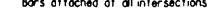


## 2-12



② 6x12-W7/W4

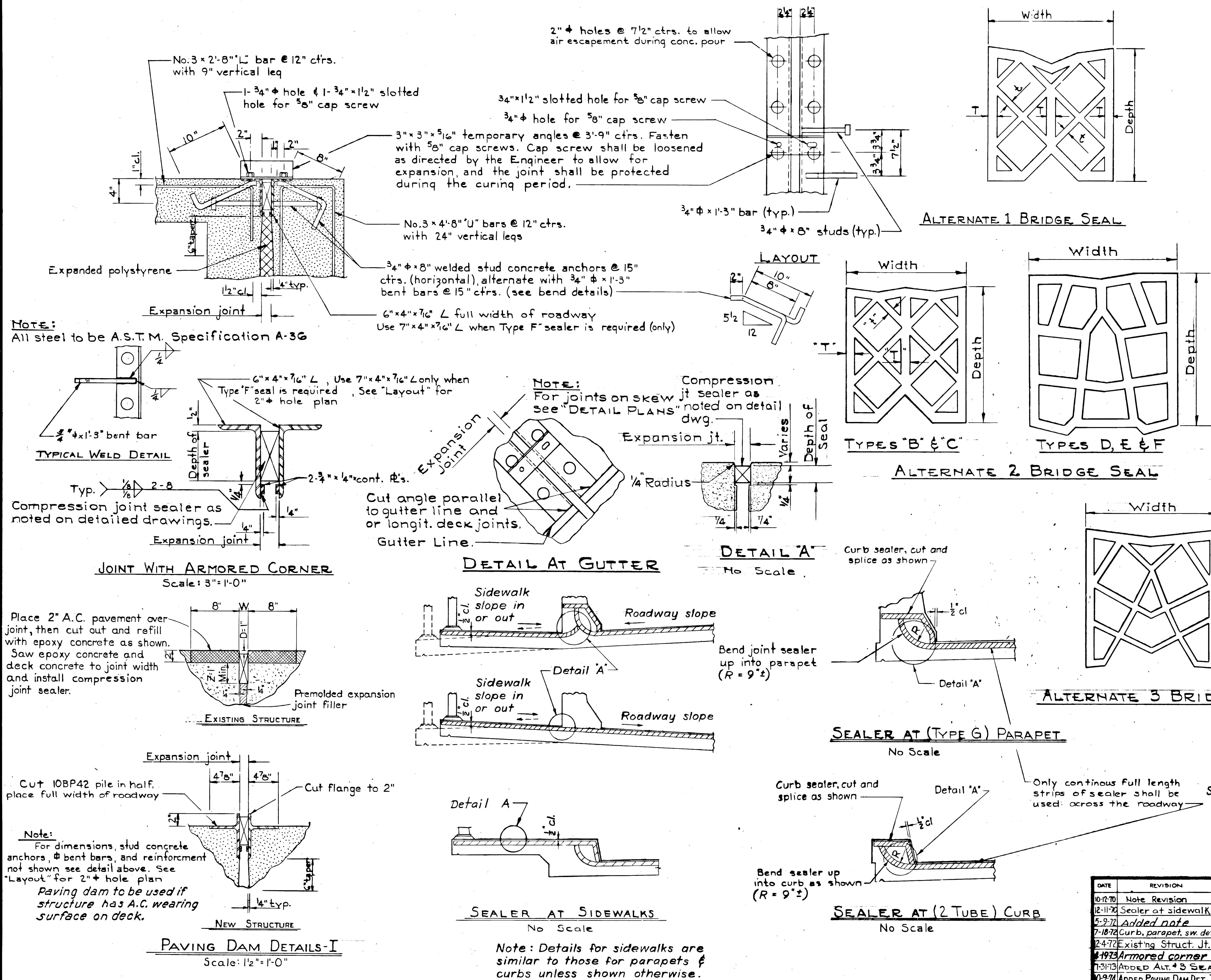
RECEIVED FOR BIDDING PLANS  
JUN 1 1967

336, 2 JUN 1966

THE SUNDAY NEW ORLEANS

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TYPE	WIDTH	DEPTH	THICKNESS	
			T	W
B	$1\frac{3}{4} \begin{smallmatrix} +0.156 \\ -0.063 \end{smallmatrix}$	$2" \pm 0.062"$	$\frac{1}{8} \begin{smallmatrix} +0.031 \\ -0.016 \end{smallmatrix}$	$\frac{3}{32} \begin{smallmatrix} +0.016 \\ -0.032 \end{smallmatrix}$
C	$2" \begin{smallmatrix} +0.156 \\ -0.063 \end{smallmatrix}$	$2\frac{1}{2} \pm 0.062"$	$\frac{1}{8} \begin{smallmatrix} +0.031 \\ -0.016 \end{smallmatrix}$	$\frac{3}{32} \begin{smallmatrix} +0.031 \\ -0.016 \end{smallmatrix}$
D	$2\frac{1}{2} \begin{smallmatrix} +0.218 \\ -0.088 \end{smallmatrix}$	$2\frac{3}{4} \begin{smallmatrix} +0.187 \\ -0.109 \end{smallmatrix}$	$\frac{3}{16} \begin{smallmatrix} +0.032 \\ -0.016 \end{smallmatrix}$	$\frac{3}{32} \begin{smallmatrix} +0.032 \\ -0.016 \end{smallmatrix}$
E	$3" \begin{smallmatrix} +0.250 \\ -0.101 \end{smallmatrix}$	$3\frac{1}{2} \pm 0.1875"$	$\frac{3}{16} \begin{smallmatrix} +0.047 \\ -0.016 \end{smallmatrix}$	$\frac{1}{8} \begin{smallmatrix} +0.032 \\ -0.016 \end{smallmatrix}$
F	$4" \begin{smallmatrix} +0.313 \\ -0.127 \end{smallmatrix}$	$4\frac{23}{32} \pm 0.250"$	$\frac{1}{4} \begin{smallmatrix} +0.047 \\ -0.032 \end{smallmatrix}$	$\frac{3}{16} \begin{smallmatrix} +0.032 \\ -0.016 \end{smallmatrix}$

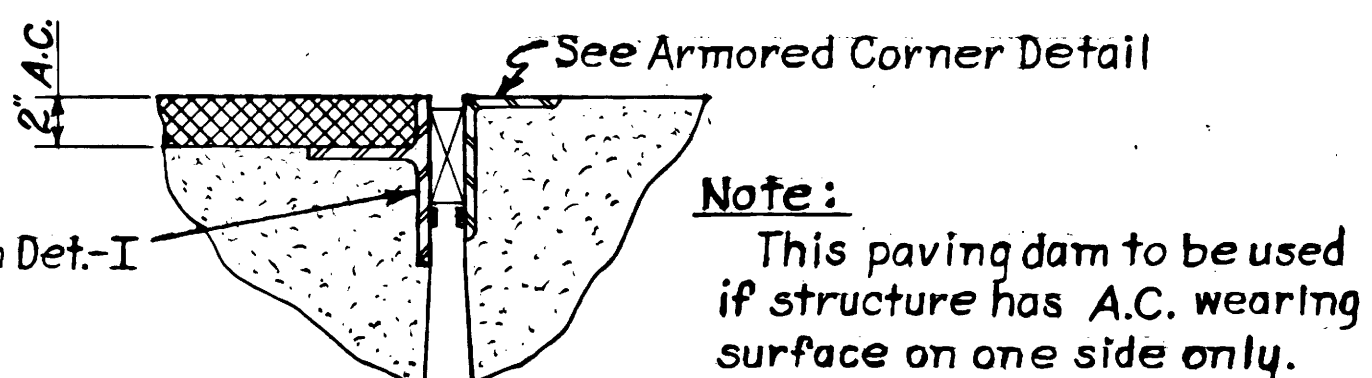
DIMENSIONS FOR ALTERNATE 2 BRIDGE SEAL				
TYPE	WIDTH	DEPTH	THICKNESS (NOTES)	
B	$1\frac{3}{4} \pm 0.156$ $-0.063$	$1\frac{3}{4} \pm 0.062$	$\frac{5}{64}$	$\frac{5}{64}$
C	$2 \pm 0.156$ $-0.063$	$2 \pm 0.062$	$\frac{3}{32}$	$\frac{3}{32}$

			JOINT LIMITS	
			MIN. (NOTE 1)	MAX. (NOTE 2)
D	$2\frac{1}{2}^{+0.1875}_{-0.00}$	$2\frac{5}{8}^{+0.125}_{-0.00}$	1"	2 <sup>1</sup> / <sub>8</sub> "
E	$3^{+0.1875}_{-0.00}$	$3\frac{1}{4}^{+0.1875}_{-0.00}$	1 <sup>1</sup> / <sub>4</sub> "	2 <sup>1</sup> / <sub>2</sub> "
F	$4^{+0.250}_{-0.00}$	$4\frac{3}{8}^{+0.1875}_{-0.00}$	1 <sup>3</sup> / <sub>4</sub> "	3 <sup>3</sup> / <sub>8</sub> "

NOTE

1. Minimum dimension: Force shall not exceed 50lbs per linear inch.
2. Maximum dimension: Force shall not be less than 10lbs per linear inch.
3. Web and shell tolerance not needed as Force - Deflection criterion assures tolerances within parameter of Movement Category.

TYPE	SEAL SIZE		GROOVE Width	
	Width	Depth	Min.(Note#1)	Max.(Note#2)
B	1 3/4"	2"	.687"	1.487"
C	2"	2"	.875"	1.70"
D	2 1/2"	2 3/4"	1.00"	2.125"
E	3"	3 3/8"	1.125"	2.55"
F	4"	4 3/8"	1.625"	3.40"



## PAVING DAM DETAILS-II

DATE	REVISION	DATE	REVISION
10-17-70	Note Revision	5-23-68	Weld Option
12-11-70	Sealer at sidewalk/Ks	5-23-68	Detail "A"
5-9-72	Added note	4-26-69	Alternate 2 Bridge Seal
7-18-72	Curb, parapet, sw. details	9-3-69	Joint Sealer Splice
2-4-72	Existing Struct. Jt.	9-14-70	Dimension
4-19-73	Armored corner	10-1-70	Alternate 2 Bridge Seal
7-31-73	ADDED ALT. #3 SEAL	APPROVED:	
10-9-74	ADDED PAVING DAM DET. II	DRAWN BY: <i>[Signature]</i>	
		BIDGE ENGINEER	
		DESIGNED:	CHECKED:
		DRAWN: GDR	CALC. BOOK:

**OREGON STATE HIGHWAY DEPARTMENT  
BRIDGE DIVISION**

## STANDARD EXPANSION JOINT DETAIL

**WITH AND WITHOUT ARMORED CORNERS**

DATE April 7, 1967 SHEET \_\_\_\_\_ OF \_\_\_\_\_

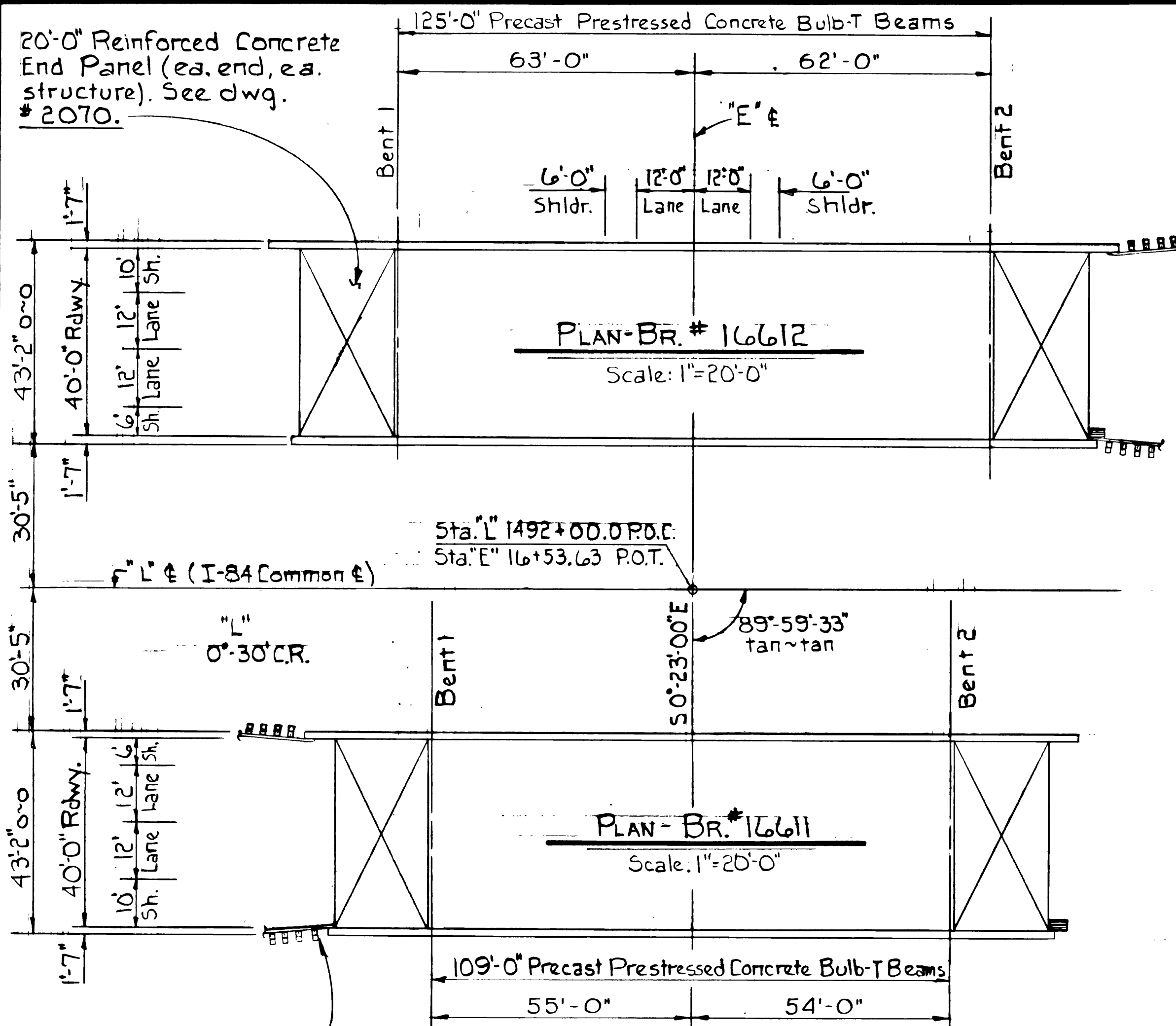
BRIDGE NO. **STANDARD** DRAWING NO. **22970**

VOLD See diag no. 40791.

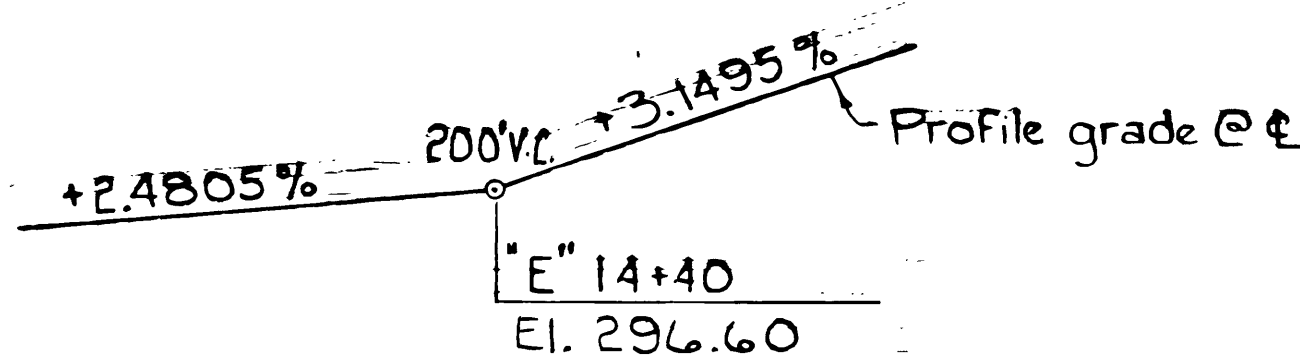
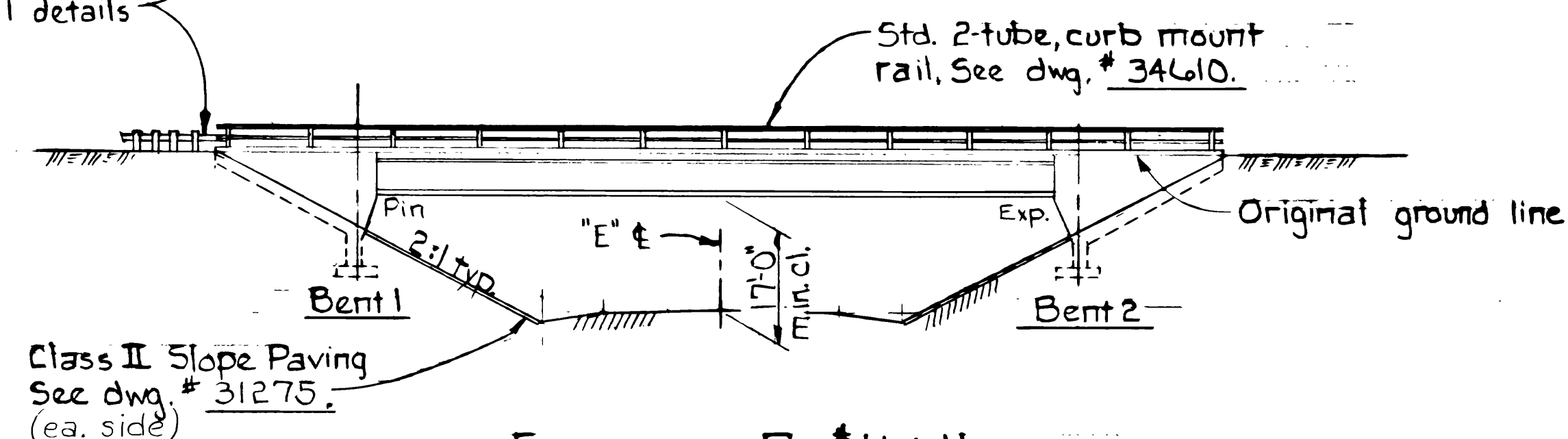




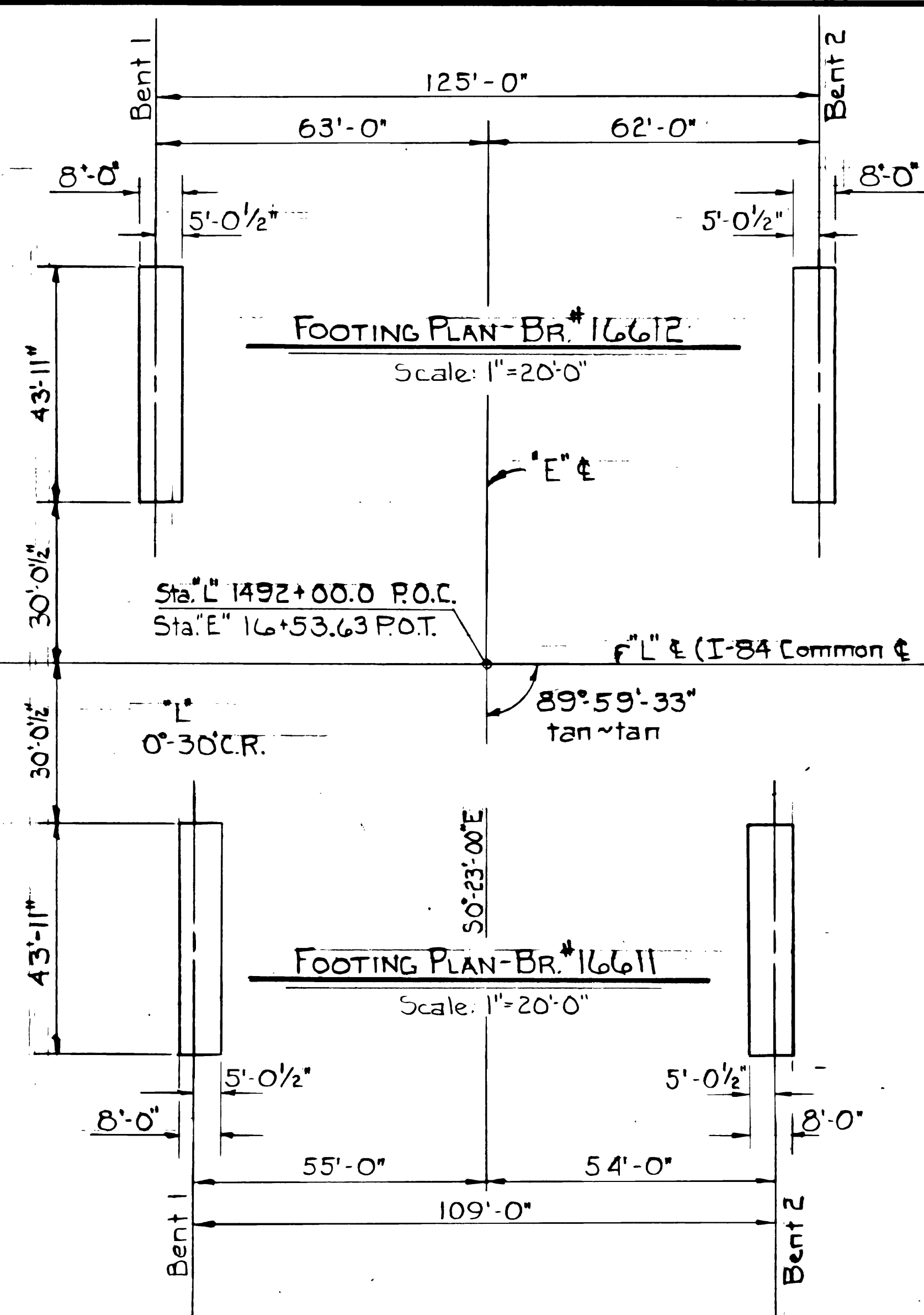
20'-0" Reinforced Concrete End Panel (ea. end, ea. structure). See dwg. #2070.



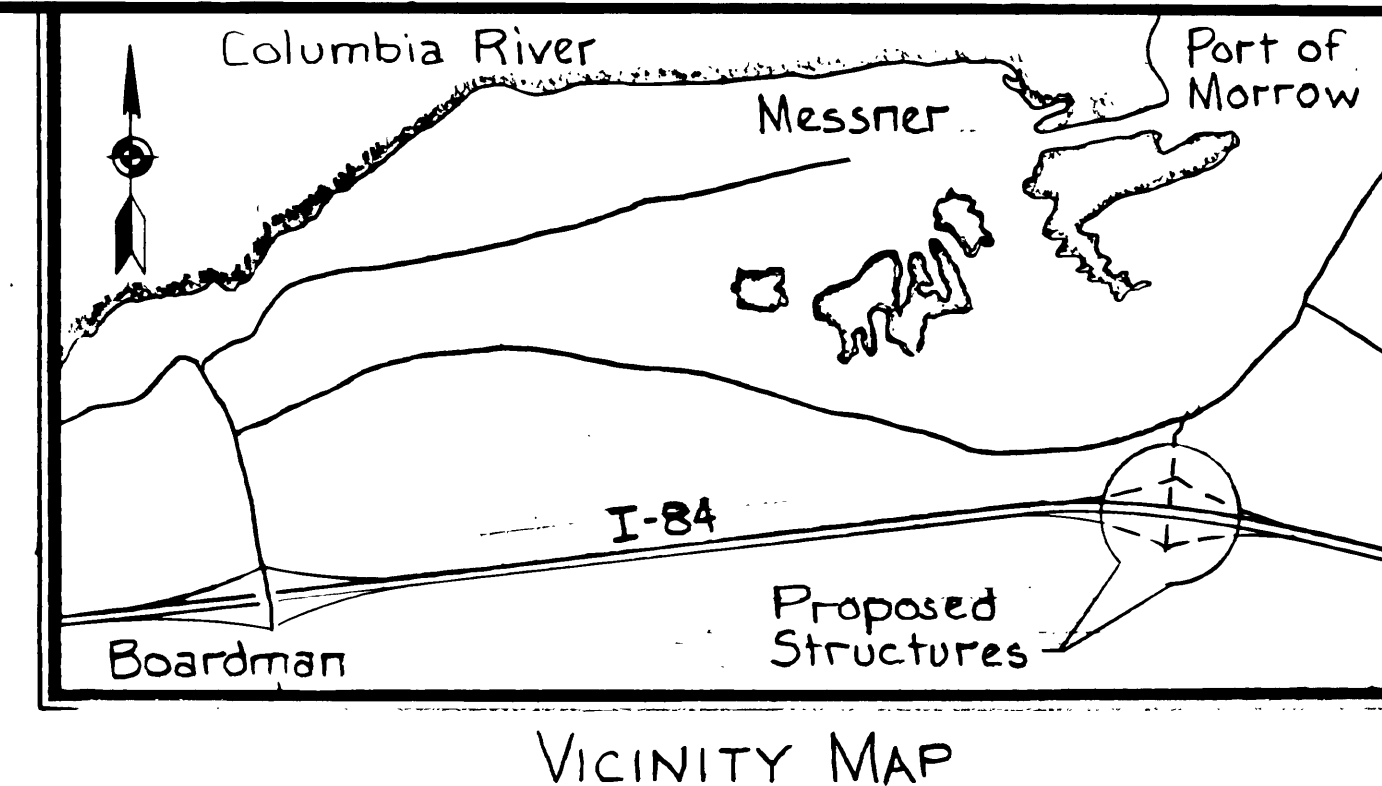
See Grading Plans for guardrail details



FOOTING PLAN - BR. # 16612  
Scale: 1"=20'-0"



NOTE:  
All bents are 90° to a chord between E's.



#### GENERAL NOTES:

All material and workmanship shall conform to the Standard Specifications for Highway Construction of the Oregon State Highway Division.

Bridges designed for H525 and Military loadings with an allowance of 50psf for future wearing surface.

Concrete members except prestressed members designed by Load Factor Design Method.

Prestressing steel shall be in accordance with detail plans.

All other reinforcing steel shall conform to ASTM Specification A615 (SI) Grade 60. The following splice lengths shall be used unless shown otherwise:

Bar Size	3	4	5	6	7	8
Splice Length	1'-0"	1'-4"	1'-8"	2'-0"	2'-9"	3'-7"

All reinforcing steel in the upper portion of the deck shall be epoxy coated. This includes top longitudinal bars and top transverse bars including "truss" bars and all bars extending from the deck into the curb.

All bars shall be placed 2" clear of the nearest face of concrete unless shown otherwise. The top bends of stirrups extending from beam stems into the top slab shall be field bent.

Reinforcing steel for abutments and wingwalls shall not be fabricated until final footing elevations have been determined in the field.

Concrete in prestressed precast beams shall be as shown on the detail plans.

Concrete in deck shall be Class 4,500-1 1/2".  
All other concrete shall be Class 3,300-1 1/2".

APPROVED: *Walter J. Robert*  
BRIDGE ENGINEER  
P.E.  
ASST. STATE HIGHWAY ENGINEER  
DESIGNED: W.M. Thompson  
DRAWN: Silbernagel  
CHECKED: Page  
REVIEWED: M. Tindall 11-27-83  
CALC. BOOK: 1919

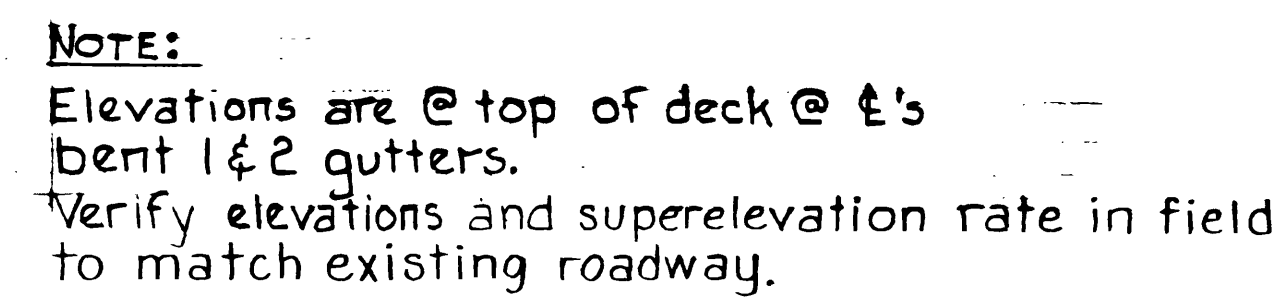
OREGON DEPARTMENT OF TRANSPORTATION  
BRIDGE DESIGN SECTION

PORT OF MORROW INTERCHANGE  
PORT OF MORROW INTERCHANGE SECTION  
COLUMBIA RIVER HWY. (I-84)  
MORROW COUNTY

PLAN & ELEVATION

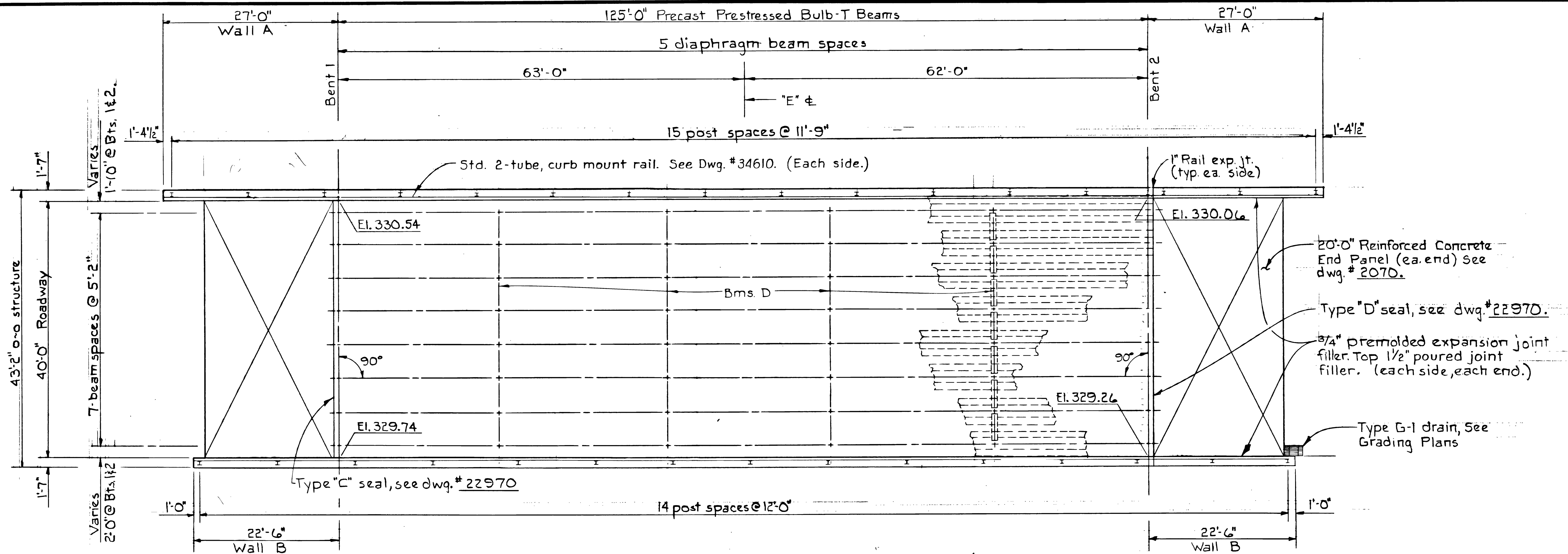
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DATE Jan. 1983 SHEET 1 OF 14  
BRIDGE NO. 16611 & 16612 DRAWING NO. 38013



PERMANING 44-131 30825

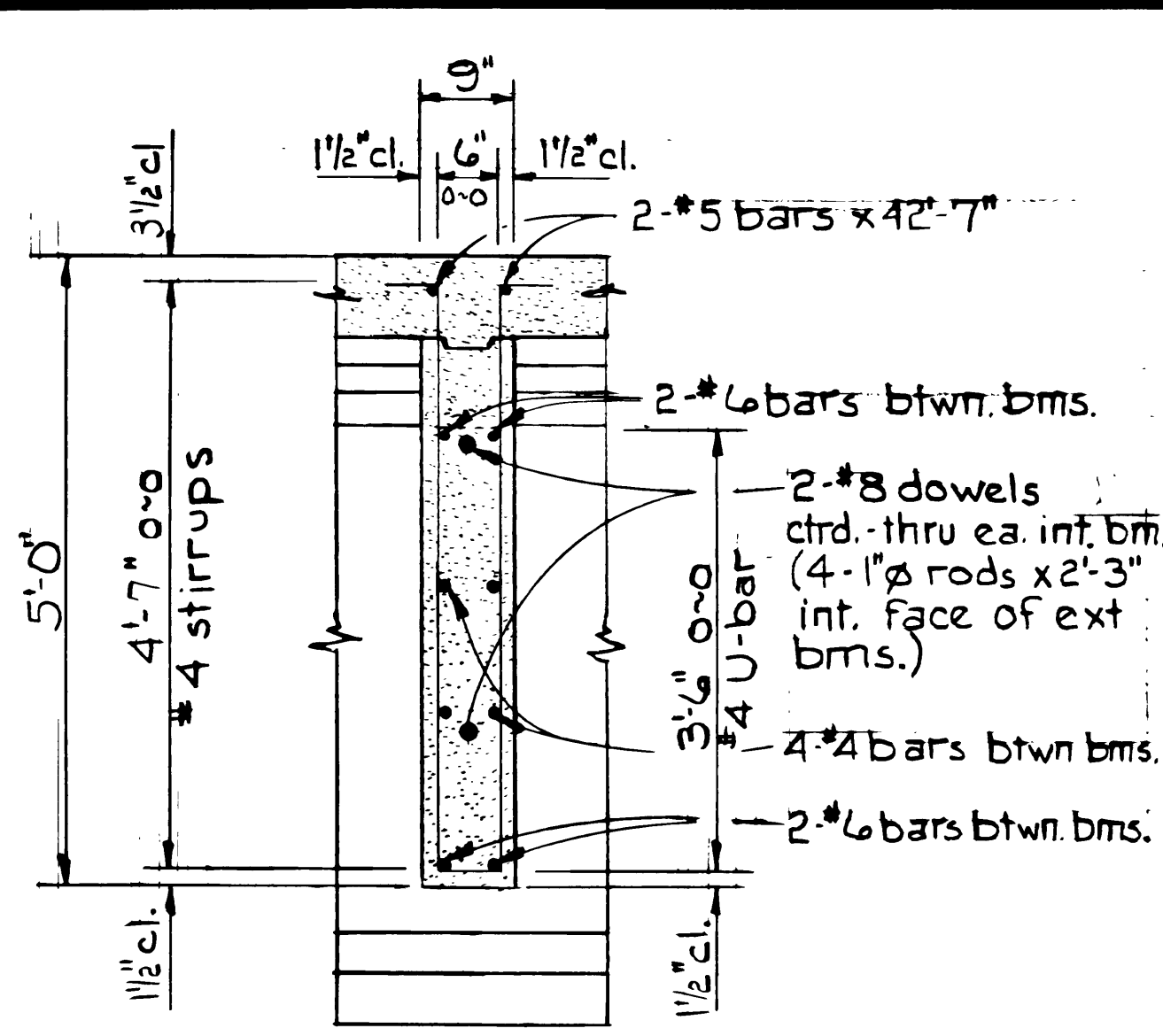
APPROVED: <u>Walter J. Blum</u> <span style="float: right;">P.E.</span> BRIDGE ENGINEER DESIGNED <u>W. M. Thompson</u> DRAWN <u>J. Silbernagel</u> CHECKED <u>Page</u> REVIEWED <u>J. M. Tindall 11/6/83</u> REVIEWED <u>J.E.B. 1-25-83</u> CALC. BOOK <u>1915</u>	OREGON DEPARTMENT OF TRANSPORTATION BRIDGE DESIGN SECTION  <div style="font-size: 2em; font-weight: bold;">PORT OF MORROW INTERCHANGE</div>																		
<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th style="width: 30%;">DATE</th> <th>REVISION</th> </tr> </thead> <tbody> <tr> <td>6/17/85</td> <td>AS CONSTRUCTED</td> </tr> <tr><td> </td><td> </td></tr> <tr><td> </td><td> </td></tr> <tr><td> </td><td> </td></tr> <tr><td> </td><td> </td></tr> <tr><td> </td><td> </td></tr> </tbody> </table>	DATE	REVISION	6/17/85	AS CONSTRUCTED											<div style="font-size: 2em; font-weight: bold;">DECK PLAN - BR. # 16611</div> <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 50%;">DATE <u>Jan. 1983</u></td> <td style="width: 50%;">SHEET <u>3</u> OF <u>14</u></td> </tr> <tr> <td>BRIDGE NO. <u>16611 &amp; 16612</u></td> <td>DRAWING NO. <u>38015</u></td> </tr> </table>	DATE <u>Jan. 1983</u>	SHEET <u>3</u> OF <u>14</u>	BRIDGE NO. <u>16611 &amp; 16612</u>	DRAWING NO. <u>38015</u>
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6/17/85	AS CONSTRUCTED																		
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BRIDGE NO. <u>16611 &amp; 16612</u>	DRAWING NO. <u>38015</u>																		



**NOTE:**  
 Elevations are @ top of deck @  
 Bent 1 & 2 gutters.  
 Verify elevations and superelevation rate  
 in field to match existing roadway.

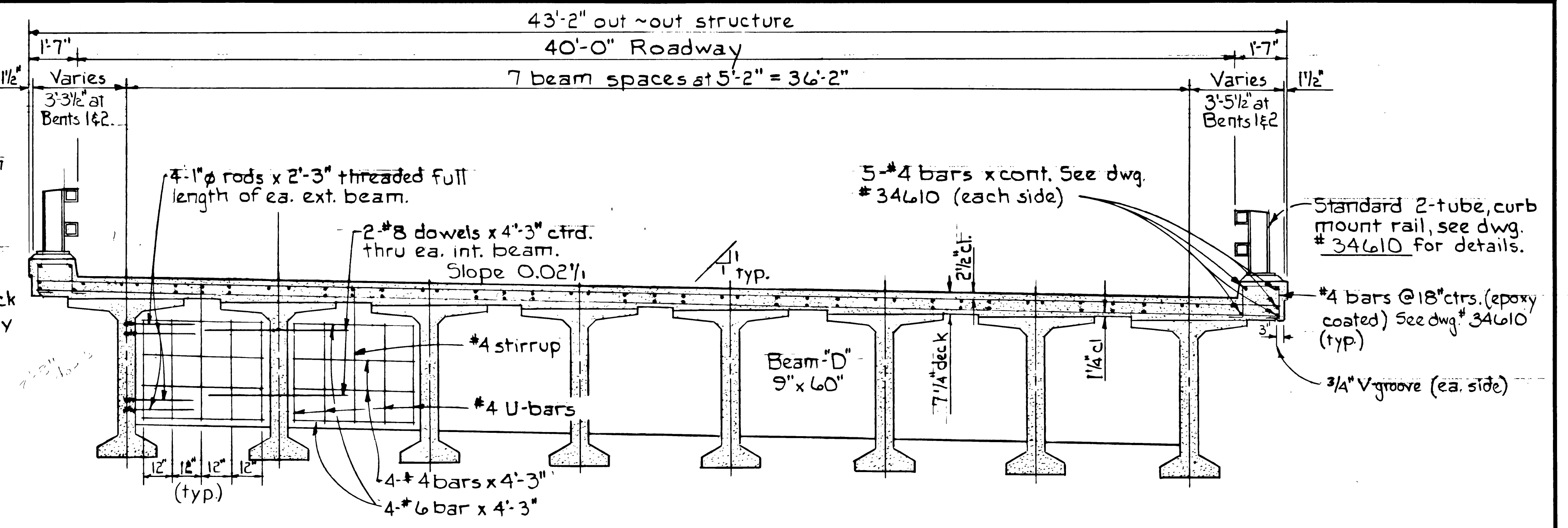
Scale: 1/8" = 1'-0"

APPROVED: <i>[Signature]</i> BRIDGE ENGINEER		OREGON DEPARTMENT OF TRANSPORTATION BRIDGE DESIGN SECTION	
DESIGNED: W.M. Thompson DRAWN: J. Silbernagel CHECKED: Page REVIEWED: J.M. Tiedall 1/5/83 CALC. BOOK: 1319		PORT OF MORROW INTERCHANGE	
DATE: 6/17/85 REVISION: AS CONSTRUCTED		DECK PLAN BR.# 16612	
DATE: Jan. 1983 BRIDGE NO. 16611 & 16612		SHEET 4 OF 14 DRAWING NO. 38016	

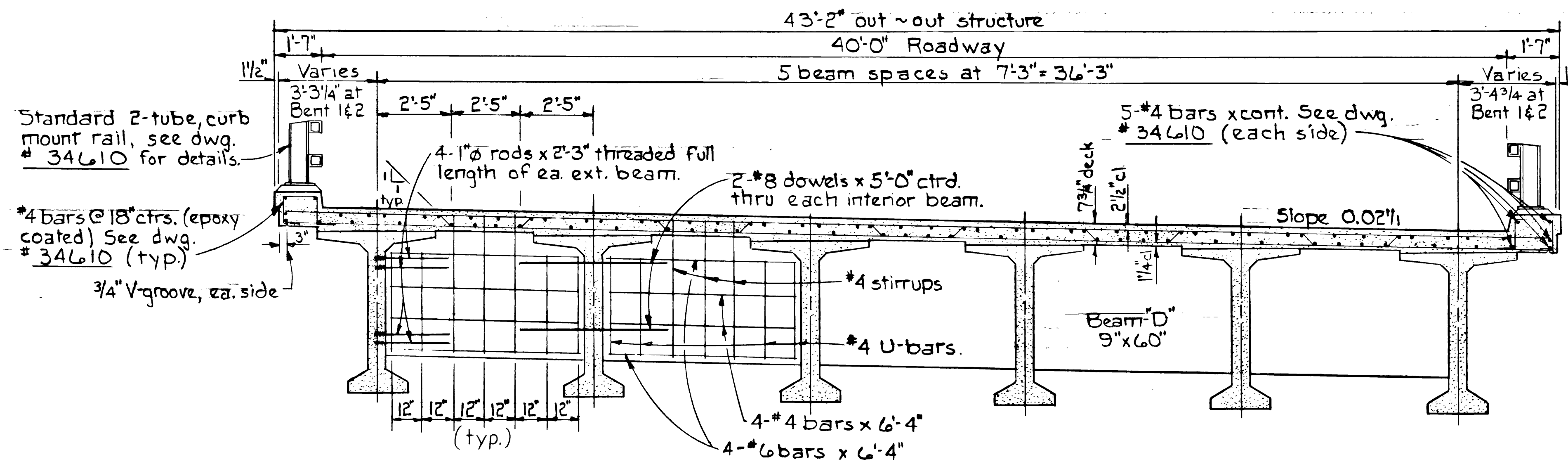


**BEAM D - BR. #16611 & 16612**  
Scale: 3/4"=1'-0"

**DECK STEEL: BR. #16612**  
#5 straight transverse bars at 10" ctrs., top & bottom.  
Place transverse bars parallel with bent bars.  
Stop transverse bars 6"± clear of all transverse beams.  
Top straight bars shall be epoxy coated.  
39 - #4 longitudinal bars in top of deck (includes hanger bars) as shown (epoxy coated).  
21 - #5 longitudinal bars in bottom of deck as shown.

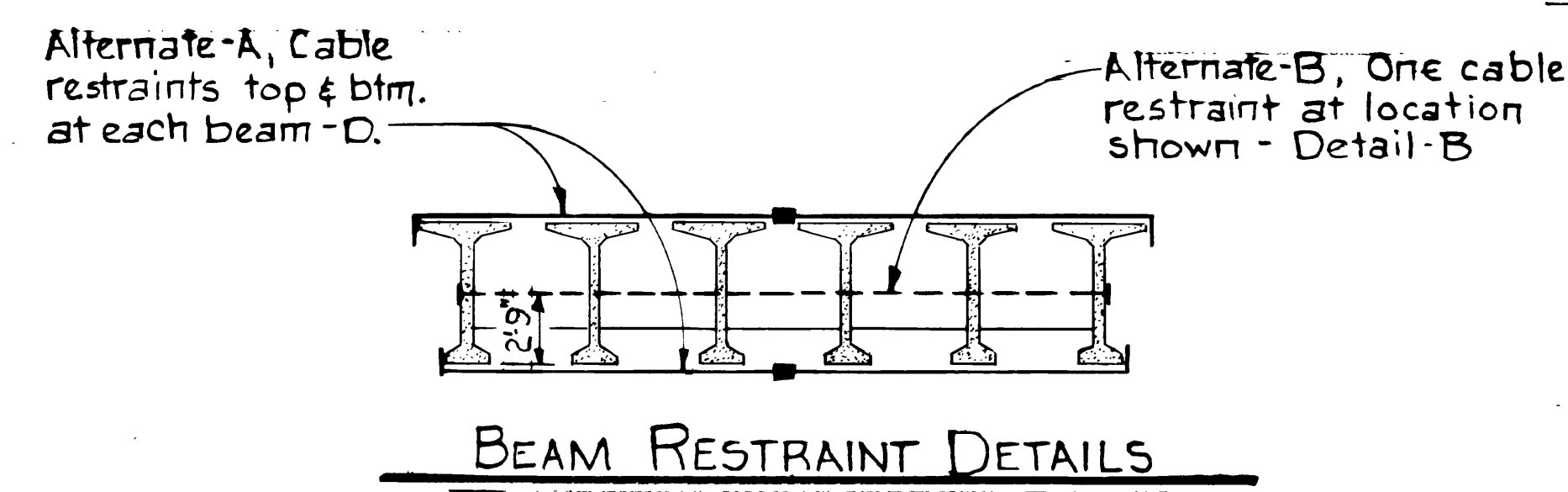
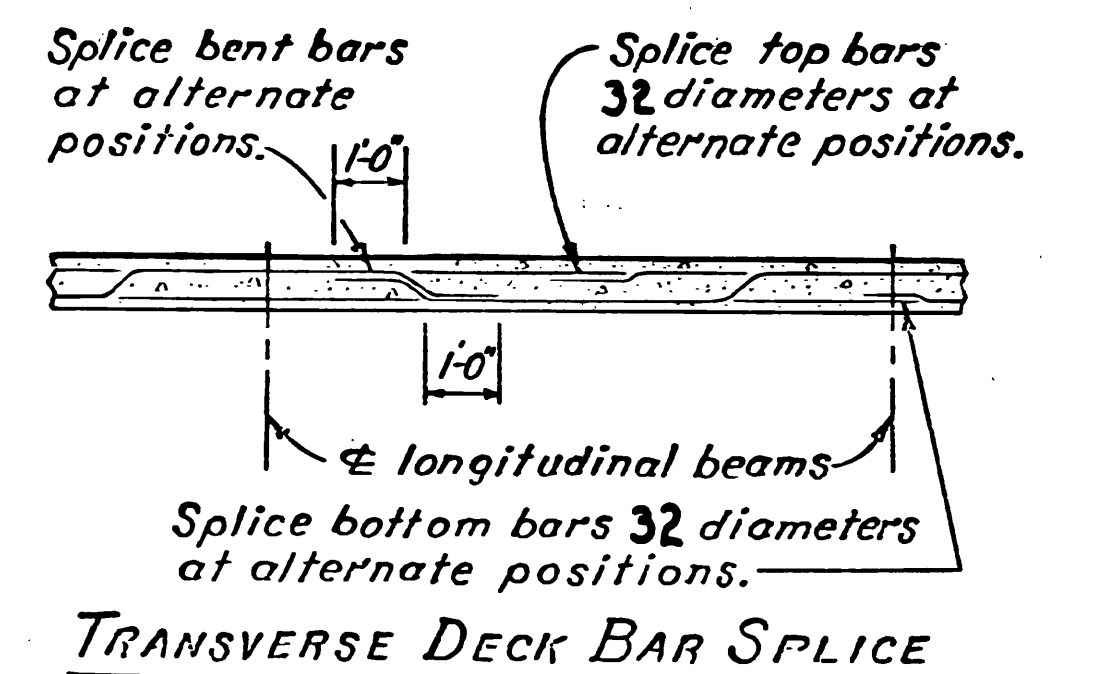


**DECK SECTION - BR. #16612**  
Scale: 3/8"=1'-0"



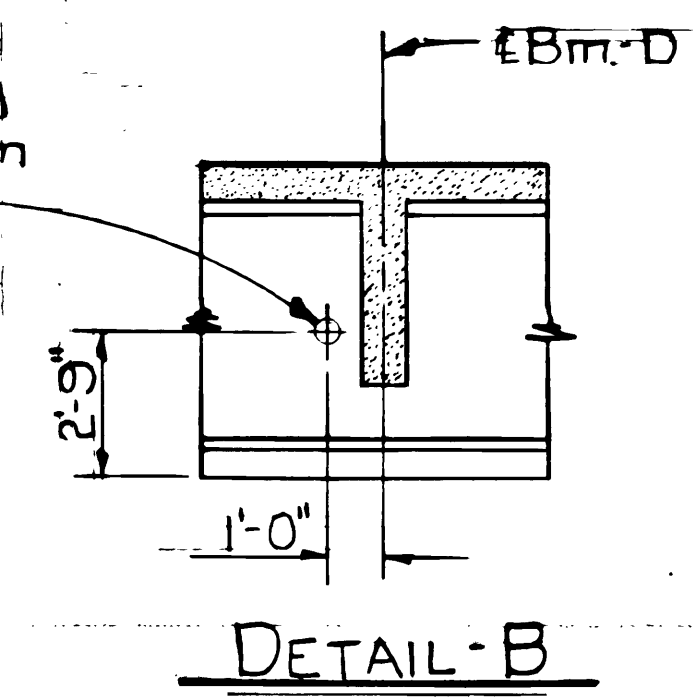
**DECK SECTION - BR. #16611**  
Scale: 3/8"=1'-0"

**DECK STEEL - BR. #16611**  
#5 straight transverse bars at 17" ctrs. top & bottom.  
#5 bent transverse bars at 17" ctrs. Alternate straight & bent bars at 8 1/2" ctrs.  
Place transverse bars parallel with bent bars.  
Stop transverse bars 6"± clear of all transverse beams.  
Top straight & bent bars shall be epoxy coated.  
44 - #4 longitudinal bars in top of deck (includes hanger bars) as shown (epoxy coated).  
32 - #5 longitudinal bars in bottom of deck as shown.



Snug fit prestressed beams against forms prior to diaphragm beam pour. Let concrete take initial set then revibrate. Restraints to remain in place a minimum of two days after completion of diaphragm pour. (Shown for Br. #16611, Br. #16612 is similar)

1" dia. hole thru all precast beams at ea. beam-D. After restraint is removed, fill hole with concrete and finish flush with surface of beam (ext. bms only)



APPROVED: *Walter J. Blum*  
BRIDGE ENGINEER  
DESIGNED: W.M. Thompson  
DRAWN: J. Silbernagel  
CHECKED: Page  
REVIEWED: J.E. B. 1-25-83  
CALC. BOOK 1913

DATE	REVISION
6/11/85	AS CONSTRUCTED

OREGON DEPARTMENT OF TRANSPORTATION BRIDGE DESIGN SECTION	
PORT OF MORROW INTERCHANGE	
DECK SECTIONS	
DATE Jan. 1983	SHEET 5 OF 14
BRIDGE NO. 16611 & 16612	DRAWING NO. 38017