CE394M: Stress-strain-strength relationship of clay

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Overview

- Stress-strain-strength relationship
- Simple shear

L-soil v D-soil

L-soils:

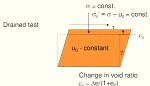
D-soils:

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Simple shear



Initial consolidation Void ratio e_n



 $\sigma = \text{const.}$

Undrained test

 $\sigma_0' = \sigma - u = changes$ $u_0 + \Delta u$ Pore pressure

changes No volume change



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NCL: L-soil (drained v undrained)

SS: LOC-soil (L-soils)

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SS: LOC-soil (L-soils) (drained v undrained)

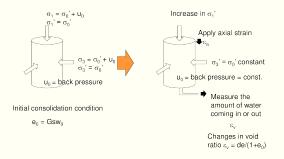


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SS: HOC-soil (D-soils) (drained v undrained)

TXC: Drained strength and volume at failure using CS



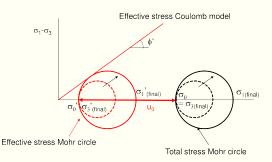
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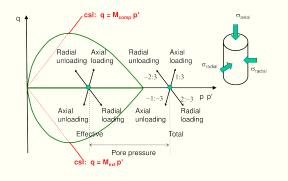
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TXC: Drained (Mohr-Coulomb ESA)



TXC: Drained Cam-Clay yield and failure



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TXC Drained (axial loading)

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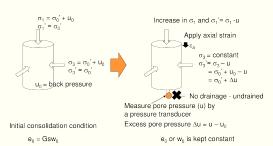
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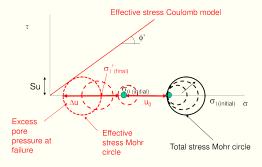
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TXC: Undrained strength and excess PWP at failure



TXC: Undrained (Mohr-Coulomb ESA)



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TXC Undrained (axial loading)

TXC Undrained (axial loading)

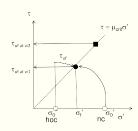
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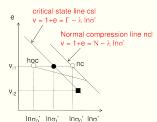
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Critical state concept





Critical state concept

Interchangeable parameters for stress at yield and $d\varepsilon^p$.

| System | Effective normal stress | Plastic normal strain | Effective shear stress | Plastic shear strain | Critical stress ratio | Plastic normal stress | Critical normal stress |
|---------|-------------------------------|-----------------------------|------------------------------|----------------------------|-----------------------------|-----------------------------|------------------------------|
| General | σ* | 8* | τ* | γ* | μ* _{crit} | σ*. | σ* _{crit} |
| SSA | σ΄ | ε | τ | γ | tan φ _{crit} | σ', | σ' _{erit} |
| BA-PS | s' | $\epsilon_{\rm v}$ | t | ϵ_{γ} | sin ф _{crit} | s′ c | s' crit |
| TA-AS | p' | $\epsilon_{\rm v}$ | q | $\epsilon_{\rm s}$ | М | p′ c | p' crit |

Plastic work and dissipation: $\sigma^*\partial \varepsilon^* + \tau^*\partial \gamma^* = \mu^*_{crit}\sigma^*\partial \gamma^*$. General yield surface: $\frac{\tau^*}{\sigma^*} = \mu^* = \mu^*_{crit} \ln \left[\frac{\sigma^*_c}{\sigma}\right]$

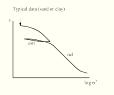
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Critical state concept: 1D compression





Plastic compression stress σ_c' is taken as the larger of the initial aggregate crushing stress and the historic maximum effective vertical stress. Clay muds are taken to begin with $\sigma_c'=1$ kPa.

Plastic compression (normal compression line): $v=v_lambda-\lambda \ln \sigma'$ for $\sigma'=\sigma'_c$.

Elastic swelling and recompression line $(\kappa$ -line): $v = v_c + \kappa (\ln \sigma'_c - \ln \sigma'_v)$.