# Deep learned one-iteration nonlinear solver for solid mechanics

Tan N. Nguyen, Jaehong Lee, Liem Dinh-Tien, Minh Dang

July 18, 2023

### **Outline**

- 1. Abstract
  - 1.1. Background and Gap Statement
  - 1.2. Goals/Aims
  - 1.3. Methodology
  - 1.4. Results and Conclusion
  - 1.5. Implications

#### **Abstract**

Nowadays, there are a lot of iterative algorithms which have been proposed for nonlinear problems of solid mechanics. The existing biggest drawback of iterative algorithms is the requirement of numerous iterations and computation to solve these problems. This can be found clearly when the large or complex problems with thousands or millions of degrees of freedom are solved. To overcome completely this difficulty, the novel one-iteration nonlinear solver (OINS) using time series prediction and the modified Riks method (M-R) is proposed in this paper.

#### **Abstract**

OINS is established upon the core idea as follows: (1) Firstly, we predict the load factor increment and the displacement vector increment and the convergent solution of the considering load step via the predictive networks which are trained by using the load factor and the displacement vector increments of the previous convergence steps and group method of data handling (GMDH):(2) Thanks to the predicted convergence solution of the load step is very close to or identical with the real one, the prediction phase used in any existing nonlinear solvers is eliminated completely in OINS. Next, the correction phase of the M-R is adopted and the OINS iteration is started at the predicted convergence point to reach the convergent solution. The training process and the applying process of GMDH are continuously conducted and repeated during the nonlinear analysis in order to predict the convergence point at the beginning of each load step. Through numerical investigations, we prove that OINS is powerful, highly accurate and only needs about one iteration per load step. Thus, OINS significantly saves number of iterations and a huge amount of computation compared with the conventional methods. Especially, OINS not only can detect limit, inflection, and other special points but also can predict exactly various types of instabilities of structures

## **Outline**

#### 1. Abstract

- 1.1. Background and Gap Statement
- 1.2. Goals/Aims
- 1.3. Methodology
- 1.4. Results and Conclusion
- 1.5. Implications

## **Background**

Nowadays, there are a lot of iterative algorithms which have been proposed for nonlinear problems of solid mechanics. The existing biggest drawback of iterative algorithms is the requirement of numerous iterations and computation to solve these problems. This can be found clearly when the large or complex problems with thousands or millions of degrees of freedom are solved.

# Nonlinear analysis of Isotropic Shells using First-order shell Deformation Theory (FSDT)

The study will evaluate the efficiency and reliability of OINS in solid mechanics via geometrically nonlinear analysis of shells using FSDT. The strain vectors using FSDT are expressed by:

$$\varepsilon = \{\varepsilon_{xx}, \ \varepsilon_{yy}, \ \gamma_{xy}\}^T = \varepsilon_0 + z\kappa_b$$
$$\gamma = \{\gamma_{xz}, \ \gamma_{yz}\}^T = \varepsilon_s,$$

with

$$\varepsilon_0 = \varepsilon_L + \varepsilon_N; \ \varepsilon_L = \begin{Bmatrix} u_{0,x} + w_0/R \\ \nu_{0,y} \\ u_{0,y} + \nu_{0,x} \end{Bmatrix}; \ \varepsilon_N = \frac{1}{2} \begin{Bmatrix} w_{0,x}^2 \\ w_{0,y}^2 \\ 2w_{0,xy} \end{Bmatrix}$$

$$\varepsilon_b = \begin{Bmatrix} \beta_{x,x} \\ \beta_{y,y} \\ \beta_{x,y} + \beta_{y,x} \end{Bmatrix}; \ \varepsilon_s = \begin{Bmatrix} -u_0/R + w_{0,x} + \beta_x \\ w_{0,y} + \beta_y \end{Bmatrix}$$

# Nonlinear analysis of Isotropic Shells using First-order shell Deformation Theory (FSDT)

The study will evaluate the efficiency and reliability of OINS in solid mechanics via geometrically nonlinear analysis of shells using FSDT. The strain vectors using FSDT are expressed by:

$$\varepsilon = \{\varepsilon_{xx}, \ \varepsilon_{yy}, \ \gamma_{xy}\}^T = \varepsilon_0 + z\kappa_b$$
$$\gamma = \{\gamma_{xz}, \ \gamma_{yz}\}^T = \varepsilon_s,$$

with

$$\varepsilon_0 = \varepsilon_L + \varepsilon_N; \ \varepsilon_L = \begin{Bmatrix} u_{0,x} + w_0/R \\ \nu_{0,y} \\ u_{0,y} + \nu_{0,x} \end{Bmatrix}; \ \varepsilon_N = \frac{1}{2} \begin{Bmatrix} w_{0,x}^2 \\ w_{0,y}^2 \\ 2w_{0,xy} \end{Bmatrix}$$

$$\kappa_b = \begin{Bmatrix} \beta_{x,x} \\ \beta_{y,y} \\ \beta_{x,y} + \beta_{y,x} \end{Bmatrix}; \ \varepsilon_s = \begin{Bmatrix} -u_0/R + w_{0,x} + \beta_x \\ w_{0,y} + \beta_y \end{Bmatrix}$$

# Nonlinear analysis of Isotropic Shells using First-order shell Deformation Theory (FSDT)

and we denote  $\Omega$  to be the initial configuration of the shell. At a specific application, i.e. focusing on the istropic shell of FSDT, NURBS basis function  $N_A$  is utilized. The study entitled "NURBS-based postbuckling analysis of functionally graded carbon nanotube-reinforced composite shells" thoroughly discussed the derivation of shell formulation based on NURBS.

At the ith iteration and mth load increment, the system of incremental equation is expressed as

The FG-CNTRC Shell Formulation based on NURBS

$$K_T(q_m)\Delta^i q_m = {}^i F_{\mathsf{ext},m} - {}^i F_{\mathsf{int},m}$$

where

$$K_{T} = \int_{\Omega} \begin{bmatrix} \begin{Bmatrix} B_{A}^{L} \\ B_{A}^{b} \\ B_{A}^{s} \end{Bmatrix} + \begin{Bmatrix} B_{A}^{N} \\ 0 \\ 0 \end{bmatrix}^{T} \begin{bmatrix} D^{p} & 0 & 0 \\ 0 & D^{b} & 0 \\ 0 & 0 & D^{s} \end{bmatrix} \begin{bmatrix} \begin{Bmatrix} B_{A}^{L} \\ B_{A}^{b} \\ B_{A}^{s} \end{Bmatrix} + \begin{Bmatrix} B_{A}^{N} \\ 0 \\ 0 \end{bmatrix} dS + \int_{\Omega} (B_{A}^{g})^{T} \begin{bmatrix} N_{x} & N_{xy} \\ N_{xy} & N_{y} \end{bmatrix} B_{A}^{g} d\Omega$$

At the ith iteration and mth load increment, the system of incremental equation is expressed as

The FG-CNTRC Shell Formulation based on NURBS

$$K_T(q_m)\Delta^i q_m = {}^i F_{\mathsf{ext},m} - {}^i F_{\mathsf{int},m}$$

where

$$K_{T} = \int_{\Omega} \begin{bmatrix} \begin{Bmatrix} B_{A}^{L} \\ B_{A}^{b} \\ B_{A}^{s} \end{Bmatrix} + \begin{Bmatrix} B_{A}^{N} \\ 0 \\ 0 \end{Bmatrix}^{T} \begin{bmatrix} D^{p} & 0 & 0 \\ 0 & D^{b} & 0 \\ 0 & 0 & D^{s} \end{bmatrix} \begin{bmatrix} \begin{Bmatrix} B_{A}^{L} \\ B_{A}^{b} \\ B_{A}^{s} \end{Bmatrix} + \begin{Bmatrix} B_{A}^{N} \\ 0 \\ 0 \end{Bmatrix} d\Omega$$
$$+ \int_{\Omega} (B_{A}^{g})^{T} \begin{bmatrix} N_{x} & N_{xy} \\ N_{xy} & N_{y} \end{bmatrix} B_{A}^{g} d\Omega$$

## FC-CNTRC based on NURBS Basis Function

Derivation of the equation is based on the related literature from the same author detailing the NURBS basis functions. We have

$$B_A^L = \begin{bmatrix} N_{A,x} & 0 & \frac{1}{R}N_A & 0 & 0 \\ 0 & N_{A,y} & 0 & 0 & 0 \\ N_{A,y} & N_{A,x} & 0 & 0 & 0 \end{bmatrix}; \quad B_A^b = \begin{bmatrix} 0 & 0 & 0 & N_{A,x} & 0 \\ 0 & 0 & 0 & 0 & N_{A,y} \\ 0 & 0 & 0 & N_{A,y} & N_{A,x} \end{bmatrix}$$

$$B_A^s = \begin{bmatrix} -\frac{1}{R}N_A & 0 & N_{A,x} & N_A & 0 \\ 0 & 0 & N_{A,y} & 0 & N_A \end{bmatrix}$$

and

$$B_A^N = AB_A^g = \begin{bmatrix} w_{0,x} & 0\\ 0 & w_{0,y}\\ w_{0,y} & w_{0,x} \end{bmatrix} \begin{bmatrix} 0 & 0 & N_{A,x} & 0 & 0\\ 0 & 0 & N_{A,y} & 0 & 0 \end{bmatrix}$$

where  $w_0$  is the radial deflector of the panel.

## FC-CNTRC based on NURBS Basis Function

Derivation of the equation is based on the related literature from the same author detailing the NURBS basis functions. We have

$$B_A^L = \begin{bmatrix} N_{A,x} & 0 & \frac{1}{R}N_A & 0 & 0\\ 0 & N_{A,y} & 0 & 0 & 0\\ N_{A,y} & N_{A,x} & 0 & 0 & 0 \end{bmatrix}; \quad B_A^b = \begin{bmatrix} 0 & 0 & 0 & N_{A,x} & 0\\ 0 & 0 & 0 & 0 & N_{A,y}\\ 0 & 0 & 0 & N_{A,y} & N_{A,x} \end{bmatrix}$$
$$B_A^s = \begin{bmatrix} -\frac{1}{R}N_A & 0 & N_{A,x} & N_A & 0\\ 0 & 0 & N_{A,y} & 0 & N_A \end{bmatrix}$$

and

$$B_A^N = AB_A^g = \begin{bmatrix} w_{0,x} & 0\\ 0 & w_{0,y}\\ w_{0,y} & w_{0,x} \end{bmatrix} \begin{bmatrix} 0 & 0 & N_{A,x} & 0 & 0\\ 0 & 0 & N_{A,y} & 0 & 0 \end{bmatrix}$$

where  $w_0$  is the radial deflector of the panel.

## FC-CNTRC based on NURBS Basis Function

Derivation of the equation is based on the related literature from the same author detailing the NURBS basis functions. We have

$$B_A^L = \begin{bmatrix} N_{A,x} & 0 & \frac{1}{R}N_A & 0 & 0\\ 0 & N_{A,y} & 0 & 0 & 0\\ N_{A,y} & N_{A,x} & 0 & 0 & 0 \end{bmatrix}; \quad B_A^b = \begin{bmatrix} 0 & 0 & 0 & N_{A,x} & 0\\ 0 & 0 & 0 & 0 & N_{A,y}\\ 0 & 0 & 0 & N_{A,y} & N_{A,x} \end{bmatrix}$$
$$B_A^s = \begin{bmatrix} -\frac{1}{R}N_A & 0 & N_{A,x} & N_A & 0\\ 0 & 0 & N_{A,y} & 0 & N_A \end{bmatrix}$$

and

$$B_A^N = AB_A^g = \begin{bmatrix} w_{0,x} & 0\\ 0 & w_{0,y}\\ w_{0,y} & w_{0,x} \end{bmatrix} \begin{bmatrix} 0 & 0 & N_{A,x} & 0 & 0\\ 0 & 0 & N_{A,y} & 0 & 0 \end{bmatrix}$$

where  $w_0$  is the radial deflector of the panel.

#### The FG-CNTRC Shell Formulation based on NURBS

$$K_T(q_m)\Delta^i q_m = {}^i F_{\mathsf{ext},m} - {}^i F_{\mathsf{int},m}$$

- $q_m$  is the displacement vector
  - $\Delta^i q_m$  is the change in load vector
  - ${}^{i}F_{\mathsf{ext}\,m}$  is the load vector
- ${}^{i}F_{\text{int},m}$  is the internal force

#### The FG-CNTRC Shell Formulation based on NURBS

$$K_T(q_m)\Delta^i q_m = {}^i F_{\mathsf{ext},m} - {}^i F_{\mathsf{int},m}$$

- $q_m$  is the displacement vector
- $\Delta^i q_m$  is the change in load vector
- ${}^{i}F_{\text{ext},m}$  is the load vector
- ${}^{i}F_{\text{int},m}$  is the internal force

#### The FG-CNTRC Shell Formulation based on NURBS

$$K_T(q_m)\Delta^i q_m = {}^i F_{\mathsf{ext},m} - {}^i F_{\mathsf{int},m}$$

- $q_m$  is the displacement vector
- $\Delta^i q_m$  is the change in load vector
- ${}^{i}F_{\mathsf{ext}.m}$  is the load vector
- ${}^{i}F_{\text{int},m}$  is the internal force

#### The FG-CNTRC Shell Formulation based on NURBS

$$K_T(q_m)\Delta^i q_m = {}^i F_{\mathsf{ext},m} - {}^i F_{\mathsf{int},m}$$

- $q_m$  is the displacement vector
- $\Delta^i q_m$  is the change in load vector
- ${}^{i}F_{\mathsf{ext}\ m}$  is the load vector
- ${}^{i}F_{\text{int},m}$  is the internal force

Using the novel OINS to solve nonlinear equation, for any load increments, an iterative process is conducted and will stop when the convergence criterion e is met as follows

#### Convergence Criterion e

$$e = \frac{\|{}^{i}\lambda_{m}F_{0} - {}^{i}F_{\mathsf{int},m}\|}{\|({}^{i}\lambda_{m} + \Delta^{i}\lambda_{m})F_{0}\|} < 10^{-3}$$

where  $\lambda$  is denoted as the load factor.

Using the novel OINS to solve nonlinear equation, for any load increments, an iterative process is conducted and will stop when the convergence criterion e is met as follows

#### Convergence Criterion e

$$e = \frac{\|i\lambda_m F_0 - iF_{\mathsf{int},m}\|}{\|(i\lambda_m + \Delta^i \lambda_m)F_0\|} < 10^{-3}$$

where  $\lambda$  is denoted as the load factor.

Note that the load vector at ithe iteration and mth load increment

$${}^{i}F_{\mathsf{ext},m} = \begin{pmatrix} {}^{i}\lambda_m + \Delta^{i}\lambda_m \end{pmatrix} \int_{\Omega} f_0 \left\{ 0 \quad 0 \quad N_A \quad 0 \quad 0 \right\}^T d\Omega = \begin{pmatrix} {}^{i}\lambda_m + \Delta^{i}\lambda_m \end{pmatrix} F_0$$
 (1)

where  $F_0$  denotes the referenced load vector while  $N_A$  stands for the NURBS (nonuniform rational B-spline) basic function.

The load factor  $\lambda$  and the displacement vector q of the iteration are updated as follows

$$i^{i+1}\lambda_m = i \lambda_m + \Delta^i \lambda_m$$

$$i^{i+1}q_m = i q_m + \Delta^i q_m$$

$$\Delta^i q_m = \Delta^i q_{R,m} + \Delta^i \lambda_m q_{F,m}$$

Moreover,  $q_{F,m}$  and  $\Delta^i q_{R,m}$  respectively denote the displacement created by the reference vector and the change caused by the residual load vectors and that

$$\Delta^{i} q_{R,m} = [K_T(q_m)]^{-1} \left( {}^{i} \lambda_m F_0 - {}^{i} F_{\mathsf{int},m} \right) \tag{2}$$

$$q_{F,m} = [K_T(q_m)]^{-1} F_0 (3)$$

Recall that  $K_T(q_m)\Delta^i q_m = {}^i F_{\text{ext},m} - {}^i F_{\text{int},m}$ .

The load factor  $\lambda$  and the displacement vector q of the iteration are updated as follows

$$i+1 \lambda_m = i \lambda_m + \Delta^i \lambda_m$$

$$i+1 q_m = i q_m + \Delta^i q_m$$

$$\Delta^i q_m = \Delta^i q_{R,m} + \Delta^i \lambda_m q_{F,m}$$

Moreover,  $q_{F,m}$  and  $\Delta^i q_{R,m}$  respectively denote the displacement created by the reference vector and the change caused by the residual load vectors and that

$$\Delta^{i} q_{R,m} = [K_T(q_m)]^{-1} \left( {}^{i} \lambda_m F_0 - {}^{i} F_{\mathsf{int},m} \right) \tag{2}$$

$$q_{F,m} = [K_T(q_m)]^{-1} F_0 (3)$$

Recall that  $K_T(q_m)\Delta^i q_m = {}^i F_{\text{ext},m} - {}^i F_{\text{int},m}$ 

The load factor  $\lambda$  and the displacement vector q of the iteration are updated as follows

$$i^{i+1}\lambda_m = i \lambda_m + \Delta^i \lambda_m$$

$$i^{i+1}q_m = i q_m + \Delta^i q_m$$

$$\Delta^i q_m = \Delta^i q_{R,m} + \Delta^i \lambda_m q_{F,m}$$

Moreover,  $q_{F,m}$  and  $\Delta^i q_{R,m}$  respectively denote the displacement created by the reference vector and the change caused by the residual load vectors and that

$$\Delta^{i} q_{R,m} = [K_T(q_m)]^{-1} \left( {}^{i} \lambda_m F_0 - {}^{i} F_{\mathsf{int},m} \right) \tag{2}$$

$$q_{F,m} = [K_T(q_m)]^{-1} F_0 (3)$$

Recall that  $K_T(q_m)\Delta^i q_m = {}^iF_{\mathsf{ext},m} - {}^iF_{\mathsf{int},m}.$ 

Rewriting equations (2), we have

$$K_T(q_m)\Delta^i q_{R,m} = {i \choose i} \lambda_m F_0 - {i \choose i} F_{\mathsf{int},m}$$
(4)

then we can rewrite the convergence criterion e as follow

$$e = \frac{\|{}^{i}\lambda_{m}F_{0} - {}^{i}F_{\mathsf{int},m}\|}{\|({}^{i}\lambda_{m} + \Delta^{i}\lambda_{m})F_{0}\|}$$
$$= \frac{\|K_{T}(q_{m})\Delta^{i}q_{R,m}\|}{\|{}^{i}F_{\mathsf{ext},m}\|} \quad [1, 4]$$

Rewriting equations (2), we have

$$K_T(q_m)\Delta^i q_{R,m} = ({}^i\lambda_m F_0 - {}^iF_{\mathsf{int},m}) \tag{4}$$

then we can rewrite the convergence criterion e as follows

$$e = \frac{\|{}^{i}\lambda_{m}F_{0} - {}^{i}F_{\mathsf{int},m}\|}{\|({}^{i}\lambda_{m} + \Delta^{i}\lambda_{m})F_{0}\|}$$
$$= \frac{\|K_{T}(q_{m})\Delta^{i}q_{R,m}\|}{\|{}^{i}F_{\mathsf{ext},m}\|} \quad [1,4]$$

Rewriting equations (2), we have

$$K_T(q_m)\Delta^i q_{R,m} = ({}^i\lambda_m F_0 - {}^iF_{\mathsf{int},m}) \tag{4}$$

then we can rewrite the convergence criterion e as follows

$$e = \frac{\|{}^{i}\lambda_{m}F_{0} - {}^{i}F_{\mathsf{int},m}\|}{\|({}^{i}\lambda_{m} + \Delta^{i}\lambda_{m})F_{0}\|}$$
$$= \frac{\|K_{T}(q_{m})\Delta^{i}q_{R,m}\|}{\|{}^{i}F_{\mathsf{ext},m}\|} \quad [1,4]$$

## **Outline**

#### 1. Abstract

- 1.1. Background and Gap Statement
- 1.2. Goals/Aims
- 1.3. Methodology
- 1.4. Results and Conclusion
- 1.5. Implications

## Goals/Aims

To overcome completely this difficulty, the novel one-iteration nonlinear solver (OINS) using time series prediction and the modified Riks method (M-R) is proposed in this paper.

## Goals/Aims

To overcome completely this difficulty, the novel one-iteration nonlinear solver (OINS) using time series prediction and the modified Riks method (M-R) is proposed in this paper.

## Modifield Riks (M-R) Method

The Modified Riks (M-R) Method is a nonlinear problem solver that uses iterative process to come up to the converging solution.

# Modifield Riks (M-R) Method

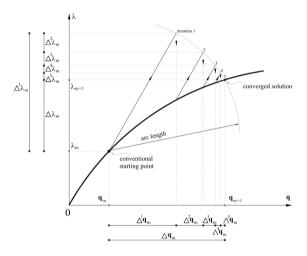


Figure: M-R Method (Nguyen, 2021)

# One-iteration Nonlinear Solver (OINS)

The **One-iteration Nonlinear Solver** is the proposed solver of the study that is developed from the M-R method implemented with the Group Method of Data Handling (GMDH)

### **One-iteration Nonlinear Solver**

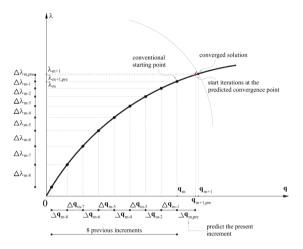


Figure: OINS (Nguyen, 2021)

## **OINS-GMDH Training Data**

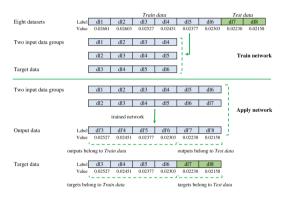


Figure: Train Network for GMDH

## **Outline**

#### 1. Abstract

- 1.1. Background and Gap Statement
- 1.2. Goals/Aims
- 1.3. Methodology
- 1.4. Results and Conclusion
- 1.5. Implications

# Methodology

OINS is established upon the core idea as follows: (1) Firstly, we predict the load factor increment and the displacement vector increment and the convergent solution of the considering load step via the predictive networks which are trained by using the load factor and the displacement vector increments of the previous convergence steps and group method of data handling (GMDH);(2) Thanks to the predicted convergence solution of the load step is very close to or identical with the real one, the prediction phase used in any existing nonlinear solvers is eliminated completely in OINS. Next, the correction phase of the M-R is adopted and the OINS iteration is started at the predicted convergence point to reach the convergent solution. The training process and the applying process of GMDH are continuously conducted and repeated during the nonlinear analysis in order to predict the convergence point at the beginning of each load step.

# Methodology

OINS is established upon the core idea as follows: (1) Firstly, we predict the load factor increment and the displacement vector increment and the convergent solution of the considering load step via the predictive networks which are trained by using the load factor and the displacement vector increments of the previous convergence steps and group method of data handling (GMDH);(2) Thanks to the predicted convergence solution of the load step is very close to or identical with the real one, the prediction phase used in any existing nonlinear solvers is eliminated completely in OINS. Next, the correction phase of the M-R is adopted and the OINS iteration is started at the predicted convergence point to reach the convergent solution. The training process and the applying process of GMDH are continuously conducted and repeated during the nonlinear analysis in order to predict the convergence point at the beginning of each load step.

# **Group Method of Data Handling**

The Group Method of Data Handling (GMDH) is a self-organizing deep learning method which is commonly and widely used for time series prediction problems. During the training stages, the number of neurons of GMDH networks continuously changes to improve the performance of the network.

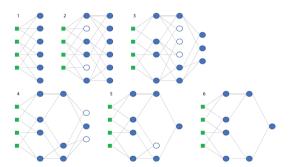


Figure: Training Process of GMDH

# Methodology

OINS is established upon the core idea as follows: (1) Firstly, we predict the load factor increment and the displacement vector increment and the convergent solution of the considering load step via the predictive networks which are trained by using the load factor and the displacement vector increments of the previous convergence steps and group method of data handling (GMDH);(2) Thanks to the predicted convergence solution of the load step is very close to or identical with the real one, the prediction phase used in any existing nonlinear solvers is eliminated completely in OINS. Next, the correction phase of the M-R is adopted and the OINS iteration is started at the predicted convergence point to reach the convergent solution. The training process and the applying process of GMDH are continuously conducted and repeated during the nonlinear analysis in order to predict the convergence point at the beginning of each load step.

### **Outline**

#### 1. Abstract

- 1.1. Background and Gap Statement
- 1.2. Goals/Aims
- 1.3. Methodology
- 1.4. Results and Conclusion
- 1.5. Implications

### **Results and Conclusion**

Through numerical investigations, we prove that OINS is powerful, highly accurate and only needs about one iteration per load step. Thus, OINS significantly saves number of iterations and a huge amount of computation compared with the conventional methods.

## The Star Players

The study compared the three nonlinear problem solvers namely:

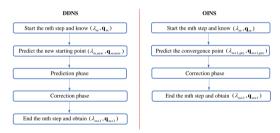
- 1. The M-R Method
- 2. The Data-driven Nonlinear Solver (DDNS) Method
- 3. The OINS Method

The study conducted an iterative comparison between these solvers showing their efficiency in solving nonlinear problem.

#### The DDNS Method

Data-driven Nonlinear Solver has been developed recently to reduce the number of iterations of nonlinear problems by having the same idea to that of OINS method: predict.

The difference between DDNS and OINS? The DDNS predicts the new starting point closer to the convergent solution while OINS predicts the converging solution which cuts a large iteration in the whole process.

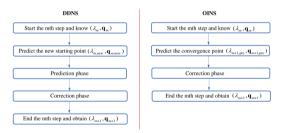


The DDNS method is thoroughly discussed in the study entitled "A novel data-driven nonlinear solver for solid mechanics using time series forecasting."

#### The DDNS Method

Data-driven Nonlinear Solver has been developed recently to reduce the number of iterations of nonlinear problems by having the same idea to that of OINS method: predict.

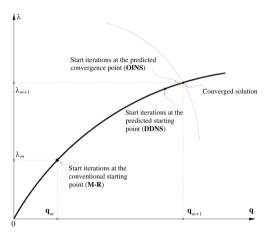
The difference between DDNS and OINS? The DDNS predicts the new starting point closer to the convergent solution while OINS predicts the converging solution which cuts a large iteration in the whole process.



The DDNS method is thoroughly discussed in the study entitled "A novel data-driven nonlinear solver for solid mechanics using time series forecasting."

## The Star Players

As a result, the study has shown that the OINS method was 77-80% faster than the M-R method and DDNS, based on literature, is 40-50% faster than M-R method.



# M-R vs OINS, continued

The Modified Riks and OINS method were tested using the problem of cylindrical panel.

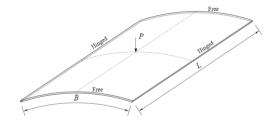
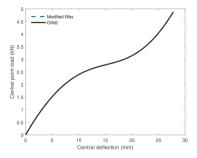


Figure: Cylindrical hinged panel under a point load  ${\cal P}$ 

### M-R vs OINS, continued

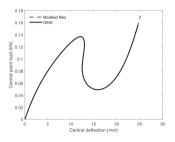
Central deflection of a hinged panel with h=25.4mm, B=508mm, L=B



Methods	Arc lengths	Total increments	Total iterations	Number of average iteration
Modified Riks	0.015	142	658	4.6
OINS			143	1.0
Modified Riks	0.012	177	816	4.6
OINS			177	1.0
Modified Riks	0.01	217	965	4.4
OINS			217	1.0

### M-R vs OINS, continued

Central deflection of a hinged panel with h=6.35mm, B=508mm, L=B



Methods	Arc lengths	Total increments	Total iterations	Number of average iteration
Modified Riks	0.025	72	336	4.7
OINS			72	1.0
Modified Riks	0.03	74	355	4.8
OINS			74	1.0
Modified Riks	0.035	62	299	4.8
OINS			62	1.0

### **Outline**

#### 1. Abstract

- 1.1. Background and Gap Statement
- 1.2. Goals/Aims
- 1.3. Methodology
- 1.4. Results and Conclusion
- 1.5. Implications

### **Impications**

Especially, OINS not only can detect limit, inflection, and other special points but also can predict exactly various types of instabilities of structures

#### Research Problem

It was noted that the solvers were applied to nonlinear problems in solid mechanics using the First-order Sheer Deformation Theory analysis of shells by the strain vectors. Given that the OINS method is proved to be efficient and accurate, we will attempt to apply the method to solve nonlinear problems in fluid mechanics by finding a set of parameters and equations that can be used in finite element analysis using OINS. Moreover, if possible, we will try to implement other deep learning method for time series prediction that might eliminate the corrective phase of the OINS method.

### **BibTex**

- [2] Data-driven Nonlinear Solver
- [1] NURBS-based postbuckling analysis.

#### References



L. A. N.-X. H. L. J. Nguyen TN, Thai CH. Nurbs-based postbuckling analysis of functionally graded carbon nanotube-reinforced composite shells, 2019.



L. J. Nguyen TN, Nguyen-Xuan H.

A novel data-driven nonlinear solver for solid mechanics using time series forecasting, 2020.