

laterally braced to the building structure. Such bracing shall be independent of any ceiling lateral force bracing. Bracing shall be spaced to limit horizontal deflection at the partition head to be compatible with ceiling deflection requirements as determined in Section 13.5.6 for suspended ceilings and elsewhere in this section for other systems.

**EXCEPTION:** Partitions that meet all of the following conditions:

1. The partition height does not exceed 9 ft (2,740 mm).
2. The linear weight of the partition does not exceed the product of 10 lb (0.479 kN) times the height (ft or m) of the partition.
3. The partition horizontal seismic load does not exceed 5 psf (0.24 kN/m<sup>2</sup>).

### 13.5.8.2 Glass

Glass in glazed partitions shall be designed and installed in accordance with Section 13.5.9.

## 13.5.9 Glass in Glazed Curtain Walls, Glazed Storefronts, and Glazed Partitions

### 13.5.9.1 General

Glass in glazed curtain walls, glazed storefronts, and glazed partitions shall meet the relative displacement requirement of Eq. 13.5-1:

$$\Delta_{\text{fallout}} \geq 1.25I_e D_p \quad (13.5-1)$$

or 0.5 in. (13 mm), whichever is greater where:

$\Delta_{\text{fallout}}$  = the relative seismic displacement (drift) at which glass fallout from the curtain wall, storefront wall, or partition occurs (Section 13.5.9.2)

$D_p$  = the relative seismic displacement that the component must be designed to accommodate (Section 13.3.2.1).  $D_p$  shall be applied over the height of the glass component under consideration

$I_e$  = the importance factor determined in accordance with Section 11.5.1

**EXCEPTION:**

1. Glass with sufficient clearances from its frame such that physical contact between the glass and frame will not occur at the design drift, as demonstrated by Eq. 13.5-2, need not comply with this requirement:

$$D_{\text{clear}} \geq 1.25D_p \quad (13.5-2)$$

where

$D_{\text{clear}}$  = relative horizontal (drift) displacement, measured over the height of the glass panel under consideration, which causes initial glass-to-frame contact. For rectangular glass panels within a rectangular wall frame

$$D_{\text{clear}} = 2c_1 \left( 1 + \frac{h_p c_2}{b_p c_1} \right) \text{ where}$$

$h_p$  = the height of the rectangular glass panel

$b_p$  = the width of the rectangular glass panel

$c_1$  = the average of the clearances (gaps) on both sides between the vertical glass edges and the frame

$c_2$  = the average of the clearances (gaps) top and bottom between the horizontal glass edges and the frame

2. Fully tempered monolithic glass in Risk Categories I, II, and III located no more than 10 ft (3 m) above a walking surface need not comply with this requirement.
3. Annealed or heat-strengthened laminated glass in single thickness with interlayer no less than 0.030 in. (0.76 mm) that is captured mechanically in a wall system glazing pocket, and whose perimeter is secured to the frame by a wet glazed gunable curing elastomeric sealant perimeter bead of 0.5 in. (13 mm) minimum glass contact width, or other approved anchorage system need not comply with this requirement.

### 13.5.9.2 Seismic Drift Limits for Glass Components

$\Delta_{\text{fallout}}$ , the drift causing glass fallout from the curtain wall, storefront, or partition shall be determined in accordance with AAMA 501.6 or by engineering analysis.

## 13.6 MECHANICAL AND ELECTRICAL COMPONENTS

### 13.6.1 General

Mechanical and electrical components and their supports shall satisfy the requirements of this section. The attachment of mechanical and electrical components and their supports to the structure shall meet the requirements of Section 13.4. Appropriate coefficients shall be selected from Table 13.6-1.

**EXCEPTION:** Light fixtures, lighted signs, and ceiling fans not connected to ducts or piping, which are supported by chains or otherwise suspended from the structure, are not required to satisfy the seismic