

# EXPERIMENT 1

## STRESS DISTRIBUTION IN A TOWER BRIDGE



# STRESS DISTRIBUTION IN A TOWER BRIDGE

**Aim:** Calculating and visualizing the Eigen values of stress matrix for simply supported beam

**Problem statement:** Find principal stresses for a two-dimensional simply supported beam by finding the eigenvalues of the stress matrix with variable components.

**Mathematical Background:** The principal stresses are the eigenvalues of the stress matrix.

- The  $2 \times 2$  stress matrix is given by  $S = \begin{bmatrix} \sigma_x & \tau_{xy} \\ \tau_{yx} & \sigma_y \end{bmatrix}$
- The principal stress at the point will be the eigenvalues of the stress matrix S

# Definitions

A nonzero vector  $\mathbf{x}$  is an **eigenvector** (or *characteristic vector*) of a square matrix  $\mathbf{A}$  if there exists a scalar  $\lambda$  such that  $\mathbf{Ax} = \lambda\mathbf{x}$ . Then  $\lambda$  is an **eigenvalue** (or *characteristic value*) of  $\mathbf{A}$ .

*Note:* The zero vector can not be an eigenvector even though  $\mathbf{A}\mathbf{0} = \lambda\mathbf{0}$ . But  $\lambda = 0$  can be an eigenvalue.

Example:

Show  $X = \begin{bmatrix} 2 \\ 1 \end{bmatrix}$  is an eigenvector for  $A = \begin{bmatrix} 2 & -4 \\ 3 & -6 \end{bmatrix}$

$$\text{Solution : } AX = \begin{bmatrix} 2 & -4 \\ 3 & -6 \end{bmatrix} \begin{bmatrix} 2 \\ 1 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

$$\text{But for } \lambda = 0, \lambda X = 0 \begin{bmatrix} 2 \\ 1 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

Thus,  $X$  is an eigen vector of  $A$ , and  $\lambda = 0$  is an eigenvalue.

# Eigenvalues

Let  $x$  be an eigenvector of the matrix  $A$ . Then there must exist an eigenvalue  $\lambda$  such that  $Ax = \lambda x$  or, equivalently,

$$Ax - \lambda x = 0 \quad \text{or}$$

$$(A - \lambda I)x = 0$$

If we define a new matrix  $B = A - \lambda I$ , then  $Bx = 0$

If  $B$  has an inverse then  $x = B^{-1}0 = 0$ . But an eigenvector cannot be zero.

Thus, it follows that  $x$  will be an eigenvector of  $A$  if and only if  $B$  does not have an inverse, or equivalently  $\det(B)=0$ , or

$$\det(A - \lambda I) = 0$$

This is called the **characteristic equation** of  $A$ . Its roots determine the eigenvalues of  $A$ .

# Examples

Example 1: Find the eigenvalues of  $A = \begin{bmatrix} 2 & -12 \\ 1 & -5 \end{bmatrix}$

$$\begin{aligned} |\lambda I - A| &= \begin{vmatrix} \lambda - 2 & 12 \\ -1 & \lambda + 5 \end{vmatrix} = (\lambda - 2)(\lambda + 5) + 12 \\ &= \lambda^2 + 3\lambda + 2 = (\lambda + 1)(\lambda + 2) \end{aligned}$$

two eigenvalues:  $-1, -2$

**Note:** The roots of the characteristic equation can be repeated. That is,  $\lambda_1 = \lambda_2 = \dots = \lambda_k$ .  
If that happens, the eigenvalue is said to be of multiplicity  $k$ .

Example 2: Find the eigenvalues of  $A = \begin{bmatrix} 2 & 1 & 0 \\ 0 & 2 & 0 \\ 0 & 0 & 2 \end{bmatrix}$

$$|\lambda I - A| = \begin{vmatrix} \lambda - 2 & -1 & 0 \\ 0 & \lambda - 2 & 0 \\ 0 & 0 & \lambda - 2 \end{vmatrix} = (\lambda - 2)^3 = 0$$

$\lambda = 2$  is an eigenvalue of multiplicity 3.

# Eigenvectors

To each distinct eigenvalue of a matrix  $\mathbf{A}$  there will correspond at least one eigenvector which can be found by solving the appropriate set of homogenous equations. If  $\lambda_i$  is an eigenvalue then the corresponding eigenvector  $\mathbf{x}_i$  is the solution of  $(\mathbf{A} - \lambda_i \mathbf{I})\mathbf{x}_i = \mathbf{0}$

Example 1 (cont.):

$$\lambda = -1 : (-1)\mathbf{I} - \mathbf{A} = \begin{bmatrix} -3 & 12 \\ -1 & 4 \end{bmatrix} \Rightarrow \begin{bmatrix} 1 & -4 \\ 0 & 0 \end{bmatrix}$$

$$x_1 - 4x_2 = 0 \Rightarrow x_1 = 4t, x_2 = t$$

$$\mathbf{x}_1 = \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = t \begin{bmatrix} 4 \\ 1 \end{bmatrix}, t \neq 0$$

$$\lambda = -2 : (-2)\mathbf{I} - \mathbf{A} = \begin{bmatrix} -4 & 12 \\ -1 & 3 \end{bmatrix} \Rightarrow \begin{bmatrix} 1 & -3 \\ 0 & 0 \end{bmatrix}$$

$$\mathbf{x}_2 = \begin{bmatrix} x_1 \\ x_2 \end{bmatrix} = s \begin{bmatrix} 3 \\ 1 \end{bmatrix}, s \neq 0$$

# Properties of Eigenvalues and Eigenvectors

1. The sum of the eigenvalues of a matrix equals the trace of the matrix.
2. A matrix is singular if and only if it has a zero eigenvalue.
3. The eigenvalues of an upper (or lower) triangular matrix are the elements on the main diagonal.
4. If  $\lambda$  is an eigenvalue of  $A$  and  $A$  is invertible, then  $1/\lambda$  is an eigenvalue of matrix  $A^{-1}$ .
5. If  $\lambda$  is an eigenvalue of  $A$  then  $k\lambda$  is an eigenvalue of  $kA$  where  $k$  is any arbitrary scalar.
6. If  $\lambda$  is an eigenvalue of  $A$  then  $\lambda^k$  is an eigenvalue of  $A^k$  for any positive integer  $k$
7. If  $\lambda$  is an eigenvalue of  $A$  then  $\lambda$  is an eigenvalue of  $A^T$

# MATLAB syntax

<b>p =poly(A)</b>	where A is an nxn matrix returns an n+1 element row vector whose elements are the coefficients of the characteristic polynomial, $\det(\lambda I - A)$ , which are stored in p
<b>r =roots(p)</b>	Returns a column vector r whose elements are the roots of the polynomial p.
<b>[V,D] = eig(A)</b>	D=diagonal matrix with eigenvalues on its diagonal; V = modal matrix whose columns are the corresponding eigenvectors.
<b>eye(n)</b>	Returns an n x n identity matrix



# Example

If  $A = \begin{bmatrix} 3 & 0 & -1 \\ 0 & 1 & 0 \\ 2 & 0 & 0 \end{bmatrix}$  Find

- Characteristic polynomial of  $A$
- Roots of Characteristic polynomial of  $A$
- Eigen values of  $A$
- Eigen vectors of  $A$
- Eigen values of  $A^{-1}$
- Eigen values of  $A^T$
- Eigen values of  $B = A^2 + 3A + 2I$

## MATLAB code

```
A=[3 0 -1; 0 1 0; 2 0 0]
Eigenvalues_of_A=eig(A)
[X,D]= eig(A)
p=poly(A)           % Coefficients of the characteristic
polynomial
r = roots(p)       % To find the roots of the
characteristic equation
sum_of_eigenvalues = sum(r)
Trace_of_A=trace(A)%Note that sum of eigenvalues is
Trace of A
product_of_eigenvalues_A=prod(r)
Determinant_of_A =det(A)   % Note that product of
the eigenvalues of A is determinant of A
Inverse_A=inv(A)
Eigenvalues_of_invA= eig(Inverse_A)
Eigenvalues_Transpose_of_A= eig(A')
% Note that eigenvalues of A and Transpose of A
are same
Eigenvalues_of_B= eig(A^2+3*A+2*eye(3))
```

# Output

>>

A =

3	0	-1
0	1	0
2	0	0

Eigenvalues\_of\_A =

2  
1  
1

X =

0.7071	0.4472	0
0	0	1.0000
0.7071	0.8944	0

D =

2	0	0
0	1	0
0	0	1

p =

1	-4	5	-2
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r =

2.0000

1.0000 + 0.0000i

1.0000 - 0.0000i

sum\_of\_eigenvalues = 4.0000

Trace\_of\_A = 4

product\_of\_eigenvalues\_A = 2.0000

Determinant\_of\_A = 2

Inverse\_A =

0 0 0.5000

0 1.0000 0

-1.0000 0 1.5000

Eigenvalues\_of\_invA =

0.5000

1.0000

1.0000

Eigenvalues\_Transpose\_of\_A =

2

1

1

Eigenvalues\_of\_B =

12

6

6

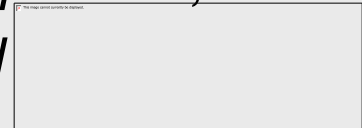
# Practice Problem

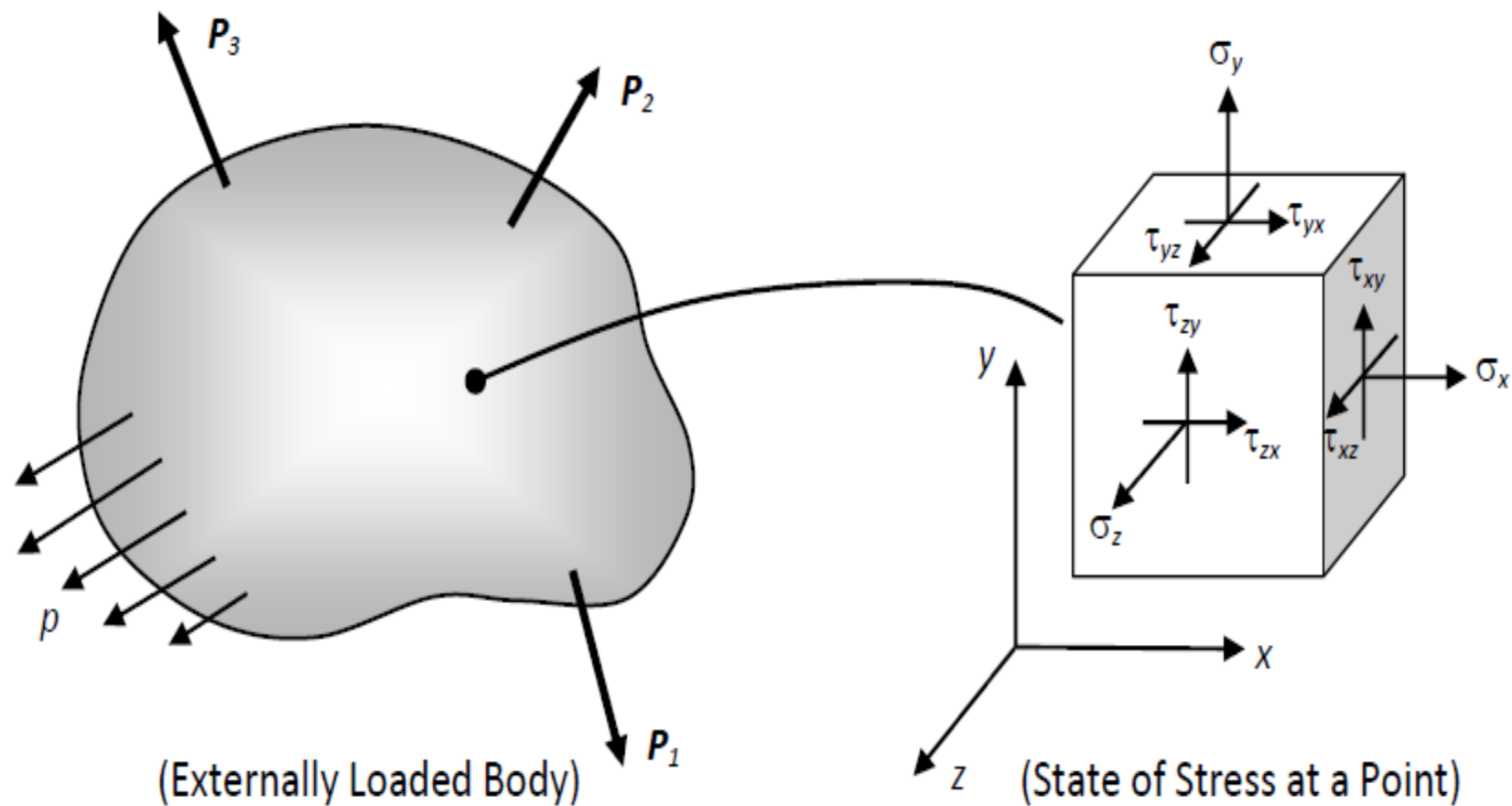
If  $A = \begin{bmatrix} 1 & 2 & 1 \\ 6 & -1 & 0 \\ -1 & -2 & -1 \end{bmatrix}$  Find

- Characteristic polynomial of  $A$
- Roots of Characteristic polynomial of  $A$
- Eigen values of  $A$
- Eigen vectors of  $A$
- Eigen values of  $A^{-1}$
- Eigen values of  $A^T$
- Eigen values of  $B = A^2 + 3A + 2I$

# STRESS ANALYSIS

- An elastic body is subjected to applied loadings, stresses are created inside the body.
- In general these stresses often vary in complicated ways from point to point and from plane to plane within the structure.
- To help characterize this situation, stresses are normally defined with respect to a given coordinate system.
- The illustration below shows that at a typical point within a loaded body, the state of stress can be characterized on a small cube of material defined with respect to a Cartesian coordinate system.
- These nine components are called the *stress components*, with  $\sigma_x, \sigma_y, \sigma_z$  referred to as normal stresses and  $\tau_{zy}, \tau_{zx}, \tau_{xz}$  called the shearing stresses.





# STRESS ANALYSIS

- The number of components and some other transformation properties, the stress can be expressed as a 3 x 3 matrix

$$\sigma = \begin{bmatrix} \sigma_x & \tau_{xy} & \tau_{xz} \\ \tau_{yx} & \sigma_y & \tau_{yz} \\ \tau_{zx} & \tau_{zy} & \sigma_z \end{bmatrix}$$

- Since the shearing stresses have the equalities  $\tau_{xy} = \tau_{yx}, \tau_{yz} = \tau_{zy}, \tau_{zx} = \tau_{xz}$ , the stress matrix is symmetric.
- If we changed the orientation of a particular plane the normal stress component  $\sigma_x$  will vary
- There exists a special orientation where the normal stress will be a maximum, and these are called *principal planes and the normal stresses acting on them are called the principal stresses*



# STRESS ANALYSIS

- The general three-dimensional case, the theory to determine principal stresses and the planes on which they act is formulated by the **eigenvalue problem**

$$[\sigma]\{n\} = \lambda\{n\}$$

- where  $\sigma$  is the stress matrix,  $\{n\}$  is the principal direction vector and  $\lambda$  (the **eigenvalue**) is the principal stress. Thus solving the **eigenvalue problem** will determine up to three distinct principal stresses and the corresponding three principal directions. It turns out for this application (3x3, symmetric real matrix) the principal directions are mutually orthogonal.
- The shear stress components will vanish on these three principal planes and so for a coordinate system that is aligned with the principal directions the stress matrix takes on the simplified diagonal form

# STRESS ANALYSIS

$$\sigma = \begin{bmatrix} \sigma_1 & 0 & 0 \\ 0 & \sigma_2 & 0 \\ 0 & 0 & \sigma_3 \end{bmatrix}$$

Where  $\sigma_1, \sigma_2, \sigma_3$  are the problem's eigenvalues (roots of the characteristic equation) and are generally referred to as the principal stresses.

# Example

**Q** Consider the bridge to be equivalent to a two-dimensional simply supported beam of thickness 1 meter. The bridge carries a concentration force  $P = 100$  Newton at the mid-point. The length of the beam is 10 times of its thickness. If the individual stresses at the point  $(0.5, 0.5)$  are given by  $\sigma_x = -4, \sigma_y = 0, \sigma_{xy} = -2.5$  then write the stress matrix and hence calculate the principal stresses.

**Ans**

The stress matrix at the point  $(0.5, 0.5)$  will be

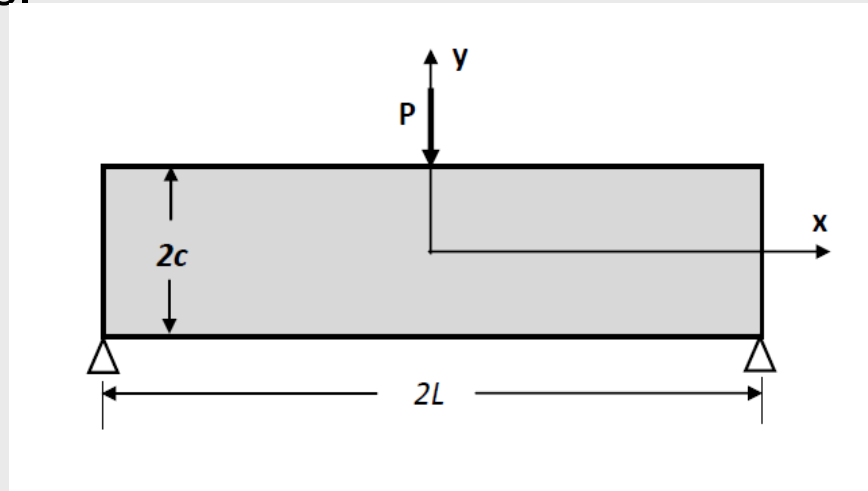
$$S = \begin{bmatrix} -4 & -2.5 \\ -2.5 & 0 \end{bmatrix}$$

# Practice Problem

**Q** If the individual stresses in the beam at any point  $(x,y)$  are given by

$$\sigma_x = -\frac{3P}{4c^3}(L-x)y, \sigma_y = 0, \tau_{xy} = -\frac{3P}{4c^3}(c^2 - y^2)$$

Where  $P$  denotes the force  $2c$  denotes the height and  $2L$  denotes the length of the beam. Write the stress matrix and hence calculate the principal stresses.



Draw the stress distribution in the beam using contour plot

# Solution

From mechanics of materials theory the in-plane stress components for the right half of the beam ( $x > 0$ ) are given by

$$\sigma_x = -\frac{3P}{4c^3}(L-x)y, \sigma_y = 0, \tau_{xy} = -\frac{3P}{4c^3}(c^2 - y^2)$$

Thus the stress matrix for this problem reduces to the 2x2 form

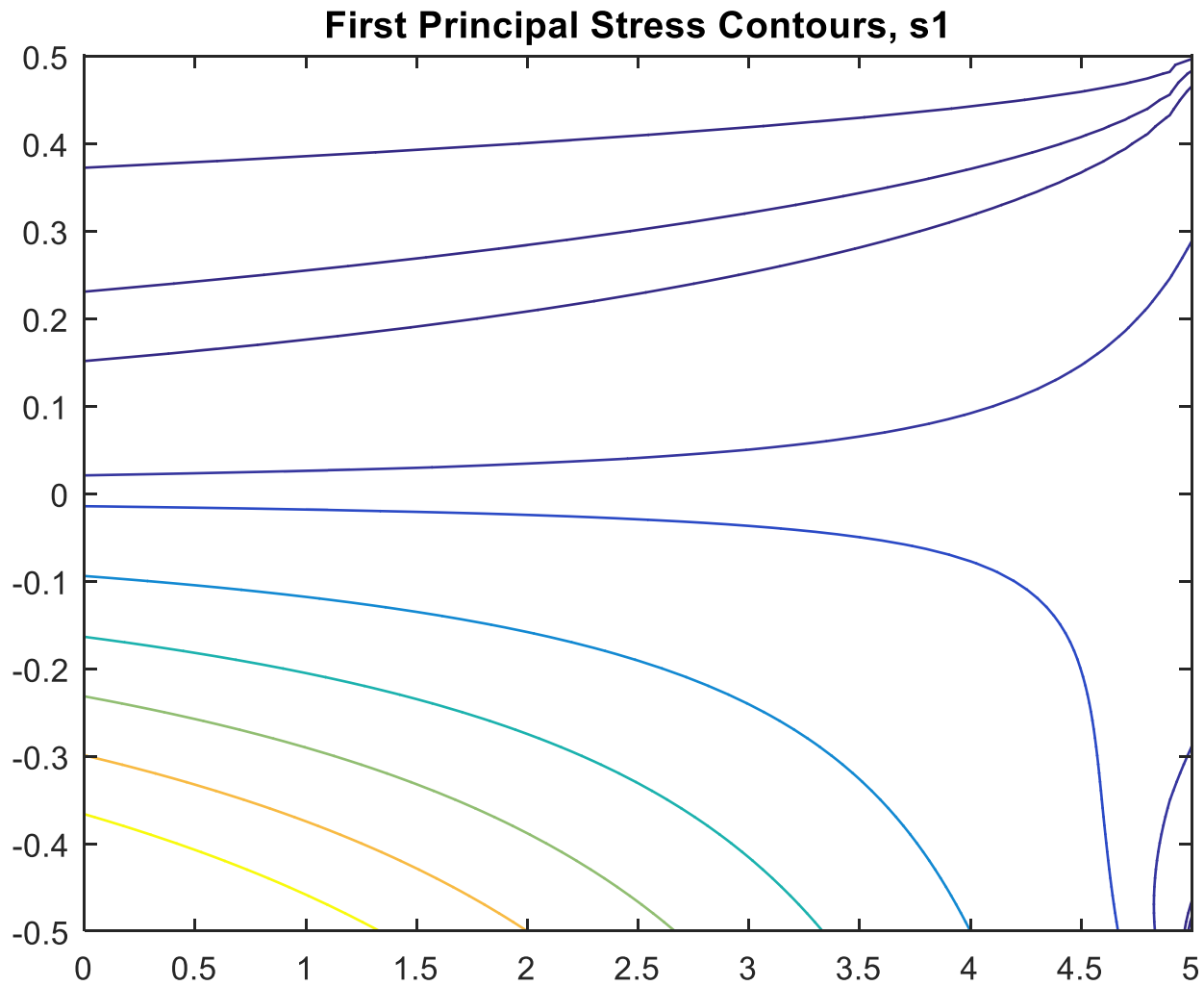
$$[\sigma] = \begin{bmatrix} \sigma_x & \tau_{xy} \\ \tau_{xy} & \sigma_y \end{bmatrix} = \begin{bmatrix} -\frac{3P}{4c^3}(L-x)y & -\frac{3P}{4c^3}(c^2 - y^2) \\ -\frac{3P}{4c^3}(c^2 - y^2) & 0 \end{bmatrix}$$

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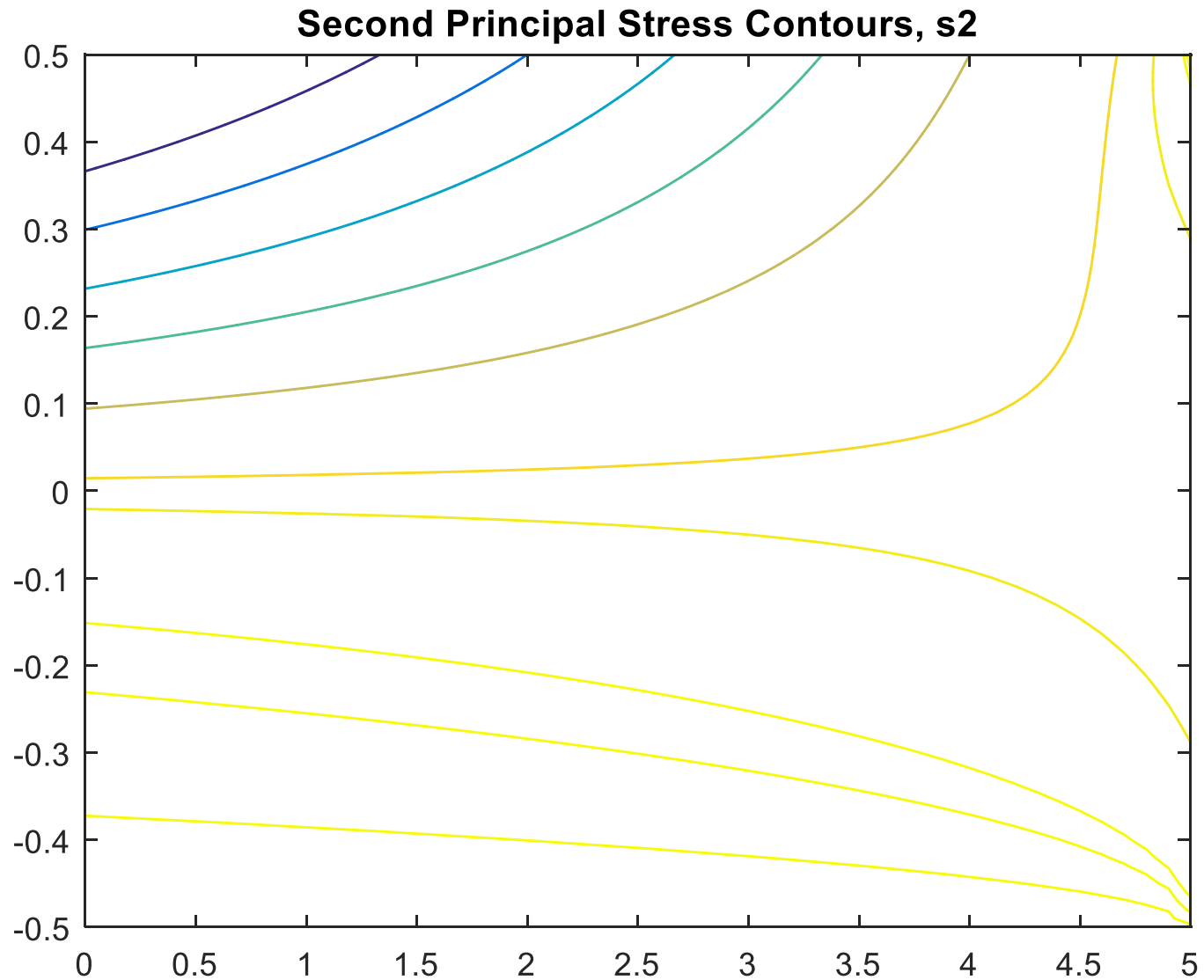
% Determine and Plot Principal Stresses in Beam Exa
clc;clear all;clf;
% Input Beam Parameters
c=0.5;L=5;P=1;
% Set up grid of x and y values
x=[0:0.1:L];y=[-c:0.01:c];
[X,Y]=meshgrid(x,y);
% Calculate Individual Stresses
sx=-(3/(4*c^3))*(L-X).*Y;
sy=zeros(length(y),length(x));
txy=-(3/(8*c^3))*(c^2-Y.^2);
% Create Stress Matrix
for i=1:length(y)
    for j=1:length(x)
s=[sx(i,j),txy(i,j);txy(i,j),sy(i,j)];
% Determine Eigenvalues(Principal Stresses)
p=eig(s);
s1(i,j)=p(2);
s2(i,j)=p(1);
    end
end
% Plot Distributions of Principal Stresses s1 and s
figure(1)
contour(X,Y,s1,[0.01,0.05,0.1,0.5,1,3,5,7,9,11])
title('First Principal Stress Contours, s1')
axis tight
figure (2)
contour(X,Y,s2,[-0.01,-0.05,-0.1,-0.5,-1,-3,-5,-7,-
axis tight
title('Second Principal Stress Contours, s2')

```

# Out put



# Out put





## Lab Assignment for Exp-1

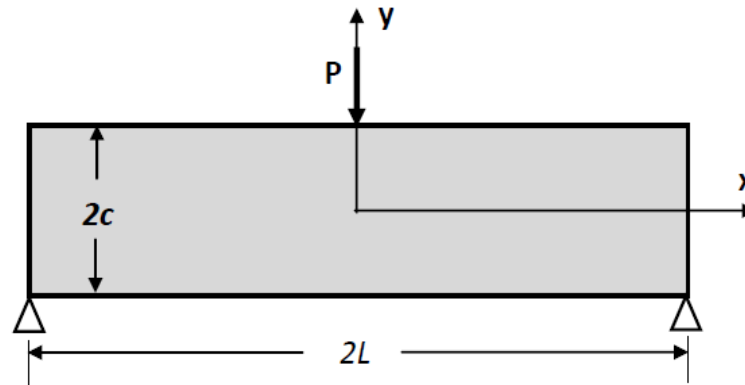
If  $A = \begin{bmatrix} 3 & -1 & 3 \\ 9 & -1 & 9 \\ 7 & -1 & 7 \end{bmatrix}$  Find

- Characteristic polynomial of  $A$
- Roots of Characteristic polynomial of  $A$
- Eigen values of  $A$
- Eigen vectors of  $A$
- Eigen values of  $A^T$
- Eigen values of  $A^{-1}$
- Eigen values of  $B = A^2 + 3A + 2I$

**Q** If the individual stresses in the beam at any point  $(x,y)$  are given by

$$\sigma_x = -\frac{3P}{4c^3}(L-x)y, \sigma_y = 0, \tau_{xy} = -\frac{3P}{4c^3}(c^2 - y^2)$$

Where  $P$  denotes the force  $2c$  denotes the height and  $2L$  denotes the length of the beam. Write the stress matrix and hence calculate the principal stresses.



Where  $L = 6$ ,  $C = 1.5$ ,  $P = 2$

Draw the stress distribution in the beam using contour plot