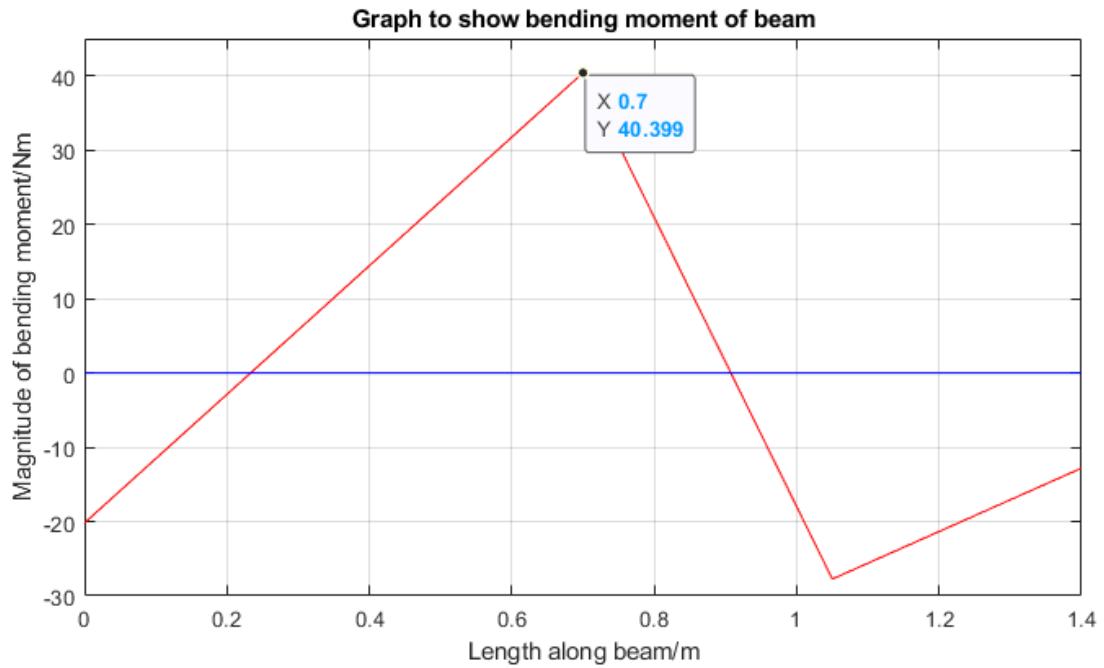


Figure 4: Graph to show bending moment along the horizontal beam.

Variable	Value
M_A	20.1995 N m
R_{Ay}	86.5694 N
R_B	-281.2393 N

Table 1: Table to show values of support reactions from MATLAB.

Hence, we know that the maximum bending moment occurs at $2L$. Let us find the yielding force at this location.

$$M(x = 2L) = -M_A + 2R_{Ay}L \quad (1.76)$$

Substituting in M_A and R_{Ay} (1.25 and 1.37):

$$M_A = \frac{2}{3}R_{Ay}L \quad (1.77)$$

$$R_{Ay} = \frac{3}{4} \left(P - \frac{WR}{L} - 2W \right) \quad (1.78)$$

$$M_A = \frac{2}{3} \left(\frac{3}{4} \left(P - \frac{WR}{L} - 2W \right) \right) L \quad (1.79)$$

$$M_A = \frac{L}{2} \left(P - \frac{WR}{L} - 2W \right) \quad (1.80)$$

$$M(x = 2L) = -\frac{1}{2}PL + \frac{1}{2}WR + WL + \frac{3}{2}PL - \frac{3}{2}WR - 3WL \quad (1.81)$$

$$M = PL - WR - 2WL \quad (1.82)$$

Let $M = M_Y$ and $P = P_Y$:

$$M_Y = P_Y L - WR - 2WL \quad (1.83)$$

$$P_Y = \frac{1}{L} (M_Y + WR + 2WL) \quad (1.84)$$

Substituting the values of M_Y , W , R , L :

$$P_Y = \frac{1}{0.35} (180.199 + 42.67 \times 0.3 + 2 \times 42.67 \times 0.35) \quad (1.85)$$

$$P_Y = 636.769 \text{ N} = 637 \text{ N (3sf)} \quad (1.86)$$

From 1.44:

$$P = 237 \text{ N (3sf)} \quad (1.87)$$

Comparing P and P_Y :

$$P < P_Y \quad (1.88)$$

$$237 \text{ N} < 637 \text{ N} \quad (1.89)$$

Therefore, the corrective load will not result in yielding.

1.4 iv

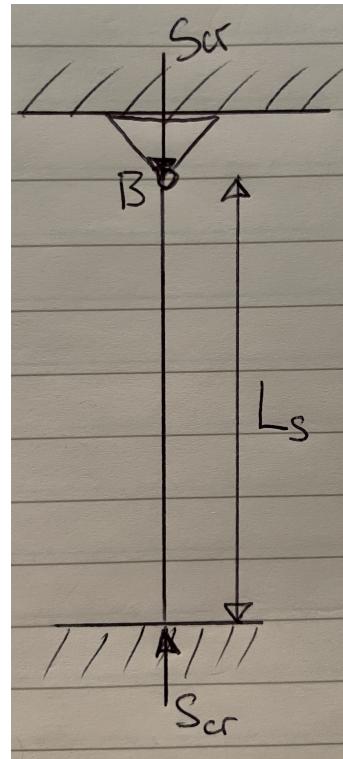


Figure 5: Graph to show arrangement of vertical strut at B.

We can find the critical load using 1.90:

$$S_{cr} = \frac{n^2 \pi^2 EI}{L_e^2} \quad (1.90)$$

Where $L_e = 0.7L_s$ for pinned-fixed columns. The second moment of inertia for the strut can be found using 1.91:

$$I = \frac{bh^3}{12} \quad (1.91)$$

Where $b = 0.04$ and $h = 0.0045$. Calculating I for each plane:

$$I_x = \frac{bh^3}{12} = \frac{0.04 \times 0.0045^3}{12} = 3.0375 \times 10^{-10} \text{ m}^4 \quad (1.92)$$

$$I_y = \frac{b^3h}{12} = \frac{0.04^3 \times 0.0045}{12} = 2.4 \times 10^{-8} \text{ m}^4 \quad (1.93)$$

Buckling occurs in the plane with the lowest I , hence buckling may occur in the x -plane. Testing:

$$S_{cr} = \frac{1 \times \pi^2 \times 71 \times 10^9 \times \frac{0.04 \times 0.0045^3}{12}}{(0.7 \times 0.96)^2} \quad (1.94)$$

$$S_{cr} = 471.342 \text{ N} = 471 \text{ N (3sf)} \quad (1.95)$$

For the value of P which we calculated in part i, R_B is pulling the strut in tension and hence is safe from buckling. We can compare the value of R_B to our S_{cr} value, when P is 0; this value can be found by adjusting the MATLAB code in part iii and generating the output for R_B .

$$R_B < S_{cr} \quad (1.96)$$

$$134 \text{ N} < 471 \text{ N} \quad (1.97)$$

Critical load is not reached, hence the strut is suitable and will not fail under these conditions.

2 Question 2

2.1 i

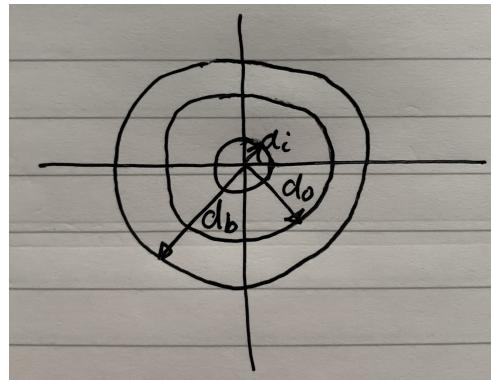


Figure 6: Sketch of cylinder arrangement.

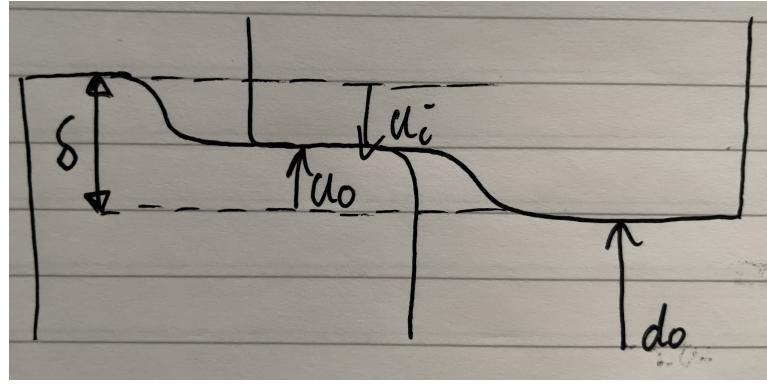


Figure 7: Diagram to show interference.

Hence:

$$\delta = u_o - u_i \quad (2.1)$$

Finding u_i . Let us start with the general equation for u :

$$u = \frac{r}{E} (\sigma_\theta - v\sigma_r) \quad (2.2)$$

$$\rightarrow u_i = \frac{r_o}{E_i} [\sigma_{\theta,i}(r_o) - v_i \sigma_{r,i}(r_o)] \quad (2.3)$$

Lame's equations:

$$\sigma_r = A - \frac{B}{r^2} \quad (2.4)$$

$$\sigma_\theta = A + \frac{B}{r^2} \quad (2.5)$$

Boundary conditions:

$$\sigma_{r,i}(r_i) = 0 = A - \frac{B}{r_i^2} \quad (2.6)$$

$$\sigma_{r,i}(r_o) - p_{int} = A - \frac{B}{r_o^2} \quad (2.7)$$

$$A = -p_{int} \frac{r_o^2}{r_o^2 - r_i^2} \quad (2.8)$$

$$B = -p_{int} \frac{r_i^2 \cdot r_o^2}{r_o^2 - r_i^2} \quad (2.9)$$

Substituting:

$$\sigma_{\theta,i} = A + \frac{B}{r^2} = -p_{int} \frac{r_o^2}{r_o^2 - r_i^2} \left(1 + \frac{r_i^2}{r^2} \right) \quad (2.10)$$

Therefore:

$$\sigma_{r,i}(r_o) = -p_{int} \quad \sigma_{\theta,i}(r_o) = -p_{int} \frac{r_o^2 + r_i^2}{r_o^2 - r_i^2} \quad (2.11)$$

$$u_i = -p_{int} \frac{r_o}{E_i} \left(\frac{r_o^2 + r_i^2}{r_o^2 - r_i^2} - v_i \right) \quad (2.12)$$

Repeating to find u_o :

$$u_o = \frac{r_o}{E_o} [\sigma_{\theta,o}(r_o) - v_o \sigma_{r,o}(r_o)] \quad (2.13)$$

Lame's equations:

$$\sigma_r = A - \frac{B}{r^2} \quad (2.14)$$

$$\sigma_\theta = A + \frac{B}{r^2} \quad (2.15)$$

Boundary conditions:

$$\sigma_{r,o}(r_o) = p_{int} = A - \frac{B}{r_o^2} \quad (2.16)$$

$$\sigma_{r,o}(r_b) = 0 = A - \frac{B}{r_b^2} \quad (2.17)$$

$$A = p_{int} \frac{r_o^2}{r_o^2 - r_b^2} \quad (2.18)$$

$$B = p_{int} \frac{r_o^2 \cdot r_b^2}{r_b^2 - r_o^2} \quad (2.19)$$

Substituting:

$$\sigma_{\theta,o} = A + \frac{B}{r^2} = p_{int} \frac{r_o^2}{r_b^2 - r_o^2} \left(1 + \frac{r_b^2}{r_o^2} \right) \quad (2.20)$$

Therefore:

$$\sigma_{r,o}(r_b) = 0 \quad \sigma_{\theta,o}(r_o) = p_{int} \frac{r_b^2 + r_o^2}{r_b^2 - r_o^2} \quad (2.21)$$

$$u_o = p_{int} \frac{r_o}{E_o} \left(\frac{r_b^2 + r_o^2}{r_b^2 - r_o^2} + v_o \right) \quad (2.22)$$

Finding δ :

$$u_i = -p_{int} \frac{r_o}{E_i} \left(\frac{r_o^2 + r_i^2}{r_o^2 - r_i^2} - v_i \right) \quad (2.23)$$

$$u_o = p_{int} \frac{r_o}{E_o} \left(\frac{r_b^2 + r_o^2}{r_b^2 - r_o^2} + v_o \right) \quad (2.24)$$

$$\delta = p_{int} r_o \left[\frac{1}{E_o} \left(\frac{r_b^2 + r_o^2}{r_b^2 - r_o^2} + v_o \right) + \frac{1}{E_i} \left(\frac{r_o^2 + r_i^2}{r_o^2 - r_i^2} - v_i \right) \right] \quad (2.25)$$

For a single material: $E_i = E_o = E$, and $v_i = v_o = v$:

$$\delta = p_{int} \frac{r_o}{E} \left[\frac{2r_o^2 (r_b^2 - r_i^2)}{(r_b^2 - r_o^2) (r_o^2 - r_i^2)} \right] \quad (2.26)$$

$$p_{int} = E \delta \left[\frac{(r_b^2 - r_o^2) (r_o^2 - r_i^2)}{2r_o^2 (r_b^2 - r_i^2)} \right] \quad (2.27)$$

Converting radius to diameters:

$$\delta = p_{int} \frac{d_o}{E} \left[\frac{d_o^2 (d_b^2 - d_i^2)}{(d_b^2 - d_o^2) (d_o^2 - d_i^2)} \right] \quad (2.28)$$

$$p_{int} = E \delta \left[\frac{(d_b^2 - d_o^2) (d_o^2 - d_i^2)}{d_o^3 (d_b^2 - d_i^2)} \right] \quad (2.29)$$

Where δ is the radial interference.

2.2 ii

We can find the internal pressure through the following equation:

$$F \leq \pi d_o L_b p_{int} \mu_s \quad (2.30)$$

$$p_{int} \geq \frac{F}{\pi d_o L_b \mu_s} \quad (2.31)$$

$$p_{int} \geq \frac{10000}{\pi(20 \times 10^{-3})(50 \times 10^{-3})(0.8)} \quad (2.32)$$

$$p_{int} \geq 3978873.577 \text{ Pa} \quad (2.33)$$

Using a value of $4 \times 10^6 \text{ Pa}$ for p_{int} , we can now substitute into 2.28:

$$\delta = \frac{(4 \times 10^6)(20 \times 10^{-3})}{210 \times 10^9} \left[\frac{(20 \times 10^{-3})^2 ((23 \times 10^{-3})^2 - (17 \times 10^{-3})^2)}{((23 \times 10^{-3})^2 - (20 \times 10^{-3})^2) ((20 \times 10^{-3})^2 - (17 \times 10^{-3})^2)} \right] \quad (2.34)$$

$$\delta = 2.554 \times 10^{-6} \text{ m} = 0.00255 \text{ mm} \quad (2.35)$$

Where δ is the radial interference. We need to make sure that this interference value does not result in the engineering failure of the internal or external cylinder. Looking at the internal cylinder, we can find the stress by utilising Lame's equations (2.10 and 2.20):

$$\sigma_{ri}(r_o) = -p_{int} = -4 \times 10^6 \text{ Pa} \quad (2.36)$$

$$\sigma_{\theta i}(r_o) = -p_{int} \frac{r_o^2 + r_i^2}{r_o^2 - r_i^2} = -4 \times 10^6 \cdot \frac{0.01^2 + 0.0085^2}{0.01^2 - 0.0085^2} = -2.483 \times 10^7 \text{ Pa} \quad (2.37)$$

$$\sigma_{ro}(r_o) = p_{int} = 4 \times 10^6 \text{ Pa} \quad (2.38)$$

$$\sigma_{\theta o}(r_o) = p_{int} \frac{r_b^2 + r_o^2}{r_b^2 - r_o^2} = 4 \times 10^6 \cdot \frac{0.0115^2 + 0.01^2}{0.0115^2 - 0.01^2} = 2.881 \times 10^7 \text{ Pa} \quad (2.39)$$

We can use the distortion-energy theory to calculate the strain energy in the tubes. This theory is justified as it is the most reliable method for calculating the stress. The strain energy associated with distortion can be found using 2.41 and this must be lower than the yield stress σ_Y :

$$\sigma_i < \sigma_Y \quad (2.40)$$

$$\sqrt{\frac{\sigma_{ri}^2 + \sigma_{\theta i}^2 + (\sigma_{ri} - \sigma_{\theta i})^2}{2}} < \sigma_Y \quad (2.41)$$

$$\sqrt{\frac{(-4 \times 10^6)^2 + (-2.483 \times 10^7)^2 + ((-4 \times 10^6) - (-2.483 \times 10^7))^2}{2}} < 4.8 \times 10^8 \quad (2.42)$$

$$2.309 \times 10^7 < 4.8 \times 10^8 \quad (2.43)$$

Hence, inner tube is not subject to yielding stress. Repeating for outer tube:

$$\sigma_o < \sigma_Y \quad (2.44)$$

$$\sqrt{\frac{\sigma_{ro}^2 + \sigma_{\theta o}^2 + (\sigma_{ro} - \sigma_{\theta o})^2}{2}} < \sigma_Y \quad (2.45)$$

$$\sqrt{\frac{(4 \times 10^6)^2 + (2.881 \times 10^7)^2 + ((4 \times 10^6) - (2.881 \times 10^7))^2}{2}} < 4.8 \times 10^8 \quad (2.46)$$

$$2.703 \times 10^7 < 4.8 \times 10^8 \quad (2.47)$$

Hence, outer tube is not subject to yielding stress.

2.3 iii

```
1 clc
2 clear
3 close all
4
5 %vars
6 pint = 4e6;
7 a = 8.5e-3;
8 b = 10e-3;
9 c = 11.5e-3;
10 ri = 0.0085;
11 ro = 0.01;
12
13 %internal
14 sigmaRI = -pint;
15 sigmaThetaI = -pint.*((b^2 + a^2)/(b^2 - a^2));
16
17 %external
18 sigmaRO = 0;
19 sigmaThetaO = pint.*((c^2 + b^2)/(c^2 - b^2));
20
21 %sigmaThetaO circle
22 p1 = nsidedpoly(1000, 'Center',[sigmaThetaO/2 0], 'Radius',abs(sigmaThetaO/2));
23 %sigmaRO circle
24 p2 = nsidedpoly(1000, 'Center',[sigmaRI/2 0], 'Radius',abs(sigmaRI/2));
25 %RI circle
26 p3 = nsidedpoly(1000, 'Center',[ (sigmaRI + sigmaThetaO)/2 0], 'Radius',abs((sigmaThetaO - sigmaRI)/2));
27
28 plot(p1)
29 hold on
30 xline(0);
31 yline(0);
32 xlabel('|\sigma')
33 ylabel('|\tau')
34 title('Graph to show Mohrs circle')
35 grid on
36 axis image
37 plot(p2)
38 plot(p3)
39 hold off
```

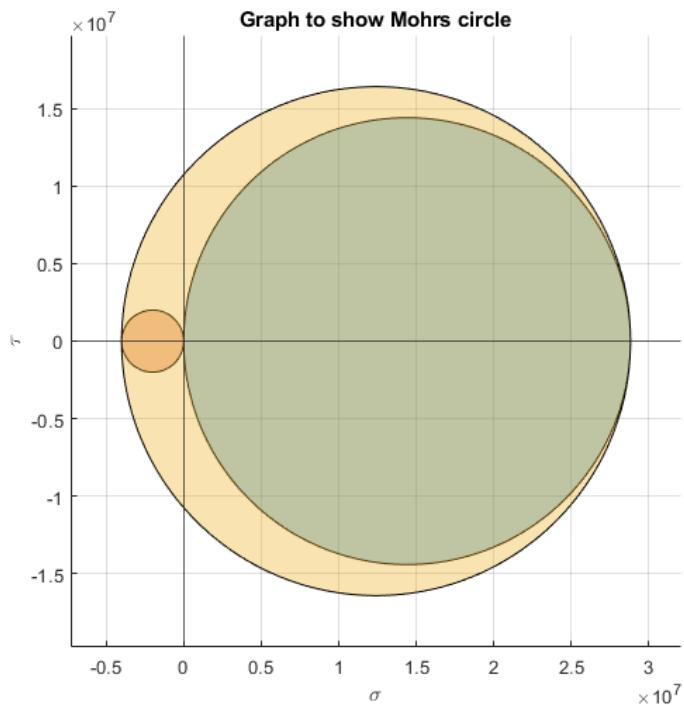


Figure 8: Graph to show Mohr's circle.

3 Question 3

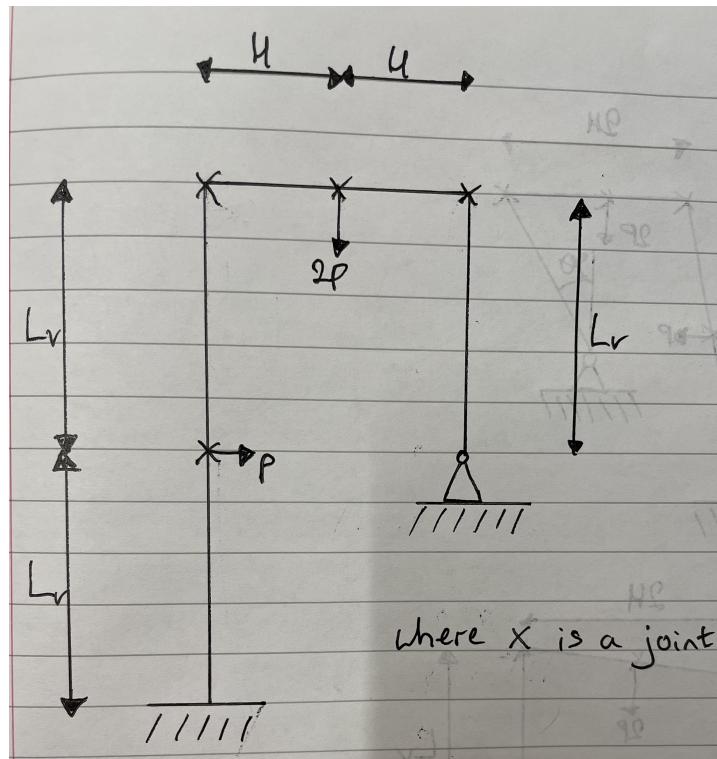


Figure 9: Sketch of portal frame.

We can now look at the potential modes of collapse of the portal frame.

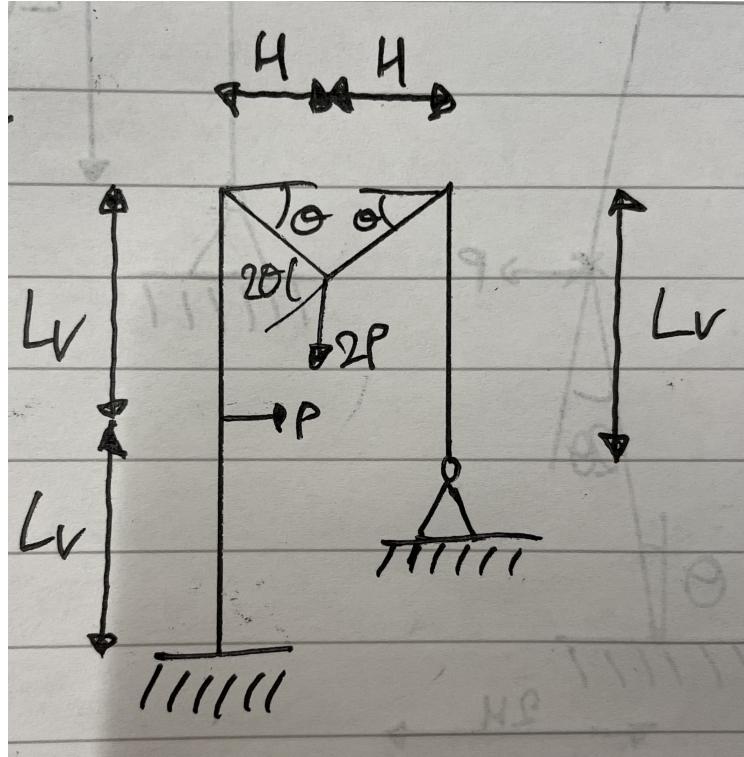


Figure 10: Sketch of portal frame undergoing beam collapse.

Here, work is done in all the joints on the top beam.

$$\sum M_P \theta = \sum Fd \quad (3.1)$$

$$M_P \theta + M_P(2\theta) + M_P \theta = 2PH\theta \quad (3.2)$$

$$4M_P \theta = 2PH\theta \quad (3.3)$$

$$P = \frac{2M_P}{H} \quad (3.4)$$

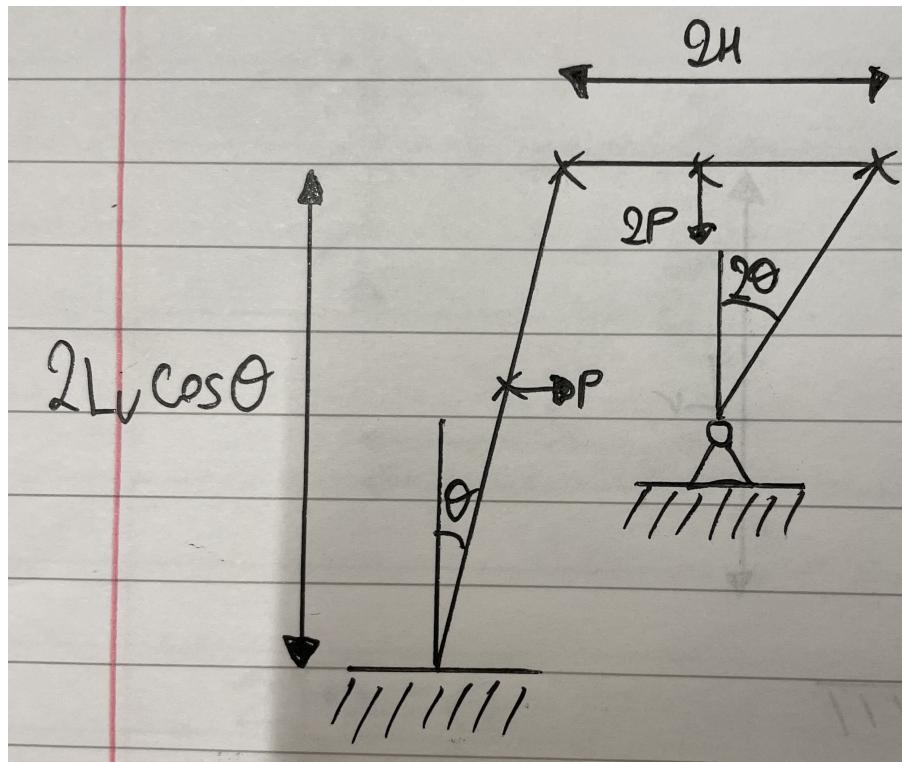


Figure 11: Sketch of portal frame undergoing sway collapse.

Here, work is done at both ends of the long column and at the top end of the short column.

$$\sum M_P \theta = \sum Fd \quad (3.5)$$

$$M_P \theta + M_P \theta + M_P(2\theta) = PL_v \theta \quad (3.6)$$

$$4M_P \theta = PL_v \theta \quad (3.7)$$

$$P = \frac{4M_P}{L_v} \quad (3.8)$$

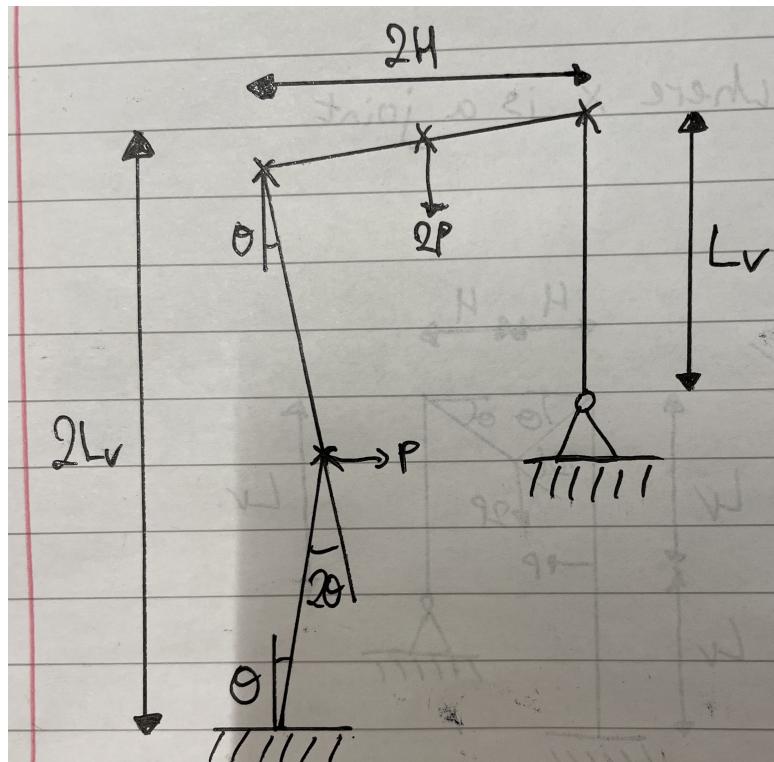


Figure 12: Sketch of portal frame undergoing column collapse.

Here, work is done at the top, bottom and middle of the long column.

$$\sum M_P \theta = \sum Fd \quad (3.9)$$

$$M_P \theta + M_P(2\theta) + M_P \theta = PL_v \theta \quad (3.10)$$

$$4M_P \theta = PL_v \theta \quad (3.11)$$

$$P = \frac{4M_P}{L_v} \quad (3.12)$$

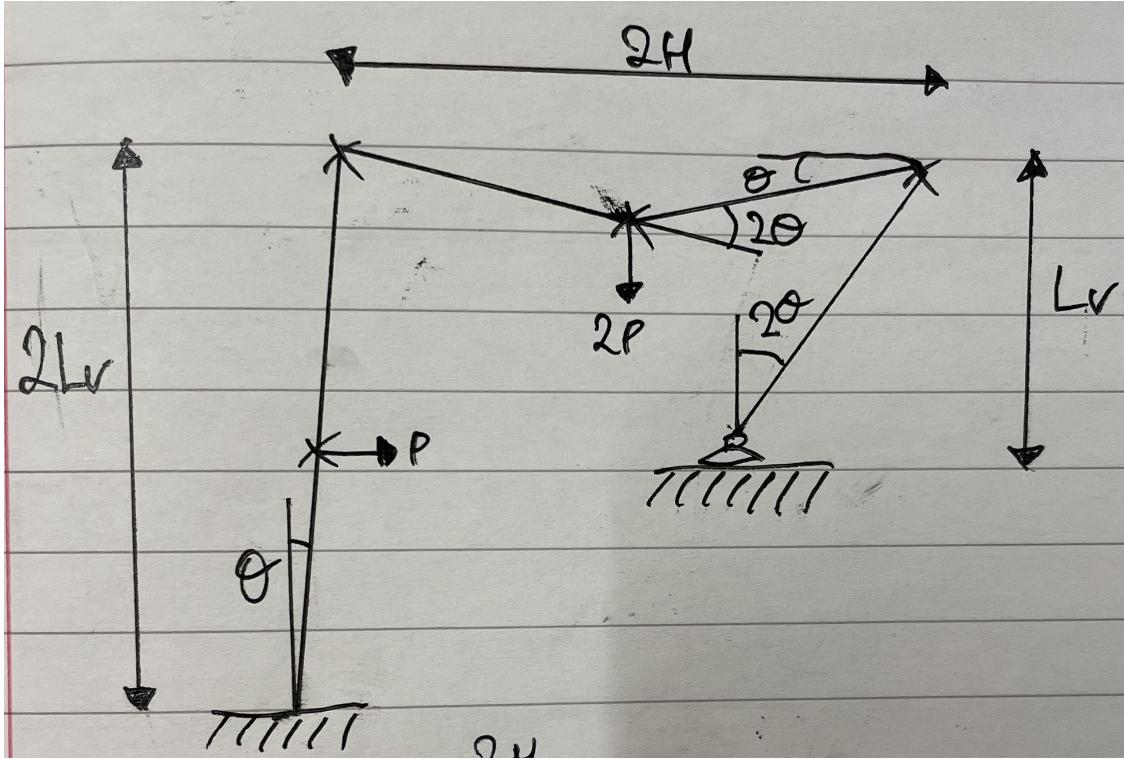


Figure 13: Sketch of portal frame undergoing beam and sway collapse.

Here, work is done at the base of the long column, the middle of the horizontal beam and the top of the short column.

$$\sum M_P \theta = \sum Fd \quad (3.13)$$

$$M_P \theta + M_P(2\theta) + M_P(3\theta) = 2PH\theta + PL_v\theta \quad (3.14)$$

$$6M_P \theta = P\theta(2H + L_v) \quad (3.15)$$

$$P = \frac{6M_P}{2H + L_v} \quad (3.16)$$

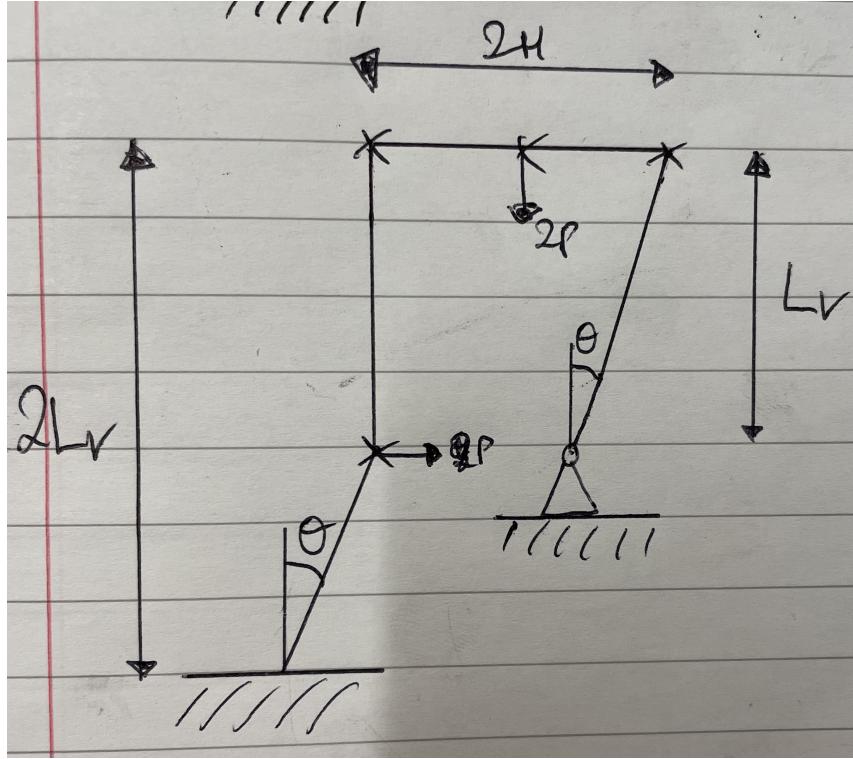


Figure 14: Sketch of portal frame undergoing column and sway collapse.

Here, work is done at the bottom and middle of the long column, as well as at the top of the short column.

$$\sum M_P \theta = \sum Fd \quad (3.17)$$

$$M_P \theta + M_P \theta + M_P \theta = PL_v \theta \quad (3.18)$$

$$3M_P \theta = PL_v \theta \quad (3.19)$$

$$P = \frac{3M_P}{L_v} \quad (3.20)$$

3.1 ii

Let us tabulate our investigations in part i:

Collapse mechanism	Collapse load
Beam collapse	$P = \frac{2M_P}{H}$
Sway collapse	$P = \frac{4M_P}{L_v}$
Column collapse	$P = \frac{4M_P}{L_v}$
Beam and sway collapse	$P = \frac{6M_P}{2H+L_v}$
Column and sway collapse	$P = \frac{3M_P}{L_v}$

Table 2: Table to show equations of P from possible collapse mechanisms of portal frame.

requested mode of collapse is column and sway. Hence, we must select our H value to ensure that column and sway collapse is the most likely to occur (when certain loading conditions are met). Using

$L_v = 0.75$:

$$P_{cr} = \frac{3M_P}{0.75} = 4M_P \quad (3.21)$$

Let us check to see if the other collapse mechanisms have a lower P_{cr} value:

$$\text{Sway collapse: } P_{cr} = \frac{4M_P}{0.75} = \frac{16M_P}{3} \quad (3.22)$$

$$\text{Column collapse: } P_{cr} = \frac{4M_P}{0.75} = \frac{16M_P}{3} \quad (3.23)$$

Sway and column collapse have higher P_{cr} values than column and sway collapse. Now we can begin to select a H value. Beam collapse:

$$4M_P < \frac{2M_P}{H} \quad (3.24)$$

$$H < 0.5 \quad (3.25)$$

Beam and sway collapse:

$$4M_P < \frac{6M_P}{2H + 0.75} \quad (3.26)$$

$$8H + 3 < 6 \quad (3.27)$$

$$H < 0.375 \quad (3.28)$$

Hence, we can see that a suitable value of H may be $H < 0.375$. Let us select a value of $H = 0.35$ m.

3.2 iii

We want our collapse to occur at $P_{cr} = \frac{2}{3}P_Y$. P_Y was calculated to be 637 N in Q1iii. Hence:

$$P_{cr} = 425 \text{ N} \quad (3.29)$$

Calculating M_P :

$$P_{cr} = \frac{3M_P}{L_v} \quad (3.30)$$

$$425 = \frac{3M_P}{0.75} \quad (3.31)$$

$$M_P = 106.25 \text{ N m} \quad (3.32)$$

Next we can check the columns of the portal frame:

$$\frac{2}{3}P_Y = 4M_P \quad (3.33)$$

$$P_Y = 6M_P = \frac{3}{2}P_{cr} \quad (3.34)$$

$$(3.35)$$

Left column analysis. We must ensure that our $P_{cr} < S_{cr}$. Since it is a fixed-fixed column, $L_e = 0.5L$:

$$P_{cr} < \frac{n^2\pi^2EI}{L_e^2} \quad (3.36)$$

$$425 < \frac{1 \times \pi^2 \times 71 \times 10^9 \times I}{0.75^2} \quad (3.37)$$

$$I > 3.41 \times 10^{-10} \text{ m}^4 \text{ (3sf)} \quad (3.38)$$

Right column analysis. We must ensure that our $P_{cr} < S_{cr}$. Since it is a pinned-fixed column, $L_e = 0.7L$:

$$P_{cr} < \frac{n^2 \pi^2 EI}{L_e^2} \quad (3.39)$$

$$425 < \frac{1 \times \pi^2 \times 71 \times 10^9 \times I}{(0.7 \times 0.75)^2} \quad (3.40)$$

$$I > 1.67 \times 10^{-10} \text{ m}^4 \quad (3\text{sf}) \quad (3.41)$$

Our second moment of area must be greater than $I \geq 3.41 \times 10^{-10} \text{ m}^4$.

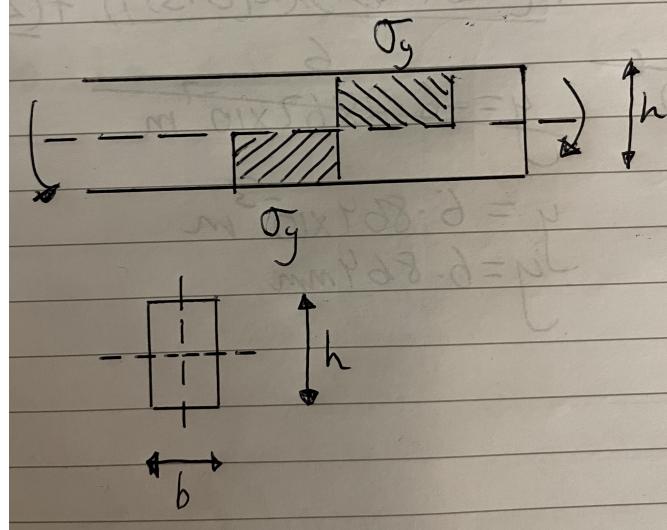


Figure 15: Sketch of portal frame undergoing column collapse.

Calculating the plastic bending moment:

$$M_P = \sigma_Y \frac{bh^2}{4} \quad (3.42)$$

$$106.25 = (186 \times 10^6) \frac{bh^2}{4} \quad (3.43)$$

$$bh^2 = 2.28 \times 10^{-6} \text{ m}^3 \quad (3.44)$$

Selecting a value of $b = 0.01 \text{ m}$:

$$h = \sqrt{\frac{2.28 \times 10^6}{0.01}} \quad (3.45)$$

$$h = 0.0151 \text{ m} \quad (3.46)$$

Now we can calculate our second moment of area and compare this to the limiting value we found earlier.

$$I = \frac{bh^3}{12} \quad (3.47)$$

$$I = \frac{0.01 \times 0.0151^3}{12} \quad (3.48)$$

$$I = 2.87 \text{ m}^4 \quad (3.49)$$

$2.87 \text{ m}^4 \geq 3.41 \times 10^{-10} \text{ m}^4$, hence $P_{cr} < S_{cr}$ is satisfied for both columns and design is suitable.

$$M_P = (186 \times 10^6) \frac{0.01 \times 0.0151^2}{4} = 106 \text{ Nm} \quad (3\text{sf}) \quad (3.50)$$

The cross-section satisfies that $P_C = \frac{2}{3} P_Y$.