Stabilized Mixed Finite Element Formulation

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Definitions

F: Deformation gradient

I: second-order unit tensor

u: Displacement

J: determinant of the deformation gradient

C: Right Cauchy-Green Strain Tensor

 $\mathcal{W}(\mathbf{F})$: strain energy function

P: first Piola-Kirchhoff stress tensor

S: second PK stress tensor

 α : cracks are represented by a scalar phase-field variable

p: Lagrange multiplier, hydrostatic pressure field

 κ : bulk modulus

$$\kappa = \frac{E}{3(1 - 2\nu)} \tag{0.1}$$

 μ : shear modulus

$$\mu = \frac{E}{2(1+\nu)}\tag{0.2}$$

 λ : Lamé modulus

$$\lambda = \frac{\nu E}{(1+\nu)(1-2\nu)}\tag{0.3}$$

For Plane Stress

$$\kappa = \frac{3 - \nu}{1 + \nu}, \quad \lambda = \frac{E\nu}{(1 - \nu)^2} \tag{0.4}$$

 \mathcal{E}_{ℓ} : potential energy functional

 $a(\alpha)$ is the decreasing stiffness modulation function

 $w(\alpha)$ is an increasing function representing the specific energy dissipation per unit of volume c_w is a normalization constant

1 Hyperelastic Phase-Field Fracture Models

Deformation Gradient

$$\mathbf{F} = \mathbf{I} + \nabla \otimes \mathbf{u} \tag{1.1}$$

where $J = \det \mathbf{F}$ and $\mathbf{C} = \mathbf{F}^T \mathbf{F}$.

The strain energy function $\mathcal{W}(\mathbf{F})$ is defined per unit reference volume such that the first PK and second PK

$$\mathbf{P} = \frac{\partial \mathcal{W}(\mathbf{F})}{\partial \mathbf{F}} \tag{1.2a}$$

$$\mathbf{S} = 2\frac{\partial \mathcal{W}(\mathbf{F})}{\partial \mathbf{C}} \tag{1.2b}$$

where P = FS.

1.1 Phase-Field Fracture Model

Non-modified strain energy function is the compressible Neo-Hookean:

$$\mathcal{W}(\mathbf{F}) = \frac{\mu}{2}(I_1 - 3 - 2\ln J) \tag{1.3}$$

For incompressible hyperelastic materials, the strain energy function is defined as

$$\widetilde{\mathcal{W}}(\mathbf{F}) = \mathcal{W}(\mathbf{F}) + p(J-1),$$
 (1.4)

We instead consider a damage dependent relaxation of the incompressibility constraint and the phase-field couples to the strain energy through the modified function

$$\widetilde{\mathcal{W}}(\mathbf{F}, \alpha) = a(\alpha)\mathcal{W}(\mathbf{F}) + a^{3}(\alpha)\frac{1}{2}\kappa (J - 1)^{2}$$
(1.5)

where the decreasing stiffness modulation function is $a(\alpha)$ and $w(\alpha)$ is an increasing function representing the specific energy dissipation per unit of volume

$$a(\alpha) = (1 - \alpha)^2 \quad w(\alpha) = \alpha \tag{1.6}$$

In the code, we have the following definition

$$b(\alpha) = (1 - \alpha)^3 = \sqrt{a^3(\alpha)}$$

To circumvent numerical difficulties, we resort to the classical mixed formulation and introduce the pressure-like field

$$p = -\sqrt{a^3(\alpha)}\kappa (J - 1)$$

$$p = -b(\alpha)\kappa (J - 1)$$
(1.7)

as an independent variable along with the displacement field.

Following the code, we have the following energy functional of the energy problem

$$\widetilde{W}(\mathbf{F}, \alpha) = (a(\alpha) + k_{\ell})\mathcal{W}(\mathbf{F}) - b(\alpha)p(J-1) - \frac{p^{2}}{2\kappa}$$

$$\widetilde{W}(\mathbf{F}, \alpha) = (a(\alpha) + k_{\ell})\frac{\mu}{2}(I_{c} - 3 - 2\ln J) - b(\alpha)p(J-1) - \frac{p^{2}}{2\kappa}$$

The first Piola-Kirchhoff stress tensor is given:

$$\mathbf{P} = \frac{\partial \widetilde{W}(\mathbf{F}, \alpha)}{\partial \mathbf{F}} \\
= \frac{\partial}{\partial \mathbf{F}} \left[a(\alpha)W(\mathbf{F}) + a^{3}(\alpha) \frac{1}{2} \kappa (J - 1)^{2} \right] \\
= a(\alpha) \frac{\partial W(\mathbf{F})}{\partial \mathbf{F}} + a^{3}(\alpha) \frac{1}{2} \kappa \frac{\partial (J - 1)^{2}}{\partial \mathbf{F}} \\
= a(\alpha) \frac{\partial W(\mathbf{F})}{\partial \mathbf{F}} + a^{3}(\alpha) \kappa (J - 1) \frac{\partial J}{\partial \mathbf{F}} \quad \text{where } \frac{\partial J}{\partial \mathbf{F}} = J\mathbf{F}^{-T} \\
= a(\alpha) \frac{\partial W(\mathbf{F})}{\partial \mathbf{F}} + a^{3}(\alpha) \kappa (J - 1) J\mathbf{F}^{-T} \quad \text{substituting in pressure equation} \\
\mathbf{P} = a(\alpha) \mu(\mathbf{F} - \mathbf{F}^{-T}) - b(\alpha) p J\mathbf{F}^{-T}$$

1.1.1 Derivation from Bin2020 Paper

Here, unlike Eq. 21 from Bin2020, we drop λ_b which is not a consideration in this formulation

$$\mathcal{E}_{\ell}(\boldsymbol{u},\alpha) = \int_{\Omega} a(\alpha) \mathcal{W}(\mathbf{F},\alpha) d\Omega + \frac{G_c}{c_w} \int_{\Omega} \left(\frac{w(\alpha)}{\ell} + \ell \|\nabla \alpha\|^2 \right) d\Omega$$
 (1.9)

The normalization constant is defined as:

$$c_w = \int_0^1 \sqrt{w(\alpha)} d\alpha \tag{1.10}$$

Starting from Eq. 25 in the 2020 Li and Bouklas paper where κ is the bulk modulus

$$\mathcal{E}_{\ell}(\boldsymbol{u}, p, \Lambda, \alpha) = \mathcal{E}_{\ell}(\boldsymbol{u}, \alpha) + \int_{\Omega} \frac{p^2}{2\kappa} d\Omega + \int_{\Omega} \Lambda(p + \sqrt{a^3(\alpha)}\kappa(J - 1)) d\Omega$$
(1.11)

Identify the stationary point of the energy functional with respect to pressure (not Λ)

$$\frac{\partial \mathcal{E}_{\ell}}{\partial p} = \int_{\Omega} \frac{p}{\kappa} d\Omega + \int_{\Omega} \Lambda d\Omega$$
$$0 = \frac{p}{\kappa} + \Lambda \to \Lambda = -p/\kappa$$

Substituting this relationship into the energy functional yields:

$$\mathcal{E}_{\ell}(\boldsymbol{u}, p, \alpha) = \mathcal{E}_{\ell}(\boldsymbol{u}, \alpha) + \int_{\Omega} \frac{p^{2}}{2\kappa} d\Omega + \int_{\Omega} -\frac{p}{\kappa} (p + \sqrt{a^{3}(\alpha)}\kappa(J - 1)) d\Omega$$

$$= \mathcal{E}_{\ell}(\boldsymbol{u}, \alpha) + \int_{\Omega} \frac{p^{2}}{2\kappa} d\Omega - \int_{\Omega} \frac{p^{2}}{\kappa} d\Omega - \int_{\Omega} \frac{p}{\kappa} \sqrt{a^{3}(\alpha)}\kappa(J - 1)) d\Omega$$

$$\mathcal{E}_{\ell}(\boldsymbol{u}, p, \alpha) = \mathcal{E}_{\ell}(\boldsymbol{u}, \alpha) - \int_{\Omega} \frac{p^{2}}{2\kappa} d\Omega - \int_{\Omega} \sqrt{a^{3}(\alpha)}p(J - 1) d\Omega$$

$$(1.12)$$

Substitute in $\mathcal{E}_{\ell}(\boldsymbol{u}, \alpha)$ and substitute Eq. 1.3

$$\mathcal{E}_{\ell}(\boldsymbol{u}, p, \alpha) = \int_{\Omega} a(\alpha) \mathcal{W}(\mathbf{F}) d\Omega + \frac{G_c}{c_w} \int_{\Omega} \left(\frac{w(\alpha)}{\ell} + \ell \|\nabla \alpha\|^2 \right) d\Omega - \int_{\Omega} \frac{p^2}{2\kappa} d\Omega - \int_{\Omega} \sqrt{a^3(\alpha)} p(J-1) d\Omega$$

The prior equation includes the full weak form, unless we want to consider linear interpolation of all fields. In that case, we can introduce the stabilization term

$$-\frac{\varpi h^2}{2\mu} \sqrt{a^3(\alpha)} \sum_{e=1}^{n_{el}} \int_{\Omega^e} J\mathbf{C}^{-1} : (\nabla p \cdot \nabla q) \, dV = 0$$

1.2 Changes for 2D Plane-Stress Models

Recalling the 1st PK stress in Eq. 1.8.

$$\mathbf{P} = a(\alpha)\mu(\mathbf{F} - \mathbf{F}^{-T}) - b(\alpha)pJ\mathbf{F}^{-T}$$

In a plane-stress case, the P_{33} component is zero:

$$\mathbf{P}_{33} = a(\alpha)\mu(\mathbf{F}_{33} - \mathbf{F}_{33}^{-1}) - b(\alpha)pJ\mathbf{F}_{33}^{-1} = 0$$

This can be multiplied by its associated test function to obtain the weak form

$$\int_{\Omega} \left(a(\alpha) \mu(\mathbf{F}_{33} - \mathbf{F}_{33}^{-1}) - b(\alpha) p J \mathbf{F}_{33}^{-1} \right) v_{F_{33}} dV = 0$$

In the FEniCS code, we expand the solution space to include displacement, pressure, and a component of the deformation gradient $\mathbf{F_{33}}$. Therefore, we include a change to the invariants of the deformation tensors:

$$J = det(F)*F33$$

 $Ic = tr(C) + F33**2$

Together with the weak form from above:

$$F_u = derivative(elastic_potential, w_p, v_q) \\ + (a(alpha)*mu*(F33 - 1/F33) - b(alpha)*p*J/F33)*v_F33*dx$$

1.2.1 Changes for 2D Discrete Crack Model

If we are considering a discrete fracture method

$$\mathcal{E}_{\ell}(\boldsymbol{u}, p, \alpha) = \int_{\Omega} a(\alpha) \mathcal{W}(\mathbf{F}) d\Omega + \frac{G_c}{c_w} \int_{\Omega} \left(\frac{w(\alpha)}{\ell} + \ell \|\nabla \alpha\|^2 \right) d\Omega - \int_{\Omega} \frac{p^2}{2\kappa} d\Omega - \int_{\Omega} \sqrt{a^3(\alpha)} p(J-1) d\Omega$$

$$\mathcal{E}_{\ell}(\boldsymbol{u}, p, \alpha) = \int_{\Omega} \mathcal{W}(\mathbf{F}) d\Omega - \int_{\Omega} \frac{p^2}{2\kappa} d\Omega - \int_{\Omega} p(J-1) d\Omega$$

where we have assumed for the energy functional

$$\widetilde{\mathcal{W}}(\mathbf{F}, \alpha) = \frac{\mu}{2} (I_c - 3 - 2 \ln J) - p(J - 1) - \frac{p^2}{2\lambda}$$

Therefore, we can calculate the 1st Piola Kirchoff Stress as:

$$\mathbf{P} = \frac{\mu}{2} (2\mathbf{F} - \frac{2}{J}J\mathbf{F}^{-T}) - pJ\mathbf{F}^{-T}$$
$$= \mu(\mathbf{F} - \mathbf{F}^{-T}) - pJ\mathbf{F}^{-T}$$

Taking the third component to be zero

$$P_{33} = \mu(F_{33} - F_{33}^{-1}) - pJF_{33}^{-1} = 0$$
$$= F_{33} - F_{33}^{-1} - \frac{pJ}{\mu}F_{33}^{-1} = 0$$
$$P_{33} = F_{33}^{2} - 1 - \frac{pJ}{\mu} = 0$$

with the stabilization term and plane stress in the weak form

$$-\frac{\varpi h^2}{2\mu} \int_{\Omega} J \mathbf{C}^{-1} : \left(\nabla p \cdot \nabla q\right) dV = 0$$
$$\int_{\Omega} \left(\mathbf{F}_{33}^2 - 1 - \frac{pJ}{\mu}\right) v_{F_{33}} dV = 0$$

1.2.2 Changes for 2D displacement formulation

Removing pressure terms

$$\mathcal{E}_{\ell}(\boldsymbol{u}, p, \alpha) = \int_{\Omega} a(\alpha) \mathcal{W}(\mathbf{F}) d\Omega + \frac{G_c}{c_w} \int_{\Omega} \left(\frac{w(\alpha)}{\ell} + \ell \|\nabla \alpha\|^2 \right) d\Omega - \int_{\Omega} \frac{p^2}{2\kappa} d\Omega - \int_{\Omega} \sqrt{a^3(\alpha)} p(J-1) d\Omega$$

$$\mathcal{E}_{\ell}(\boldsymbol{u}, p, \alpha) = \int_{\Omega} a(\alpha) \mathcal{W}(\mathbf{F}) d\Omega + \frac{G_c}{c_w} \int_{\Omega} \left(\frac{w(\alpha)}{\ell} + \ell \|\nabla \alpha\|^2 \right) d\Omega$$

with plane stress in the weak form (no need for stabilization terms)

$$\int_{\Omega} \left(a(\alpha)\mu(F_{33} - F_{33}^{-1}) \right) v_{F_{33}} dV = 0$$

We have assumed the modified energy functional

$$\widetilde{W}(\mathbf{F},\alpha) = a(\alpha)\frac{\mu}{2}(I_c - 3 - 2\ln J)$$
(1.13)

Therefore, we can calculate the 1st Piola Kirchoff Stress as:

$$\mathbf{P} = a(\alpha) \frac{\mu}{2} (2\mathbf{F} - \frac{2}{J} J \mathbf{F}^{-T})$$
$$= a(\alpha) \mu (\mathbf{F} - \mathbf{F}^{-T})$$

Taking the third component to be zero

$$P_{33} = a(\alpha)\mu(F_{33} - F_{33}^{-1}) = 0$$

2 Obtaining the Critical Stretch

In order to obtain the critical stretch of a particular specimen in Krishnan 2008, we can equate our dissipated energy or critical fracture energy density

$$\int_{\Gamma} G_c d\mathbf{S} = \frac{G_c}{c_w} \int_{\Omega} \left(\frac{w(\alpha)}{\ell} + \ell \|\nabla \alpha\|^2 \right) d\Omega$$

to Eq. 15 for the energy release rate of the strip geometry.

$$J = h\mu \left(\lambda_a - \frac{1}{\lambda_a}\right)^2$$

where the total height of the strip is 2h and μ is the shear modulus

3 Strain energy decomposition

The Heaviside function is defined as

$$H(x) = \frac{x + |x|}{2x} = \begin{cases} 1, & x \ge 0, \\ 0, & x < 0 \end{cases}$$

The Macaulay bracket is defined

$$M(x) = \frac{x + |x|}{2} = \begin{cases} x, & x \ge 0, \\ 0, & x < 0 \end{cases}$$

3.1 Following Ye 2020 and Tang 2019

In the Ye 2020 paper, the internal energy is expressed as:

$$W_{int}(\mathbf{F}, \alpha, \nabla \alpha) = [a(\alpha) + k_{\ell}]W_{act} + W_{pas} + G_c\left(\frac{\alpha^2}{2\ell} + \frac{\ell}{2}|\nabla \alpha|^2\right)$$

Now in section 3.2.3 of Ye 2020, is stated the decomposition for a Mooney Rivlin constitutive law:

$$W_{MR}(I_1, I_2) = C_1(I_1 - 3) + C_2(I_2 - 3)$$

$$W_{MR}(\lambda_1, \lambda_1, \lambda_3) = C_1(\lambda_1^2 + \lambda_2^2 + \lambda_3^2 - 3) + C_2(\lambda_1^{-2} + \lambda_2^{-2} + \lambda_3^{-2} - 3)$$

$$= C_1 \sum_{i=1}^{3} (\lambda_i^2 - 1) + C_2 \sum_{i=1}^{3} (\lambda_i^{-2} - 1)$$

Which can be decomposed to active and passive internal energy terms. First we can rewrite:

$$W_{MR}(\lambda_1, \lambda_1, \lambda_3) = C_1 \sum_{i=1}^3 H(\lambda_i - 1)(\lambda_i^2 - 1) + C_2 \sum_{i=1}^3 H(\lambda_i - 1)(\lambda_i^{-2} - 1) + C_1 \sum_{i=1}^3 H(1 - \lambda_i)(\lambda_i^2 - 1) + C_2 \sum_{i=1}^3 H(1 - \lambda_i)(\lambda_i^{-2} - 1)$$

The active and passive terms can be stated as follows where the active part represents the crackdriven energy.

$$W_{act} = C_1 \sum_{i=1}^{3} H(\lambda_i - 1)(\lambda_i^2 - 1) + C_2 \sum_{i=1}^{3} H(\lambda_i - 1)(\lambda_i^{-2} - 1)$$

$$W_{pas} = C_1 \sum_{i=1}^{3} H(1 - \lambda_i)(\lambda_i^2 - 1) + C_2 \sum_{i=1}^{3} H(1 - \lambda_i)(\lambda_i^{-2} - 1)$$

One way to better understand these is to consider some cases 1) triaxial tension $\lambda_i > 1$ 2) other stress states where $\lambda_i < 1$:

For
$$\lambda_i > 1$$
:
$$W_{act} = C_1 \sum_{i=1}^{3} (\lambda_i^2 - 1) + C_2 \sum_{i=1}^{3} (\lambda_i^{-2} - 1)$$

$$W_{pas} = 0$$
For $\lambda_i < 1$:
$$W_{act} = 0$$

$$W_{pas} = C_1 \sum_{i=1}^{3} (\lambda_i^2 - 1) + C_2 \sum_{i=1}^{3} (\lambda_i^{-2} - 1)$$

For this second case, we end up with a negative energy component (first term of the passive energy).

We can also note the definitions within Tang 2019 for Model M_I . In this model, we consider the free energy density of a neo-Hookean constitutive law:

$$W(\mathbf{F}) = \frac{\mu}{2}(I_1 - 3 - 2\ln J) + \frac{\kappa}{2}(\ln J)^2$$

which can also be rephrased in terms of stretches

$$W(\lambda_1, \lambda_2, \lambda_3) = W_1 + W_2$$

= $\frac{\mu}{2} \sum_{i=1}^{3} (\lambda_i^2 - 1 - 2 \ln \lambda_i) + \frac{\kappa}{2} (\ln J)^2$

where we note that W_1 is a linear function of $\ln \lambda_i$ and W_2 is a nonlinear function of $\ln J$. The free energy is stated as

$$G_{rub} = [(1 - K)\alpha^2 + K]W^+ + W^-$$

Note that K is not κ . No definition is provided in the paper. This is another way of coupling the damage to the free energy density, and I believe we can rewrite our own version where:

$$G_{rub} = [a(\alpha) + k_{\ell}]W^{+} + W^{-}$$

Now we turn to the definition of W^+ and W^- which refers to the energy with tensile stretching

$$W^{+} = W(\lambda_{i}^{+}, J^{+}) = \frac{\mu}{2} \sum_{i=1}^{3} ((\lambda_{i}^{+})^{2} - 1 - 2 \ln \lambda_{i}^{+}) + \frac{\kappa}{2} (\ln J^{+})^{2}$$

and the energy with compression respectively.

$$W^{-} = W(\lambda_{i}^{-}, J^{-}) = \frac{\mu}{2} \sum_{i=1}^{3} ((\lambda_{i}^{-})^{2} - 1 - 2 \ln \lambda_{i}^{-}) + \frac{\kappa}{2} (\ln J^{-})^{2}$$

The definitions for these superscript + and - terms gives us

$$\lambda_{i}^{+} = \begin{cases} \lambda_{i}, & \lambda_{i} > 1, \\ 1, & \lambda_{i} \leq 1 \end{cases} \qquad J^{+} = \begin{cases} J, & J > 1, \\ 1, & J \leq 1 \end{cases}$$
$$\lambda_{i}^{-} = \begin{cases} \lambda_{i}, & \lambda_{i} < 1, \\ 1, & \lambda_{i} \geq 1 \end{cases} \qquad J^{-} = \begin{cases} J, & J < 1, \\ 1, & J \geq 1 \end{cases}$$

This isn't the definition for the heaviside function, but it could be a shifted Macaulay bracket

$$M_s(x) = \frac{x-1+|x-1|}{2} + 1 = \begin{cases} x, & x > 1, \\ 1, & x \le 1 \end{cases}$$

Now we can consider some examples.

For
$$J > 1$$
:

$$W^{+} = \frac{\mu}{2} \sum_{i=1}^{3} ((\lambda_{i}^{+})^{2} - 1 - 2 \ln \lambda_{i}^{+}) + \frac{\kappa}{2} (\ln J)^{2}$$

$$W^{-} = \frac{\mu}{2} \sum_{i=1}^{3} ((\lambda_{i}^{-})^{2} - 1 - 2 \ln \lambda_{i}^{-})$$

For J < 1:

$$W^{+} = \frac{\mu}{2} \sum_{i=1}^{3} ((\lambda_{i}^{+})^{2} - 1 - 2 \ln \lambda_{i}^{+})$$

$$W^{-} = \frac{\mu}{2} \sum_{i=1}^{3} ((\lambda_{i}^{-})^{2} - 1 - 2 \ln \lambda_{i}^{-}) + \frac{\kappa}{2} (\ln J)^{2}$$

If we consider the same stress states as in Ye2020, 1) triaxial tension $\lambda_i > 1$ 2) all other stress states $\lambda_i < 1$:

For
$$J > 1$$
, $\lambda_i > 1$:
$$W^+ = \frac{\mu}{2} \sum_{i=1}^3 (\lambda_i^2 - 1 - 2 \ln \lambda_i) + \frac{\kappa}{2} (\ln J)^2$$
$$W^- = 0$$

For $J < 1, \lambda_i > 1$:

$$W^{+} = \frac{\mu}{2} \sum_{i=1}^{3} (\lambda_i^2 - 1 - 2 \ln \lambda_i)$$
$$W^{-} = \frac{\kappa}{2} (\ln J)^2$$

Now for all other stress states:

For
$$J > 1$$
, $\lambda_i < 1$:
$$W^+ = \frac{\kappa}{2} (\ln J)^2$$

$$W^- = \frac{\mu}{2} \sum_{i=1}^3 (\lambda_i^2 - 1 - 2 \ln \lambda_i)$$
 For $J < 1$, $\lambda_i < 1$:
$$W^+ = 0$$

$$W^- = \frac{\mu}{2} \sum_{i=1}^3 (\lambda_i^2 - 1 - 2 \ln \lambda_i) + \frac{\kappa}{2} (\ln J)^2$$

This should be roughly equivalent to the considerations in the Ye2020 paper.

3.2 Our strain energy decomposition

Following the section above, we can consider our modified strain energy

$$\widetilde{W}(\mathbf{F},\alpha) = a(\alpha)\frac{\mu}{2}(I_c - 3 - 2\ln J) - b(\alpha)p(J - 1) - \frac{p^2}{2\kappa} \quad \text{where } p = -b(\alpha)\kappa(J - 1)$$

$$\widetilde{W}(\mathbf{F},\alpha) = a(\alpha)\frac{\mu}{2}(I_c - 3 - 2\ln J) + \frac{\kappa}{2}a(\alpha)^3(J - 1)^2$$

We can also rewrite the first term with regards to stretches

$$\widetilde{W}(\mathbf{F}, \alpha) = a(\alpha) \frac{\mu}{2} \sum_{i=1}^{3} (\lambda_i^2 - 1 - 2 \ln \lambda_i) + \frac{\kappa}{2} a(\alpha)^3 (J - 1)^2$$

Following Tang 2019 we rewrite the strain energy as

$$\widetilde{W}(\mathbf{F}, \alpha) = \widetilde{W}_{\text{act}}(\mathbf{F}, \alpha) + \widetilde{W}_{\text{pas}}(\mathbf{F}, \alpha)$$
 (3.1)

where the active and passive parts of the strain energy can be written as:

$$\widetilde{W}_{act}(\mathbf{F}, \alpha) = a(\alpha) \frac{\mu}{2} \sum_{i=1}^{3} ((\lambda_i^+)^2 - 1 - 2 \ln \lambda_i^+) + \frac{\kappa}{2} a(\alpha)^3 (J^+ - 1)^2$$

$$\widetilde{W}_{pas}(\mathbf{F}, \alpha) = a(\alpha) \frac{\mu}{2} \sum_{i=1}^{3} ((\lambda_i^-)^2 - 1 - 2 \ln \lambda_i^-) + \frac{\kappa}{2} a(\alpha)^3 (J^- - 1)^2$$

where the definitions of the superscript + and - terms remain the same as in Tang2020:

For J > 1:

$$\widetilde{W}_{act}(\mathbf{F}, \alpha) = a(\alpha) \frac{\mu}{2} \sum_{i=1}^{3} ((\lambda_i^+)^2 - 1 - 2 \ln \lambda_i^+) + \frac{\kappa}{2} a(\alpha)^3 (J - 1)^2$$

$$\widetilde{W}_{pas}(\mathbf{F}, \alpha) = a(\alpha) \frac{\mu}{2} \sum_{i=1}^{3} ((\lambda_i^-)^2 - 1 - 2 \ln \lambda_i^-)$$
For $J \le 1$:
$$\widetilde{W}_{act}(\mathbf{F}, \alpha) = a(\alpha) \frac{\mu}{2} \sum_{i=1}^{3} ((\lambda_i^+)^2 - 1 - 2 \ln \lambda_i^+)$$

$$\widetilde{W}_{pas}(\mathbf{F}, \alpha) = a(\alpha) \frac{\mu}{2} \sum_{i=1}^{3} ((\lambda_i^-)^2 - 1 - 2 \ln \lambda_i^-) + \frac{\kappa}{2} a(\alpha)^3 (J - 1)^2$$

Now considering the same two cases of, triaxial tension and

For $J > 1, \, \lambda_i > 1$:

$$\widetilde{W}_{act}(\mathbf{F}, \alpha) = a(\alpha) \frac{\mu}{2} \sum_{i=1}^{3} (\lambda_i^2 - 1 - 2 \ln \lambda_i) + \frac{\kappa}{2} a(\alpha)^3 (J - 1)^2$$

$$\widetilde{W}_{pas}(\mathbf{F}, \alpha) = 0$$

For $J \leq 1$, $\lambda_i > 1$:

$$\widetilde{W}_{act}(\mathbf{F}, \alpha) = a(\alpha) \frac{\mu}{2} \sum_{i=1}^{3} (\lambda_i^2 - 1 - 2 \ln \lambda_i)$$
$$\widetilde{W}_{pas}(\mathbf{F}, \alpha) = \frac{\kappa}{2} a(\alpha)^3 (J - 1)^2$$

all other cases:

For
$$J > 1$$
, $\lambda_i < 1$:

$$\widetilde{W}_{act}(\mathbf{F}, \alpha) = \frac{\kappa}{2} a(\alpha)^3 (J - 1)^2$$

$$\widetilde{W}_{pas}(\mathbf{F}, \alpha) = a(\alpha) \frac{\mu}{2} \sum_{i=1}^{3} (\lambda_i^2 - 1 - 2 \ln \lambda_i)$$

For $J \leq 1$, $\lambda_i < 1$:

$$\widetilde{W}_{act}(\mathbf{F}, \alpha) = 0$$

$$\widetilde{W}_{pas}(\mathbf{F}, \alpha) = a(\alpha) \frac{\mu}{2} \sum_{i=1}^{3} (\lambda_i^2 - 1 - 2 \ln \lambda_i) + \frac{\kappa}{2} a(\alpha)^3 (J - 1)^2$$

These can be concisely summarized with the following expressions, where the active part of the strain energy is

$$\widetilde{W}_{\text{act}}(\mathbf{F}, \alpha) = a(\alpha) \frac{\mu}{2} \sum_{i=1}^{3} H(\lambda_i - 1) \left(\lambda_i^2 - 1 - 2\ln \lambda_i\right) + a^3(\alpha) H(J - 1) \frac{1}{2} \kappa (J - 1)^2,$$
(3.3)

and the passive part of the strain energy is

$$\widetilde{W}_{\text{pas}}(\mathbf{F}, \alpha) = a(\alpha) \frac{\mu}{2} \sum_{i=1}^{3} H(1 - \lambda_i) \left(\lambda_i^2 - 1 - 2 \ln \lambda_i \right) + a^3(\alpha) H(1 - J) \frac{1}{2} \kappa (J - 1)^2,$$
 (3.4)

3.2.1 Compute the principal stretches λ_i

The eigenvalues of Cauchy-Green strain tensor C are λ_i^2 , i=1,2,3. With following definitions

$$d = \frac{Tr\mathbf{C}}{3}, \quad e = \sqrt{\frac{Tr(\mathbf{C} - d\mathbf{I})^2}{6}}, \quad f = \frac{1}{e}(\mathbf{C} - d\mathbf{I}), \quad g = \frac{\det f}{2}, \tag{3.5}$$

and assuming the eigenvalues satisfying $\lambda_3^2 \leq \lambda_2 \leq \lambda_1$, we could obtain (?)

$$\lambda_1^2 = d + 2e\cos\left(\frac{\arccos g}{3}\right), \quad \lambda_3^2 = d + 2e\cos\left(\frac{\arccos g}{3} + \frac{2\pi}{3}\right), \quad \lambda_2^2 = 3d - \lambda_1^2 - \lambda_3^3.$$
 (3.6)

3.3 Hybrid Formulation

The principal stretches can be computed as shown above, but for spherical stretch ($\mathbf{C} = constant\mathbf{I}$) leading to NaN error. This means that for 3D strain decomposition using the explicit eigenvalue formulation, the computation of the first variation and second variation are nontrivial. FEniCS auto-differential function cannot detect these special cases.

The workaround is to consider the Hybrid model in Ambati 2015: A review on phase-field models of brittle fracture and a new fast hybrid formulation.

$$\sigma(\mathbf{u}, \alpha) = (1 - \alpha)^2 \frac{\partial W(\epsilon)}{\partial \epsilon}$$
$$-l^2 \nabla^2 \alpha + \alpha = \frac{2l}{G_c} (1 - \alpha) \mathcal{H}^+$$

Again, we consider our modified strain energy

$$\widetilde{W}(\mathbf{F},\alpha) = a(\alpha)\frac{\mu}{2}(I_c - 3 - 2\ln J) + \frac{\kappa}{2}a(\alpha)^3(J - 1)^2$$

Then the active and

$$\widetilde{W}_{\text{act}}(\mathbf{F}, \alpha) = a(\alpha) \frac{\mu}{2} \sum_{i=1}^{3} H(\lambda_i - 1) \left(\lambda_i^2 - 1 - 2\ln \lambda_i\right) + a^3(\alpha) H(J - 1) \frac{1}{2} \kappa (J - 1)^2,$$
(3.7)

the passive part of the strain energy is

$$\widetilde{W}_{\text{pas}}(\mathbf{F}, \alpha) = \frac{\mu}{2} \sum_{i=1}^{3} H(1 - \lambda_i) \left(\lambda_i^2 - 1 - 2 \ln \lambda_i \right) + H(1 - J) \frac{1}{2} \kappa (J - 1)^2,$$
(3.8)

4 Following Borden: Derivations of Analytical Phase Field

Note the full potential energy functional, which can also be called the lagrangian

$$\mathcal{E}_{\ell}(\boldsymbol{u}, p, \alpha) = \int_{\Omega} a(\alpha) \mathcal{W}(\mathbf{F}) d\Omega - \int_{\Omega} \sqrt{a^{3}(\alpha)} p(J-1) d\Omega - \int_{\Omega} \frac{p^{2}}{2\kappa} d\Omega + \frac{G_{c}}{c_{w}} \int_{\Omega} \left(\frac{w(\alpha)}{\ell} + \ell \|\nabla \alpha\|^{2} \right) d\Omega$$

We can use the Euler-Lagrange equations to arrive at the equations of motion by taking the derivative with respect to displacement, pressure, and the scalar damage field. Starting with displacement:

$$\frac{\partial \mathcal{E}_{\ell}}{\partial \mathbf{u}} = \int_{\Omega} a(\alpha) \frac{\partial \mathcal{W}(\mathbf{F})}{\partial \mathbf{u}} d\Omega$$

$$\frac{\partial \mathcal{E}_{\ell}}{\partial p} = -\int_{\Omega} \frac{p}{\kappa} d\Omega - \int_{\Omega} \sqrt{a^3(\alpha)} (J-1) d\Omega$$

$$\frac{\partial \mathcal{E}_{\ell}}{\partial \alpha} = -\int_{\Omega} 2(1-\alpha) \, \mathcal{W}(\mathbf{F}) \, d\Omega + \int_{\Omega} 3p(1-\alpha)^2 (J-1) d\Omega + \frac{G_c}{c_w} \int_{\Omega} \left[\frac{1}{\ell} + 2\ell \nabla^2 \alpha \right] d\Omega$$

Therefore we have three equations: First is mechanical eq,

$$\frac{\partial \mathcal{W}(\mathbf{F})}{\partial u_i} = 0$$

$$\frac{\partial}{\partial x_j} \left(\frac{\partial \mathcal{W}}{\partial \epsilon_{ij}} \right) = 0$$

$$\frac{\partial}{\partial x_j} = 0$$

Second is an equation for pressure,

$$-\frac{p}{\kappa} - \sqrt{a^3(\alpha)}(J-1) = 0$$
$$-\frac{p}{\kappa} - (1-\alpha)^3(J-1) = 0$$
$$-\kappa(J-1)(1-\alpha)^3 = p$$

Lastly,

$$-2(1-\alpha)\mathcal{W}(\mathbf{F}) + 3p(1-\alpha)^2(J-1) + \frac{G_c}{c_w} \left[\frac{1}{\ell} + 2\ell\nabla^2\alpha \right] = 0$$

Substitute second equation into third

$$-2(1-\alpha)\mathcal{W}(\mathbf{F}) - 3\kappa(1-\alpha)^5(J-1)^2 + \frac{G_c}{c_w} \left[\frac{1}{\ell} + 2\ell\nabla^2\alpha \right] = 0$$

4.1 Homogeneous Solution

We can study the homogeneous solution by ignoring spatial derivatives of α . If we don't substitute p:

$$-2(1-\alpha_h)\mathcal{W}(\mathbf{F}) + 3p(1-\alpha_h)2(J-1) + \frac{G_c}{c_w\ell} = 0$$

or if we substitute pressure

$$-2(1-\alpha_h)\mathcal{W}(\mathbf{F}) - 3\kappa(1-\alpha_h)^5(J-1)^2 + \frac{G_c}{c_w\ell} = 0$$

4.2 Non-Homogeneous Solution

Now for the Non-homogenous solution, we have the following

$$-2(1-\alpha)\mathcal{W}(\mathbf{F}) + 3p(1-\alpha)^2(J-1) + \frac{G_c}{c_w} \left[\frac{1}{\ell} + 2\ell \nabla^2 \alpha \right] = 0$$

Multiply by $d\alpha/dx$

$$\frac{d\alpha}{dx} \left[-2(1-\alpha) \mathcal{W}(\mathbf{F}) + 3p(1-\alpha)^2 (J-1) + \frac{G_c}{c_w} \left(\frac{1}{\ell} + 2\ell \nabla^2 \alpha \right) \right] = 0$$

$$\frac{d}{dx} \int \left[-2(1-\alpha) \mathcal{W}(\mathbf{F}) + 3p(1-\alpha)^2 (J-1) + \frac{G_c}{c_w} \left(\frac{1}{\ell} + 2\ell \nabla^2 \alpha \right) \right] d\alpha = 0$$

$$\frac{d}{dx} \left[(1-\alpha)^2 \mathcal{W}(\mathbf{F}) - p(1-\alpha)^3 (J-1) + \frac{G_c}{c_w} \left(\frac{\alpha}{\ell} + 2\ell \nabla^2 \alpha \right) \right] = 0$$

now integrate from x to infinity

$$\left[(1 - \alpha)^2 \mathcal{W}(\mathbf{F}) - p(1 - \alpha)^3 (J - 1) + \frac{G_c}{c_w} \left(\frac{\alpha}{\ell} + 2\ell \nabla^2 \alpha \right) \right]_0^{\infty} = 0$$

$$(1 - \alpha)^2 \mathcal{W}(\mathbf{F}) - p(1 - \alpha)^3 (J - 1) + \frac{G_c}{c_w} \left(\frac{\alpha}{\ell} + 2\ell \nabla^2 \alpha \right)$$

$$- \left[(1 - \alpha_h)^2 \mathcal{W}(\mathbf{F}) - p(1 - \alpha_h)^3 (J - 1) + \alpha_h \frac{G_c}{c_w \ell} \right] = 0$$

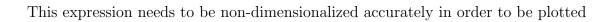
with some rearrangement we can call the bracketed section

$$a_{hom} = (1 - \alpha_h)^2 \mathcal{W}(\mathbf{F}) - p(1 - \alpha_h)^3 (J - 1) + \alpha_h \frac{G_c}{c_{col}}$$

$$\tag{4.1}$$

which can yield an expression that can solve for the phase field profile

$$-(1-\alpha)^2 \mathcal{W}(\mathbf{F}) + p(1-\alpha)^3 (J-1) - \frac{G_c}{c_w} \frac{\alpha}{\ell} + \left[a_{hom} \right] = 2\ell \nabla^2 \alpha \frac{G_c}{c_w}$$
$$\frac{c_w}{2\ell G_c} \left[-(1-\alpha)^2 \mathcal{W}(\mathbf{F}) + p(1-\alpha)^3 (J-1) \right] - \frac{\alpha}{2\ell^2} + \frac{c_w}{2\ell G_c} \left[a_{hom} \right] = \frac{d^2 \alpha}{dx^2}$$



(4.2)