# Finite element solution for a single element

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#### **Contents**

- definte parameters
- define element info
- define numerical integration parameters
- take a numerical integration
- Show K matrix and its eigenvalues
- initialization displacement and force vector
- define constraint point and others
- impose constraints
- define Known displacement and force
- calculate Unknown displacement and force
- Draw displacement distribution and force distribution

### definte parameters

material parameters young modulus

```
E = 10;
% poisson's ratio
nu = 0.33;
% model parameters
% dimensionality
D = 3;
% number of nodes
nnde = 4;
% order of numerical integration
nint = 3;
% all degrees of freedom
nf = D* nnde;
```

### define element info

### coordinates

### define numerical integration parameters

```
SF = GenerateShapeFunction(D, nnde, nint);
% calculate elast tensor
CC = ElastTensor(E,nu);
```

## take a numerical integration

```
[K, M]=IntKMLoc(SF, CC, x(ix, :));
```

## Show K matrix and its eigenvalues

```
disp('K = : ');
disp(K);
disp('eig = : ')
disp(eig(K));
```

```
K = :
 列 1 至 7
 132.7552
          -0.0000
                   -0.0000 -80.4762 39.6506 -37.2910 -52.2790
  -0.0000
          33.6841
                    0.0000 20.4261 -20.4193 -0.0000 -20.4261
  -0.0000
           0.0000
                     33.6841 -19.2105
                                      -0.0000 -20.4193 -12.1178
 -80.4762
          20.4261 -19.2105 72.1270 -36.4185
                                               34.2512
                                                         26.2161
  39.6506 -20.4193
                    -0.0000 -36.4185
                                      72.1511 -34.2625
                                                         -3.2321
 -37.2910
          -0.0000 -20.4193 34.2512 -34.2625
                                              67.9442
                                                         22.0310
                                              22.0310
 -52.2790 -20.4261 -12.1178 26.2161
                                      -3.2321
                                                         37.3332
 -39.6506 -13.2648
                    0.0000 15.9924 -33.8648 15.2650
                                                         23.6582
 -23.5228
          0.0000 -13.2648
                            21.8246
                                      -2.6149
                                               22.8920
                                                        14.0353
  -0.0000
            0.0000
                   31.3283 -17.8670
                                       -0.0000 -18.9912 -11.2703
  -0.0000 -0.0000 -0.0000
                            -0.0000 -17.8670
                                              18.9975
                                                          0.0000
                    -0.0000 -36.8653
  60.8138
           -0.0000
                                       36.8775 -70.4169 -23.9485
 列 8 至 12
 -39.6506 -23.5228
                    -0.0000
                              -0.0000
                                       60.8138
 -13.2648
           0.0000
                     0.0000
                              -0.0000
                                       -0.0000
   0.0000 -13.2648
                    31.3283
                              -0.0000
                                       -0.0000
  15.9924
          21.8246 -17.8670
                              -0.0000
                                     -36.8653
 -33.8648
          -2.6149
                   -0.0000 -17.8670
                                      36.8775
                             18.9975 -70.4169
  15.2650
          22.8920 -18.9912
          14.0353
  23.6582
                   -11.2703
                              0.0000 -23.9485
  58.3999 21.6124
                    0.0000 -11.2703 -36.8775
  21.6124
          34.7910 -12.3371 -18.9975 -44.4182
   0.0000 -12.3371
                   29.1373
                              0.0000
                                      -0.0000
                                       -0.0000
 -11.2703 -18.9975
                    0.0000
                              29.1373
 -36.8775 -44.4182
                    -0.0000
                            -0.0000 114.8351
eig = :
  1.0e+02 *
```

```
3.3650 + 0.0000i
```

```
0.9669 + 0.0000i

0.8936 + 0.0000i

0.8568 + 0.0000i

0.5278 + 0.0000i

0.5497 + 0.0000i

0.0000 + 0.0000i

0.0000 + 0.0000i

-0.0000 + 0.0000i

-0.0000 + 0.0000i

-0.0000 + 0.0000i
```

## initialization displacement and force vector

```
u = zeros(D*nnde,1);
fext = zeros(D*nnde,1);
```

### define constraint point and others

```
pu2 = [1 2 3 4 8 12];
pu1 = setxor((1:nf), pu2);
```

### impose constraints

```
u(pu2) = 0;
fext(:) = 0;
% extrude the 4th node a distance
u(12) = 0.1;
```

## define Known displacement and force

```
u2 = u(pu2);
f1 = fext(pu1);
```

### calculate Unknown displacement and force

```
u(pu1) = K(pu1, pu1) \ (f1 - K(pu1, pu2) * u2);
fext(pu2) = K(pu2, :) * u;
```

警告: 矩阵接近奇异值,或者缩放错误。结果可能不准确。RCOND = 8.950083e-19。

### Draw displacement distribution and force distribution

reshape force and displacement matrix

```
fext0 = reshape(fext,[3,4])';
```

```
u0 = reshape(u, [3, 4])';
% get the current location of node
xu = x+u0;
% convert vertex matrix to face matrix
fx = VerToFace(x, ix);
% difine title
ftl ={'Force in x', 'Force in y', 'Force in z'};
% draw all force distribution
for i = 1:3
figure
% draw the frame
patch('vertices', x, 'faces', fx, 'facecolor', 'none', 'edgecolor', 'b');
hold on
% draw force distribution
mypatch(x, fx, 'interp', 'none', fext0(:,i), 0.5, 0,ftl{i},'x axis','y axis','z axis');
view(35, 20)
colorbar
% mark node number
gui label(3, x, fx, 4, 1, ix);
end
%define title
dtl ={'Displacement in x', 'Displacement in y', 'Displacement in z'};
% draw all displacement distribution
for i =1:3
figure
% draw frame
patch('vertices', x, 'faces', fx, 'facecolor', 'none', 'edgecolor', 'b');
hold on
% draw displacement distribution
mypatch(xu, fx, 'interp', 'none', u0(:,i), 0.5, 1,dtl{i},'x axis','y axis','z axis');
view(35, 20)
colorbar
% mark node number
gui_label(3, x, fx, 4, 1, ix);
end
```











