

# **MECHANICAL SYSTEM DYNAMICS**

**Finite Elements Method (FEM) in structural dynamics:  
software implementation in Matlab environment**

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1. Mesh generation [**USER INPUT: \*.inp**]
2. Definition of the global and local reference systems [**USER INPUT: \*.inp**]
3. Removal of external constraints and introduction of corresponding constraint forces [**USER INPUT: \*.inp**]
4. Energy functions formulation in the local nodal coordinates of each element [**FEM PROGRAM: loadstructure()**]
5. Coordinate transformation from the local to the global reference system [**FEM PROGRAM: assem(), calling el\_tra()**]
6. Matrix assembling, for the entire structure [**FEM PROGRAM: assem()**]

```
Main.m
% structure data
m = ...

% check max element length
Lmax = ...

% build *.inp file (mesh, constraints)

loadstructure()
    % nodes definition
    % elements definition

% draw structure
dis_stru()

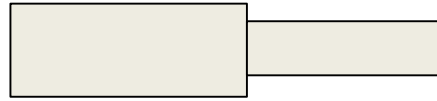
% build and assemble matrices
assem()
    el_tra() % build M and K local ref.
% assemble total M and K
```



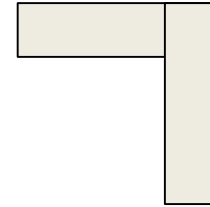
Must consider

- Discontinuities
- Element length:

Properties change



Geometry change



**The element must work in quasi-static region!**

The element first natural frequency is

$$\omega_k^{(1)} = \left( \frac{\pi}{L_k} \right)^2 \sqrt{\frac{E J_k}{m_k}}$$

Given the problem frequency range of interest (  $\Omega_{max}$  )

$$\omega_k^{(1)} > \eta \Omega_{max}$$

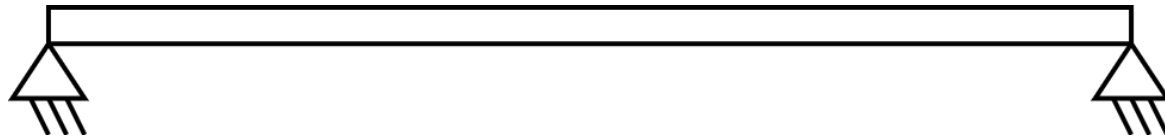
with  $\eta$  safety coefficient (e.g. 1.5), thus

$$L_{max} = \sqrt{\frac{\pi^2}{\eta \Omega_{max}}} \sqrt{\left( \frac{E J}{m} \right)_{min}}$$

## Example - properties of the system

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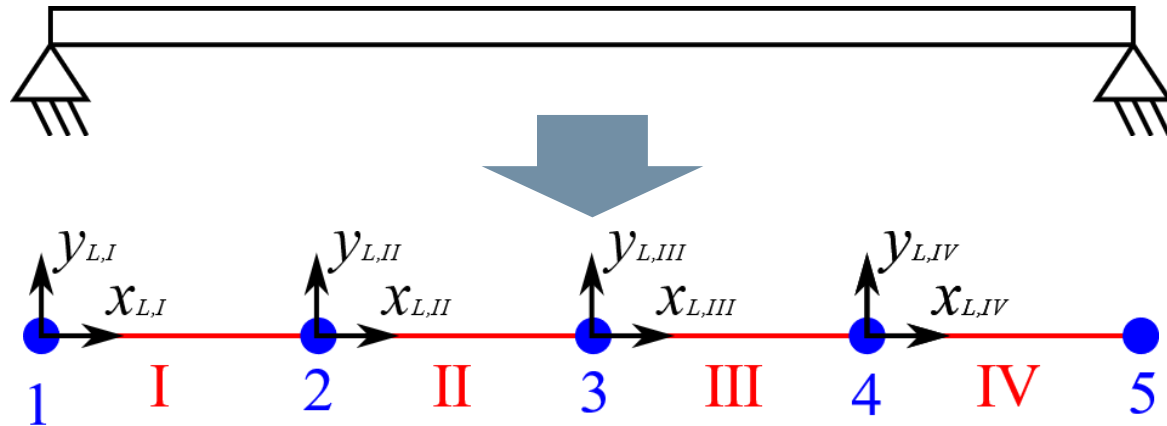
A simple supported aluminum beam with rectangular constant cross-section



Parameter	symbol	unit	value
Lenght	$L$	mm	1200
Thickness	$h$	mm	8
Width	$b$	mm	40
Density	$\rho$	kg/m <sup>3</sup>	2700
Young's Modulus	$E$	GPa	68

$$\Omega_{max} = 100 \cdot 2\pi, \quad \eta = 1.5 \quad \rightarrow \quad L_{max} = \sqrt{\frac{\pi^2}{\eta \Omega_{max}}} \sqrt{\left(\frac{EJ}{m}\right)_{min}} = 348 \text{ mm}$$





4 beam elements (300 mm) and 5 nodes

We must define:

- Nodes:
  - Constraints (x, y,  $\theta$ )
  - Coordinates (x, y)
- Elements:
  - Connected nodes
  - Properties (m, EA, EJ)

Node number

Constraint on x: 1 true, 0 false

```
*NODES
n - constr.(x,y,theta) - coord. x,y
1 1 0 0 0
2 0 0 0 0.3 0
3 ...
```

coordinates

```
*BEAMS
n - input node - output node - property
1 1 2 1
2 2 3 1
3 ...
```

Element property number

Connected nodes

Element number

Property number

```
*PROPERTIES
1 0.864 2.176e7 1.1605e3
```

m, EA, EJ



```

! FEM(1)
! 1st Exercise
! -----
! list of nodes :
*NODES
! n. of node - constraint code (x,y,theta) - x coordinate- y coordinate.
1 1 1 0 0.0 0.0
2 0 0 0 0.3 0.0
3 0 0 0 0.6 0.0
4 0 0 0 0.9 0.0
5 1 1 0 1.2 0.0
! end card *NODES
*ENDNODES
! -----
! list of elements :
*BEAMS
! n. of elem. - n. of input node - n. of output node - n. of prop.
1 1 2 1
2 2 3 1
3 3 4 1
4 4 5 1
*ENDBEAMS
! -----
! List of properties
*PROPERTIES
! N. of prop. - m - EA - EJ
1 0.864 2.176e7 1.1605e3
*ENDPROPERTIES

```

Start comments with «!», compiler ignores following characters

\*NODES start definition of nodes

\*ENDNODES end definition of nodes

\*BEAMS start definition of beam finite elements

\*ENDBEAMS end definition of beam finite elements

\*PROPERTIES start definition of element properties

\*ENDPROPERTIES end definition of beam finite elements properties

- **loadstructure()**

Processes the \*.inp file and return some usefull variables

```
[file_i,xy,nnod,sizew,idb,ngdl,incid,l,gamma,m,EA,EJ,posiz,nbeam]=loadstructure;
```

- **assem()**

Taking info from previous function outputs, assemble M and K matrices

```
[M,K]=assem(incid,l,m,EA,EJ,gamma,idb);
```

The final matrices M and K are of the whole system (free and constrained), as we'll see in the following.



## loadstructure () function

### Call

```
[file_i,xy,nnod,sizew,idb,ngdl,incid,l,gamma,m,EA,EJ,posiz,nbeam]=loadstructure;
```

### Outputs

<b>file_i</b>	name of the *.inp file analysed
<b>xy</b>	$N \times 2$ matrix containing the coordinates of the nodes
<b>nnod</b>	total number of nodes of the structure
<b>sizew</b>	maximum dimension of the structure
<b>idb</b>	$N \times 3$ matrix, numbering each degree of freedom (free and constrained) with different progressive numbers. $N$ is the number of nodes, 3 are the degrees of freedom for each node $(1, 2, 3) = (x, y, \theta)$
<b>ndof</b>	number of total degrees of freedom





## `loadstructure()` function

### Outputs

<b>incid</b>	incidence matrix $N \times 6$ , same idea as for <code>idb</code> , but with $N$ number of elements and 6 the degrees of freedom of each element: (1, 2, 3, 4, 5, 6) = (x1, y1, $\theta$ 1, x2, y2, $\theta$ 2)
<b>l</b>	vector containing the length of each element
<b>alpha</b>	angle of rotation of each element with respect to the global
<b>m</b>	vector containing the mass per unit length of each element
<b>EA, EJ</b>	vectors containing EA and EJ for each elements
<b>posit</b>	$N \times 2$ containing the xy positions defined of the elements
<b>nbeam</b>	number of elements

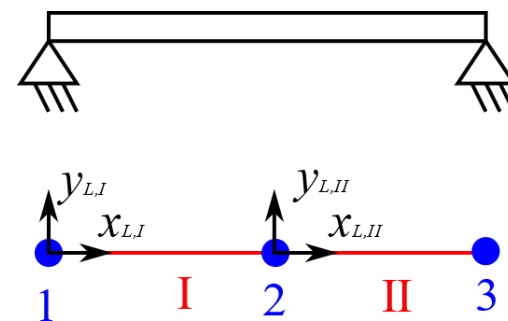


## Input file processing: `loadstructure()`

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... what are **idb** and incidence matrix **incid** for?

Considering the simple example



$$\text{idb} = \begin{bmatrix} 6 & 7 & 1 \\ 2 & 3 & 4 \\ 8 & 9 & 5 \end{bmatrix}$$

$$\text{incid} = \begin{bmatrix} 6 & 7 & 1 & 2 & 3 & 4 \\ 2 & 3 & 4 & 8 & 9 & 5 \end{bmatrix}$$

If we wanted to know which is the index of the row, in the assembled matrices, corresponding to the  $\theta$  DoF of the second node (mid span), we use **idb** matrix as:

$$\text{index} = \text{idb}(2, 3) = 4$$

Whereas if we wanted to know which is the index of the row corresponding to the  $y$  DoF of the second node of the second element (II), we would type:

$$\text{index} = \text{incid}(2, 3+2) = 9$$

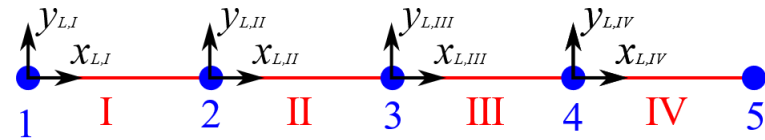
which, in this case, is constrained!



## Example: main code

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Going back to the example



```
clear all
close all
clc

L = 1.2;
E = 68e9;
b = 40e-3;
h = 8e-3;
r = 2700;
m = r*b*h;          % [kg/m]
J = 1/12*b*h^3;
A = b*h;
EA = E*A;
EJ = E*J;

Omax = 100*2*pi;
a = 1.5;
Lmax = sqrt( pi^2/a/Omax * sqrt(EJ/m) );

% build the inp file

[file_i,xy,nnod,sizew,idb,ngdl,incid,l,gamma,m,EA,EJ,posit,nbeam]=loadstructure;

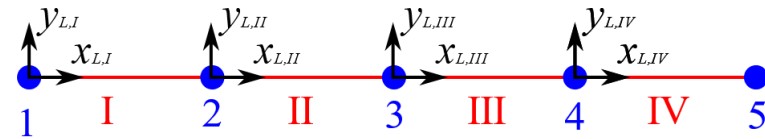
figure();
dis_stru(posit,l,gamma,xy);

[M,K]=assem(incid,l,m,EA,EJ,gamma,idb);
```



# Outputs matrices M and K

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$$M = \begin{bmatrix} 0.0018 & 0 & 0.0096 & -0.0013 & 0 & 0 & 0.0163 & 0 & 0 \\ 0 & 0.3456 & 0 & 0 & 0 & 0.0864 & 0 & 0.0864 & 0 \\ 0.0096 & 0 & 0.0051 & 0 & -0.0096 & 0 & 0.0667 & 0 & 0.0667 \\ -0.0013 & 0 & 0 & 0.0036 & -0.0013 & 0 & -0.0096 & 0 & 0.0096 \\ 0 & 0 & -0.0096 & -0.0013 & 0.0018 & 0 & 0 & 0 & -0.0163 \\ 0 & 0.0864 & 0 & 0 & 0 & 0.1728 & 0 & 0 & 0 \\ 0.0163 & 0 & 0.0667 & -0.0096 & 0 & 0 & 0.1925 & 0 & 0 \\ 0 & 0.0864 & 0 & 0 & 0 & 0 & 0.1728 & 0 & 0 \\ 0 & 0 & 0.0667 & 0.0096 & -0.0163 & 0 & 0 & 0 & 0.1925 \end{bmatrix}$$

Submatrices identified in the matrix M:

- $M_{FF}$  (top-left 5x5 block)
- $M_{FC}$  (top-right 5x4 block)
- $M_{CF}$  (bottom-left 4x5 block)
- $M_{CC}$  (bottom-right 4x4 block)

$K = 1.0e+10 *$

$$\begin{bmatrix} 0.0774 & 0 & -0.1934 & 0.0387 & 0 & 0 & 0.1934 & 0 & 0 \\ 0 & 0.0073 & 0 & 0 & 0 & -0.0036 & 0 & -0.0036 & 0 \\ -0.1934 & 0 & 0.12894 & 0 & 0.1934 & 0 & -0.6447 & 0 & -0.6447 \\ 0.0387 & 0 & 0 & 0.1547 & 0.0387 & 0 & 0.1934 & 0 & -0.1934 \\ 0 & 0 & 0.1934 & 0.0387 & 0.0774 & 0 & 0 & 0 & -0.1934 \\ 0 & -0.0036 & 0 & 0 & 0 & 0.0036 & 0 & 0 & 0 \\ 0.1934 & 0 & -0.6447 & 0.1934 & 0 & 0 & 0.6447 & 0 & 0 \\ 0 & -0.0036 & 0 & 0 & 0 & 0 & 0.0036 & 0 & 0 \\ 0 & 0 & -0.6447 & -0.1934 & -0.1934 & 0 & 0 & 0 & 0.6447 \end{bmatrix}$$

Submatrices identified in the matrix K:

- $K_{FF}$  (top-left 5x5 block)
- $K_{FC}$  (top-right 5x4 block)
- $K_{CF}$  (bottom-left 4x5 block)
- $K_{CC}$  (bottom-right 4x4 block)

