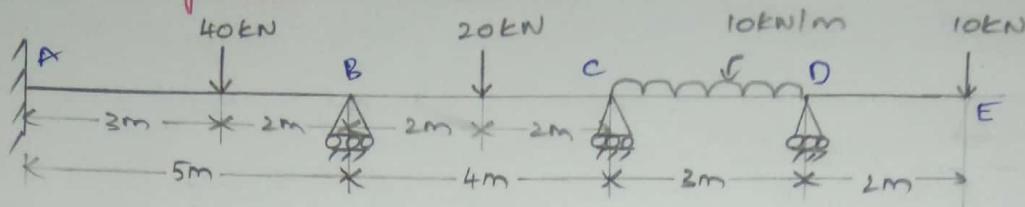


MOMENT DISTRIBUTION METHOD

Analyze the beam as shown in figure by moment distribution method and draw the B.M. Assume the EI is constant throughout the beam.



$$\bar{M}_{AB} = -\frac{w a b^2}{l^2} \Rightarrow -\frac{40 \times 3 \times 2^2}{5^2} \Rightarrow -19.2 \text{ kN.m}$$

$$\bar{M}_{BA} = \frac{w a^2 b}{l^2} \Rightarrow \frac{40 \times 3^2 \times 2}{5^2} \Rightarrow 28.8 \text{ kN.m}$$

$$\bar{M}_{BC} = -\frac{w l}{8} \Rightarrow -\frac{20 \times 4}{8} \Rightarrow -10 \text{ kN.m}$$

$$\bar{M}_{CB} = \frac{w l}{8} \Rightarrow \frac{20 \times 4}{8} \Rightarrow 10 \text{ kN.m}$$

$$\bar{M}_{CD} = -\frac{w l^2}{12} \Rightarrow -\frac{10 \times 2^2}{12} \Rightarrow -7.5 \text{ kN.m}$$

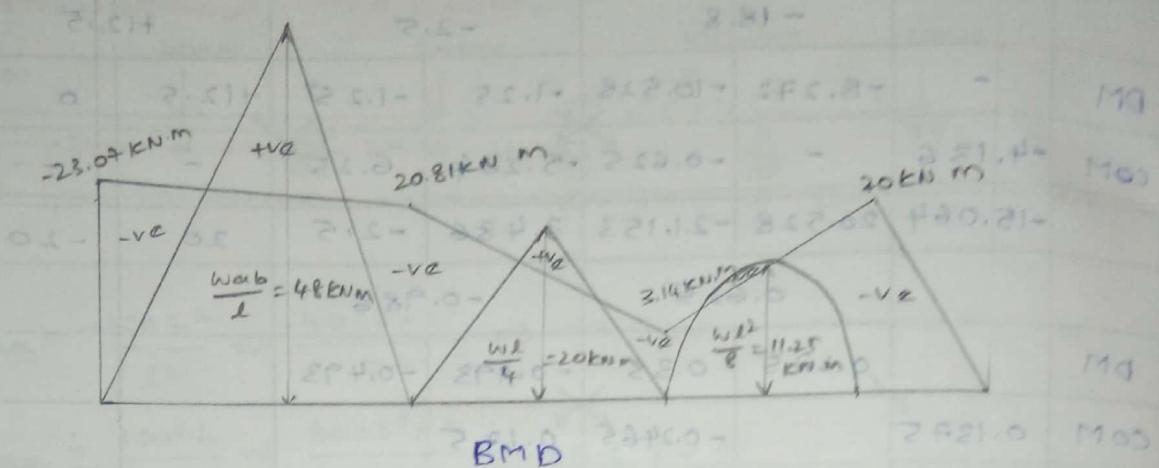
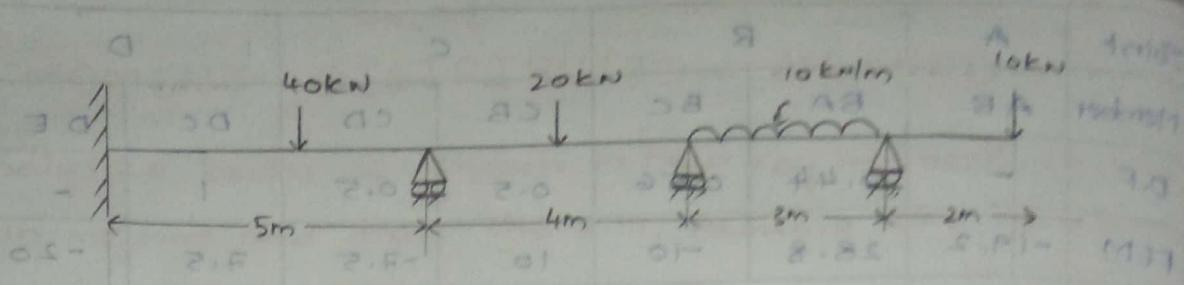
$$\bar{M}_{DC} = \frac{w l^2}{12} \Rightarrow \frac{10 \times 2^2}{12} \Rightarrow 7.5 \text{ kN.m}$$

$$\bar{M}_{DE} = -10 \times 2 \Rightarrow -20 \text{ kN.m}$$

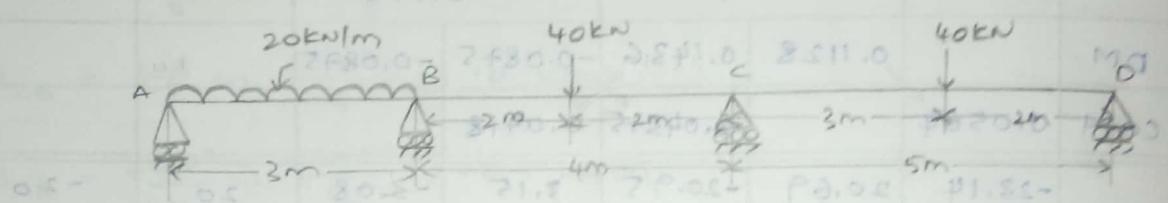
$$\bar{M}_{ED} = 10 \times 2 \Rightarrow 20 \text{ kN.m}$$

Joint	Member	K	$\sum K$	Distribution factor $= \frac{K}{\sum K}$
B	BA	$\frac{4EI}{L} \Rightarrow \frac{4}{5}$	$\frac{9}{5}$	0.44
	BC	$\frac{4EI}{L} \Rightarrow 1$		0.56
C	CB	$\frac{4EI}{L} \Rightarrow 1$	2	0.5
	CD	$\frac{2EI}{L} = 1$		0.5
D	DC	$\frac{4EI}{L} \Rightarrow \frac{4}{3}$	$\frac{4}{3}$	1

Joint	A	B	C	D	
FEM	-19.2	28.8	-10	-7.5	-20
				6.25	+12.5
	-19.2	28.8	-10	-12.5	-20
			-18.8	-2.5	
		-8.272	-10.528	-1.25	-1.25
COM	-4.136		-0.625	-5.264	
	-23.336	20.528	-21.153	-3.486	-2.5
			0.625	-0.986	
		0.275	0.35	-0.493	-0.493
COM	0.1375		-0.246	0.175	
	-23.19	20.803	-21.049	-3.168	-2.993
			0.246	-0.175	
		0.108	0.137	-0.087	-0.087
COM	0.054		-0.043	0.0685	
	-23.13	20.91	-20.95	-3.149	-3.08
			0.04	-0.069	
	0.017	0.022	-0.0345	-0.0345	
COM	0.0085		-0.017	0.011	
	-23.12	20.92	-20.94	-3.12	-3.11
			0.02	-0.01	
		0.088	0.011	-0.005	-0.005
COM	0.0044		-0.0025	0.0056	
Final moments	-23.11	20.92	-20.93	-3.12	-3.12
			200.0	200.0	
		2100.0	200.0	2100.0	
			2100.0	2100.0	
		0.848	0.848	28.81	28.81



2.



Analyse the continuous beam by Moment Distribution Method. Draw SFD & BMD. Assume EI is constant throughout the section.

$$\text{So } M_{AB} = \frac{-wl^2}{12} \Rightarrow \frac{-20 \times 3^2}{12} \Rightarrow -15 \text{ kNm}$$

$$M_{BA} = \frac{wl^2}{12} \Rightarrow \frac{20 \times 3^2}{12} \Rightarrow 15 \text{ kNm}$$

$$M_{BC} = \frac{-wl}{8} \Rightarrow \frac{-40 \times 4}{8} \Rightarrow -20 \text{ kNm}$$

$$M_{CB} = \frac{wl}{8} \Rightarrow \frac{40 \times 4}{8} \Rightarrow 20 \text{ kNm}$$

$$M_{CD} = \frac{-wab^2}{l^2} \Rightarrow \frac{-40 \times 3 \times 2^2}{5^2} \Rightarrow -19.2 \text{ kNm}$$

$$M_{DC} = \frac{wab^2}{l^2} \Rightarrow \frac{40 \times 3^2 \times 2}{5^2} \Rightarrow 28.8 \text{ kNm}$$

Joint	Member	$K = \frac{EI}{L}$	$\sum K$	$D.F = \frac{k}{\sum K}$
B	BA	$\frac{3EI}{3} = 1$	2	$\frac{1}{2}$
	BC	$\frac{4EI}{4} = 1$		$\frac{1}{2}$
C	CB	$\frac{4EI}{4} = 1$	$\frac{8}{5}$	$\frac{5}{8}$
	CD	$\frac{3EI}{5} = \frac{3}{5}$		$\frac{3}{8}$

Joint	A		B		C	
Member	AB	BA	BC	CB	CD	DC
D.F	-	$\frac{1}{2}$	$\frac{1}{2}$	$\frac{5}{8}$	$\frac{3}{8}$	-
F.E.M	-15	15	-20	20	-19.2	28.8
	+15	7.5			-14.4	-28.8
Sum	0	22.5	-20	20	-33.6	0
com		-2.5			13.6	
DM		-1.25	-1.25	8.5	5.1	
COM			4.25	-0.625		
Sum	0	21.25	-17	27.875	-28.5	0
com		-4.25			0.625	
DM		-2.125	-2.125	0.390	0.234	
COM			0.195	-1.062		
Sum	0	19.125	-18.93	27.203	-28.266	0
com		-0.195			1.063	
DM		-0.0975	-0.0975	0.664	0.398	
COM			0.332	-0.048		
Sum	0	19.027	-18.995	27.819	-27.868	0
		-0.032			0.049	
DM		-0.016	-0.016	0.0306	0.0183	
COM			0.0153	-0.008		
Sum	0	19.011	-18.995	27.841	-27.849	0
		-0.016			0.008	
DM		-0.008	-0.008	0.005	0.003	
COM			0.0025	-0.004		
Final moments	0	19.003	-19.000	27.842	-27.846	0

Reactions:-

$$R_A + R_{B1} = 60 \text{ kN}$$

$$\Sigma M_A = 0$$

$$R_B \times 3 - 20 \times 3 \times \frac{3}{2} - 19 = 0$$

$$R_{B1} = 36.33 \text{ kN}$$

$$R_A = 23.67 \text{ kN}$$

$$R_{B2} + R_{C1} = 40 \text{ kN}$$

$$\Sigma M_B = 0$$

$$R_C \times 4 - 40 \times 2 - 27.84 \times 19 = 0$$

$$R_{C1} = 22.21 \text{ kN}$$

$$R_{C2} = 17.79 \text{ kN}$$

$$R_B = R_{B1} + R_{B2} = 36.33 + 17.79$$

$$R_B = 54.12 \text{ kN}$$

$$R_{C2} + R_D = 40 \text{ kN}$$

$$\Sigma M_C = 0$$

$$R_D \times 5 - 40 \times 3 + 27.84 = 0$$

$$R_D = 18.43 \text{ kN}$$

$$R_{C2} = 21.57 \text{ kN}$$

$$R_c = R_{C1} + R_{C2} = 22.21 + 21.57$$

$$R_c = 43.78$$

$$R_A + R_B + R_C + R_D = 140 \text{ kN}$$

$$23.67 + 54.12 + 43.78 + 18.43 = 140$$

$$140 = 140$$

$$P_{A1} = 0 \quad P_{B1} = 0 \quad P_{C1} = 0 \quad P_{D1} = 0$$

$$P_{A2} = 200.0 \quad P_{B2} = 200.0 \quad P_{C2} = 200.0 \quad P_{D2} = 200.0$$

$$P_{A3} = 200.0 \quad P_{B3} = 200.0 \quad P_{C3} = 200.0 \quad P_{D3} = 200.0$$

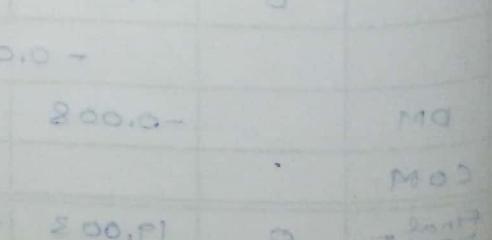
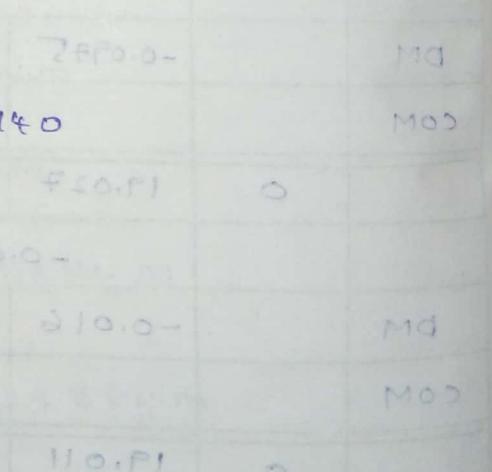
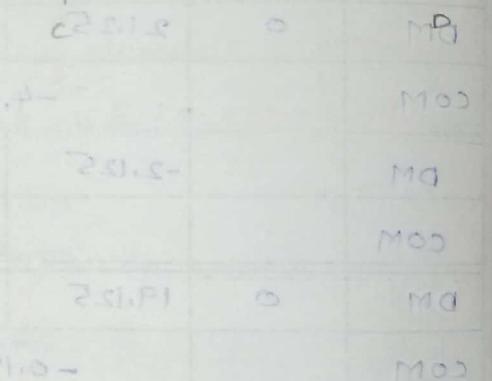
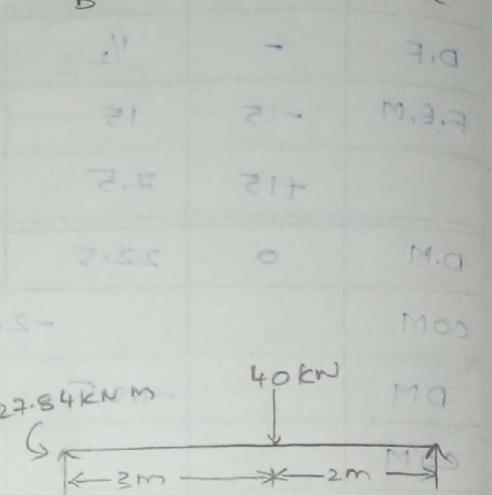
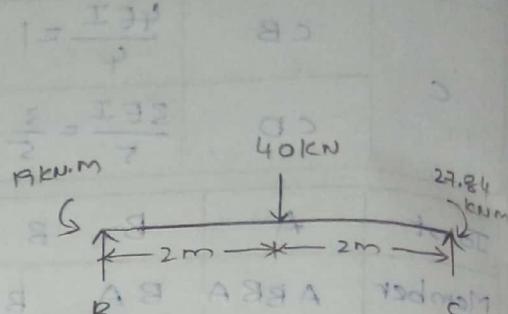
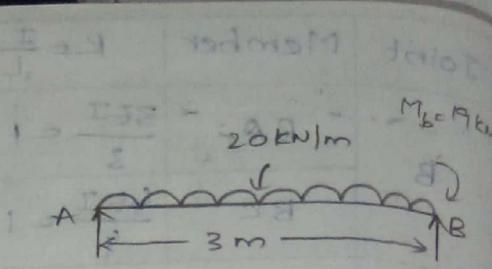
$$P_{A4} = 200.0 \quad P_{B4} = 200.0 \quad P_{C4} = 200.0 \quad P_{D4} = 200.0$$

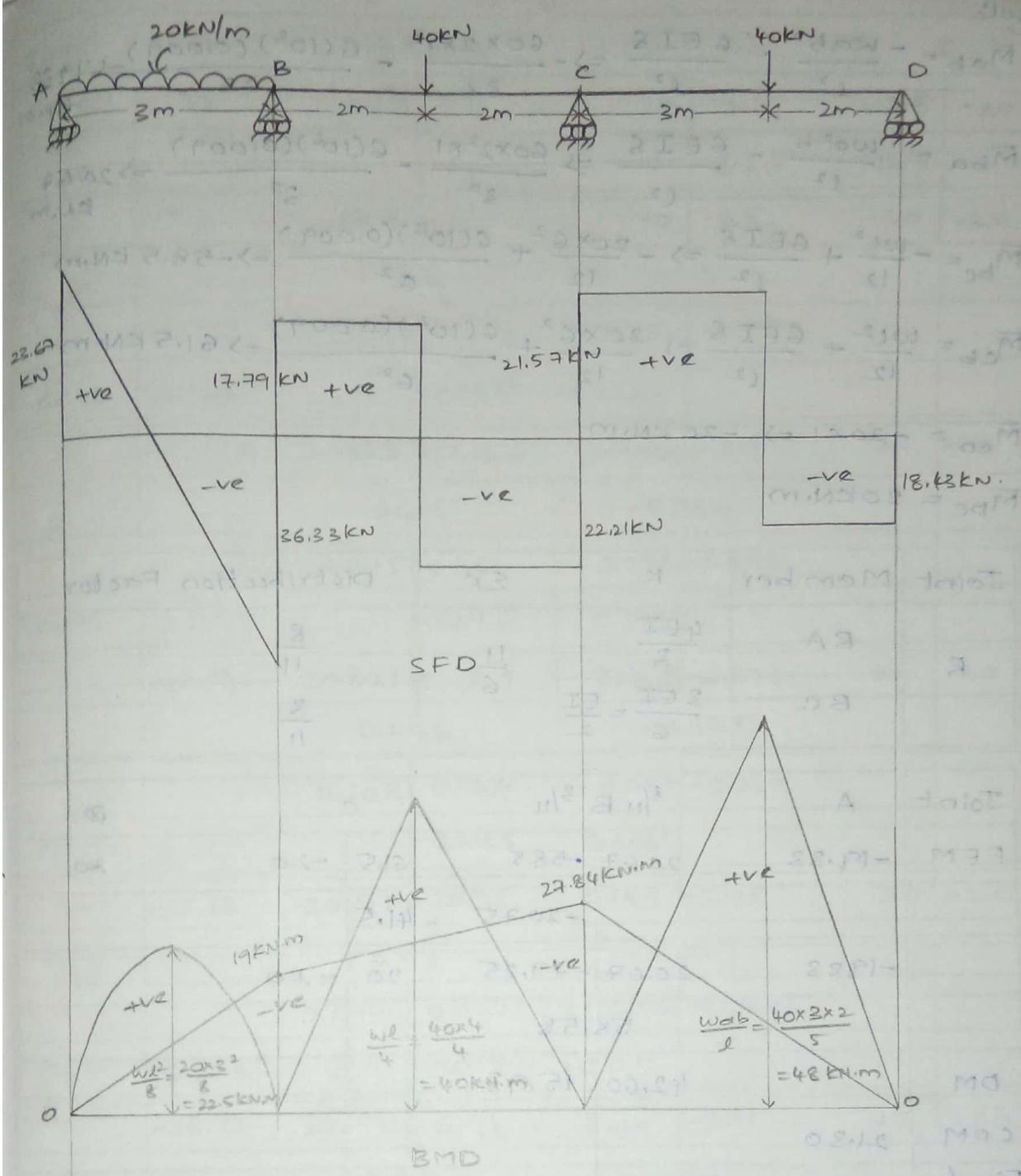
$$P_{A5} = 200.0 \quad P_{B5} = 200.0 \quad P_{C5} = 200.0 \quad P_{D5} = 200.0$$

$$P_{A6} = 200.0 \quad P_{B6} = 200.0 \quad P_{C6} = 200.0 \quad P_{D6} = 200.0$$

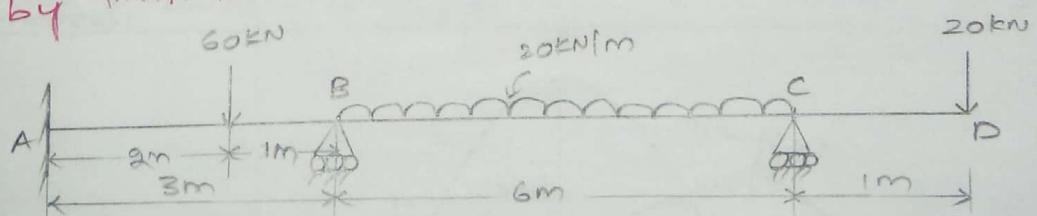
$$P_{A7} = 200.0 \quad P_{B7} = 200.0 \quad P_{C7} = 200.0 \quad P_{D7} = 200.0$$

$$P_{A8} = 200.0 \quad P_{B8} = 200.0 \quad P_{C8} = 200.0 \quad P_{D8} = 200.0$$





3. Analyse the continuous beam as shown in fig by moment distribution method. Support B sinks by 9mm. Take $EI = 1 \times 10^{12} \text{ N.mm}^2$. Draw BMD



$$EI = 1 \times 10^6 \text{ N.mm}^2$$

$$EI = 1 \times 10^{12} \times (10^{-3})^2$$

$$EI = 1 \times 10^6 \text{ N.m}^2$$

$$EI = 1 \times 10^6 \times 10^{-3} \text{ KN.m}^2$$

$$EI = 1 \times 10^3 \text{ KN.m}^2$$

SOL:

$$\bar{M}_{ab} = -\frac{w_{ab} b^2}{l^2} - \frac{6EI\delta}{l^2} \Rightarrow -\frac{60 \times 2 \times 1^2}{3^2} - \frac{6(10^3)(0.009)}{3^2} = -19.33 \text{ kN.m}$$

$$\bar{M}_{ba} = \frac{w_{ab} b}{l^2} - \frac{6EI\delta}{l^2} \Rightarrow \frac{60 \times 2^2 \times 1}{3^2} - \frac{6(10^3)(0.009)}{3^2} = 20.67 \text{ kN.m}$$

$$\bar{M}_{bc} = -\frac{w l^2}{l^2} + \frac{6EI\delta}{l^2} \Rightarrow -\frac{20 \times 6^2}{12} + \frac{6(10^3)(0.009)}{6^2} = -58.5 \text{ kN.m}$$

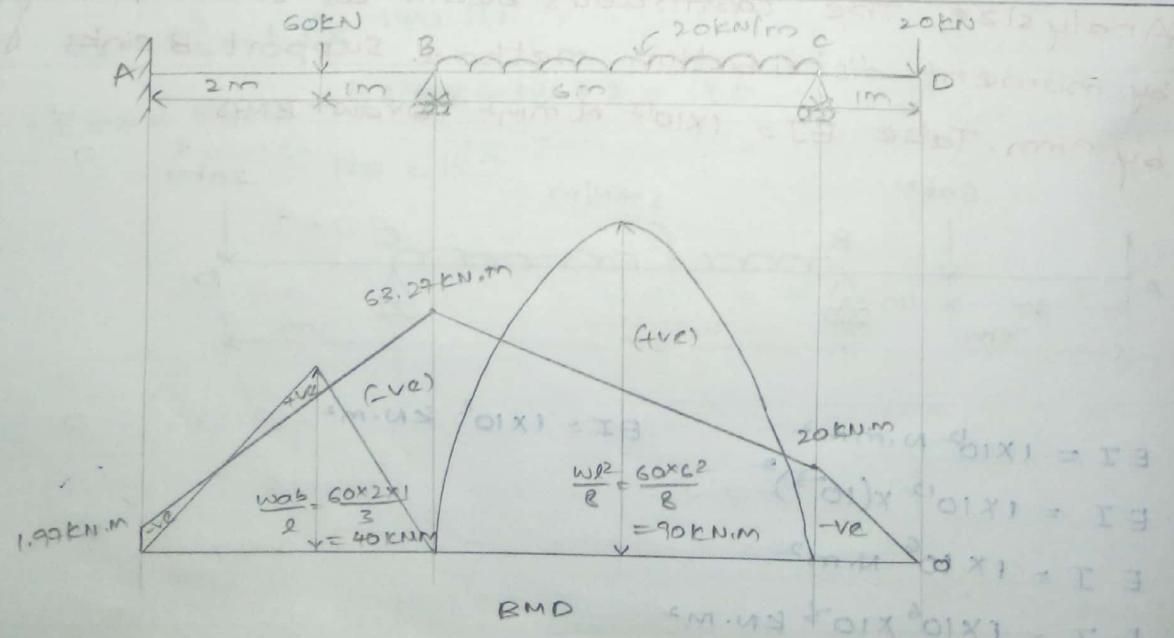
$$\bar{M}_{cb} = \frac{w l^2}{l^2} + \frac{6EI\delta}{l^2} \Rightarrow \frac{20 \times 6^2}{12} + \frac{6(10^3)(0.009)}{6^2} = 61.5 \text{ kN.m}$$

$$\bar{M}_{cd} = -20 \times 1 \Rightarrow -20 \text{ kN.m}$$

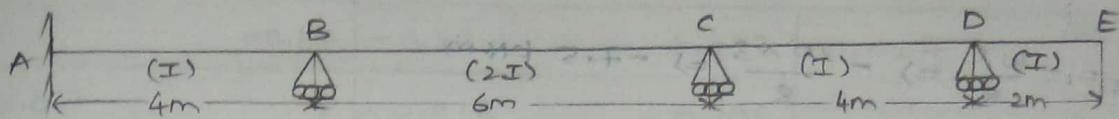
$$\bar{M}_{dc} = 20 \text{ kN.m}$$

Joint	Member	K	ΣK	Distribution Factor
B	BA	$\frac{4EI}{3}$	$\frac{11}{6}$	$\frac{8}{11}$
	BC	$\frac{3EI}{6} = \frac{EI}{2}$		$\frac{3}{11}$

Joint	A	$\frac{8}{11} B \frac{3}{11}$	C	D	
FEM	-19.33	20.67	-58.5	61.5	-20
			-20.75	-41.5	
	-19.33	20.67	-79.25	20	-20
			58.58		
DM		42.60	15.97		
COM	21.30				
Final moments	1.97	63.27	-63.28	20	-20



4. Analyse the continuous beam by MDM. If support B sinks by 12mm. Given $E = 200 \text{ kN/mm}^2$ $I = 20 \times 10^6 \text{ mm}^4$. Draw the BMD.



$$\text{Sol: } M_{AB} = -\frac{GEI\delta}{l^2} \Rightarrow -\frac{6 \times (200 \times 10^6) \times (2 \times 10^{-5}) \times 0.012}{4^2}$$

$$M_{AB} = -18 \text{ KN.m}$$

$$\bar{M}_{BA} = \frac{-GEI\delta}{l^2} \Rightarrow +\frac{6 \times (200 \times 10^6) \times (2 \times 10^{-5}) \times 0.012}{4^2}$$

$$\bar{M}_{BA} = +18 \text{ KN.m}$$

$$\bar{M}_{BC} = \frac{GEI\delta}{l^2} \Rightarrow \frac{6 \times (200 \times 10^6) \times (2 \times 10^{-5}) \times 0.012}{6^2}$$

$$\bar{M}_{BC} = 16 \text{ KN.m}$$

$$\bar{M}_{CB} = \frac{GEI\delta}{l^2} \Rightarrow \frac{6 \times (200 \times 10^6) \times (2 \times 10^{-5}) \times 0.012}{6^2}$$

$$\bar{M}_{CB} = 16 \text{ KN.m}$$

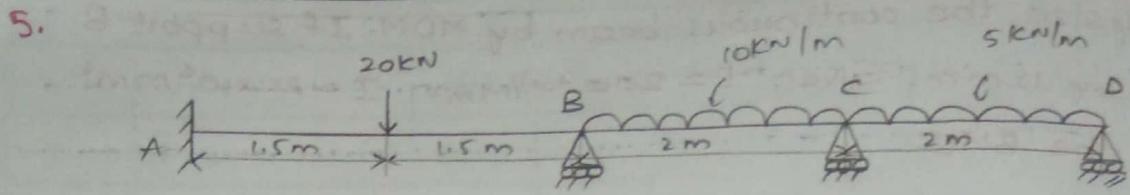
$$\bar{M}_{CD} = \bar{M}_{DC} = 0$$

Joint	Member	K	ΣK	$D.F = \frac{K}{\Sigma K}$
B	BA	$\frac{4EI}{4} = 1$	$\frac{7}{3}$	$\frac{3}{7}$
	BC	$\frac{8EI}{6} = \frac{4}{3}$		$\frac{4}{7}$
	CB	$\frac{8EI}{6} = \frac{4}{3}$		$\frac{16}{25}$
	CD	$\frac{3EI}{4} = \frac{3}{4}$		$\frac{9}{25}$

A	B	C	D	
-18	-18	16	16	0
			-16	
0.857	1.142	-10.24	-5.76	
0.428	5.12	0.571		
-17.572	-17.143	22.262	6.331	-5.76
			0	
	-5.12		-0.571	
-2.19	-2.92	-0.365	-0.205	82.8
-1.09	-0.182	1.46		

To be continued... ☺

A	$3/4$	B	$4/4$	$16/25$	C	$9/25$	D
-18.662	-19.33	19.16		7.426	-5.965		0
	0.17				-1.461		
	0.072	0.097		-0.935	-0.526		
0.036		-0.467	0.048				
-18.62	-19.25	18.79	6.539	-6.491		0	
	0.47			-0.048			
	0.201	0.268	-0.030	-0.017			
0.1		-0.015	0.134				
-18.52	-19.06	19.043	6.643	-6.508		0	
	0.017			-0.135			
	0.007	0.0097	-0.0864	-0.0486			
0.0036		-0.0432	0.0048				
-18.51	-19.05	19.00	6.56	-6.45		0	
	0.05			-0.11			
	0.021	0.028	-0.07	-0.039			
0.010		-0.035	0.014				
-18.5	-19.03	19.01	6.50	6.49		0	



$$\bar{M}_{AB} = -\frac{w l}{8} \Rightarrow -\frac{20 \times 3}{8} \Rightarrow -7.5 \text{ KN.m}$$

$$\bar{M}_{BA} = \frac{w l}{8} \Rightarrow \frac{20 \times 3}{8} \Rightarrow 7.5 \text{ KN.m}$$

$$\bar{M}_{BC} = -\frac{w l^2}{12} \Rightarrow -\frac{10 \times 2^2}{12} \Rightarrow -3.33 \text{ KN.m}$$

$$\bar{M}_{CB} = \frac{w l^2}{12} \Rightarrow \frac{10 \times 2^2}{12} \Rightarrow 3.33 \text{ KN.m}$$

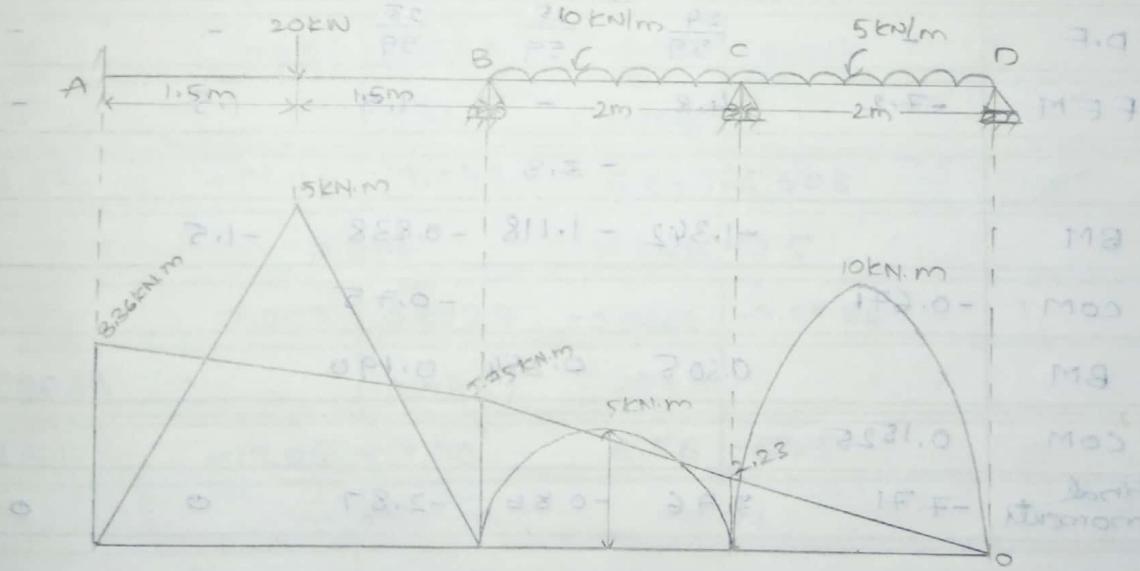
$$\bar{M}_{CD} = -\frac{w l^2}{12} \Rightarrow -\frac{5 \times 2^2}{12} \Rightarrow -1.67 \text{ KN.m}$$

$$\bar{M}_{DC} = \frac{w l^2}{12} \Rightarrow \frac{5 \times 2^2}{12} \Rightarrow 1.67 \text{ KN.m}$$

Joint	Member	K	ΣK	$D.F = \frac{K}{\Sigma K} M$
B	BA	$\frac{4EI}{L} \Rightarrow \frac{4}{3}$	$\frac{10}{3}$	$\frac{2}{5}$
	BC	$\frac{4EI}{L} = \frac{4}{2} = 2$		$\frac{3}{5}$
C	CB	$\frac{4EI}{L} = \frac{4}{2} = 2$	$\frac{7}{2}$	$\frac{4}{7}$
	CD	$\frac{3EI}{2} = \frac{3}{2}$		$\frac{8}{7}$

A	B	C	D
-7.5	7.5	-3.33	3.33
-7.5	7.5	-3.33	3.33
-0.83	-4.17	-0.825	-0.825
-8.33	5.84	-6.06	1.609
0.044	0.088	0.132	0.713
-8.28	5.92	-5.57	2.388

A	2/5 B 3/5	A to D	4/7 C 3/7	D
-8.28	5.92	-5.57	2.388	-2.323
		-0.35		-0.065
	-0.14	-0.21	-0.037	-0.027
-0.07		-0.018	-0.105	
-8.35	5.78	-5.79	2.246	-2.35
		0.01	0.104	
	0.004	0.006	0.059	0.044
0.002		0.029	0.003	
-8.348	5.784	-5.755	+2.184	-2.306
		-0.029	0.122	
	-0.011	-0.017	0.069	0.052
-0.0055		0.034	-0.0085	
-8.3535	5.773	-5.732	2.244	-2.254
		-0.041	0.01	
	-0.016	-0.024	0.005	0.00428
-0.0082		0.002	-0.012	
-8.36	5.75	-5.75	2.237	-2.249



6 A continuous beam ^{ABC} fixed at A; is supported on an elastic column BD loaded as shown in figure. Analysis frame and draw BMD. $AB = 2I$, $BC = BD = I$

$$\text{Sol: } \bar{M}_{AB} = -\frac{wab^2}{l^2} \Rightarrow -\frac{10 \times 2 \times 3^2}{5^2}$$

$$\bar{M}_{AB} = -7.2 \text{ kN.m}$$

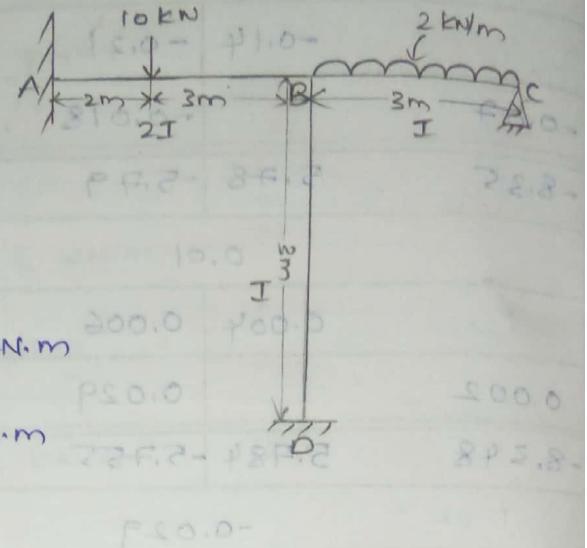
$$\bar{M}_{BA} = \frac{w a^2 b}{l^2} \Rightarrow \frac{10 \times 2^2 \times 3}{5^2} \Rightarrow$$

$$\bar{M}_{BA} = 4.8 \text{ kN.m}$$

$$\bar{M}_{BC} = -\frac{wl^2}{l^2} \Rightarrow -\frac{2 \times 3^2}{12} \Rightarrow -1.5 \text{ kN.m}$$

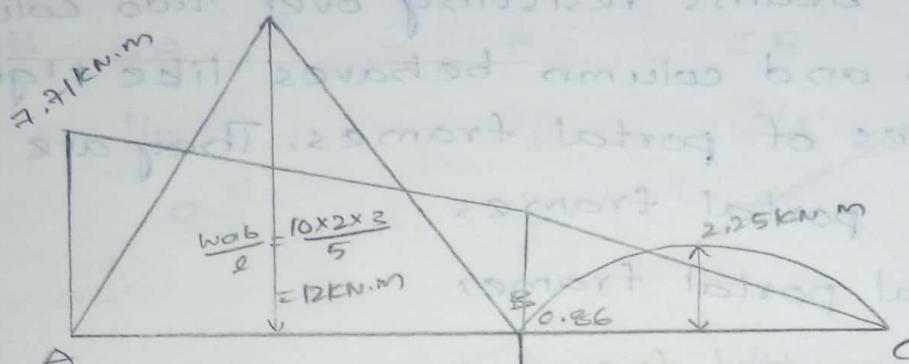
$$\bar{M}_{CB} = \frac{wl^2}{l^2} \Rightarrow \frac{2 \times 3^2}{12} \Rightarrow 1.5 \text{ kN.m}$$

$$\bar{M}_{BD} = \bar{M}_{DB} = 0$$



Joint	Member	$\frac{K}{EI}$	$\sum K$	$D.F. = \frac{K}{\sum K}$
B	BA	$\frac{4EI}{L} \Rightarrow \frac{4(2EI)}{5}$	+20.0	$\frac{24}{59}$
	BC	$\frac{3EI}{L} = \frac{3EI}{3}$	59	$\frac{15}{59}$
	BD	$\frac{4EI}{L} = \frac{4EI}{3}$	20.0	$\frac{20}{59}$

Joint	A	B	C	D
Member	AB	BA	BC	CD
D.F.	-	$\frac{24}{59}$	$\frac{15}{59}$	-
FEM	-7.2	4.8	-1.5	1.5
		-3.3		
BM		-1.342	-1.118	-0.838
COM	-0.671		-0.75	
BM		0.305	0.254	0.190
COM	0.1525			
Final moments	-7.71	3.76	-0.86	-2.89
			0	0



$$\frac{w_{ab}}{2} = \frac{10 \times 2 \times 2}{5} = 12 \text{ kN.m}$$

$$\frac{w_{bc}^2}{8} = 2.25$$

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$\frac{w_{bc}^2}{8} = 2.25$

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Portal Frames:-

It consists of beams resisting over two columns. The junction beam and column behaves like rigid joints. There are 2 types of portal frames. They are

1. Symmetrical portal frames
2. Unsymmetrical portal frames

1. Symmetrical portal frames:-

In which both columns are of same length having similar end conditions i.e., hinged or fixed. Moment of Inertia (I), Young's Modulus (E) are subjected to symmetrical loading.

2. Unsymmetrical portal frames:-

In which both columns are not of same length, in this frame it is subjected to horizontal moment known as sway to one side or other.

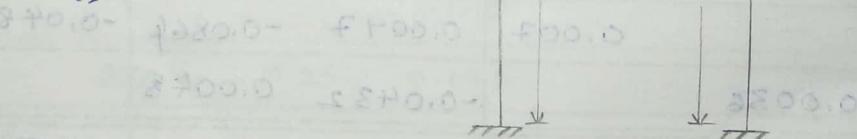
So in rigid joint both columns and beams have motion of translation.

1. A portal frame ABCD is loaded as shown in figure is loaded with a UDL of 2kN/m on the horizontal member. The MOI of AB, CD = I and BC = 3I. Find the support reactions and BM in frame by MDM and draw BMD.

Sol: $\bar{M}_{AB} = \bar{M}_{BA} = \bar{M}_{CD} = \bar{M}_{DC} = 0$

$$\bar{M}_{BC} = -\frac{WL^2}{12} \Rightarrow -\frac{2 \times 6^2}{12} = -6 \text{ kNm}$$

$$\bar{M}_{CB} = \frac{WL^2}{12} \Rightarrow \frac{2 \times 6^2}{12} = 6 \text{ kNm}$$



Joint	Member	K	ΣK	$D.F = \frac{K}{\Sigma K}$
B	BA	$\frac{4EI}{L} = \frac{4EI}{4}$	8EI	$\frac{1}{3}$
	BC	$\frac{4E(3)}{L} = \frac{12EI}{6}$		
C	CB	$\frac{4E(3)}{L} = \frac{12EI}{6}$	3EI	$\frac{2}{3}$
	CD	$\frac{4EI}{L} = \frac{4EI}{4}$		

A

 $\frac{1}{3} B \frac{2}{3}$ $\frac{2}{3} C \frac{1}{3}$

D

0

0 -6

6 0

0

1000 0-

6 1000

1000

-6

1000 0+

02.1

2 4

-4 -2

02.1+

1

-2

2

-1

1

2 -4

4 -2

-1

2

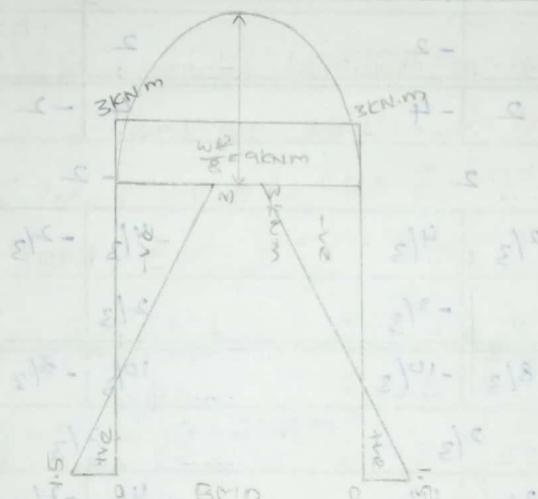
-2

 $\frac{2}{3} \quad \frac{4}{3}$ $-\frac{4}{3} \quad -\frac{2}{3}$ $\frac{1}{3}$ $-\frac{2}{3}$ $\frac{2}{3}$ $-\frac{1}{3}$ $\frac{4}{3}$ $\frac{8}{3} \quad -\frac{10}{3}$ $\frac{10}{3} \quad -\frac{8}{3}$ $-\frac{2}{3}$ $\frac{2}{3}$ $-\frac{2}{3}$ $\frac{2}{9} \quad \frac{4}{9}$ $-\frac{4}{9} \quad -\frac{2}{9}$ $\frac{1}{9}$ $-\frac{2}{9}$ $\frac{2}{9}$ $-\frac{1}{9}$ $\frac{13}{9}$ $\frac{26}{9} \quad -\frac{28}{9}$ $\frac{28}{9} \quad -\frac{26}{9}$ $-\frac{7}{9}$ $\frac{2}{9}$ $-\frac{2}{9}$ $\frac{2}{27} \quad \frac{4}{27}$ $-\frac{4}{27} \quad -\frac{2}{27}$ $-\frac{1}{27}$ $\frac{1}{27}$ $-\frac{2}{27}$ $\frac{2}{27}$ $-\frac{1}{27}$ $\frac{40}{27}$ $\frac{80}{27} \quad -\frac{82}{27}$ $\frac{82}{27} \quad -\frac{80}{27}$ $-\frac{22}{27}$ $\frac{2}{27}$ $-\frac{2}{27}$ $\frac{2}{81} \quad \frac{4}{81}$ $-\frac{4}{81} \quad -\frac{2}{81}$ $-\frac{1}{81}$ $\frac{1}{81}$ $-\frac{2}{81}$ $\frac{2}{81}$ $-\frac{1}{81}$

1.5

 $\frac{242}{81} \quad -\frac{244}{81}$ $\frac{244}{81} \quad \frac{242}{81}$ $-\frac{67}{81}$

A	$\frac{1}{3}$	B $\frac{2}{3}$	$\frac{2}{3}$	C $\frac{1}{3}$	D $\frac{1}{3}$
$\frac{121}{81}$	$\frac{242}{81}$	$-\frac{244}{81}$	$+\frac{244}{81}$	$-\frac{242}{81}$	$-\frac{67}{81}$
	$\frac{2}{81}$		$-\frac{2}{81}$		
	$\frac{2}{243}$	$\frac{4}{243}$	$-\frac{4}{243}$	$-\frac{8}{243}$	
$\frac{1}{243}$		$-\frac{2}{243}$	$\frac{2}{243}$		$-\frac{1}{243}$
$\frac{364}{243}$	2.995	-3.004	+3.004	-2.995	$-\frac{364}{243}$
	0.009			-0.009	
	0.003	0.006	-0.006	-0.003	
0.0015		-0.003	0.003		-0.0015
1.449	2.998	3.001	-3.004	2.998	-1.449
	0.003		-0.003		
	0.001	0.002	-0.002	-0.001	
+0.0001		-0.001	0.001		-0.0001
1.50	3	-3	3	-3	1.50



2. Analyse the symmetric portal frame as shown in figure by MDM. Draw SFD and BMD.

$$\text{Sol: } M_{AB} = -\frac{wl^2}{12} \Rightarrow -\frac{12 \times 4^2}{12} \Rightarrow -16 \text{ kN.m}$$

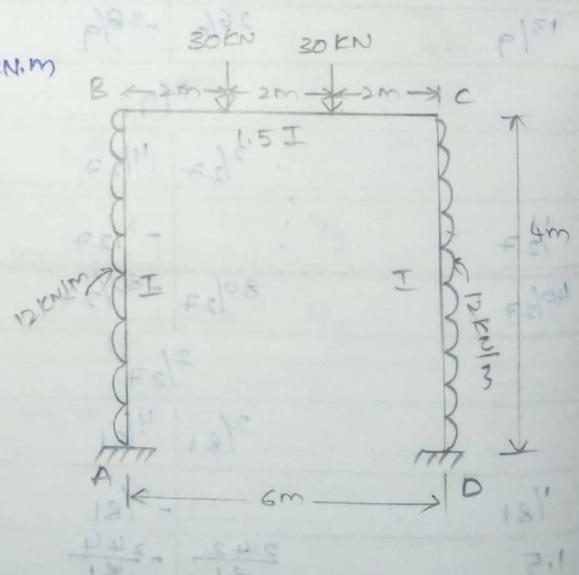
$$M_{BA} = \frac{wl^2}{12} \Rightarrow \frac{12 \times 4^2}{12} \Rightarrow 16 \text{ kN.m}$$

$$M_{BC} = -\frac{wab^2}{l^2} + \frac{wab^2}{l^2}$$

$$= -\frac{30 \times 2 \times 4^2}{6^2} + \left(-\frac{20 \times 4 \times 4^2}{6^2} \right)$$

$$M_{BC} = -40 \text{ kN.m}$$

$$M_{CB} = \frac{wab^2}{l^2} + \frac{wab^2}{l^2}$$



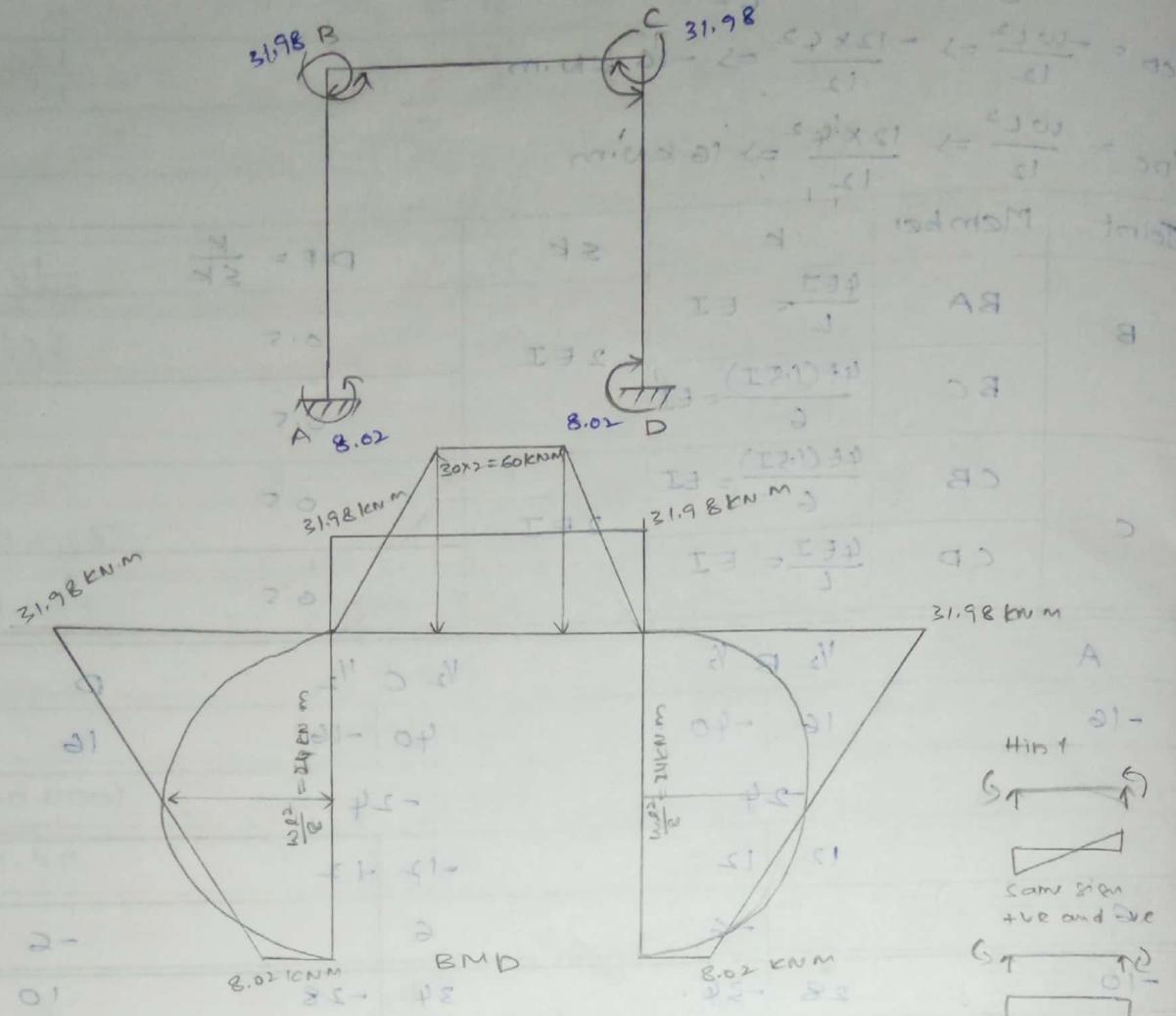
$$= \frac{30 \times 2^2 \times 4}{6^2} + \frac{30 \times 4^2 \times 2}{6^2} \Rightarrow 40 \text{ kN.m}$$

$$M_{CD} = -\frac{wL^2}{12} \Rightarrow -\frac{12 \times 4^2}{12} \Rightarrow -16 \text{ kN.m}$$

$$M_{DC} = \frac{wL^2}{12} \Rightarrow \frac{12 \times 4^2}{12} \Rightarrow 16 \text{ kN.m}$$

Joint	Member	K	$\sum K$	$D.F = \frac{K}{\sum K}$
B	BA	$\frac{4EI}{L} = EI$	2EI	0.5
	BC	$\frac{4E(1.5I)}{6} = EI$		0.5
C	CB	$\frac{4E(1.5I)}{6} = EI$	2EI	0.5
	CD	$\frac{4EI}{L} = EI$		0.5

A	B	C	D
-16	16 -40	40 -16	16
	24	-24	
	12 12	-12 -12	
6	-6	6	-6
-10	28 -34	34 -28	10
	6	-6	
	2 3	-3 -0.3	
1.5	-1.5	1.5	-1.5
-8.5	31 -32.5	32.5 -31	8.5
	1.5	-1.5	
	0.75 0.75	-0.75 -0.75	
0.375	-0.375	0.375	-0.375
-8.125	31.75 -32.125	32.125 -31.75	8.125
	0.375	-0.375	
	0.1875 0.1875	-0.1875 -0.1875	
0.09375	-0.09375	0.09375	-0.09375
-8.03	31.93 -32.03	32.03 -31.93	8.03
	0.1	-0.1	
	0.05 0.05	-0.05 -0.05	
0.025	-0.025	0.025	-0.025
-8	31.98 -31.98	31.98 -31.98	8.02



S.F's

$$R_A + R_B = 48 \text{ kN}$$

$$\sum M_A = 0$$

$$R_B \times 4 + 8.02 - 12 \times 4 \times \frac{4}{2} - 31.98 = 0$$

$$R_B = 29.99 \text{ kN}$$

$$R_A = 18.01 \text{ kN}$$

$$R_B + R_C = 60 \text{ kN}$$

$$\sum M_B = 0$$

$$R_B \times 6 - 20 \times 4 - 20 \times 2 - 31.98 + 31.98 = 0$$

$$R_C = 20 \text{ kN}$$

$$R_B = 20 \text{ kN}$$

$$R_C + R_D = 48 \text{ kN}$$

$$\sum M_C = 0$$

$$R_D \times 4 - 8.02 - 12 \times 4 \times \frac{4}{2} + 31.98 = 0$$

$$R_D = 18.01 \text{ kN}$$

$$R_{DC} = 29.99 \text{ kN}$$

$$200.0 -$$

$$200.0 -$$

$$200.0 -$$

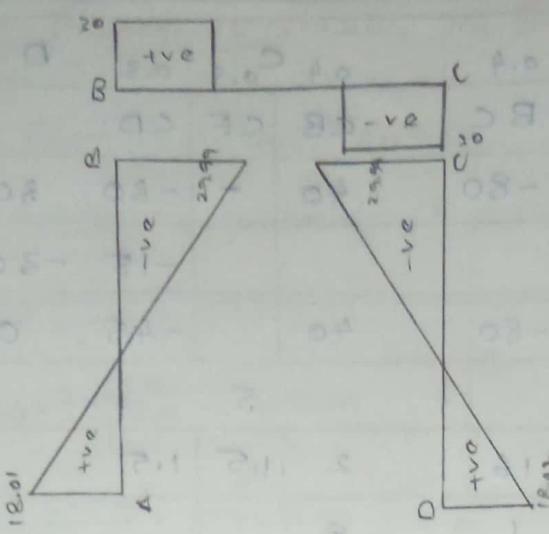
$$200.0 -$$

$$60.8 -$$

$$8P.12 - 8P.12 -$$

$$8P.12 - 8P.12 -$$

$$8 -$$



3. Analyze the frame as shown in figure by MDM and draw SFD and BMD.

$$\text{Ans: } M_{AB} = -\frac{wL^2}{12} \Rightarrow -\frac{30 \times 4^2}{12} \Rightarrow -40 \text{ kN.m}$$

$$M_{BA} = \frac{wL^2}{12} = \frac{30 \times 4^2}{12} \Rightarrow 40 \text{ kN.m}$$

$$M_{BC} = -\frac{wab^2}{l^2} \Rightarrow -\frac{90 \times 2 \times 4^2}{6^2} \Rightarrow -80 \text{ kN.m}$$

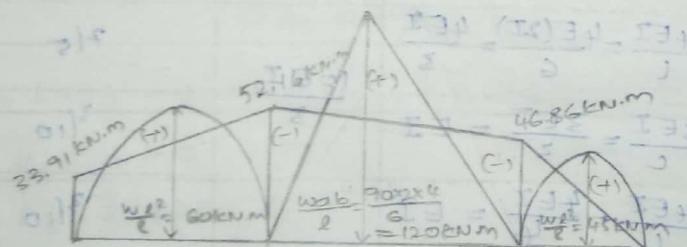
$$M_{CB} = \frac{wab^2}{l^2} \Rightarrow \frac{90 \times 2^2 \times 4}{6^2} \Rightarrow 40 \text{ kN.m}$$

$$M_{CD} = -\frac{wL^2}{12} = -\frac{40 \times 3^2}{12} \Rightarrow -30 \text{ kN.m}$$

$$M_{DC} = \frac{wL^2}{12} = \frac{40 \times 3^2}{12} \Rightarrow 30 \text{ kN.m} \quad M_{BE} = M_{EF} = M_{CF} = M_{FC} = 0$$

Joint	Member	K	ΣK	$P/F = \frac{K}{\Sigma K}$
B	BA	$\frac{4EI}{L} = \frac{4EI}{4} = EI$	310.0	$\frac{3}{10}$
	BC	$\frac{4EI}{L} = \frac{4E(2I)}{8} = \frac{4EI}{3}$	430.0	$\frac{10}{31}$
	BE	$\frac{4EI}{L} = \frac{4EI}{4} = 2EI$	310.0	$\frac{3}{10}$
C	CB	$\frac{4EI}{L} = \frac{4E(2I)}{3} = \frac{4EI}{3}$	10EI	$\frac{2}{5}$
	CD	$\frac{3EI}{L} = \frac{3EI}{3} = EI$	3EI	$\frac{3}{10}$
	CF	$\frac{4EI}{L} = \frac{4EI}{4} = EI$	3EI	$\frac{3}{10}$

A	0.3	B _{0.3}	0.4	0.4	C _{0.3}	0.3	D	EB	FC
	BA	BE	BC	CB	CF	CD			
-40	40	-	-80	40	-	-30	30	-	-
						-15	-30		
-40	40	-	-80	40		-45	0	-	-
	40				5				
	12	12	16	2	1.5	1.5			
6			1	8			6	0.75	
-34	52	12	-63	50	1.5	-43.5	6	0.75	
		-1		mus - 8					
-0.3	-0.3	-0.4		-3.2	-2.4	-2.4			
-0.15			-1.6	-0.2			-0.15	-0.12	
-34.15	51.7	11.7	-65	46.6	-0.9	-45.9	+5.85	-0.45	
	1.6			0.2					
0.48	0.48	0.64	0.08	0.06	0.06		0.2		
0.24			0.04	0.32			0.24	0.03	
-33.91	52.18	12.18	-64.32	47	-0.84	-45.84	6.09	-0.42	
	-0.04			-0.32					
-0.012	-0.012	-0.016	-0.128	-0.096	-0.096				
-0.006			-0.064	-0.008			+0.006	-0.048	
-33.91	52.16	12.16	-64.4	46.86	-0.93	-45.93	6.08	-0.46	
-33.91	52.16	-52.24		46.86	-46.86		6.08	-0.46	



Analyze the frame as shown in figure. Draw SFD and BMD.

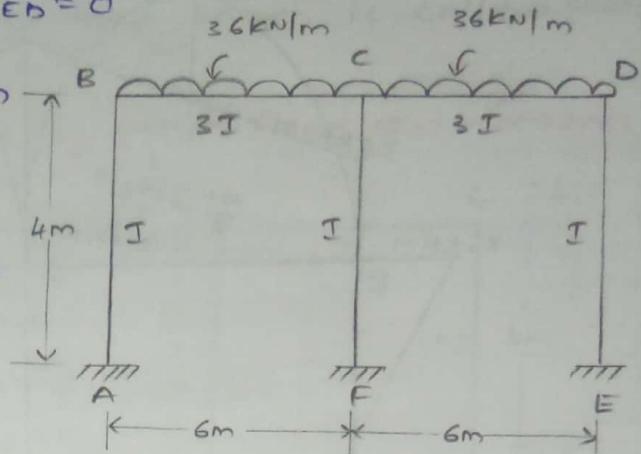
$$\therefore M_{AB} = M_{BA} = M_{CF} = M_{FC} = M_{DE} = M_{ED} = 0$$

$$M_{BC} = -\frac{WL^2}{12} \Rightarrow -\frac{36 \times 6^2}{12} = -108 \text{ KN.m}$$

$$M_{CB} = \frac{WL^2}{12} \Rightarrow \frac{36 \times 6^2}{12} = 108 \text{ KN.m}$$

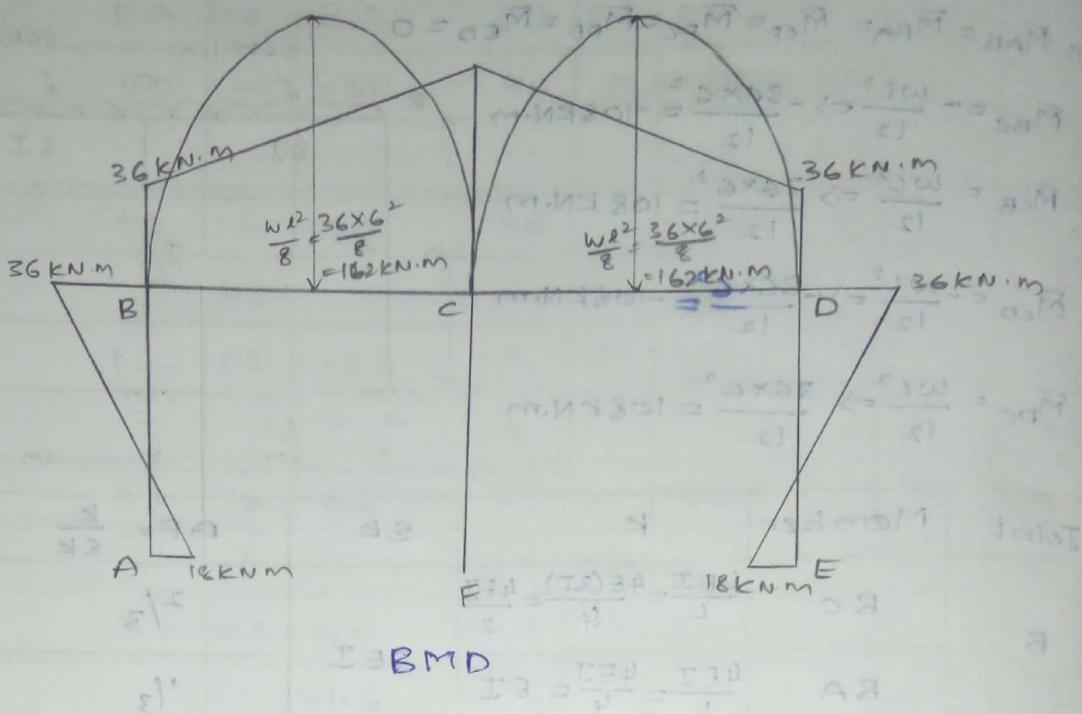
$$M_{CD} = -\frac{WL^2}{12} \Rightarrow -\frac{36 \times 6^2}{12} = -108 \text{ KN.m}$$

$$M_{DC} = \frac{WL^2}{12} \Rightarrow \frac{36 \times 6^2}{12} = 108 \text{ KN.m}$$



Joint	Member	K	ΣK	$D.F = \frac{k}{\Sigma K}$
B	BC	$\frac{4EI}{L} = \frac{4E(3I)}{6} = \frac{4EI}{2}$	$3EI$	$\frac{2}{3}$
	BA	$\frac{4EI}{L} = \frac{4EI}{4} = EI$		$\frac{1}{3}$
C	CB	$\frac{4EI}{L} = \frac{4E(3I)}{6} = \frac{4EI}{2}$	$5EI$	$\frac{2}{5}$
	CD	$\frac{4EI}{L} = \frac{4E(3I)}{6} = \frac{4EI}{2}$		$\frac{2}{5}$
D	CF	$\frac{4EI}{L} = \frac{4EI}{4} = EI$	$3EI$	$\frac{1}{3}$
	DC	$\frac{4EI}{L} = \frac{4E(3I)}{6} = \frac{4EI}{2}$		$\frac{2}{3}$
E	DE	$\frac{4EI}{L} = \frac{4EI}{4} = EI$	$3EI$	$\frac{1}{3}$

A	$\frac{1}{3}$	B	$\frac{2}{3}$	$\frac{2}{5}$	C	$\frac{1}{5}$	$\frac{2}{5}$	$\frac{2}{3}$	D	$\frac{1}{3}$	$\frac{2}{3}$	EF			
BA	0	BC	0	CB	0	CF	0	CD	0	DC	0	DE	0	ED	0
O	0	-108		108	0	-108	108	0	108		-108				
	36	72		0	0	0	-72	-36							
18		0		36		-36	0					-18			
18	36	-36		144	0	-144	36	-36				-18			



BMD

sway frame

When the joints of the beam are having lateral displacement/deflections then the beam is called sway frame.

Analyze the portal frame by MDM. Fixed end moments in all the

Fixed end moments in all the members is equal to zero.

Hence there is no non-sway moments only sway force is calculated but here they given

force equal to 80KN.

\therefore Sway force = 80KN

Non sway Analysis :-

since there is no fixed end moments non sway analysis is zero.

Sway Force :-

$$S = 80\text{KN}$$

Sway Analysis :-

$$\frac{M_{AB}}{M_{CD}} = \frac{M_{f_1}}{M_{f_2}} = \frac{\frac{-6EI_1\Delta}{L_1^2}}{\frac{-6EI_2\Delta}{L_2^2}}$$

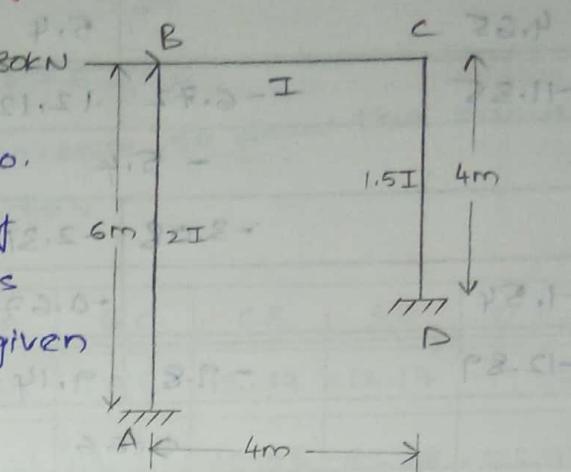
$$\Rightarrow \frac{-6E(2I)\Delta}{B_1^2} \times \frac{4^2}{-6E(1.5I)\Delta}$$

$$2F \frac{2 \times 4^2}{6^2 \times 1.5} \Rightarrow \frac{16}{27}$$

Let us assume fixed end moments developed in AB & BA = -16 kN.m and fixed end moments developed in CD & DC = 27 kN.m

For these arbitrary FEM moment distribution is carried out.

Joint	Member	K	ΣK	$D.F = \frac{K}{\Sigma K}$
B	BA	$\frac{4EI}{L} = \frac{4E(2I)}{6} = \frac{4EI}{3}$	$\frac{7}{3}$	$\frac{4}{7}$
	BC	$\frac{4EI}{L} = \frac{4EI}{4} = EI$		$\frac{3}{7}$
C	CB	$\frac{4EI}{L} = \frac{4EI}{4} = EI$	$\frac{5}{2}$	$\frac{2}{5}$
	CD	$\frac{4E(1.5I)}{4} \Rightarrow 1.5EI$		$\frac{3}{5}$



A	$\frac{4}{7} B \frac{3}{7}$	$\frac{2}{5} C \frac{3}{5}$	D
-16	-16	-27	-27
16	27		
9.28	6.72	10.8	16.2
4.65	5.4	3.36	8.1
-11.35	-6.72	12.12	14.16
	-5.4	-3.36	-18.9
-3.08	-2.31	-1.34	-2.01
-1.54	-0.67	-1.15	-1.00
-12.89	-9.8	9.14	11.66
	0.66	1.15	-12.81
0.377	0.282	0.46	0.69
0.188	0.23	0.141	0.345
-12.70	-9.42	9.65	12.26
	-0.23	-0.14	-12.12
-0.131	-0.098	-0.056	-0.084
-0.065	-0.028	-0.049	-0.042
-12.76	-9.55	9.52	12.15
	0.03	0.05	-12.2
0.017	0.012	0.02	0.03
0.008	0.01	0.006	0.015
-12.75	-9.53	9.54	12.17
	-12.17	-19.57	

$$\Sigma M_B = 0$$

$$H_A \times 6 - 12.45 - 9.53 = 0$$

$$H_A = +3.71 \text{ kN}$$

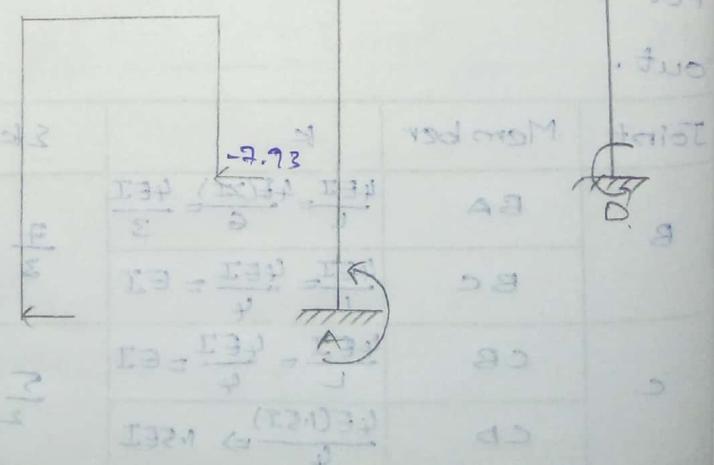
$$H_B = -3.71 \text{ kN}$$

$$\Sigma M_C = 0$$

$$H_D \times 4 - 12.27 - 19.57 = 0$$

$$H_D = 7.93 \text{ kN}$$

$$H_C = -7.93 \text{ kN}$$



$$s' = H_A + H_D \\ = 2.74 + 7.95$$

$$s' = 11.69$$

sway correction factor, $K = \frac{s}{s'}$

$$= \frac{80}{11.69}$$

$$K = 6.843$$

Final End moments:

By finding the final end moments to draw the table with the sway correction factor.

Member	AB	BA	BC	CB	CD	DC
Assumed sway moments	-12.75	-9.53	9.53	12.17	-12.17	-19.57
Actual sway moments = (A.S.M * S.C.F)	-87.24	-65.21	65.21	83.28	-83.28	-133.91
Non sway moments	-	-	-	-	-	-
Final end moments	-87.24	-65.21	65.21	83.28	-83.28	-133.91

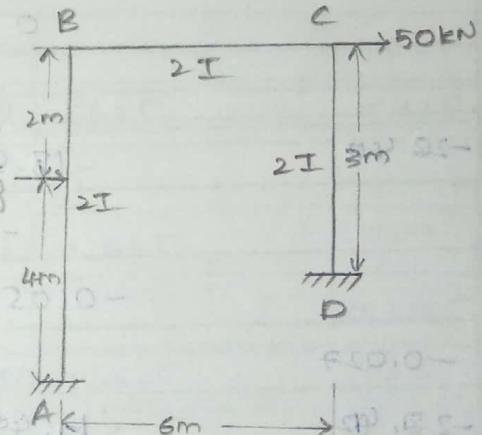
2. Analyze the frame as shown in figure by MDM and draw BMD.

Sol: Non-sway Analysis:-

$$M_{AB} = -\frac{w a^2 b}{l^2} \Rightarrow -\frac{30 \times 4 \times 2^2}{6^2} \Rightarrow -13.33 \text{ kNm}$$

$$M_{BA} = \frac{w a^2 b}{l^2} \Rightarrow \frac{30 \times 4^2 \times 2}{6^2} \Rightarrow 26.67 \text{ kNm}$$

$$M_{BC} = M_{CB} = M_{CD} = M_{DC} = 0$$



Joint	Member	K	EF	D/F = $\frac{K}{EF}$
B	BA	$\frac{4EI}{L} = \frac{4E(2I)}{6} = \frac{4EI}{3}$	$\frac{8EI}{3}$	$\frac{K}{EF} = \frac{4EI}{8EI/3} = \frac{3}{8}$
	BC	$\frac{4EI}{L} = \frac{4E(2I)}{6} = \frac{4EI}{3}$	$\frac{8EI}{3}$	$\frac{K}{EF} = \frac{4EI}{8EI/3} = \frac{3}{8}$
C	CB	$\frac{4EI}{L} = \frac{4E(2I)}{6} = \frac{4EI}{3}$	$\frac{4EI}{3}$	$\frac{K}{EF} = \frac{4EI}{4EI/3} = \frac{3}{4}$
	CD	$\frac{4EI}{3}$	$\frac{8EI}{3}$	$\frac{K}{EF} = \frac{4EI/3}{8EI/3} = \frac{1}{2}$

$$O = O_2 + O_4 + A_4 + O_2 + 2 \\ O = O_2 + S_2 F_1 + F_1 C_1 - O_2 + 2 \\ W_2 F_2 - O_2 = 2$$

A	$\frac{1}{2}$ B $\frac{1}{2}$	$\frac{1}{2}$ C $\frac{1}{2}$	D
-13.33	26.67		
	-26.67		
-13.335	-13.335		
-6.6675		-6.6675	
-19.99	13.33 -13.33	-6.6675	0
	0	6.6675	
-	-	3.33	3.33
-	1.66	1.66	1.66
-19.99	13.33 -14.67	-3.33 3.33	1.66
	-1.66		
-0.83	-0.83	-	-
-0.415		-0.415	
-20.40	12.5 -12.5	-3.74 3.33	1.66
	0	0.41	
0	0	0.205	0.205
	0.102		0.102
-20.40	12.5 -12.39	-3.535 3.535	1.762
	-0.11	0	
-0.055	-0.055	-	-
-0.027		-0.027	-
-20.42	12.45 -12.45	-3.55 3.535	1.762

$$\Sigma M_B = 0$$

$$6H_A + 20.412 + 30 \times 2 - 12.45 = 0$$

$$H_A = -11.327 \text{ kN}$$

$$\Sigma M_C = 0$$

$$3H_D - 1.772 - 3.542 = 0$$

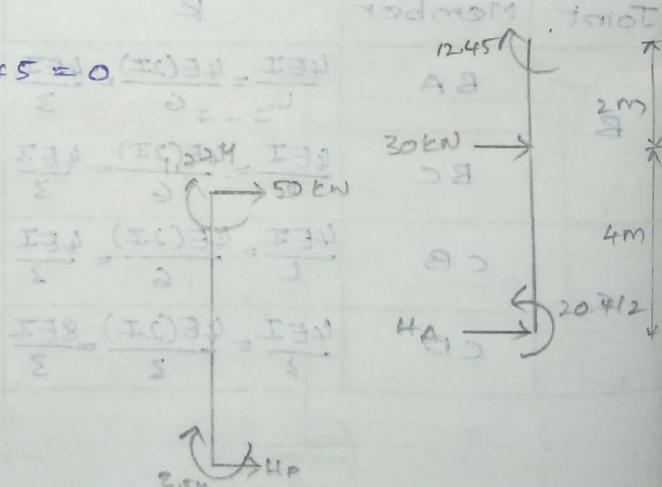
$$H_D = 1.772$$

$$\Sigma H = 0$$

$$-S + 30 + H_A + H_D + 50 = 0$$

$$-S + 30 - 11.327 + 1.772 + 50 = 0$$

$$S = 70.445 \text{ kN}$$



Sway Analysis :-

$$\frac{M_{AB}}{M_{CD}} = \frac{\frac{-GEI_1\Delta_1}{L^2}}{\frac{-GEI_2\Delta_2}{L^2}} \Rightarrow \frac{6(2I)}{6^2} \times \frac{3^2}{6EI}$$

$$\frac{M_{AB}}{M_{CD}} = \frac{1}{2}$$

Let us assume the FEM in column AB & BA = $-1 \times 10 = -10 \text{ kNm}$

Let us assume the FEM in column CD & DC = $-2 \times 10 = -20 \text{ kNm}$

A	$\frac{1}{2}B$	$\frac{1}{2}$	$\frac{1}{2}C$	$\frac{1}{2}D$
-10	-10		-20	-20
	10		20	
2.5	5	5	10	10
		5	2.5	5
-7.5	-5	10	12.5	-10
			-10	-15
	-5		-2.5	
	-2.5	-2.5	-1.25	-1.25
-1.25		-0.625	-1.25	-0.625
-8.75	-7.5	6.875	10	-11.25
			-11.25	-15.625
	0.625		1.25	
0.156	0.3125	0.3125	0.625	0.625
		0.3125	0.156	0.3125
-8.59	-7.18	7.5	10.781	-10.625
			-10.625	-15.31
	-0.32		-0.156	
	-0.16	-0.16	-0.078	-0.078
-0.08		-0.039	-0.08	-0.039
-8.67	-7.34	7.309	10.62	-10.70
			-10.70	-15.349
	0.03		0.08	
0.015	0.015	0.04	0.04	
0.0075	0.02	0.0075		0.02
-8.68	-7.32	7.33	10.66	-10.66
			-10.66	-15.33

$$\sum M_B = 0$$

$$H_A' \times 6 + 8.66 + 7.33 = 0$$

$$H_A' = -2.66 \text{ kN}$$

$$\sum M_C = 0$$

$$H_D' \times 3 + 15.23 + 10.66 = 0$$

$$H_D' = -8.63 \text{ kN}$$

$$\sum H = 0$$

$$S' + H_A' + H_D' = 0$$

$$S' - 2.66 - 8.63 = 0$$

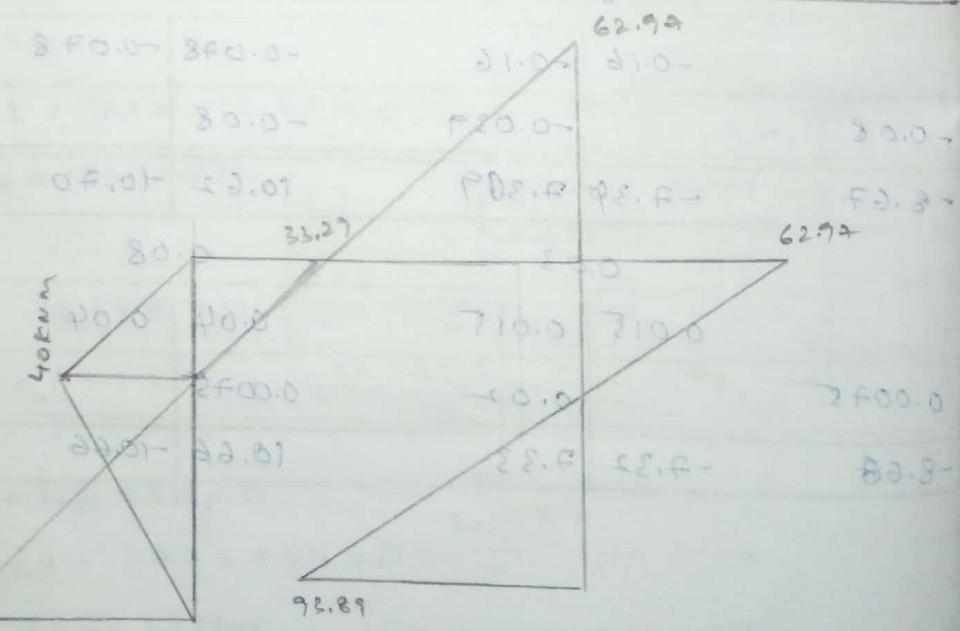
$$S' = 11.29 \text{ kN}$$

Sway correction factor

$$k = \frac{S}{S'} = \frac{70.445}{11.29} = 6.24$$

$$k = 6.24$$

Member	AB	BA	BC	CB	CD	DC
Assumed sway moments	-8.66	-7.32	7.33	10.66	-10.66	-15.33
Actual sway moments (ASMXk)	-54.02	-45.67	45.74	66.51	-66.51	-95.66
Non sway moments	-20.412	12.45	-12.45	-3.54	3.54	1.77
Final Moments	-74.43	-33.22	33.29	62.97	-62.97	-93.89



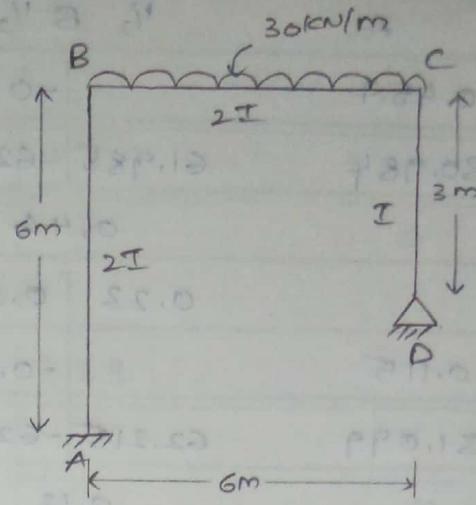
Jon-Swung Analysis :-

$$M_{AB} = \bar{M}_{BA} = 0$$

$$\bar{M}_{BC} = -\frac{WL^2}{12} = -\frac{30 \times 6^2}{12} \Rightarrow 90 \text{ kNm}$$

$$\bar{M}_{CB} = \frac{WL^2}{12} = \frac{30 \times 6^2}{12} \Rightarrow 90 \text{ kNm}$$

$$\bar{M}_{CD} = \bar{M}_{DC} = 0$$



Joint	Member	K	ΣK	$D.F = \frac{K}{\Sigma K}$
B	BA	$\frac{4EI}{L} = \frac{4E(2I)}{6} = \frac{8EI}{6} = \frac{4EI}{3}$	$\frac{8EI}{3}$	$\frac{1}{2}$
	BC	$\frac{4EI}{L} = \frac{4E(2I)}{6} = \frac{8EI}{6} = \frac{4EI}{3}$		$\frac{1}{2}$
C	CB	$\frac{4EI}{L} = \frac{4E(2I)}{6} = \frac{8EI}{6} = \frac{4EI}{3}$	$\frac{7EI}{3}$	$\frac{4}{7}$
	CD	$\frac{3EI}{L} = \frac{3EI}{3} = EI$		$\frac{3}{7}$

A	B	$\frac{1}{2}$	$\frac{1}{2}$	C	$\frac{3}{7}$	D
0	0	-90		90	0	0
		90		-90		
45	45		-51.42	-38.57		
22.5		-25.71		22.5		
22.5	45	-70.71	61.08	-38.57		
	25.71		-22.51			
12.855	12.855		-12.86	-9.647		
6.427		-6.431	6.427			
28.927	57.855	-64.286	54.647	-48.217		
	6.431		-6.43			
3.215	3.215		-3.67	-2.75		
1.60		-1.83	1.60			
30.527	61.07	-62.90	52.57	-50.96		
	1.83		-1.61			
0.915	0.915		-0.92	-0.67		

A	B	C	D
0.457	-0.46	0.457	
30.984	61.985 - 62.445	52.107 - 51.65	0
	0.46	-0.457	
	0.23	0.23	-0.195
0.115	-0.130	0.115	
31.099	62.215 - 62.345	51.961 - 51.845	0
	0.13	-0.116	
	0.065	0.065	-0.0497
0.0325	-0.033	0.0325	
31.13	62.28 - 62.31	51.92 - 51.89	0
	0.03	-0.03	
	0.015	0.015	-0.0128
0.0075	-0.0085	0.0075	
31.13	62.295 - 62.303	51.91 - 51.90	



$$\sum M_B = 0$$

$$\sum M_C = 0$$

$$GHA - 31.13 - 62.295 = 0 \quad 3HD + 51.9 = 0$$

$$HA = 15.57 \text{ kN}$$

$$HB = -17.3 \text{ kN}$$

$$\sum H_O = -15.57 + 17.3 \quad F.O.R.$$

Sway Analysis :-

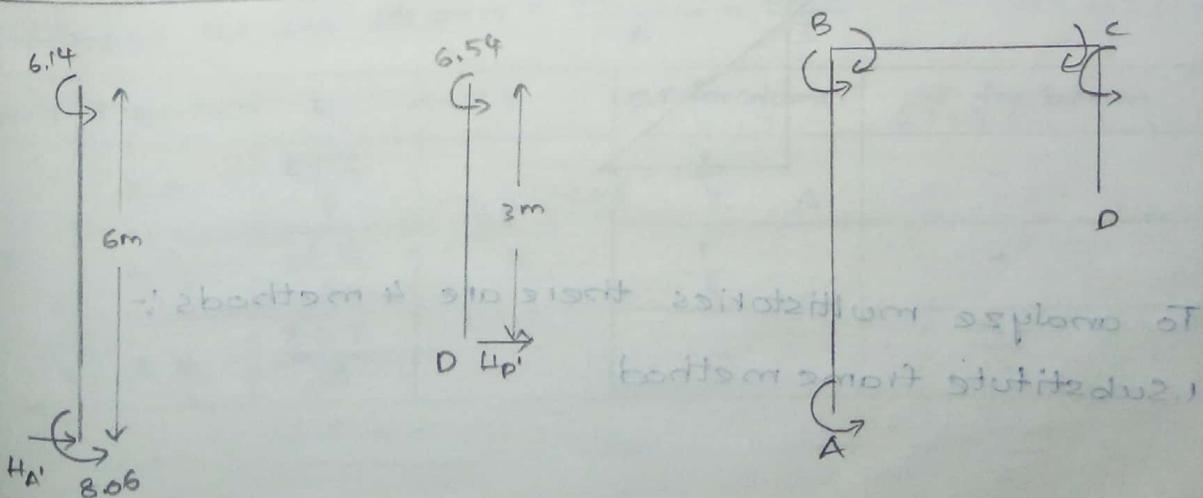
$$\frac{M_{F1AB}}{M_{F2CD}} = \frac{\frac{-6EI_1\Delta_1}{L_1^2} - \frac{12}{G^2}}{\frac{-6EI_2\Delta_2}{L_2^2} - \frac{6}{G^2}} \Rightarrow \frac{1}{2} \quad ; \quad \frac{S_1}{S_2} = \frac{17.3}{15.57} \quad F.O.R.$$

Fixed end moments :-

$$M_{AB} = M_{BA} = -1 \times 10 = -10 \text{ kN.m}$$

$$M_{CD} = M_{DC} = -2 \times 10 = -20 \text{ kN.m}$$

A	$\frac{1}{2}$	B	$\frac{1}{2}$	$\frac{1}{4}$	C	$\frac{3}{4}$	D
-10	-10				-20		-20
-10	-10				+10		+20
-10	-10				-10		0
	10			10			
	5	5		5.71	4.28		
2.5		2.85		2.5			
-7.5	-5	7.85		8.21	-5.72	0	
	-2.85			-2.49			
	-1.425	-1.425		-1.422	-1.06		
-0.7125		-0.711		-0.7125			
-8.212	-6.425	5.714		6.07	-6.78	0	
	+0.711			0.71			
	0.355	0.355		0.405	0.304		
0.1775		0.202		0.1775			
-8.03	-6.07	6.271		6.652	-6.476	0	
	-0.201			-0.176			
	-0.1	-0.1		-0.100	-0.075		
-0.05		-0.05		-0.05			
-8.08	-6.17	6.121		6.502	-6.551	0	
	+0.049			0.049			
	0.0245	0.0245		0.028	0.021		
0.0122		0.014		0.0122			
-8.06	-6.14	6.15		6.54	-6.53	0	



$$\sum M_B = 0$$

$$6H_A' + 8.06 + 6.14 = 0$$

$$\sum M_C = 0$$

$$3H_D' + 6.23 = 0$$

$$H_D' = 2.07 \text{ kN}$$

$$H_A' = -2.367 \text{ kN}$$

$$\sum H = 0$$

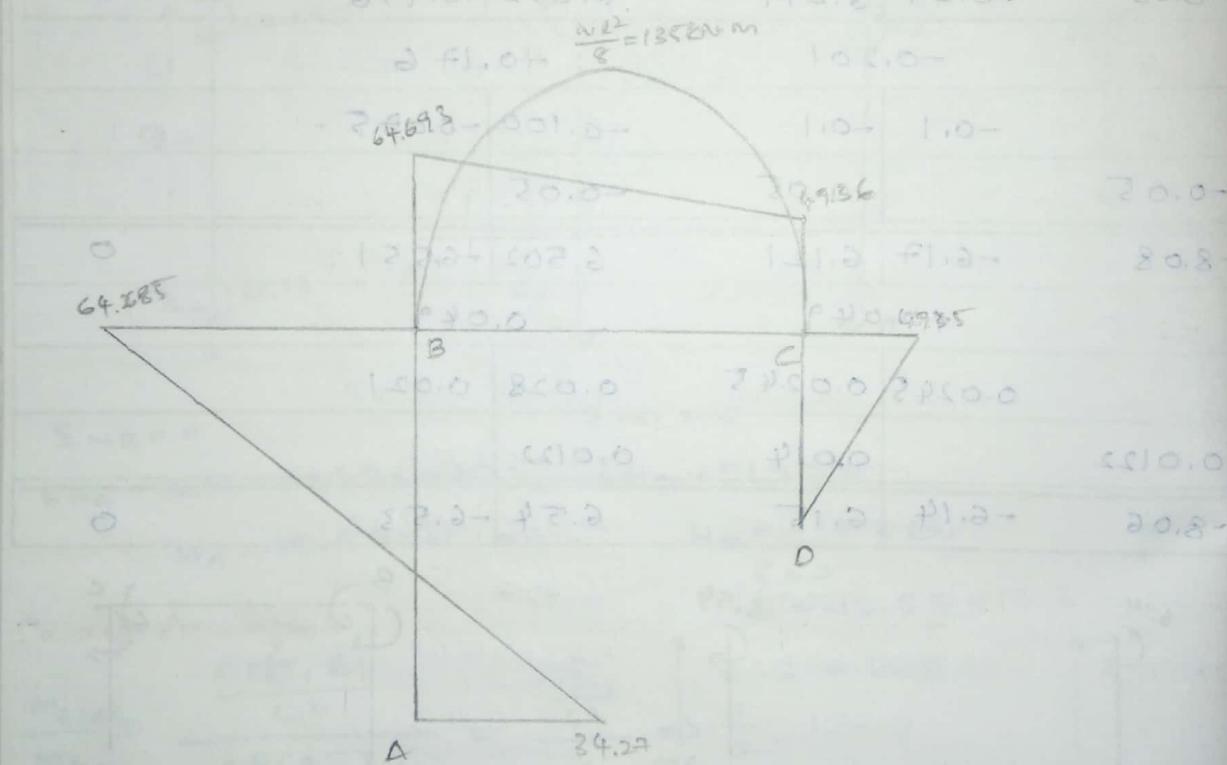
$$S' + H_A' + H_D' = 0$$

$$S' = 2.367 + 2.07$$

$$S' = 4.437 \text{ kN}$$

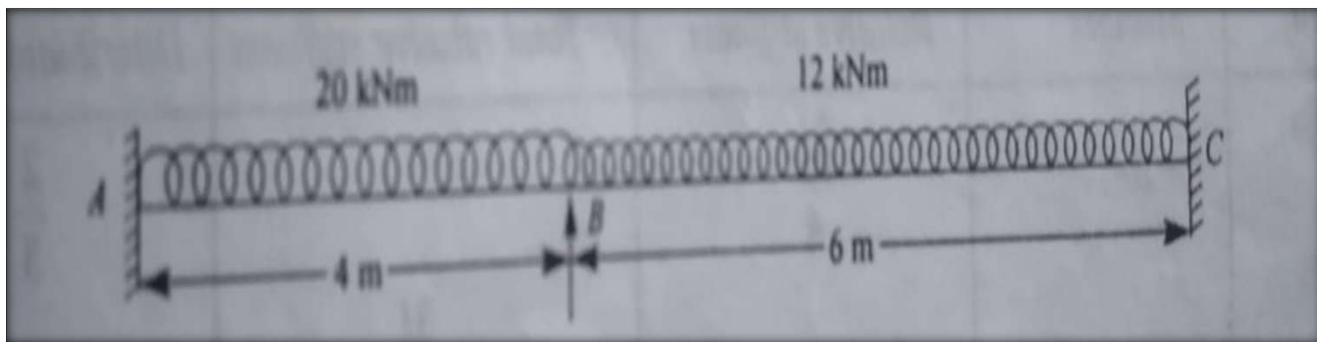
$$K = \frac{S}{S'} = -\frac{1.75}{4.437} = -0.39$$

Member	AB	BA	BC	CB	CD	DC
Assumed sway moments	-8.06	-6.14	6.15	6.54	-6.53	-
Actual sway moments (ASMXk)	3.14	2.39	-2.39	-2.55	2.55	-
Non sway moments	31.13	62.295	-62.303	51.91	-51.90	-
Final moments	34.27	64.685	-64.693	49.36	-49.35	-



To analyze multistories there are 4 methods :-

1. Substitute frame method

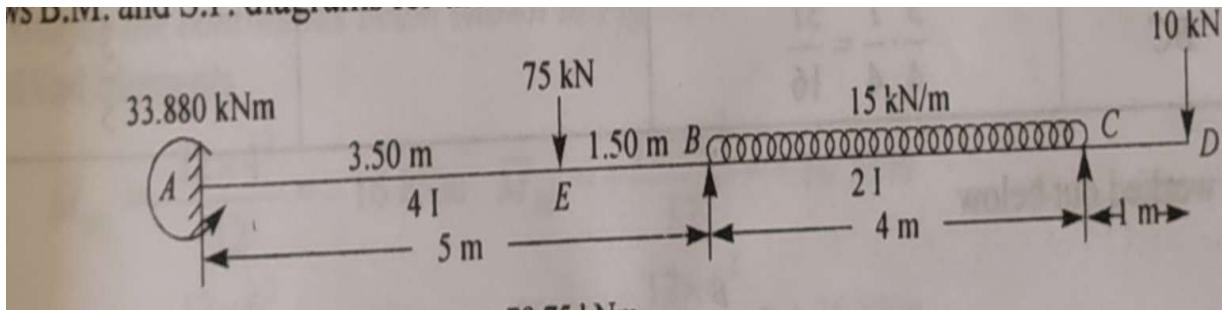


FIXED END MOMENTS	
MFAB	-26.67
MFBA	26.67
MFBC	-36
MFCB	36

DISTRUBUTION TABLE			
JOINT	MEMBER	K	$\Sigma K/K$
B	BA	$4EI/4$	0.60
	BC	$4EI/6$	0.4



JOINT	A		B			C
DF			0.60	0.4		
F.E.M	-26.67		26.67	-36		36
BALANCE						
I.M	-26.67		26.67	-36		36
SUM			9.3333333333			
DISTRIBUTION			5.6	3.733333		
CARRY OVER	2.8				1.866667	
NET	-23.87		32.26667	-32.2667		37.86667
SUM						
DISTRIBUTION						
CARRY OVER						
NET						
SUM						
DISTRIBUTION						
CARRY OVER						



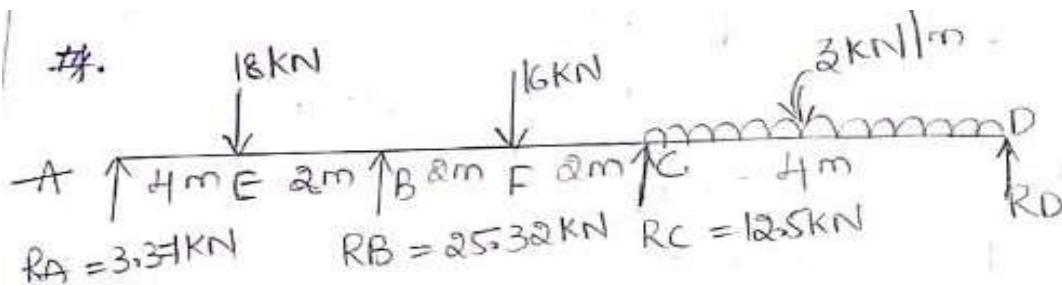
FIXED END MOMENTS	
MFAB	-23.63
MFBA	55.13 AS PER PROBLEM
MFBC	-20.00 MCD 10
MFCB	20.00

DISTRUBUTION TABLE

JOINT	MEMBER	joint condn	E	M.O.I	LENGTH	K	$\Sigma K/K$
B	BA	FIXED	4		4	5	3.20
	BC	hinged	3		2	4	1.50



JOINT	A	B	C
DF		0.68	0.32
F.E.M	-23.63	55.13	-20.00
BALANCE			-5.00 ← -10.00
I.M	-23.63	55.13	-25.00
SUM		-30.13	
DISTRIBUTION		-20.51	-9.61
CARRY OVER	-10.26 ←		
NET	-33.88	34.61	-34.61
SUM			10.00
DISTRIBUTION			
CARRY OVER			
NET			
SUM			
DISTRIBUTION			
CARRY OVER			



FIXED END MOMENTS	
MFAB	-8.00
MFBA	16.00
MFBC	-8.00
MFCB	8.00
MFCD	-4.00
MFDC	4.00

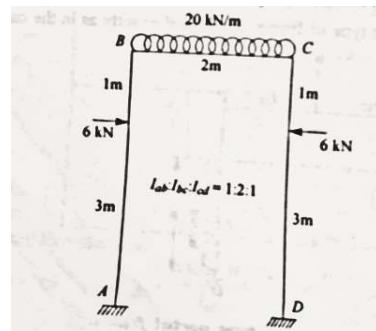
DISTRUBUTION TABLE

JOINT	MEMBER	joint condn	E	M.O.I	LENGTH	K	$\Sigma K/K$
B	BA	HINGED	3		1	6	0.50
	BC	hinged	3		1	4	0.75
C	CB	HINGED	3		1	4	0.75
	CD	hinged	3		1	4	0.75



JOINT	A		B		C		D
DF			0.40	0.60		0.50	0.50
F.E.M	-8.00		16.00	-8.00		8.00	-4.00
BALANCE	8.00	→	4.00			-2.00	← -4.00
I.M	0.00		20.00	-8.00		8.00	-6.00
SUM			-12.00			-2.00	
DISTRIBUTION			-4.80	-7.20		-1.00	-1.00
CARRY OVER	0.00			-0.50	→	-3.60	
NET	0.00		15.20	-15.70		3.40	-7.00
SUM			0.50			3.60	
DISTRIBUTION			0.20	0.30	→	1.80	1.80
CARRY OVER				-0.90	→	-0.15	
NET			15.40	-15.40		5.05	-5.20
SUM							
DISTRIBUTION							
CARRY OVER							

FIXED END MOMENTS	
MFAB	-1.12
MFBA	3.37
MFBC	-6.67
MFCB	6.67
MFCD	-3.37
MFDC	1.12



DISTRUBUTION TABLE							
JOINT	MEMBE	joint condn	E	M.O.I	LENGTH	K	$\Sigma K/K$
B	BA	FIXED	4		1	4	1.00
	BC	FIXED	4		2	4.00	0.80
C	CB	FIXED	4		2	4.00	0.80
	CD	FIXED	4		1	4	1.00
AS PER PROBLEM							
MCD		10					

BALANCE → SUM (add negative sign) → DISTRIBUTION → CARRYOVER (half of the moment) → NET MOMENT (I.M+D.M) → REPEAT

JOINT	A		B		C		D
DF			0.20	0.80			
F.E.M	-1.12		3.37	-6.67		6.67	-3.37
BALANCE	1.12		0.56				1.12
I.M	-1.12		3.37	-6.67		6.67	-3.37
SUM			3.30			-3.30	
DISTRIBUTION			0.66	2.64		-2.64	-0.66
CARRY OVER	0.33			-1.32		1.32	
NET	-0.79		4.03	-5.35		5.35	-4.03
SUM			1.32			-1.32	
DISTRIBUTION			0.26	1.06		-1.06	-0.26
CARRY OVER	0.13			-0.90		-0.15	
NET	-0.66		4.29	-5.19		4.14	-4.29
SUM			0.90			0.15	
DISTRIBUTION			0.18	0.72		0.12	-0.86
CARRY OVER	0.09			0.06		0.36	
NET	-0.57		4.47	-4.41		4.62	-5.15
SUM							
DISTRIBUTION							
CARRY OVER							

WE BALANCE
1.FAR END IS HINGED
2. FAR END HAS OVERHANG
WE CARRY OVER
1.FAR END IS FIXED