

**Assignment 9: Cam-Clay**  
**Assigned: 29th April 2019**  
**Due: 10th May 2019**

## 1 Simple shear

1. Kaolin is reconstituted to a slurry and is then permitted to re-consolidate one-dimensionally. It eventually reaches a vertical effective stress of 100 kPa in a Simple Shear Apparatus:
  - (a) Estimate its water content
  - (b) Predict its undrained shear strength
  - (c) It is then permitted to drain while it continues shearing; predict its drained shear strength.
  - (d) What volumetric change will the sample eventually suffer during its drained shearing? How would this estimate be changed if a pre-consolidation stress of 1000 kPa had first been imposed during the initial setting-up.
2. London Clay is normally consolidated to 1000 kPa and then permitted to swell back into equilibrium (with zero pore pressure) under a normal stress of 50 kPa
  - (a) Predict both drained and undrained shear strengths using SSA Cam Clay.
  - (b) Estimate the pore water pressure consistent with your estimate of the undrained shear strength. Comment on the magnitude in relation to the probable behavior of heavily over-consolidated London Clay exposed in an excavation.

## 2 Triaxial tests

1. Establish expression for TX compression Cam Clay parameter  $M$  as a function of  $\phi_{crit}$ . Assume that the test eventually come to mobilize  $\phi_{crit}$  in the vertical plane. Hint: Use earth pressure co-efficient to relate different component of stresses.
2. A saturated clay is characterized by these Cam-Clay parameters:  $M = 0.87$ ,  $\lambda = 0.091$ ,  $\kappa = 0.035$  and  $\Gamma = 2.072$  at  $p' = 1kPa$ . Consider two different soil specimens consolidated to the same  $p'_c = 100kPa$ . Specimen A is isotropically consolidated to  $p'_0 = 100kPa$ , while Specimen B is anisotropically consolidated to  $p'_0 = 100kPa$  with  $K_c = \sigma'_{1c}/\sigma'_{3c} = 2.0$ .
  - (a) Sketch the initial states, paths and yield surfaces for each specimen in  $q - p'$  and  $v - \ln p'$  space.
  - (b) Use the OCC and MCC models to predict the undrained shear strength fro the two specimens. Compare your results.

- (c) Use the OCC and MCC models to predict the drained  $q_f$  at failure.
3. Weald clay is reconstituted as a saturated slurry and isotropically consolidated to  $p' = 100 \text{ kPa}$ , before being allowed to swell back to  $70 \text{ kPa}$ .
- (a) What will be its water content?
- (b) It is then to be subjected to undrained triaxial compression. At what deviatoric stress  $q$  might the sample yield? Estimate the axial strain at yield (assuming effective Poisson's ratio of 0.15).
- (c) If  $q$  is allowed to increase a further 10% as the undrained test progresses, search for a consistent value of the mean effective stress  $p'$  at that stage.
- (d) What ultimate undrained strength  $q_u$  should be recorded?
- (e) What volumetric strain should occur if the sample were finally allowed to drain while shearing continued, and what would be the ultimate strength?

• Parameter values which fit soil data

	London Clay	Weald Clay	Kaolin	Dog's Bay Sand	Ham River Sand
$\lambda^*$	0.161	0.093	0.26	0.334	0.163
$\kappa^*$	0.062	0.035	0.05	0.009	0.015
$\Gamma^*$ at 1 kPa	2.759	2.060	3.767	4.360	3.026
$\sigma_{c, \text{ virgin}}^*$ kPa	1	1	1	Loose 500 Dense 1500	Loose 2500 Dense 15000
$\phi_{\text{crit}}$	23°	24°	26°	39°	32°
$M_{\text{comp}}$	0.89	0.95	1.02	1.60	1.29
$M_{\text{extn}}$	0.69	0.72	0.76	1.04	0.90
$w_L$	0.78	0.43	0.74	-----	-----
$w_p$	0.26	0.18	0.42	-----	-----
$G_s$	2.75	2.75	2.61	2.75	2.65

Note: 1) parameters  $\lambda^*$ ,  $\kappa^*$ ,  $\Gamma^*$ ,  $\sigma_c^*$  should depend to a small extent on the deformation mode, e.g. SSA, BA-PS, TA-AS, etc. This may be neglected unless further information is given.  
 2) Sand which is loose, or loaded cyclically, compacts more than Cam Clay allows.