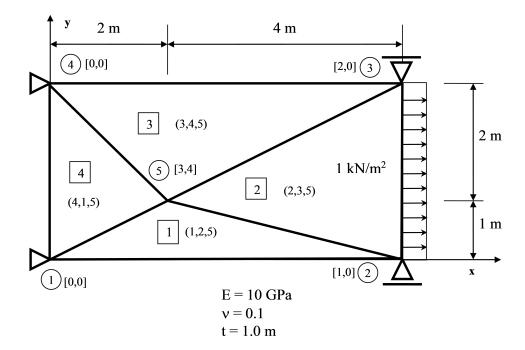
# Cvičení č. 9 - Rovinná napjatost

## Příklad č. 1 - Tahový patch test



#### Slabé řešení:

$$\delta \mathbf{r}^{\mathsf{T}} \qquad \left\{ \sum_{e=1}^{n} \mathbf{L}^{e\mathsf{T}} \left[ \overbrace{\int_{\Omega} \mathbf{B}^{e}(\mathbf{x})^{\mathsf{T}} \mathbf{D}^{e}(\mathbf{x}) \mathbf{B}^{e}(\mathbf{x}) \ d\Omega} \mathbf{L}^{e} \mathbf{r} - \overbrace{\int_{\Omega} \mathbf{N}^{e}(\mathbf{x})^{\mathsf{T}} \overline{\mathbf{X}}(\mathbf{x}) d\Omega} \right] \right\} = 0$$

$$- \underbrace{\int_{\Gamma_{p}} \mathbf{N}^{e}(\mathbf{x})^{\mathsf{T}} \overline{\mathbf{p}}(\mathbf{x}) \ d\Gamma_{p}}_{\mathbf{f}_{p}^{e}} \right] \right\} = 0$$

$$\varepsilon^{e}(x) \approx B^{e}(x) r^{e}$$

$$\sigma^{e}(x) \approx D^{e}(x) B^{e}(x) r^{e}$$

#### Derivace interpolačních funkcí:

$$B^e = egin{bmatrix} rac{\partial N_1}{\partial x} & 0 & rac{\partial N_2}{\partial x} & 0 & rac{\partial N_3}{\partial x} & 0 \ 0 & rac{\partial N_1}{\partial y} & 0 & rac{\partial N_2}{\partial y} & 0 & rac{\partial N_3}{\partial y} \ rac{\partial N_1}{\partial y} & rac{\partial N_1}{\partial x} & rac{\partial N_2}{\partial y} & rac{\partial N_2}{\partial x} & rac{\partial N_3}{\partial y} & rac{\partial N_3}{\partial x} \ \end{bmatrix} \ B^e = egin{bmatrix} y_{23} & 0 & y_{31} & 0 & y_{12} & 0 \ 0 & x_{32} & 0 & x_{13} & 0 & x_{21} \ x_{32} & y_{23} & x_{13} & y_{31} & x_{21} & y_{12} \end{bmatrix}$$

Matice tuhosti pro rovinnou napjatost:

$$D = rac{E}{1-
u^2} egin{bmatrix} 1 & 
u & 0 \ 
u & 1 & 0 \ 0 & 0 & rac{1-
u}{2} \end{bmatrix}$$

Matice tuhosti prvku:

$$K^e = \int_{\Omega_e} B^{eT} D^e B^e d\Omega = B^{eT} D^e B^e \int_{\Omega_e} d\Omega = At B^{eT} D^e B^e$$

# Okrajové podmínky:

Předepsané zatížení na hranici:

$$f_{\Gamma_p} = \int_{\Gamma_p} N^e(x)^T ar{p}(x) d\Gamma_p$$

```
In [16]: % funkce pocita plochu trojuhelnikoveho prvku
         % In:
         % xe - vektor x-ovych souradnic uzlu prvku (x1,x2,x3)
         %
            ye - vektor y-ovych souradnic uzlu prvku (y1,y2,y3)
         % Out:
         % Ae - plocha prvku
         function Ae = area_triangle (xe,ye)
         Ae=(1/2)*((xe(2)*ye(3)-xe(3)*ye(2))-(xe(1)*ye(3)-xe(3)*ye(1))+(xe(1)*ye(2)-xe(2)*ye(1)));
         end
         % funkce pocita delky stran trojuhelnikoveho prvku
         % xe - vektor x-ovych souradnic uzlu prvku (x1,x2,x3)
            ye - vektor y-ovych souradnic uzlu prvku (y1,y2,y3)
         %
         % Out:
         % le - delky hran prvku (3,1)
         function le = length_triangle (xe,ye)
         le = zeros(3,1);
         le(1) = sqrt((xe(2)-xe(1))*(xe(2)-xe(1))+(ye(2)-ye(1))*(ye(2)-ye(1)));
         le(2) = sqrt((xe(3)-xe(2))*(xe(3)-xe(2))+(ye(3)-ye(2))*(ye(3)-ye(2)));
         le(3) = sqrt((xe(1)-xe(3))*(xe(1)-xe(3))+(ye(1)-ye(3))*(ye(1)-ye(3)));
         % funkce pocita matici tuhosti 2d trojuhelnikoveho stenoveho prvku s linearni aproximaci
         % In:
         % xe - vektor x-ovych souradnic uzlu prvku (x1,x2,x3)
         % ye - vektor y-ovych souradnic uzlu prvku (y1,y2,y3)
         % E - Younguv modul pruznosti
         % nu - Poissonuv soucinitel
         % Out:
         % ke - matice tuhosti prvku (6,6)
         % dbe - matice na vypocet napeti (3,6)
            de - materialova matice (3,3)
            be - mative derivaci bazovych funkci (3,6)
         function [ke,dbe,de,be] = plane_stress (xe,ye,E,nu)
         Ae = area_triangle(xe,ye);
         y23 = ye(2)-ye(3); y31 = ye(3)-ye(1); y12 = ye(1)-ye(2);
         x32 = xe(3)-xe(2); x13 = xe(1)-xe(3); x21 = xe(2)-xe(1);
         be = (1/(2*Ae))*[y23, 0, y31, 0, y12, 0;
                           0, x32, 0, x13, 0, x21;
                           x32,y23,x13,y31,x21,y12];
         de = (E/(1-nu*nu))*[1, nu, 0;
                             nu, 1, 0;
                             0, 0, (1-nu)/2 ];
         dbe = de*be ;
         ke = Ae*be'*de*be;
         end
         % funkce pro lokalizaci matice tuhosti prvku
         % do matice tuhosti konstrukce
         % vyzaduje existenci globalni matice kodovych cisel - lm
         %
         % Vstup:
         % k - matice tuhosti konstrukce
         % ke- matice tuhosti prvku
         %
            lm- matice kodovych cisel
            e - cislo prvku
         % Vystup:
```

```
k - matice tuhosti konstrukce
function k = assembly (k,ke,lm,e)
ndof = size(ke,1);
for i=1:ndof
         ia = lm(e,i);
         if ia ~=0
             for j=1:ndof
                 ja=lm(e,j);
                 if ja ~=0
                   k(ia,ja)=k(ia,ja)+ke(i,j);
                 end
             end
        end
    end
end
% funkce pro lokalizaci matice prave strany
% vyzaduje existenci globalni matice kodovych cisel - lm
%
% Vstup:
% f - matice prave strany
% fe- matice prave strany prvku
% Lm- matice kodovych cisel
% e - cislo prvku
%
% Vystup:
\% f - matice prave strany
function f = assemblyf (f,fe,lm,e)
ndof = size(fe,1);
for i=1:ndof
        ia = lm(e,i);
         if ia ~=0
            f(ia,1)=f(ia,1)+fe(i,1);
         end
    end
end
```

```
In [17]: %pocet prvku
         nelem=4;
         nnodes=5;
         E=10;
         nu = 0.1;
         % pole kodovych cisel uzlu
         id = [ 5 6; 1 7; 2 8; 9 10; 3 4 ];
         %pole uzlovych cisel prvku
         ide = [1 2 5; 2 3 5; 3 4 5; 4 1 5];
         % pocet neznamych
         neq = 10;
         % pole souradnic
         x = [0.0, 6.0, 6.0, 0.0, 2.0];
         y = [0.0, 0.0, 3.0, 3.0, 1.0];
         ke = zeros(6,6);
         dbe = zeros(3,6);
         be = zeros(3,6);
         de = zeros(3,3);
         k = zeros(neq,neq);
         f = zeros(neq,1);
         for i = 1:nelem
             xe = [x(ide(i,:))];
             ye = [y(ide(i,:))];
             [ke,dbe,de,be] = plane_stress(xe,ye,E,nu);
             lm = [id(ide(i,1),:), id(ide(i,2),:), id(ide(i,3),:)];
             k=assembly(k,ke,lm,1);
         f = [1.5, 1.5, 0.0, 0.0];
         %reseni posunu
         kuu=k(1:4,1:4)
         fuu=f(:)
         u=kuu\fuu;
         ug = zeros(neq,1);
         ug(1:4)=u(:)
         nodal_u = [];
         % Vypis posunu podle uzlu
         for i=1:size(id,1)
             nodal_u = [nodal_u; ug(id(i,1)), ug(id(i,2))];
         end
         nodal_u
```

```
kuu =
    7.07071 -2.18855 -7.07071
                                            2.77778
   -2.18855
              5.89226 -3.53535
                                          -2.77778

      -7.07071
      -3.53535
      31.81818

      2.77778
      -2.77778
      0.00000

                                            0.00000
                                            50.56818
fuu =
   1.50000
   1.50000
   0.00000
   0.00000
u =
   5.9400e-01
   5.9400e-01
   1.9800e-01
   5.6938e-19
ug =
   0.59400
   0.59400
   0.19800
   0.00000
   0.00000
   0.00000
   0.00000
   0.00000
   0.00000
```

### Vyhodnocení výsledků:

0.00000

0.00000

0.59400

0.59400

0.00000

0.19800

0.00000

0.00000

0.00000

0.00000

0.00000

nodal\_u =

Vnitřní síly na hranách:

h: 
$$g^e = \left\{egin{array}{c} \sigma_n \ au \end{array}
ight\} = \left[egin{array}{c} cos(lpha) & sin(lpha) \ -sin(lpha) & cos(lpha) \end{array}
ight] \left\{egin{array}{c} p_x \ p_y \end{array}
ight\} = \left[egin{array}{c} cos(lpha) & sin(lpha) \ -sin(lpha) & cos(lpha) \end{array}
ight] \left\{egin{array}{c} \sigma_x & au_{xy} \ \tau_{yx} & \sigma_y \end{array}
ight] \left\{egin{array}{c} cos(lpha) \ sin(lpha) \end{array}
ight\}$$

```
In [18]: % funkce pocita napeti na hrnach 2d trojuhelnikoveho stenoveho prvku s linearni aproximaci
         % In:
         % xe - vektor x-ovych souradnic uzlu prvku (x1,x2,x3)
         % ye - vektor y-ovych souradnic uzlu prvku (y1,y2,y3)
            sig - matice napeti v tezisti (3,1)
         %
         % Out:
         % pse - napeti na hranach (6,1)
         function pse = boundary_stress (xe,ye,sig)
         pse = zeros(6,1);
         A = [ sig(1), sig(3);
               sig(3), sig(2)];
         y23 = ye(2)-ye(3); y31 = ye(3)-ye(1); y12 = ye(1)-ye(2);
         x32 = xe(3)-xe(2); x13 = xe(1)-xe(3); x21 = xe(2)-xe(1);
         le = length_triangle (xe,ye) ;
         c1 = -y12/le(1);
         s1 = -x21/le(1);
         T = [c1, s1; -s1, c1];
         temp = T*A*[c1;s1];
         pse(1) = temp(1,1);
         pse(2) = temp(2,1);
         c1 = -y23/le(2);
         s1 = -x32/le(2);
         T = [c1, s1; -s1, c1];
         temp = T*A*[c1;s1];
         pse(3) = temp(1,1);
         pse(4) = temp(2,1);
         c1 = -y31/le(3);
         s1 = -x13/le(3);
         T = [c1, s1; -s1, c1];
         temp = T*A*[c1;s1];
         pse(5) = temp(1,1);
         pse(6) = temp(2,1);
         end
         %reseni relativnich deformaci a napeti
         for i = 1:nelem
            xe = [x(ide(i,:))];
             ye = [y(ide(i,:))];
             [ke,dbe,de,be] = plane_stress(xe,ye,E,nu);
             lm = [id(ide(i,1),:), id(ide(i,2),:), id(ide(i,3),:)];
             ul = [ug(lm)];
             eps = be*ul;
             sig = dbe*ul;
             pse = boundary_stress (xe,ye,sig);
             eps'
             sig'
             pse'
         end
         % Plot deformed configuration over the initial configuration
         for i = 1:nnodes
             xnew(i) = x(i) + ug(id(i,1));
             ynew(i) = y(i) + ug(id(i,2));
         end
         figure;
```

```
for i = 1:nelem
    xe = [x(ide(i,:))];
    ye = [y(ide(i,:))];
    plot(xe,ye,'k');hold on;
    xe = [xnew(ide(i,:))];
    ye = [ynew(ide(i,:))];
    plot(xe,ye,'r');hold on;
end
title('Initial and deformed structure'); xlabel('X'); ylabel('Y');
```

0.099000 0.000000 0.000000

ans =

1.0000e+00 1.0000e-01 1.1102e-16

ans =

1.0000e-01 -1.1102e-16 1.5294e-01 -2.1176e-01 2.8000e-01 3.6000e-01

ans =

0.099000 0.000000 -0.000000

ans =

1.0000e+00 1.0000e-01 -6.4702e-19

ans =

1.0000e+00 -6.4702e-19 2.8000e-01 3.6000e-01 1.5294e-01 -2.1176e-01

ans =

0.099000 -0.000000 0.000000

ans =

1.0000e+00 1.0000e-01 -5.5511e-17

ans =

1.0000e-01 5.5511e-17 5.5000e-01 -4.5000e-01 2.8000e-01 3.6000e-01

ans =

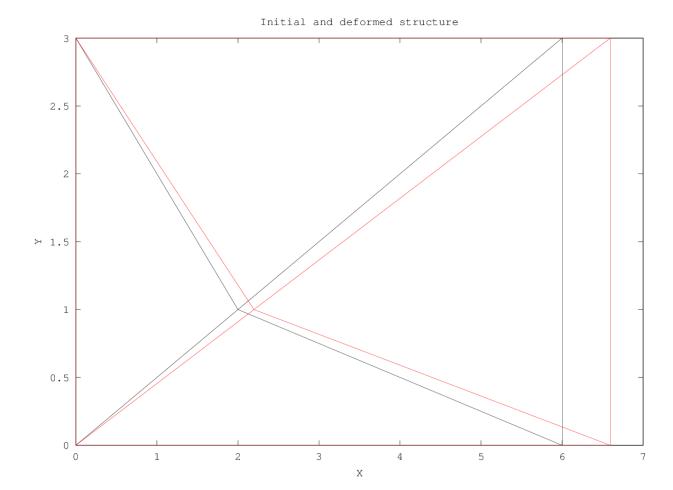
0.099000 0.000000 0.000000

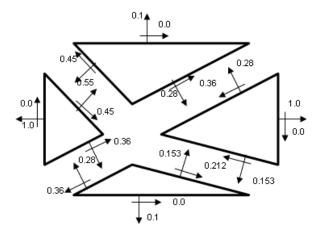
ans =

1.0000e+00 1.0000e-01 1.2940e-18

ans =

1.0000e+00 1.2940e-18 2.8000e-01 3.6000e-01 5.5000e-01 -4.5000e-01





### Analytické řešení:



Předpokládáme, že je aproximace posunutí na prvku lineární (implikuje konstantní relativní deformace a konstantní napětí):

$$u=ax,v=0$$

Z geometrických rovnic:

$$egin{aligned} arepsilon_x &= rac{\partial u}{\partial x} = a \ arepsilon_y &= rac{\partial v}{\partial y} = 0 \ \gamma_{xy} &= rac{\partial u}{\partial y} + rac{\partial v}{\partial x} = 0 \end{aligned}$$

Z konstitutivních rovnic: \$\$

# \left{\begin{array}{c} \sigma\_x \ \sigmay \ \tau{xy} \end{array}\right}

 $\label{eq:frac} $$ \frac{E}{1-\frac2}\left(E^2\right) = \frac{1-nu^2}{E}$ 

$$1 \quad \nu \quad 0 \quad \nu \quad 1 \quad 0 \quad 0 \quad 0 \quad \frac{1-i}{2}$$

\sigma\_x = 1.0

Celkem:

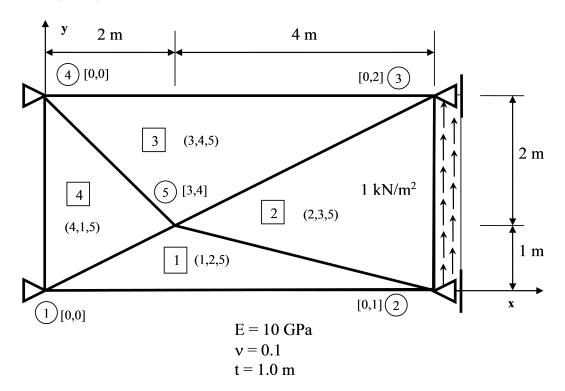
 $\sigma_x = \frac{E}{1-\ln^2}a=1.0 \le a=0.099=\sqrt{x}$ 

 $\sigma = \frac{E}{1-\ln^2}\ln a = 0.1$ 

 $tau\{xy\} = 0$ 

u(6)=ax=0.099 \cdot 6=0.594 \$\$

Příklad č. 2 - Smykový patch test



```
In [19]: %pocet prvku
         nelem=4;
         nnodes=5;
         E=10;
         nu = 0.1;
         % pole kodovych cisel uzlu
         id = [ 5 6; 7 1; 8 2; 9 10; 3 4 ];
         %pole uzlovych cisel prvku
         ide = [1 2 5; 2 3 5; 3 4 5; 4 1 5];
         % pocet neznamych
         neq = 10;
         % pole souradnic
         x = [0.0, 6.0, 6.0, 0.0, 2.0];
         y = [0.0, 0.0, 3.0, 3.0, 1.0];
         ke = zeros(6,6);
         dbe = zeros(3,6);
         be = zeros(3,6);
         de = zeros(3,3);
         k = zeros(neq,neq);
         f = zeros(neq,1);
         for i = 1:nelem
             xe = [x(ide(i,:))];
             ye = [y(ide(i,:))];
             [ke,dbe,de,be] = plane_stress(xe,ye,E,nu);
             lm = [id(ide(i,1),:), id(ide(i,2),:), id(ide(i,3),:)];
             k=assembly(k,ke,lm,1);
         f = [1.5, 1.5, 0.0, 0.0];
         %reseni posunu
         kuu=k(1:4,1:4)
         fuu=f(:)
         u=kuu\fuu;
         ug = zeros(neq,1);
         ug(1:4)=u(:);
         nodal_u = [];
         % Vypis posunu podle uzlu
         for i=1:size(id,1)
             nodal_u = [nodal_u; ug(id(i,1)), ug(id(i,2))];
         end
         nodal_u
```

```
kuu =
```

11.23737	-6.35522	2.77778	-11.23737
-6.35522	9.36448	-2.77778	-5.61869
2.77778	-2.77778	31.81818	0.00000
-11.23737	-5.61869	0.00000	50.56818

### fuu =

- 1.50000
- 1.50000
- 0.00000
- 0.00000

#### u =

- 1.3200e+00
- 1.3200e+00
- 1.9979e-17
- 4.4000e-01

### nodal\_u =

- 0.00000 0.00000
- 0.00000 1.32000
- 0.00000 1.32000
- 0.00000 0.00000
- 0.00000 0.44000

```
In [20]: %reseni relativnich deformaci a napeti
         for i = 1:nelem
             xe = [x(ide(i,:))];
             ye = [y(ide(i,:))];
             [ke,dbe,de,be] = plane_stress(xe,ye,E,nu);
             lm = [id(ide(i,1),:), id(ide(i,2),:), id(ide(i,3),:)];
             ul = [ug(lm)];
             eps = be*ul;
             sig = dbe*ul;
             pse = boundary_stress (xe,ye,sig);
             eps'
             sig'
             pse'
         end
         % Plot deformed configuration over the initial configuration
         for i = 1:nnodes
             xnew(i) = x(i) + ug(id(i,1));
             ynew(i) = y(i) + ug(id(i,2));
         figure;
         for i = 1:nelem
             xe = [x(ide(i,:))];
             ye = [y(ide(i,:))];
             plot(xe,ye,'k');hold on;
             xe = [xnew(ide(i,:))];
             ye = [ynew(ide(i,:))];
             plot(xe,ye,'r');hold on;
         end
         title('Initial and deformed structure'); xlabel('X'); ylabel('Y');
```

0.00000 0.00000 0.22000

ans =

2.2204e-16 1.7764e-15 1.0000e+00

ans =

1.7764e-15 -1.0000e+00 4.7059e-01 -8.8235e-01 -8.0000e-01 -6.0000e-01

ans =

-4.9948e-18 5.5511e-17 2.2000e-01

ans =

6.0570e-17 8.8313e-16 1.0000e+00

ans =

6.0570e-17 1.0000e+00 -8.0000e-01 -6.0000e-01 4.7059e-01 -8.8235e-01

ans =

0.00000 -0.00000 0.22000

ans =

-5.5511e-17 -4.4409e-16 1.0000e+00

ans =

-4.4409e-16 -1.0000e+00 1.0000e+00 -2.2204e-16 -8.0000e-01 -6.0000e-01

ans =

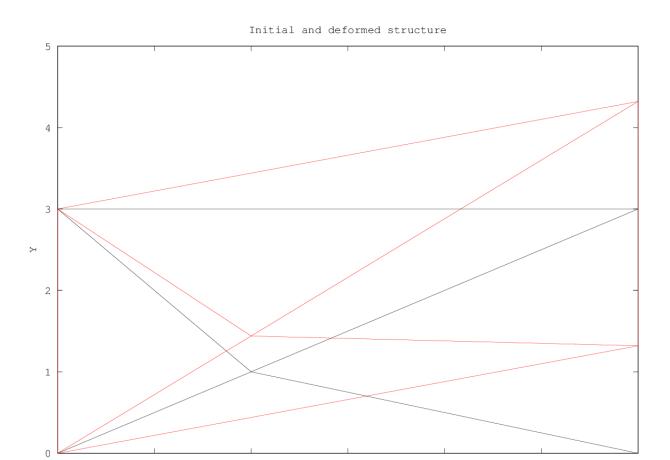
0.00000 0.00000 0.22000

ans =

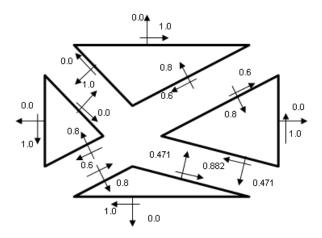
1.0090e-16 1.0090e-17 1.0000e+00

ans =

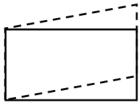
1.0090e-16 1.0000e+00 -8.0000e-01 -6.0000e-01 1.0000e+00 -5.5511e-17



Х



### Analytické řešení:



Předpokládáme, že je aproximace posunutí na prvku lineární (implikuje konstantní relativní deformace a konstantní napětí):

$$u=0, v=ax$$

Z geometrických rovnic:

$$egin{aligned} arepsilon_x &= rac{\partial u}{\partial x} = 0 \ arepsilon_y &= rac{\partial v}{\partial y} = 0 \ \gamma_{xy} &= rac{\partial u}{\partial y} + rac{\partial v}{\partial x} = a \end{aligned}$$

Z konstitutivních rovnic: \$\$

# \left{\begin{array}{c} \sigma\_x \ \sigmay \ \tau{xy} \end{array}\right}

\frac{E}{1-\nu^2}\left[

1 
$$\nu$$
 0  $\nu$  1 0 0 0  $\frac{1-\nu}{2}$ 

Zokrajových podmínek:

 $\text{tau}\{xy\} = 1.0$ 

Celkem:

v(6)=ax=0.22 \cdot 6 = 1.32 \$\$