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CODE

## **COMMENTARY**

of higher grades, these limits are replaced by the expressions  $\varepsilon_{ty} + 0.003$  and  $\varepsilon_{ty} + 0.008$ , respectively. The first expression is the same expression as used for the limit on  $\varepsilon_t$  for classification of tension-controlled members in Table 21.2.2; this expression is further described in Commentary R21.2.2. The second expression provides a limit on  $\varepsilon_t$  with Grade 420 reinforcement that is approximately the same value as the former constant of 0.010.

Table 8.4.2.2.4—Maximum modified values of  $\gamma_f$  for nonprestressed two-way slabs

| · · · · · · · · · · · · · · · · · · · |                           |                         |                                      |   |
|---------------------------------------|---------------------------|-------------------------|--------------------------------------|---|
| Column location                       | Span direction            | $v_{uv}$                | $\varepsilon_t$ (within $b_{slab}$ ) | Maximum modified $\gamma_f$   |
| Corner column                         | Either direction          | ≤0.5\psi v <sub>c</sub> | $\geq \varepsilon_{ty} + 0.003$      | 1.0   |
|                                       | Perpendicular to the edge | ≤0.75¢v <sub>c</sub>    | $\geq \varepsilon_{ty} + 0.003$      | 1.0   |
| Edge column                           | Parallel to the edge      | ≤0.4¢v <sub>c</sub>     | $\geq \varepsilon_{ty} + 0.008$      | $\frac{1.25}{1 + \left(\frac{2}{3}\right)\sqrt{\frac{b_1}{b_2}}} \le 1.0$ |
| Interior column                       | Either direction          | ≤0.4\psi\v_c            | $\geq \epsilon_{ty} + 0.008$         | $\frac{1.25}{1 + \left(\frac{2}{3}\right)\sqrt{\frac{b_1}{b_2}}} \le 1.0$ |

- **8.4.2.2.5** Concentration of reinforcement over the column by closer spacing or additional reinforcement shall be used to resist moment on the effective slab width defined in 8.4.2.2.2 and 8.4.2.2.3.
- **8.4.2.2.6** The fraction of  $M_{sc}$  not calculated to be resisted by flexure shall be assumed to be resisted by eccentricity of shear in accordance with 8.4.4.2.

## **8.4.3** *Factored one-way shear*

- **8.4.3.1** For slabs built integrally with supports,  $V_u$  at the support shall be permitted to be calculated at the face of support.
- **8.4.3.2** Sections between the face of support and a critical section located d from the face of support for nonprestressed slabs and h/2 from the face of support for prestressed slabs shall be permitted to be designed for  $V_u$  at that critical section if (a) through (c) are satisfied:
  - (a) Support reaction, in direction of applied shear, introduces compression into the end regions of the slab.
  - (b) Loads are applied at or near the top surface of the slab.
  - (c) No concentrated load occurs between the face of support and critical section.

## 8.4.4 Factored two-way shear

## **R8.4.4** Factored two-way shear

The calculated shear stresses in the slab around the column are required to conform to the requirements of 22.6.

