

$$\Gamma_1 = \frac{\bar{W}_1}{\sum_{i=1}^n w_i \phi_{i1}} \quad (18.5-4)$$

where

$h_i$  = the height above the base to Level  $i$

$h_r$  = the height of the structure above the base to the roof level

$w_i$  = the portion of the total effective seismic weight,  $W$ , located at or assigned to Level  $i$

The fundamental period,  $T_1$ , shall be determined either by dynamic analysis using the elastic structural properties and deformational characteristics of the resisting elements, or using Eq. 18.5-5 as follows:

$$T_1 = 2\pi \sqrt{\frac{\sum_{i=1}^n w_i \delta_i^2}{g \sum_{i=1}^n f_i \delta_i}} \quad (18.5-5)$$

where

$f_i$  = lateral force at Level  $i$  of the structure distributed in accordance with Section 12.8.3

$\delta_i$  = elastic deflection at Level  $i$  of the structure due to applied lateral forces  $f_i$

#### 18.5.2.4 Fundamental Mode Seismic Response Coefficient

The fundamental mode seismic response coefficient,  $C_{S1}$ , shall be determined using Eq. 18.5-6 or 18.5-7:

For  $T_{1D} < T_S$ ,

$$C_{S1} = \left( \frac{R}{C_d} \right) \frac{S_{D1}}{\Omega_0 B_{1D}} \quad (18.5-6)$$

For  $T_{1D} \geq T_S$ ,

$$C_{S1} = \left( \frac{R}{C_d} \right) \frac{S_{D1}}{T_{1D} (\Omega_0 B_{1D})} \quad (18.5-7)$$

where

$S_{DS}$  = the design spectral response acceleration parameter in the short period range

$S_{D1}$  = the design spectral response acceleration parameter at a period of 1 s

$B_{1D}$  = numerical coefficient as set forth in Table 18.6-1 for effective damping equal to  $\beta_{mD}$  ( $m = 1$ ) and period of the structure equal to  $T_{1D}$

#### 18.5.2.5 Effective Fundamental Mode Period Determination

The effective fundamental mode period at the design earthquake,  $T_{1D}$ , and at the maximum consid-

ered earthquake,  $T_{1M}$ , shall be based on explicit consideration of the post-yield force deflection characteristics of the structure or shall be calculated using Eqs. 18.5-8 and 18.5-9:

$$T_{1D} = T_1 \sqrt{\mu_D} \quad (18.5-8)$$

$$T_{1M} = T_1 \sqrt{\mu_M} \quad (18.5-9)$$

#### 18.5.2.6 Residual Mode Base Shear

Residual mode base shear,  $V_R$ , shall be determined in accordance with Eq. 18.5-10:

$$V_R = C_{SR} \bar{W}_R \quad (18.5-10)$$

where

$C_{SR}$  = the residual mode seismic response coefficient as determined in Section 18.5.2.8

$\bar{W}_R$  = the effective residual mode effective weight of the structure determined using Eq. 18.5-13

#### 18.5.2.7 Residual Mode Properties

Residual mode shape,  $\phi_{iR}$ , participation factor,  $\Gamma_R$ , effective residual mode seismic weight of the structure,  $\bar{W}_R$ , and effective period,  $T_R$ , shall be determined using Eqs. 18.5-11 through 18.5-14:

$$\phi_{iR} = \frac{1 - \Gamma_1 \phi_{i1}}{1 - \Gamma_1} \quad (18.5-11)$$

$$\Gamma_R = 1 - \Gamma_1 \quad (18.5-12)$$

$$\bar{W}_R = W - \bar{W}_1 \quad (18.5-13)$$

$$T_R = 0.4 T_1 \quad (18.5-14)$$

#### 18.5.2.8 Residual Mode Seismic Response Coefficient

The residual mode seismic response coefficient,  $C_{SR}$ , shall be determined in accordance with Eq. 18.5-15:

$$C_{SR} = \left( \frac{R}{C_d} \right) \frac{S_{DS}}{\Omega_0 B_R} \quad (18.5-15)$$

where

$B_R$  = numerical coefficient as set forth in Table 18.6-1 for effective damping equal to  $\beta_R$ , and period of the structure equal to  $T_R$

#### 18.5.2.9 Design Lateral Force

The design lateral force in elements of the seismic force-resisting system at Level  $i$  due to fundamental mode response,  $F_{i1}$ , and residual mode