limits given in **Table 5.3** may be increased by up to 50%. For values of $0.5 < s_1/c < 1.0$ linear interpolation may be used. The additional straight links should also conform to the rules in **5.4.5.5** and **5.4.5.6**.

5.4.5.5 – The diameter, d_{bw}, of the additional straight links referred to in **5.4.5.4** should be at least 6 mm. When transverse links are employed to delay local flange buckling as described in **5.4.5.4**, d_{bw} should be not less than the value given by **Eq.(5.8)**.

5.4.5.6 – The additional straight links referred to in **5.4.5.4** should be welded to the flanges at both ends and the capacity of the welds should be not less than the tensile yield strength of the straight links. A clear concrete cover of at least 20 mm, but not exceeding 40 mm, should be provided to these links.

5.4.5.7 – The design of partially-encased composite members may take into account the resistance of the steel section alone, or the composite resistance of the steel section and of concrete encasement.

5.4.5.8 – The design of partially-encased members in which only the steel section is assumed to contribute to member resistance may be carried out in accordance with the provisions of **Chapter 4**, but the capacity design provisions of **5.3.1.2** and **5.3.2.3** should be applied.

5.4.6. Filled composite columns

5.4.6.1 – The allowable slenderness d/t or h/t is as given in **Table 5.3**.

5.4.6.2 – The shear resistance of dissipative columns should be determined on the basis of the structural steel section or on the basis of the reinforced concrete section with the steel hollow section taken only as shear reinforcement.

5.4.6.3 – In non-dissipative members, the shear resistance of the column should be determined in accordance with EN 1994-1-1.