

CE394M: Stress paths and invariants

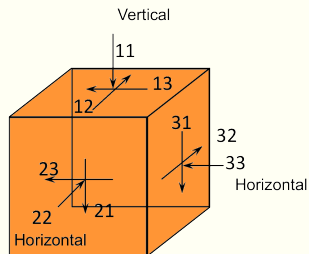
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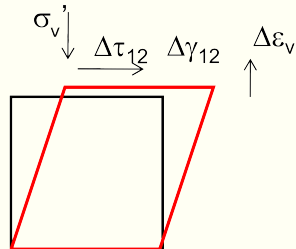
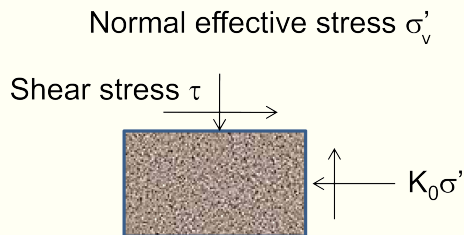
1D consolidation / simple shear

2D plane strain



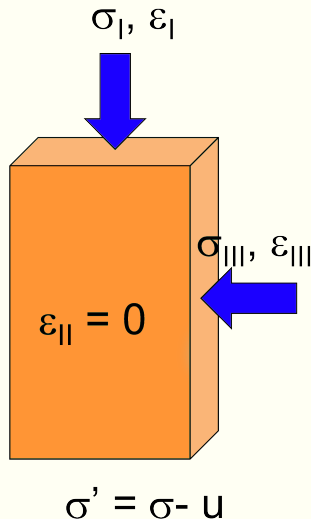
3D general (axi-symmetric as a special case)

1D simple shear



2D plane strain / Mohr-Coulomb model

Stresses and strains: independent components

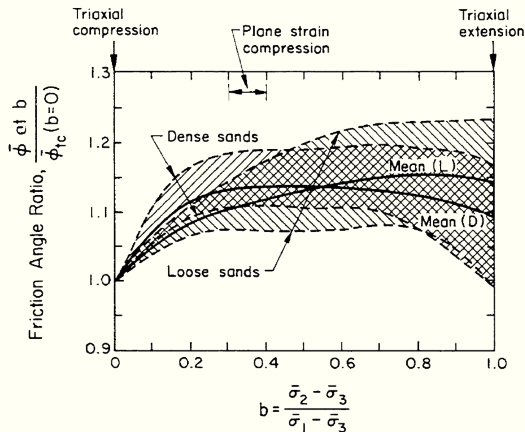


2D Mohr circle

Mohr circle to 2D stress components

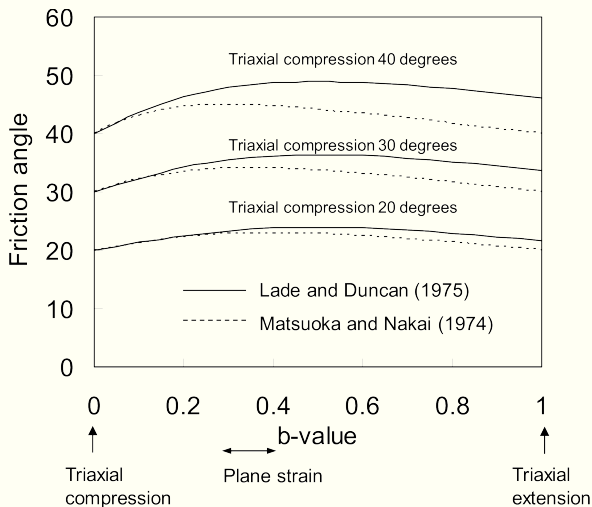
Successive Mohr circles and stress path for constant σ_3 and increasing σ_1 :

Bishop (1966) defined **b-value**:



Effect of σ_{II} on friction angle (Kulhawy and Mayne, 1990)

Effect of σ_{II} on friction



Chapter 11., Mitchell and Soga, 2005

Effect of σ_{II} on friction

Various models fit the experimental data showing the intermediate stress effect. Lade and Duncan (1975)

$$\frac{l_1^3}{l_3} = \text{const}$$

Matsuoka and Nakai (1974)

$$\frac{l_1 l_2}{l_3} = \text{const}$$

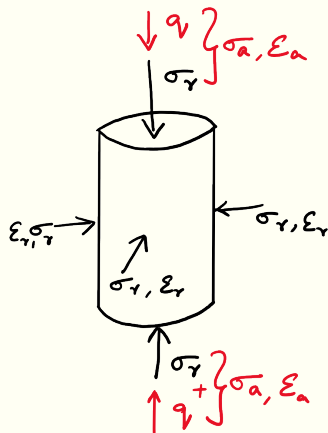
$$l_1 = \sigma_1 + \sigma_2 + \sigma_3$$

$$l_2 = \sigma_1 \sigma_2 + \sigma_2 \sigma_3 + \sigma_3 \sigma_1$$

$$l_3 = \sigma_1 \sigma_2 \sigma_3$$

Matsuoka and Nakai's model gives the same friction angle for compression and extension, whereas Lade and Duncan's model gives the ratio of the TXE friction angle (ϕ'_{TXE}) to the TXC friction angle (ϕ'_{TXC}) to be 1.08 at $\phi'_{TXC} = 20^\circ$ to 1.15 at $\phi'_{TXC} = 40^\circ$.

Triaxial stresses and strains: independent components



Triaxial deviatoric strain

- Deviatoric / shear strain: $\varepsilon_s = 2/3(\varepsilon_a - \varepsilon_r)$
- Why 2/3?:

Triaxial stress - strain relationship

Relationship in terms of axial and radial direction:

$$\begin{bmatrix} \varepsilon_a \\ \varepsilon_r \end{bmatrix} = \frac{1}{E} \begin{bmatrix} 1 & -2\nu \\ -\nu & (1 - \nu) \end{bmatrix} \begin{bmatrix} \sigma_a \\ \sigma_r \end{bmatrix} \quad (1)$$

Relationship in terms of 'p' and 'q':

$$\begin{bmatrix} p \\ q \end{bmatrix} = \begin{bmatrix} 1/3 & 2/3 \\ 1 & -1 \end{bmatrix} \begin{bmatrix} \sigma_a \\ \sigma_r \end{bmatrix} \quad \text{invert} \rightarrow \begin{bmatrix} \sigma_a \\ \sigma_r \end{bmatrix} = \begin{bmatrix} 1 & 2/3 \\ 1 & -1/3 \end{bmatrix} \begin{bmatrix} p \\ q \end{bmatrix} \quad (2)$$

Strain:

$$\begin{bmatrix} \varepsilon_p \\ \varepsilon_q \end{bmatrix} = \begin{bmatrix} 1 & 2 \\ 2/3 & -2/3 \end{bmatrix} \begin{bmatrix} \varepsilon_a \\ \varepsilon_r \end{bmatrix} \quad (3)$$

Volume - shear coupling

Equation (1) and eq. (2) in eq. (3) to relate $\varepsilon_p, \varepsilon_q$ to p, q :

$$\begin{bmatrix} \varepsilon_p \\ \varepsilon_q \end{bmatrix} = \begin{bmatrix} 3(1 - 2\nu)/E & 0 \\ 0 & 2(1 + \nu)/3E \end{bmatrix} \begin{bmatrix} p \\ q \end{bmatrix}$$

Volume shear coupling

Usually volume change and shear deformation are decoupled (i.e. bulk modulus and shear modulus), but in soils they are coupled:

Mohr circle to 3D stress components

Successive Mohr circles and stress path for constant σ_3 and increasing σ_1 :

Different stress paths for initially hydrostatic stress conditions:

6 stresses and strains

- Mean pressure:
- Deviator stress: $q = \sqrt{3/2} \sqrt{s_{11}^2 + s_{22}^2 + s_{33}^2 + 2\tau_{12}^2 + 2\tau_{23}^2 + 2\tau_{31}^2}$
- where $s_{11} = \sigma'_{11} - p'$, $s_{22} = \sigma'_{22} - p'$, $s_{33} = \sigma'_{33} - p'$
- Volumetric strain:
- Deviatoric strain:
$$\varepsilon_s = \sqrt{2/3} \sqrt{e_{11}^2 + e_{22}^2 + e_{33}^2 + \gamma_{12}^2/2 + \gamma_{23}^2/2 + \gamma_{31}^2/2}$$
- where $e_{11} = \varepsilon_{11} - \varepsilon_v/3$, $e_{22} = \varepsilon_{22} - \varepsilon_v/3$, $e_{33} = \varepsilon_{33} - \varepsilon_v/3$

3 Principal stresses and strains

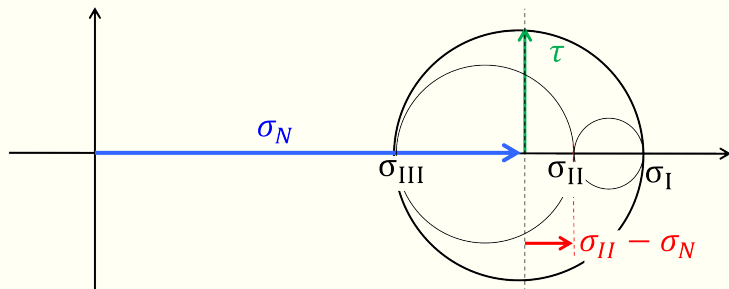
- Mean pressure:
- Deviator stress: $q = \sqrt{1/2} \sqrt{(\sigma'_I - \sigma'_{II})^2 + (\sigma'_{II} - \sigma'_{III})^2 + (\sigma'_{III} - \sigma'_I)^2}$
- Volumetric strain:
- Deviatoric strain: $\varepsilon_s = \frac{\sqrt{2}}{3} \sqrt{(\varepsilon'_I - \varepsilon'_{II})^2 + (\varepsilon'_{II} - \varepsilon'_{III})^2 + (\varepsilon'_{III} - \varepsilon'_I)^2}$

In triaxial condition (principal stresses/strains)

The missing stress invariant

- **General stresses:**
- **Principal stresses:**

The Lode parameter

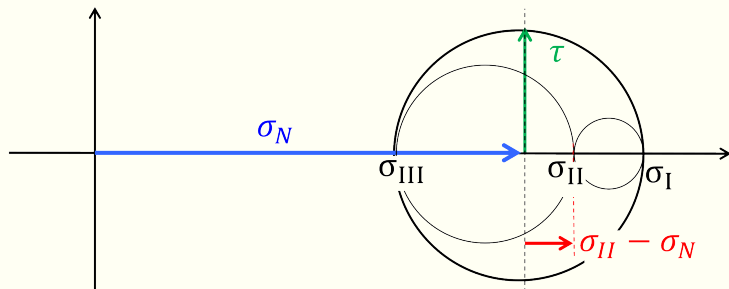


Maximum shear stress:

Normal stress:

Position of intermediate principal stress:

The Lode parameter



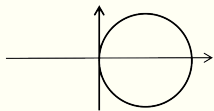
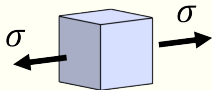
Maximum shear stress:

Normal stress:

Position of intermediate principal stress:

Which states have the same Lode parameter?

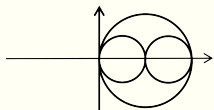
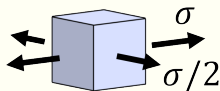
- Uniaxial tension



$$\left. \begin{aligned} \sigma_m &= \sigma/3 \\ \bar{\sigma} &= \sigma \end{aligned} \right\}$$

$$\sigma_{II} = \sigma_{III} = 0 \Rightarrow L = -1$$

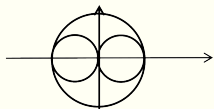
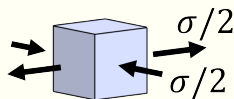
- Plane strain tension



$$\left. \begin{aligned} \sigma_m &= \sigma/2 \\ \bar{\sigma} &= \sqrt{3}/2 \sigma \end{aligned} \right\}$$

$$\sigma_{II} = (\sigma_I + \sigma_{III})/2 \Rightarrow L = 0$$

- Pure shear



$$\left. \begin{aligned} \sigma_m &= 0 \\ \bar{\sigma} &= \sqrt{3}/2 \sigma \end{aligned} \right\}$$

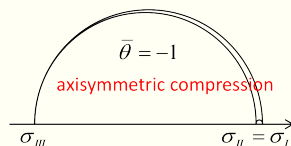
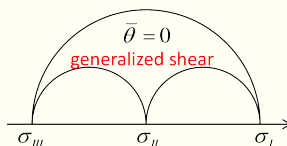
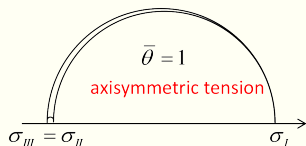
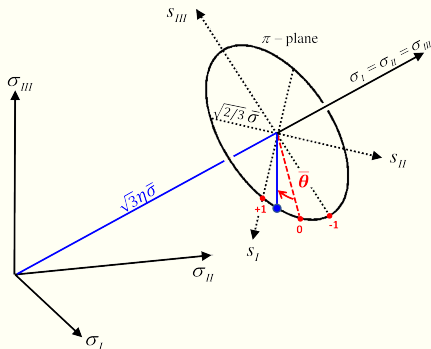
$$\sigma_{II} = (\sigma_I + \sigma_{III})/2 \Rightarrow L = 0$$

Lode angle parameter

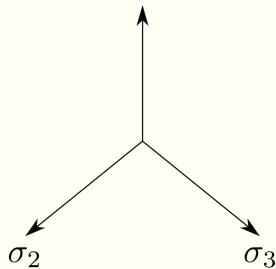
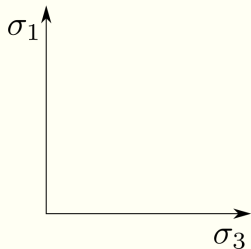
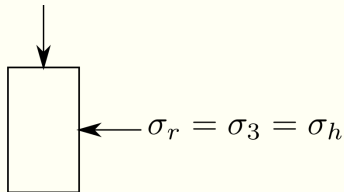
Lode angle parameter
(Lode, 1926)

$$L = \frac{2\sigma_{II} - \sigma_I - \sigma_{III}}{\sigma_I - \sigma_{III}}$$

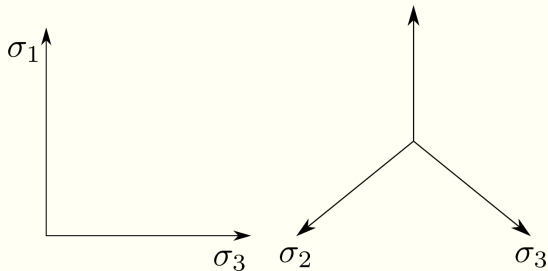
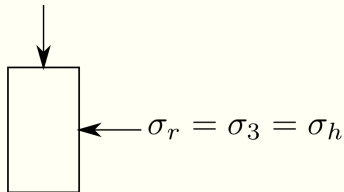
Lode angle $\bar{\theta} \approx -L$



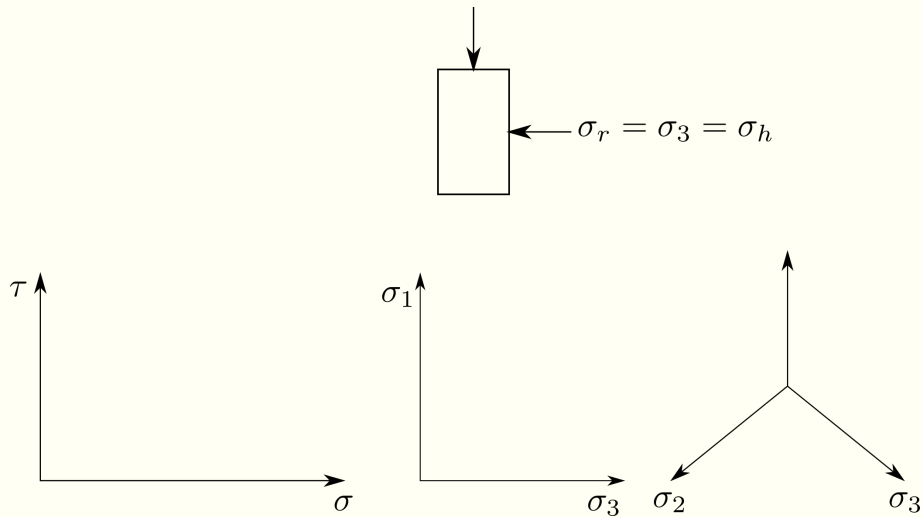
π plane: Triaxial compression



π plane: Triaxial extension



π plane: Random stress paths



Stress invariant

Principal stresses (3 components) \Leftrightarrow Invariants (3 components)

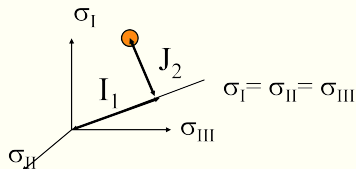
- $I_1 =$

- $s_{ij} =$

- $J_2 =$

- $J_2 =$

- $q =$



Lode angle:

- $\sin 3\theta = \frac{-3\sqrt{3}}{2} \frac{J_3}{J_2^{3/2}}.$

- $\theta = \tan^{-1} \left[\frac{1}{\sqrt{3}} \left(2 \frac{(\sigma_2 - \sigma_3)}{(\sigma_1 - \sigma_3)} - 1 \right) \right]$

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