

Representation of Heterogeneity in “Single Collector Efficiency” Equation for Multi Walled Carbon Nanotubes

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Abstract- Carbon nanotubes (type of engineered nanoparticles) are identified as a group of new and emerging contaminants. Because of their unique characteristics, novel models need to be developed in order to forecast their transport and fate in the saturated porous media. The movement of nanoparticles through saturated porous media has been previously modelled by clean-bed filtration theory. In this theory single collector efficiency (SCE) evaluates the fraction of transported particles that come into contact with the collector grain and can be removed from the aqueous phase. This equation assumes spherical geometry for nanoparticles. This assumption was identified as a short coming concerning carbon nanotubes (CNTs). To address this limitation, single collector efficiency equation was modified to accommodate the cylindrical shape of Multi-Walled Carbon Nanotubes (MWCNT), however, the underlying assumption for this empirical model is uniformity and homogeneity of the porous media. In this paper, further modification of the above-mentioned equation is suggested in order to represent the heterogeneity of a natural porous media through replacing the “collector diameter”. This was achieved through utilising field measured properties such as hydraulic conductivity, porosity, and grain-size distribution. The resulting equation overcomes the limitations of current approaches and shows remarkable agreement with exact theoretical predictions of the single collector efficiency over a range of conditions commonly encountered in natural groundwater systems. Furthermore, the theoretical effect of natural heterogeneity on the movement of CNTs in saturated porous media is assessed. It was established that the use of an average grain size can greatly over-estimate the movement of CNTs while representing heterogeneity through the modified equation reduces the modelled mobility of CNTs. In addition, increasing heterogeneity (smaller uniformity coefficient) resulted in mobility reduction for CNTs.

Keywords: Groundwater Contamination, Heterogeneity, Carbon Nanotube, Filtration Theory, Single Collector Efficiency, Groundwater Modelling, Exposure Assessment, CNT.

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1. Introduction

Rapid developments in nanotechnology and ever-increasing volumes of engineered nanomaterials, together with largely neglected risk-based studies regarding the potential harm done by these particles have become a concern for communities (Maynard et al., 2006). Production of these particles has led to their release in the urban and subsequently the natural environment (Nowack and Bucheli, 2007).

Very few studies have explored the movement of engineered nanoparticles in natural aquatic environments including -but not limited to- saturated soil (Li et al., 2008). However, based on information for other processes, it is likely that the behaviour of these materials will be different from non-particulate contaminants and hence new models and paradigms need to be developed for engineered nanoparticles in the saturated soil environment (Boxall, 2012).

Studies have explored the uptake and effects of some nanomaterials on a range of species (Oberdorster, 2004; Lovern and Klaper, 2006; Oberdorster et al., 2006; Kashiwada, 2006). The environmental impacts of

nanomaterials seem to be determined by a range of characteristics including dissolution potential, aggregation potential, particle surface properties, the characteristics of the exposure environment, the level, and the frequency of exposure (Dhawan *et al.*, 2006, Rogers *et al.*, 2007).

Carbon nanotubes (CNTs) are a group of engineered nanoparticles which has attracted significant attention in the past decade. Because of their unique and distinctive characteristics, various types of CNTs have been used in actuators, sensors, composites, paints, coatings, biological agents, electronics and other applications (Balasubramanian and Burghard, 2005).

These particles were shown to form stable dispersions in aquatic environments in the presence of natural organic matter (NOM) and travel for potentially long distances. This behaviour highlights the importance of predictive modelling in order to forecast the fate and transport of CNTs in the environment.

Empirical governing equations have been developed to explain and imitate break-through curves (BTCs) for CNTs. While these equations have been effectively used in laboratory-based studies (using well characterised and perfectly sorted porous media), they offer little value for real-world modelling of CNT transport through a heterogeneous natural porous media.

Single collector efficiency (SCE) is a key element in filtration theory as it represents the potential and consequence of contacts between the particles and the porous medium's single grains. This work is dedicated to developing a modified equation for single collector efficiency, in which heterogeneity of the porous medium is taken into account to model the transport of multi-walled carbon nanotubes (MWCNTs) in natural saturated porous media. Modification is pursued through replacing 'collector diameter' by a term inclusive of heterogeneity, using permeability measurements and grain size distribution.

The relationship between grain size and hydraulic characteristics is investigated and several methods of estimating hydraulic conductivity based on grain size, and grain size distribution are evaluated. Subsequently, the most suitable and versatile equation is selected based on the modelling criteria, theoretical value, data availability, and the purpose of the study. A modified equation is developed, and the results are compared with an empirical equation Liu *et al.* (2009) investigated based on laboratory experiments, to explain MWCNTs break through curves.

The resulting equation can be used for exposure assessment and transport modelling studies for CNTs as well as any other cylindrical nanoparticle through a natural heterogeneous saturated porous media.

2. Mobility and Toxicity of MWCNTs

Carbon nanotubes are highly hydrophobic and therefore have very low water solubility (Lam *et al.* 2004); however they can form stable dispersions in water in the presence of

organic matter. Hyung *et al.* (2007) studied the stability of MWCNTs in the Suwannee River (Georgia USA) where these nanoparticles were in the form of dispersed individuals and remained stable for over a month. Their findings suggested that MWCNTs in the natural, aqueous environment might occur to an unexpected extent.

This study establishes the basis for considering a mechanism, previously neglected in environmental fate and transport of CNTs in surface water as well as groundwater bodies. For instance, in a typical sandstone shallow aquifer with a hydraulic conductivity between 30 to 300 mm per hour (approximately 0.7 to 7 meters per day) a single pulse of these nanoparticles will travel between 20 to 200 meters in the first month in the presence of organic matter. This value is swiftly increased in the case of a sandy, costal aquifer where hydraulic conductivity reaches 40 to 45 meters per day. Fractured systems provide another example where the distances travelled by dispersed CNTs can substantially increase over a short period of time.

The cytotoxicity of carbon nanotubes has been reviewed by Hussain *et al.* (2009) who observed that invariably, CNTs are toxic to cells when used as a suspension in cell culture media in any given experiment. In contrast, they appear as nontoxic to a matrix or to a culture dish if immobilized. The findings of Cui *et al.* (2005), Davoren *et al.* (2007), Ding *et al.* (2005), and Raja *et al.* (2007) are in agreement with those of Hussain *et al.* (2009) on the adverse effects of CNTs on various human cells. A recent study by Poland *et al.* (2008) on mice suggested asbestos-like pathogenicity, however it is premature to conclude that nanotubes should be considered to have a toxicological profile similar to asbestos (Pulskamp *et al.*, 2007, Kang *et al.*, 2007). In the future therefore, we need to bring the exposure and effects studies closer together in order to determine whether or not nanomaterials can pose a risk to humans and the environment (Tiede *et al.*, 2008; SCENIHR, 2007).

2. 1. Modified MWCNT Single Collector Efficiency

Classic filtration theory, also known as clean-bed theory was primarily developed to evaluate the efficiency of man-made filtration systems. However in principle it could be -and indeed has been- used to simulate the movement of colloids (suspended particles) in saturated porous media.

This theory assumes spherical collector (grains of the porous media) and spherical particle geometries. Based on filtration theory Eq. (1), a fraction of transported particles which come into contact with a collector (grain) can be removed from the aqueous phase. The assumed three mechanisms through which a particle comes into contact with a collector are; inertial impaction, settling due to gravity and diffusion.

$$\eta_0 = \eta_I + \eta_G + \eta_D \quad (1)$$

where η_i , η_g , and η_d represent theoretical values for single collector efficiency (SCE) when the only transport mechanisms are interception, sedimentation, and diffusion respectively.

Particle shape was shown to have a major influence on filtration, hence this equation was modified to accommodate the cylindrical shape of CNTs (Yao, 1968 and Yao et al. 1971, Liu et al. 2009). In this equation, η_0 is calculated by Eq. (2) assuming all [significant] contacts are side contact.

$$\eta_0 = \left[\frac{1}{2} \left(\frac{d_p}{d_c} \right)^2 \left(3 - \frac{d_p}{d_p + d_c} \right) \right] + \eta_G + \eta_D \quad (2)$$

where

$$\eta_G = \left(\frac{d_p}{l} \right)^{\frac{2}{3}} \left(\frac{0.146(\rho_p - \rho)g \left(\frac{d_p^2}{l} \right)^{\frac{2}{3}}}{\mu v_0 \left[1 - \left(\frac{d_p}{l} \right)^2 \right]^{\frac{1}{2}}} \right) \ln \left(\frac{1 + \left(1 - \left(\frac{d_p}{l} \right)^2 \right)^{\frac{1}{2}}}{\frac{d_p}{l}} \right) \quad (2a)$$

and

$$\eta_D = 4.03 \left[kT \ln \left(\frac{1 + \left(1 - \left(\frac{d_p}{l} \right)^2 \right)^{\frac{1}{2}}}{\frac{d_p}{l}} \right) \right]^{\frac{2}{3}} \times \left(\frac{3\pi\mu l_c v_0 \left(\frac{3}{2} \frac{d_p^2}{l} \right)^{\frac{1}{3}} \left(\frac{l}{d_p} \right)^{\frac{2}{3}} \left(1 - \left(\frac{d_p}{l} \right)^2 \right)^{\frac{1}{2}}}{\left(\frac{d_p}{l} \right)^{\frac{2}{3}}} \right)^{-\frac{2}{3}} \quad (2b)$$

where d_p and l are particle diameter and length respectively, d_c is collector (grain) diameter, ρ_p is particle density, ρ is the fluid density, μ is the fluid viscosity, v_0 is fluid velocity, T is the absolute temperature, and k is the Boltzmann constant.

3. Representation of Heterogeneity

While Eq. (2) was shown to provide improved results for CNT break through curves in homogeneous soil columns, it fails to represent the heterogeneity of a natural porous medium. Previous colloidal movement modelling studies suggest that colloid size, heterogeneity of the porous media and attachment efficiency are significant in colloid mobility (Sun et al., 2001, Bhattacharjee et al., 2002, Jaisi et al., 2008). Complex modelling of colloids has been done by a number of researchers (Maxwell et al., 2003 and 2007, Scheibe, 2007, Tufenkji et al. 2003) where the results showed much improvement following the utilization of field-measured hydraulic conductivity data and a better representation of field natural heterogeneities. These studies show the importance of representation of heterogeneity for modelling the transport of colloids and nanocolloids such as CNTs.

In order to represent the heterogeneity of the porous media, soil measurements such as grain size distribution, porosity, and hydraulic conductivity can be used to replace the grain size (d_c or collector diameter) in Eq. (2) and Eq. (2b). Note that assigning a single value to d_c implies a homogeneous and uniform porous media in which all grains are uniformly sized.

3.1. Grain Size Distribution and Hydraulic Conductivity

The relationship between grain size distribution and hydraulic conductivity has been studied by numerous scientists and there have been, therefore, many attempts in order to offer a robust relationship between grain size distribution and hydraulic conductivity. Some of the more commonly used methods and theories are discussed subsequently.

In 1893 Hazen developed a relationship between hydraulic conductivity and grain size distribution. His method has been vastly used since, however, many ensuing studies refined and improved his methods and extended the limitations of his equation. Kozeny (1927), Carmen (1937), Masch and Denny (1966), Uma et al. (1989), Alyamani and Sen (1993), and many more have developed empirical equations, all which performs best under a given set of assumed conditions. Vukovic and Soro (1992) have summarised several methods from these studies and have commented on each one's limitations and inherent assumptions. Most of these equations apply to granular aquifers and include the grain size distribution of the component particles from mechanical sieving. The grain diameter is d_x , where x is the size fraction (by weight) finer than a given sieve size. The uniformity coefficient, C_u , is defined as the ratio of d_{60} to d_{10} and is a measure of the distribution's spread.

Hazen's equation (Eq. (3)) can be applied when effective diameter (d_{10}) is between 0.1 and 3 mm and C_u is less than 5.

$$K = C d_{10}^2 (0.70 + 0.03T) \quad (3)$$

where:

K is hydraulic conductivity (m/day)

C is Hazen Coefficient calculated based on porosity [Eq. (4)]

d_{10} is effective grain diameter (mm).

T is temperature in °C

$$C = 400 + 40(n - 26) \quad (4)$$

where:

n is porosity as a percentage (not as a fraction).

For a typical groundwater sample with a temperature of 10 °C, Eq. (3) can be re-written as:

$$K = Cd_{10}^2 \quad (5)$$

Eq. (5) is also known as the Beyer equation when temperature is not taken into consideration. In the Beyer equation hydraulic conductivity is estimated in m/s units and

C is calculated as:

$$C = (4.5 \times 10^{-3}) \log \left[\frac{500}{C_{\mu}} \right] \quad (6)$$

The Kozeny-Carman equation (Eq. (7)) also considers porosity as well as the effective grain diameter:

$$K = 5400 \frac{n^3}{(1-n)^2} d_{10}^2 \quad (7)$$

Alyamani and Sen (1993) introduced a new variable to their equation: *intercept* (I_o) is the value at which no material passes from the set of sieves. This value is usually very small and theoretically, equal to or less than d_{10} . Intercept has a direct relationship with hydraulic conductivity. They also suggested a relationship between hydraulic conductivity and the slope which defines the rate of grain diameter change within the sample. They derived a relationship as:

$$K = 1300[I_o + 0.025(d_{50} - d_{10})]^2 \quad (8)$$

The use of Intercept makes Alyamani-Sen equation applicable to a much wider range of soil types with various size distribution properties.

3.2. Modified Equation to Represent Heterogeneity

In a homogeneous porous medium such as those used in the soil column laboratory experiments of Liu et al. (2009), d_{10} is very similar to d_{50} and d_{60} as a result of near-perfect

sorting in preparations. Hence, C_{μ} tends towards 1 and any of the given equations can be rewritten with d_{10} , d_{50} , and d_{60} replaced by a single value for the grain size (collector diameter).

Using d_{50} as the representative average grain-size, will under-estimate collector efficiency. To further demonstrate this concept ten characterised soil samples from Australia were used (Table 1). The SCE for each sample was calculated using d_{50} based on the common practice of taking the average value as a representative measurement, followed by calculations using d_{10} as the most frequently used “effective grain size” reported in the literature.

Note that using d_{50} virtually assumes a larger grain size as a representative diameter. On the other hand, it is in fact the finer grains that control the hydraulic conductivity (and hence the ease of water flow) of a porous medium. It's also observed that SCE shows a very clear inverse relationship with hydraulic conductivity (plotted on a secondary axis) when effective grain size (d_{10}) is used (Fig. 1).

Table 1. Characterised Australian soil samples*.

Sample	Hydraulic Conductivity (m/d)	d_{10}^{**}	d_{50}	d_{60}	St. Dev.	C_{μ}	Porosity (n)***
1	950.4	0.91	1.04	1.07	0.11	1.18	0.46
2	864	0.65	0.75	0.79	0.11	1.22	0.46
3	777.6	0.54	1.75	1.97	0.63	3.65	0.38
4	172.8	0.33	0.46	0.48	0.08	1.45	0.45
5	86.4	0.32	0.36	0.37	0.04	1.16	0.46
6	69.1	0.24	0.29	0.3	0.03	1.25	0.46
7	25.9	0.18	0.45	0.51	0.56	2.83	0.41
8	17.3	0.18	0.37	0.45	0.4	2.5	0.42
9	17.3	0.19	0.57	0.94	0.69	4.95	0.36
10	17.3	0.16	0.21	0.22	0.03	1.38	0.45

* Data taken from Alyamani and Sen (1993)

** All diameters are in mm

*** Porosity of each sample was calculated from the Vukovic and Soro (1992) equation; $n = 0.255 (1 + 0.83C_{\mu})$

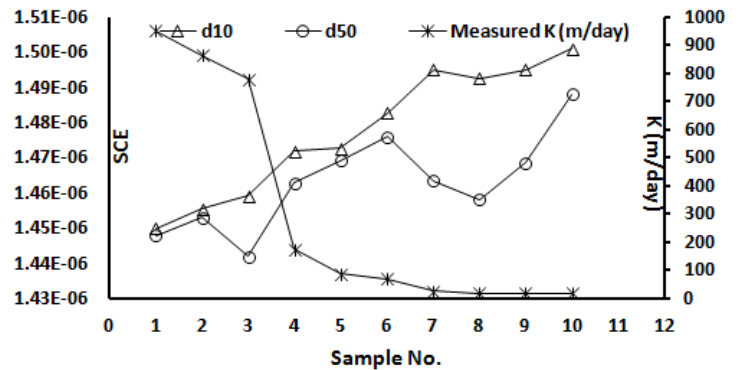


Fig. 1. SCE calculated based on d_{10} and d_{50} for 10 heterogeneous samples.

Assuming a poorly sorted (well graded), homogeneous porous medium at 10 °C, each one of Equations 5 (for both Hazen and Beyer), 7, and 8 can be rearranged as Equations 9, 10 and 11 in terms of d_c (equivalent to d_{10}).

The Hazen equation (Eq. (5)) at 10 °C can be expressed as:

$$K = (400 + 40.(n - 26))d_{10}^2$$

$$\Rightarrow d_{10} = d_c = \sqrt{\frac{K}{400 + 40(n - 26)}} \quad (9)$$

The Beyer equation (Eq. (5)) can be rearranged as:

$$K = 4.5 \times 10^{-3} \log \frac{500}{C_\mu} \times d_{10}^2$$

$$\Rightarrow d_{10} = d_c = \sqrt{\frac{K}{4.5 \times 10^{-3} \log \frac{500}{C_\mu}}} \quad (10)$$

The Kozeny-Carman equation becomes:

$$K = 5400 \frac{n^3}{(1 - n)^2} d_{10}^2$$

$$\Rightarrow d_{10} = d_c = \sqrt{\frac{K(n - 1)^2}{5400n^3}} \quad (11)$$

The Alyamani and Sen equation (Eq. (8)) does not consider porosity: instead it is concerned with the difference between the mean and effective grain sizes. This difference defines a slope which, in turn, defines the *intercept* (I_0) at the plot origin. In a well-sorted (homogeneous) porous medium, the trend line between d_{10} and d_{50} is essentially a vertical line parallel to Y axis. In such a case I_0 , d_{10} , and d_{50} will have the same value as d_c .

Hence d_c can be expressed as:

$$K = 1300[I_0 + 0.025(d_{50} - d_{10})]^2$$

$$\Rightarrow d_{10} = d_c = \frac{-\sqrt{\frac{K}{1300}} + I_0 + d_{50}}{0.025} \quad (12)$$

Each one of these replacement equations offers a different level of representation for the natural heterogeneity

of the porous media. In order to evaluate the deviation from the original theoretical equation (Eq. (2)), d_c has been replaced by Eqs. (9)-(12) in Eq. (2) for a set of soil samples. Table 2 shows a list of various scenarios for which these calculations were carried out. In these scenarios, grain size changes between 0.001 and 1 millimetre. These scenarios cover a range of very fine to coarse grain size.

By developing these virtual scenarios the deviation of each method's result can be calculated from the theoretical value (calculated by Eq. (2)) selected method can be then evaluated using laboratory measurements of factual samples.

Table. 2. Validation scenarios.

Collector Diameter (mm)	Hazen K (m/day)	Beyer K (m/day)	Kozeny-Carman K (m/day)	Alyamani-Sen K (m/day)
0.001	0.0012	1.049359538	1.802518519	0.0013
0.01	0.12	10.49359538	18.02518519	0.130
0.1	12	104.9359538	180.2518519	13.00
1	1200	1049.359538	1802.518519	1300

For all scenarios particle diameter, particle length, temperature, and all other relevant parameters are the same and the uniformity coefficient is always 1 as the theoretical material is assumed to be homogeneous for validation purposes.

Table 3 contains the results of the SCE calculations for each method and the standard deviation from the original equation (Eq. (2)). Results presented in Table 3 strongly favour the Alyamani-Sen equation which has replicated the SCE calculated by Eq. (2) most closely. Based on this validation of the various options available, we adapted this equation for up-scaling Eq. (2).

Table 3. Single Collector Efficiency (SCE) calculation results and their standard deviation from Eq. (2).

Grain Size	Eq. (2)	Hazen	Beyer	Kozeny-Carman	Alyamani-Sen
0.001	7.68E-5	1.047E+1	2.41E-1	5.49E+0	3.50E-6
0.01	2.61E-6	1.04E-1	2.41E-2	5.36E-1	1.87E-6
0.1	1.53E-6	1.05E-3	2.41E-3	5.50E-2	1.52E-6
1	1.44E-6	1.19E-5	2.43E-4	5.50E-3	1.44E-6

The values of SCE for the previously mentioned Australian soil samples have been compared between Eq. (2) -with the limitation of assuming d_{50} or d_{10} as d_c and hence neglecting the heterogeneity- and the proposed modified equation where the Alyamani-Sen equation (Eq. (12)) has

replaced d_c in Eq. (2). Figure 2 illustrates the results of this comparison.

The Alyamani-Sen equation offers an even more interesting observation. Despite following a general inverse relationship with hydraulic conductivity, it estimates SCEs below and above those calculated based on d_{10} only. This is believed to be due to other characteristics of the soil such as uniformity, porosity, effective porosity, pore connectivity, and size distribution.

Note that the Hazen, Beyer, and Kozeny-Carman equations calculated values of approximately 1, 2, and 3 orders of magnitude larger than those calculated by Alyamani and Sen. The Hazen equation has been recommended for conditions under which $0.1 < d_{10} < 3$ mm and $C_u < 5$ (Vukovic and Soro, 1992). While these conditions are true for all ten soil samples tested in this paper, the Hazen method predicts much higher values for SCE.

Alyamani-Sen SCE values seem to increasingly deviate from theoretical values by decreasing grain size. However, this equation continually offers the closest predictions to the theoretical calculations based on Eq. (2).

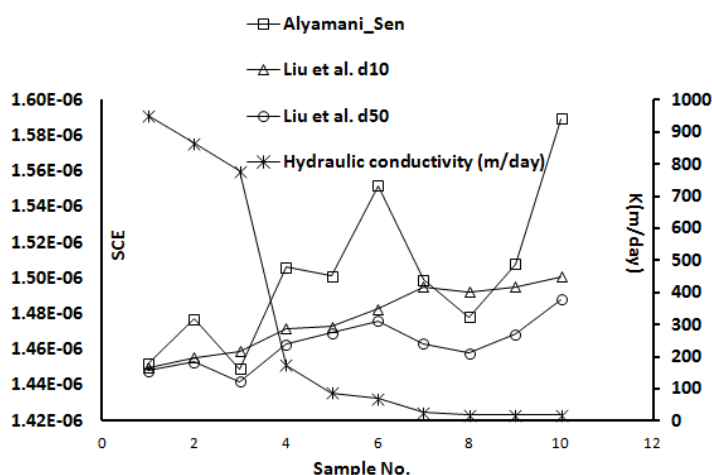


Fig. 2. Comparison of SCE results using Eq. (2) with d_{10} , d_{50} , and Eq.(12).

4. Conclusion

The mobility of CNTs in the natural environment is significant in predictive modelling of their fate and transport when considering risk assessment aspects of the release of these engineered nanoparticles. This study highlights the importance of considering the natural heterogeneity of natural porous media when modelling the factors relating to the mobility of CNTs.

Although clean-bed filtration (traditional filtration) theory has proven useful in preliminary predictions of CNT travel distance in saturated porous media, it falls short in representing the heterogeneity of the natural subsurface.

Virtual and real scenarios tested in this study have illustrated a difference in values estimated with and without the heterogeneity represented in the predictions. These

observations plant the seeds for a plausible argument that the mobility of CNTs can be significantly different from those estimated under the assumption of uniformity and homogeneity.

Based on the scenarios tested in this study, it is expected that CNTs would become less mobile as heterogeneity increases [note that this is not true for fractured rock systems]. Thus it can be anticipated that they will be immobilised in the vicinity of release points. Although this might reduce the chances of groundwater pollution and long travel distances initially, it keeps the option of remobilisation (as a result of change of conditions) animate and pending.

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