T_n = Natural period of n'th mode [s]
t_f = Flange thickness of a seismic link
t_w = Web thickness of a seismic link
V_b = Base shear in the earthquake direction considered
V_{bx} = Base shear in x earthquake direction
V_{bCx} = Base shear obtained by modal combination in x earthquake direction
V_{by} = Base shear in x earthquake direction
V_{bCy} = Base shear obtained by modal combination in y earthquake direction
V_{Ed} = Shear force obtained from analysis for the seismic design situation
V'_{Ed} = Design shear force determined in accordance with capacity design rule
$V_{Ed,E}$ = Shear force due to design seismic action
$V_{Ed,G}$ = Shear force due to non-seismic actions in seismic design situation
$V_{Ed,M}$ = Shear force due to application of plastic moment resistances at the two ends
V_i = i'th storey seismic shear in the earthquake direction considered
V_{ic} = Sum of seismic shear forces of all columns at the i'th storey in the earthquake direction considered
V_{is} = Sum of seismic shear forces in the earthquake direction considered at the i'th storey columns where strong column – weak beam condition is satisfied at both bottom and top joints
$V_{pl,Rd}$ = Design value of shear resistance of a member in accordance with EN 1993-1-1: 2004
$V_{wb,Rd}$ = Shear buckling resistance of the web panel
$V_{wp,Ed}$ = Design shear force in web panel due to design seismic action effects
$V_{wp,Rd}$ = Shear resistance of the web panel in accordance with EN 1993- 1-8:2004, 6.2.4.1
W_i = Seismic weight of i'th storey of building
W_t = Total seismic weight of building corresponding to total mass, M_t
α = Confinement effectiveness factor; ratio of the smaller bending moments $M_{Ed,A}$ at one end of the link in the seismic design situation, to the greater bending moments $M_{Ed,B}$ at the end where the plastic hinge develops, both moments being taken as absolute values.
α_G = Coefficient used for determining the gap size of a seismic joint
α_i = Ratio of V_{is} / V_{ic} calculated for any i'th storey
Δ_{ji} = Reduced storey drift of the j'th vertical element at i'th storey
$(\Delta_i)_{avg}$ = Average reduced storey drift of the i'th storey
δ_{ji} = Effective storey drift of the j'th vertical element at i'th storey
$(\delta_i)_{max}$ = Maximum effective storey drift of the i'th storey
ΔF_N = Additional equivalent seismic load acting on the N'th storey (top) of building
ε = Shear amplification factor of wall
ε_a = Total strain of steel at Ultimate Limit State
ε_{cg} = Upper limit (capacity) of concrete compressive strain in the extreme fiber inside the confinement reinforcement
ε_{cu2} = Ultimate compressive strain of unconfined concrete
ε_s = Upper limit (capacity) of strain in steel reinforcement
$\varepsilon_{sy,d}$ = Design value of steel strain at yield
η_{ti} = Torsional Irregularity Factor defined at i'th storey of building
η_{ci} = Strength Irregularity Factor defined at i'th storey of building
η_{ki} = Stiffness Irregularity Factor defined at i'th storey of building
Φ_{xin} = In buildings with floors modelled as rigid diaphragms, horizontal component of n'th mode shape in the x direction at i'th storey of building