## Fluid Flow and Transport

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## Chapter 1

## Fluid Flow

#### 1.0.1 Multiphase Fluid Flow

It is assumed that Darcy's law holds for a multiphase system, with a specific discharge for each component, and a coupling between the flow equations done by the relative permeability, that is in turn based on the relative saturations. For example, for a 2 component flow,

$$q_w = \frac{K_{wr}K\rho_w g}{\mu_w} \vec{\nabla} h \tag{1.1}$$

$$q_{w} = \frac{K_{wr}K\rho_{w}g}{\mu_{w}} \vec{\nabla}h$$

$$q_{NAPL} = \frac{K_{wr}K\rho_{NAPL}g}{\mu_{NAPL}} \vec{\nabla}h$$

$$(1.1)$$

On a more general basis, Darcy's law for a multiphase flow with variable density is equal to

$$q_{iCh} = -\frac{k_{Ch}K_i}{\mu_{Ch}} \left(\partial P_i + \delta_{i3}\rho_i g\right) \tag{1.3}$$

with  $K_i$  being the relative permeability.

#### Single Phase Fluid Flow: The Freshwater Equivalent Head

The freshwater equivalent head was developed by Frind [?] to deal with problems when the solute/contaminant changes the density of the groundwater flow, like saline intrusion and brine dissolution. Also, the transient state for these systems can be quite long.

For a system with different densities, a equivalent (or reference) hydraulic head with respect to fresh water is defined as

$$\psi = \frac{p}{\rho_0 g} + z \tag{1.4}$$

for  $\rho_0$  is the reference density of fresh water. The Darcy's equation for variable density reads

$$q_i = -\frac{k_{ij}}{\mu} \left( \frac{\partial p}{\partial x_j} + \rho g n_j \right) \tag{1.5}$$

for

•  $k_{ij}$  is the permeability tensor

- $\mu$  is the dynamic viscosity
- $\rho$  is the fluid density, dependent of position
- $n_j$  is a delta that is 1 in the vertical and zero if horizontal.

Replacing equation (1.4) in equation (1.5), Darcy's law reads

$$p = \rho_0 g \left( \psi - z \right) \tag{1.6}$$

$$q_{i} = -\frac{k_{ij}}{\mu} \left( \frac{\partial \rho_{0}g \left( \psi - z \right)}{\partial x_{j}} + \rho g n_{j} \right)$$

$$\tag{1.7}$$

$$q_i = -\frac{k_{ij}\rho_0 g}{\mu} \left( \frac{\partial \psi}{\partial x_j} + \frac{\rho - \rho_0}{\rho_0} n_j \right)$$
 (1.8)

$$q_i = -K_{ij} \left( \frac{\partial \psi}{\partial x_j} + \rho_r n_j \right) \tag{1.9}$$

with a hydraulic conductivity independent of the variations of the fluid properties,

$$K_{ij} = \frac{k_{ij}\rho_0 g}{\mu} \tag{1.10}$$

and an equivalent density,  $\rho_r$ , equal to

$$\rho_r = \frac{\rho - \rho_0}{\rho} \tag{1.11}$$

Considering that the concentration of the solute is sufficiently small, a relation between density and concentration can be established:

$$\rho = \rho_0 (1 + \gamma c) \tag{1.12}$$

where

$$\gamma = \frac{\rho_{max}}{\rho_0} - 1 \tag{1.13}$$

and again where  $\rho_0$  is the maximum density of the fluid. This way the equivalent density is equal to

$$\rho_r = \gamma c \tag{1.14}$$

Using (1.12) and (1.14), Darcy's law for a single phase flow with varying density is equal to

$$q_i = -K_{ij} \left( \frac{\partial \psi}{\partial x_j} + \gamma c n_j \right) \tag{1.15}$$

So now the Darcy's law for varying density is written in function of the concentration, that Frind [?] states that this solution is much more stable numerically. This is caused by the large static pressure values and the small variations in pressure, while concentration varies from 0 to 1.

When the flow has several solutes but only one phase the equivalent density is written as

$$\rho_r = \frac{1}{\rho_0} \sum_{k=1}^{N} c_k - \sum_{k=1}^{N} \frac{c_k}{\rho_k}$$
 (1.16)

### 1.0.3 Continuity Equation

The continuity equation for the equivalent freshwater head is written as

$$\frac{\partial}{\partial x_i} \left[ K_{ij} \left( \frac{\partial \psi}{\partial x_j} + \gamma c n_j \right) \right] = S_s \frac{\partial \psi}{\partial t}$$
(1.17)

where  $S_s$  is the specific storage [1/L], equal to

$$S_s = \rho_0 g(\alpha + \theta \beta) \tag{1.18}$$

and

- $\beta$  is the compressibility of the fluid  $[LT^2/M]$
- $\theta$  is the porosity [-]
- $\alpha$  is the compressibility of the porous medium  $[LT^2/M]$

The continuity equation for a single phase, non-saturated flow, with sources or sinks, and several solutes, is equal to

$$\frac{\partial}{\partial x_i} \left[ K_{ij} K_{wr} \left( \frac{\partial \psi}{\partial x_j} + \gamma c n_j \right) \right] - \sum_{k=1}^N Q_k(x_1, x_2, x_3, t) = S_s S_w \frac{\partial \psi}{\partial t} + \theta \frac{\partial S_w}{\partial t}$$
 (1.19)

with the same parameters as before, and  $S_w(t)$  is the water saturation, and  $K_{wr}$  is the relative permeability, given by

$$K_{wr} = \left(\frac{S_w - S_{wr}}{1 - S_{wr}}\right)^4 \tag{1.20}$$

and  $S_{wr}$  is the irreducible water saturation.

#### 1.1 Transport

#### 1.1.1 Total Flux of an Extensive Quantity

Flux is defined as a quantity that passes through an unit area per unit time, in the direction normal to the unit area.

The total flux of a extensive quantity is given by

$$J^{\vec{t}E} = e\vec{V^E} \tag{1.21}$$

$$J^{t\vec{E}\gamma} = e^{\gamma} V^{\vec{E}\gamma} \tag{1.22}$$

with

$$J^{\vec{t}E} = \sum_{\gamma} J^{\vec{t}E\gamma} \sum_{\gamma} e^{\gamma} V^{\vec{E}\gamma}$$
 (1.23)

where

• e is the quantity of the extensive quantity (grandeza) E per unit volume.

The velocity of one component, in the reference frame of the particle, is defined as

$$V^{\vec{E}\gamma} = \frac{\partial x^{\vec{\epsilon}\gamma}}{\partial t} \tag{1.24}$$

with  $\epsilon$  being the material coordinates (Lagrangian coordinates). The mass-weighted velocity is an average velocity in a fixed coordinate reference frame, and it's an average with respect of the masses of the components. If there is only one component, the particle velocity is equal to the average velocity. The average velocity is given in a fixed point (Eulerian description) by

$$\vec{V} = \sum_{\gamma} c_{\gamma} \vec{V_{\gamma}} \tag{1.25}$$

where, as noted before,  $\vec{V_{\gamma}}$  is the Lagrangian speed of each particle of component  $\gamma$ .  $\vec{V}$  should be understood as an averaged speed by each component's masses.

Subtracting the particle velocity of the extensive quantity from the average velocity, diffusion with respect of the average velocity is defined:

$$e\vec{V^E} = e\vec{V} + e\left(\vec{V^e} - \vec{V}\right) \tag{1.26}$$

The diffusive flux with respect to the advective flux velocity is equal to

$$j^{\vec{E}} = e\left(V^e - \vec{V}\right) \tag{1.27}$$

By component, the diffusive flux is equal to

$$j^{\vec{E}\gamma} = \sum_{\gamma}^{N} e\left(\vec{V}^e - \vec{V}\right) \tag{1.28}$$

with the conservation equations:

$$\sum_{\gamma}^{N} j^{\vec{E}\gamma} = \sum_{\gamma}^{N} \sum_{\gamma}^{N} = \sum_{\gamma}^{N} (\vec{V} - \vec{V}) = 0$$
 (1.29)

#### 1.1.2 Advective Flux

Advective Transport describes the mass amount that crosses an unit area per unit time, in the porous medium, in the direction normal to the unit area. Defined as

$$\vec{J_{adv}} = S_{\gamma} \vec{v} c^{\gamma} \tag{1.30}$$

the same expression as eq. (1.26), as expected. The only difference is the saturation:

- $S_{\gamma}$  saturation
- $\vec{v}$  average medium velocity
- $c^{\gamma}$  component concentration

with the average medium velocity,  $\vec{v}$ , being defined as

$$\vec{v} = \frac{\vec{q}}{\theta} \tag{1.31}$$

where  $\vec{q}$  is the specific discharge vector.

#### 1.1.3 Fick's Law - Diffusion

Fick's Law in microscopic for is equal to

$$\vec{j}^{\gamma} = c^{\gamma} \left( \vec{V} - \vec{V}^{\gamma} \right) = -\rho D^{\gamma \delta} \vec{\nabla} w^{\gamma} \tag{1.32}$$

where the quantities are defined as

- $w^{\gamma} = \rho^{\gamma}/\rho$ , mass fraction of component  $\gamma$
- $\rho^{\gamma} = m^{\gamma}/V$ , density of component  $\gamma$
- $D^{\gamma\delta}$  diffusion coefficient, independent of concentration but dependent of temperature and pressure

If the fluid is homogeneous, Fick's Law is equal to

$$\vec{j}^{\gamma} = -D^{\gamma} \vec{\nabla} c^{\gamma} \tag{1.33}$$

where  $c^{\gamma}$  is the concentration of the component  $\gamma$ . The negative sign, as in the Darcy's Law, indicates that the flow of the component goes from the higher to lower concentration.

#### Macroscopic Fick's Law

The macroscopic version for Fick's Law has the same form of the microscopic one, but a averaging process was done on all quantities. The diffusion equation is equal to

$$\vec{J}^{\gamma} = -D^{\prime \gamma}(\theta) \vec{\nabla} c^{\gamma} = -D^{\prime \gamma}_{ij}(\theta) \partial_j c^{\gamma}$$
(1.34)

 $D^{\prime\gamma}(\theta)$  or  $D^{\prime\gamma}(\phi)$  is the second order diffusion tensor in porous media, proportional to either porosity  $(\phi)$  or saturation  $(\theta)$ . It can be decomposed as

$$D^{\prime \gamma}(\theta) = D^{\gamma} T(\theta) \tag{1.35}$$

where  $D^{\gamma}$  is the diffusion scalar and  $T(\theta)$  is the second order tensor for tortuosity, the property of a curve being sinuous. The tortuosity tensor is a geometric coefficient, that accounts for the effects of the porous surface in diffusion.

For varying density the macroscopic Fick's Law is

$$\vec{J}^{\gamma} = -\rho D_{ij}^{'\gamma}(\theta) \vec{\nabla} \frac{\rho^{\gamma}}{\rho} \tag{1.36}$$

### 1.2 Hydrodynamic Dispersion

Experiments of the spread of a solute in a fluid in motion show that instead of a sharp interface between the region with and without the solute, there is a region with a concentration gradient,  $1 \le c \le 0$ , and that region grows with time. One interesting characteristic is that the solute spreads even in the direction *perpendicular* to the fluid motion direction.

Hydrodynamic dispersion is composed by a mechanical component, called *mechanical dis*persion, that is caused by microscopic velocities variations both in magnitude and direction, caused in turn by the irregular pore structure and the velocity profiles between pores. It is also composed by diffusion, as explained in section (1.1.3). The mechanical dispersion component is not present when the fluid is at rest, but diffusion always contribute for the spread of the solute, even with zero fluid velocity. Diffusion is also responsible for the crossing of solute streamlines, what is impossible with only dispersion caused by velocity.

#### 1.2.1 Dispersive Flux

The dispersive flux is a macroscopic quantity that quantifies the mechanical dispersion and molecular diffusion. It can be modeled with a Fickian-type law:

$$\vec{J}^* = -D^* \vec{\nabla} c \tag{1.37}$$

 $D^*$  is the coefficient of hydrodynamic dispersion for a saturated multiphase flow, with different densities. There are several ways to define this coefficient:

$$D_{ij}^* = a_{ijkl} \frac{V_k V_l}{V} f(Re) \tag{1.38}$$

 $a_{ijkl}$  is a tensor that gives the relations of the interfaces between solid-liquid and others. It's a positive definite tensor, with 36 components. In an isotropic porous media,  $a_{ijkl}$  has only two components,  $a_L$  and  $a_T$ , where L stands for longitudinal and T for transversal component. The magnitude of  $a_L$  is the same of the size of the pores, and  $a_T$  is 8 to 24 times smaller. The coefficient of hydrodynamic dispersion is then equal to

$$D_{ij}^* = \left[ a_T \delta_{ij} + (a_L - a_T) \frac{V_i V_j}{V^2} \right] V$$
 (1.39)

In this case, the tensor  $D^*$  has principal components. For flow in the x direction and the other components with zero velocity, the hydrodynamic tensor is equal to

$$D^* = \begin{pmatrix} a_L & 0 & 0 \\ 0 & a_T & 0 \\ 0 & 0 & a_T \end{pmatrix} V \tag{1.40}$$

#### 1.2.2 Total Flux

The total flux of a solute with concentration c and velocity  $\vec{V}$ , hydrodynamic dispersion coefficient  $D^*$ , molecular diffusion coefficient  $D^{'}$  and  $D = D^* + D^{'}$  is equal to

$$\vec{J_t} = c\vec{V} - D\vec{\nabla}c \tag{1.41}$$

for a single phase flow. For multiphase, multi-component flow,

$$\vec{J_t^{\gamma}} = c\vec{V^f} - D^{\gamma}\vec{\nabla}c^f \tag{1.42}$$

where f denotes different phases.