



ME 1071: Applied Fluids

Lecture 7 Flow in Open Channels

Spring 2021

Weekly Study Plan



| Weeks | Dates | Lectures |
|-----------|----------------|---|
| 1 | Mar. 9 | Course Introduction, Fluids Review |
| 2 | Mar. 16 | Chapter 8: Internal Incompressible Viscous Flow |
| 3 | Mar. 23 | Chapter 8: Internal Incompressible Viscous Flow |
| 4 | Mar. 30 | Chapter 8/Exam I Review |
| 5 | Apr. 6 | Exam I |
| 6 | Apr. 13 | Chapter 9: External Incompressible Viscous Flow |
| 7 | Apr. 20 | Chapter 9: External Incompressible Viscous Flow |
| 8 | Apr. 25 | Chapter 11: Flow in Open Channels |
| 9 | Apr. 27 | Chapter 11: Flow in Open Channels |
| 10 | May. 11 | Exam II Review |
| 11 | May. 18 | Exam II |
| 12 | May. 25 | Chapter 12: Introduction to Compressible Flow |
| 13 | Jun. 1 | Chapter 12: Introduction to Compressible Flow |
| 14 | Jun. 8 | Chapter 12: Introduction to Compressible Flow |
| 15 | Jun. 15 | Chapter 5: CFD Related Topics |
| 16 | Jun. 22 | Final Exam Review |

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Outlines



- **Flow in Open Channel**
- **The Hydraulic Jump**

Outlines



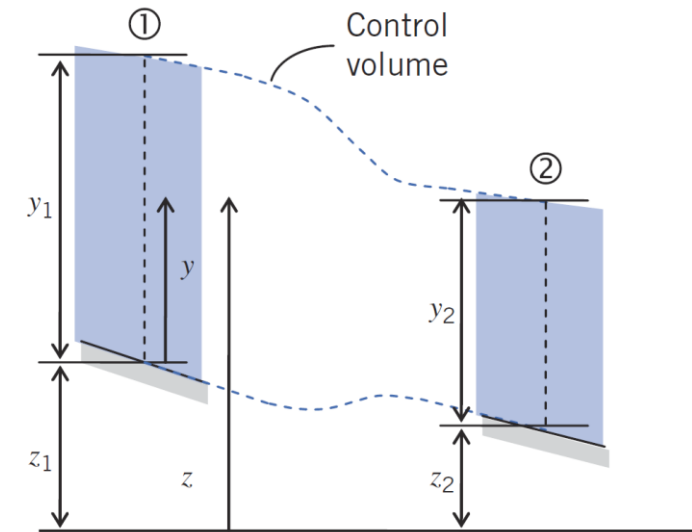
- **Flow in Open Channel**
- **The Hydraulic Jump**

Energy Equation for Open-Channel Flows



Assumptions

1. Steady flow.
2. Incompressible flow.
3. Uniform velocity at a section.
4. Gradually varying depth so that pressure distribution is hydrostatic.
5. Small bed slope.
6. $W_s = W_{\text{shear}} = W_{\text{other}} = 0$.



Energy Equation for Open-Channel Flow

$$\frac{V_1^2}{2g} + y_1 + z_1 = \frac{V_2^2}{2g} + y_2 + z_2 + H_l$$

Total Head or Energy Head

$$H = \frac{V^2}{2g} + y + z$$

$$H_1 - H_2 = H_l$$

Specific Energy

$$E = \frac{V^2}{2g} + y$$

$$E_1 - E_2 + z_1 - z_2 = H_l$$

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Energy Equation for Open-Channel Flows



The Specific Energy

- E indicates actual energy (kinetic plus potential/pressure per unit mass flow rate) being carried by the flow

$$E = \frac{V^2}{2g} + y$$

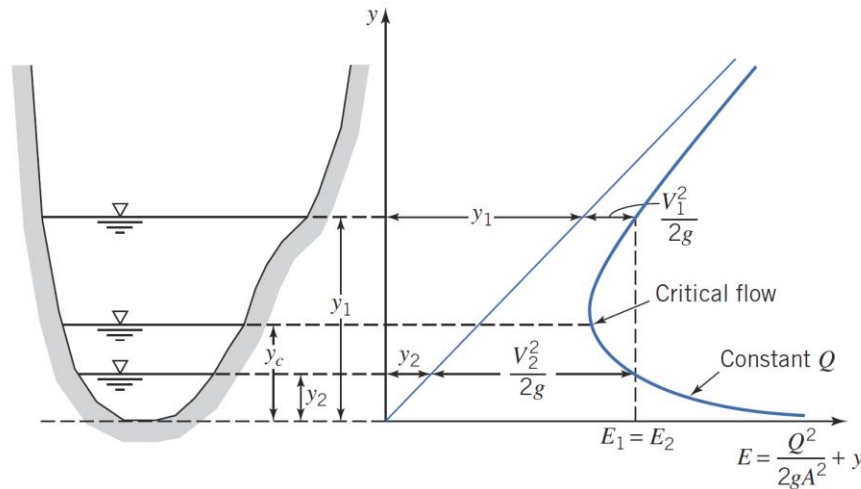


Fig. 11.7 Specific energy curve for a given flow rate.

Critical Depth ($Fr = 1$)

$$Q^2 = \frac{gA_c^3}{b_{sc}}$$

$$V_c = \sqrt{gy_{hc}}$$

Minimum Specific Energy

- the specific energy is at its minimum at critical conditions, i.e., $Fr = 1$.

$$y_c = \left[\frac{Q^2}{gb^2} \right]^{1/3} \quad E_{\min} = \frac{3}{2}y_c \quad (\text{Rectangular channel})$$

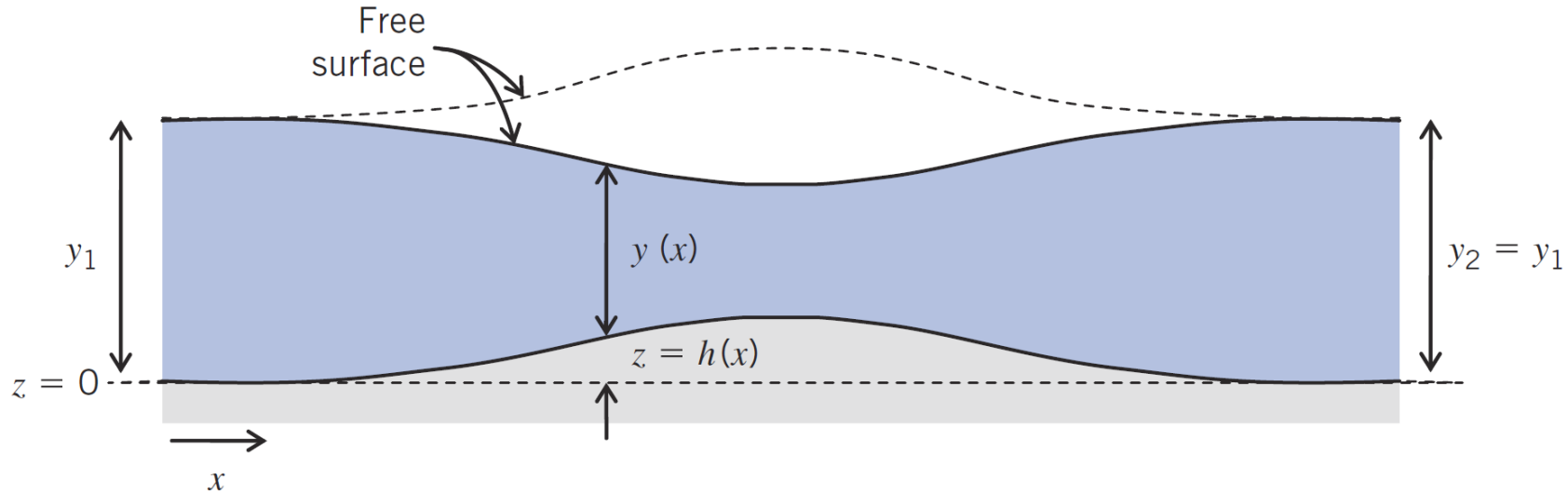
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Localized Effect of Area Change



Flow over a Bump

$$\frac{V_1^2}{2g} + y_1 + z_1 = \frac{V_2^2}{2g} + y_2 + z_2 = \frac{V^2}{2g} + y + z = \text{const}$$



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Localized Effect of Area Change



Flow over a Bump



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Localized Effect of Area Change



Example

A rectangular channel 2 m wide has a flow of 2.4 m³/s at a depth of 1.0 m. Determine whether critical depth occurs at:

- a) A section where a bump of height $h = 0.20$ m is in the channel bed.
- b) A side wall constriction that reduces the channel width to 1.7 m.
- c) A combined bump and side wall constrictions.

$$y_c = \left[\frac{Q^2}{gb^2} \right]^{1/3}$$

$$E_1 = y_1 + \frac{Q^2}{2gA^2} = y_1 + \frac{Q^2}{2gb^2y_1^2}$$

$$E_{min} = \frac{3}{2}y_c$$

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The Hydraulic Jump



Phenomenon



Hydraulic Jump:

The jump or standing wave formed when the depth of flow of water changes from supercritical to subcritical state

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The Hydraulic Jump

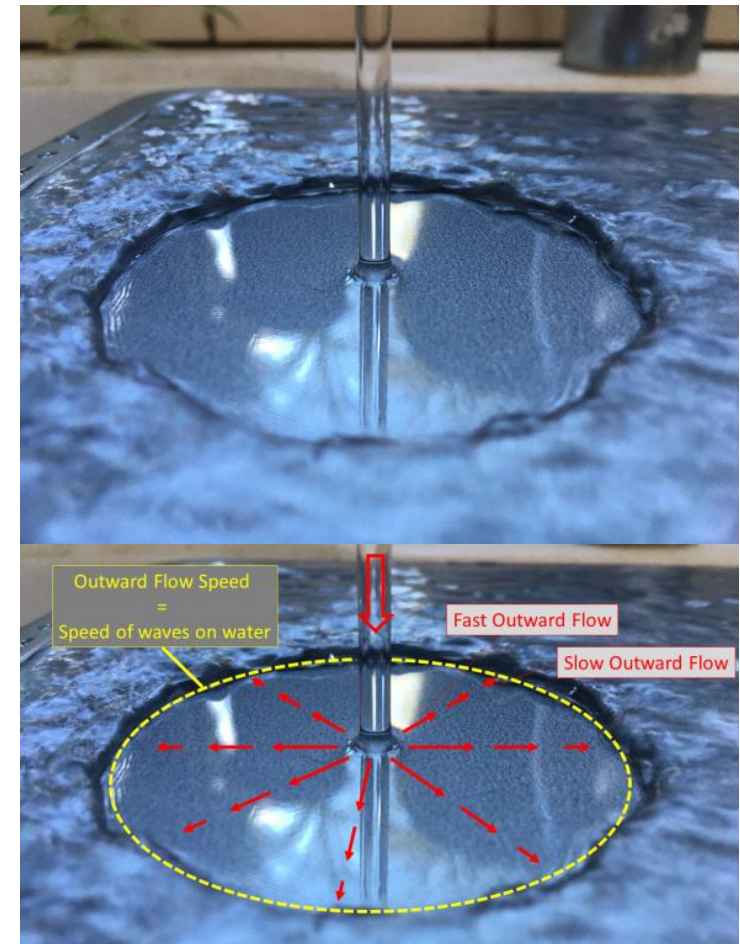
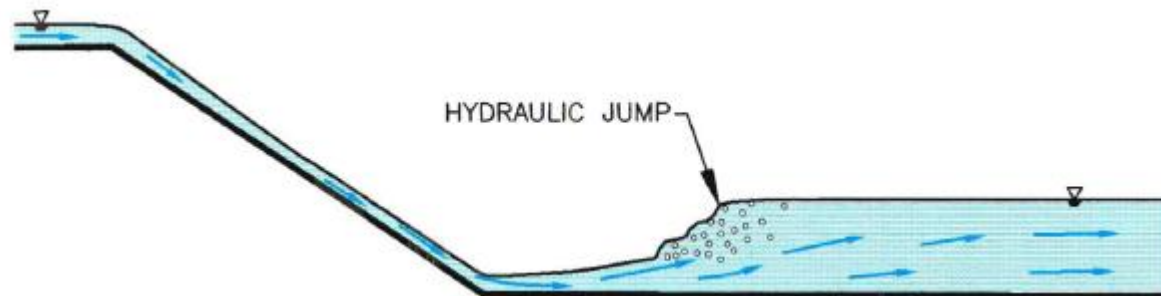


Subcritical Flow $Fr < 1$

- Disturbances move upstream, smooth adjustment

Supercritical Flow $Fr > 1$

- Disturbances cannot move upstream
- The transition from supercritical to subcritical flow occurs abruptly through a **hydraulic jump**.

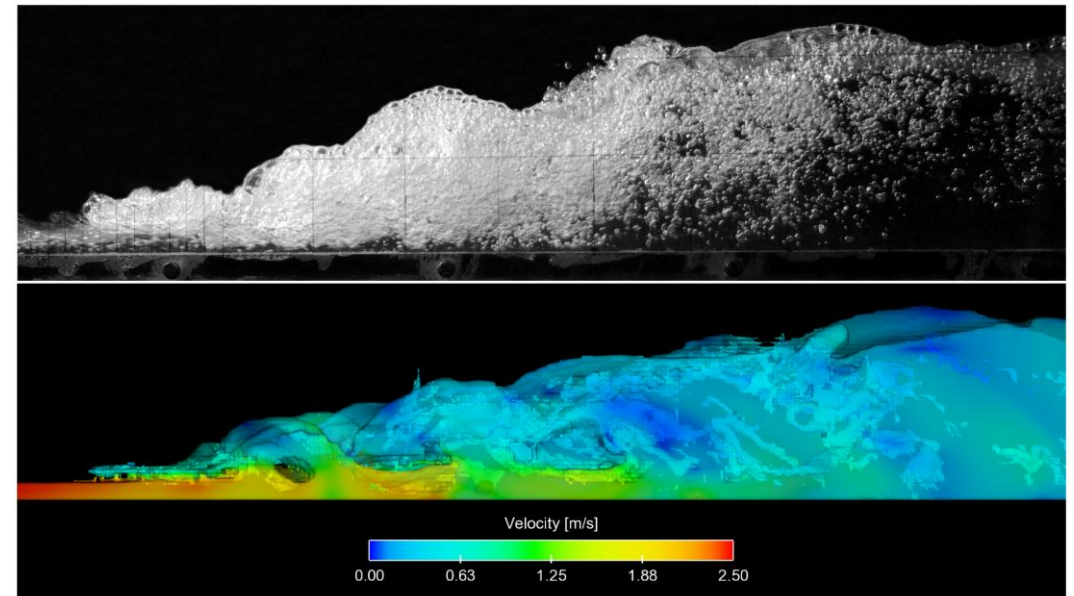
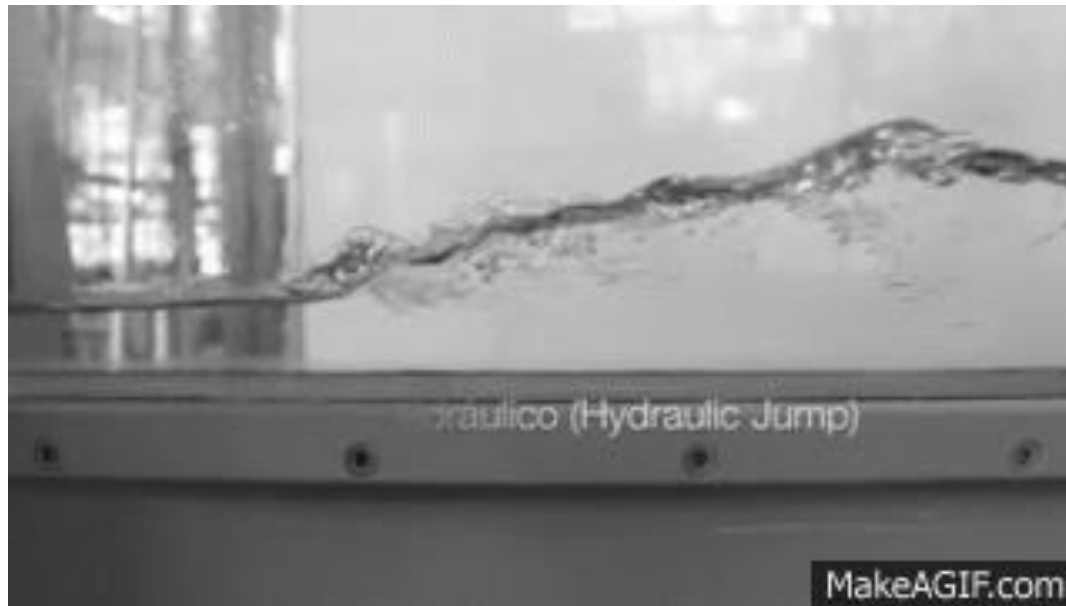


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The Hydraulic Jump



More Examples of Hydraulic Jump



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The Hydraulic Jump

More Examples of Hydraulic Jump



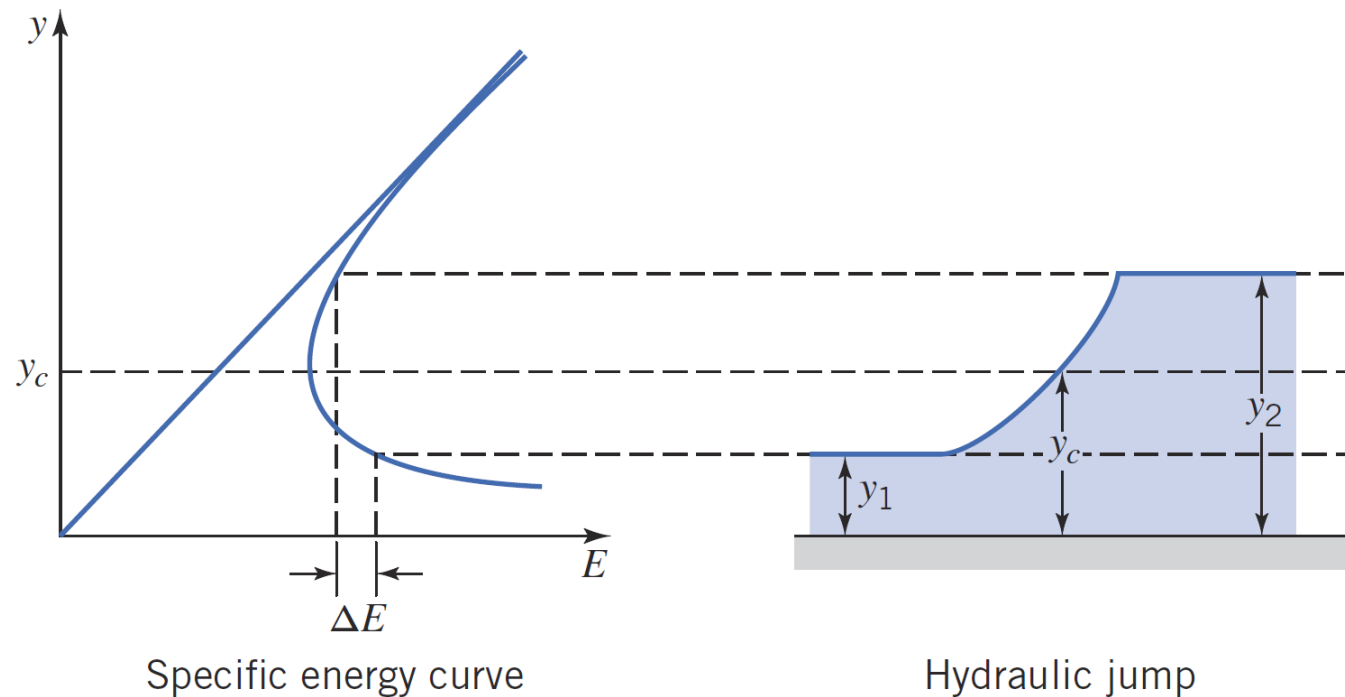
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The Hydraulic Jump



Specific Energy Curve for Hydraulic Jump

- Unlike the changes due to phenomena such as a bump, the abrupt change in depth involves a **significant loss of mechanical energy through turbulent mixing**.



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The Hydraulic Jump



Governing Equations for Hydraulic Jump

$$\text{Continuity} \quad V_1 y_1 = V_2 y_2$$

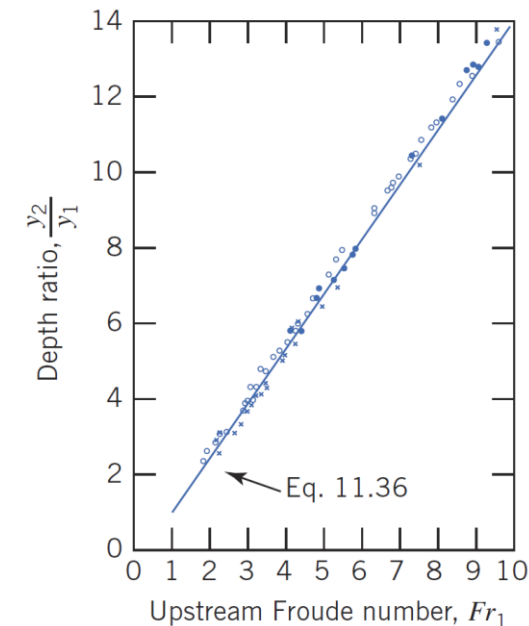
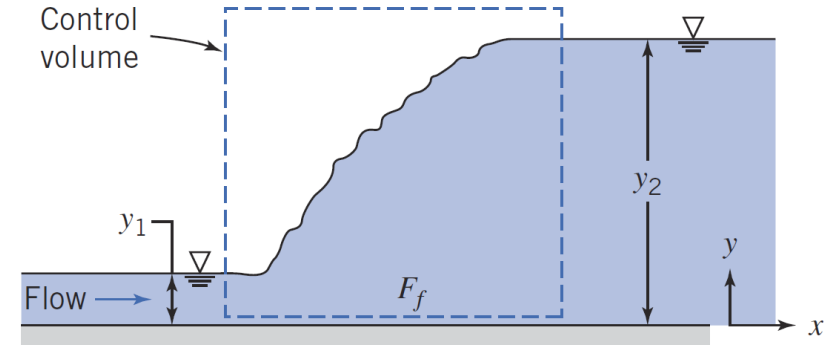
$$\text{Momentum} \quad \frac{V_1^2 y_1}{g} + \frac{y_1^2}{2} = \frac{V_2^2 y_2}{g} + \frac{y_2^2}{2}$$

$$\text{Energy} \quad E_1 = \frac{V_1^2}{2g} + y_1 = \frac{V_2^2}{2g} + y_2 + H_l = E_2 + H_l$$

Depth Increase Across a Hydraulic Jump

- The ratio of downstream to upstream depths across a hydraulic jump is only a function of the upstream Froude number.

$$\frac{y_2}{y_1} = \frac{1}{2} [\sqrt{1 + 8Fr_1^2} - 1], \quad Fr_1 > 1$$



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The Hydraulic Jump



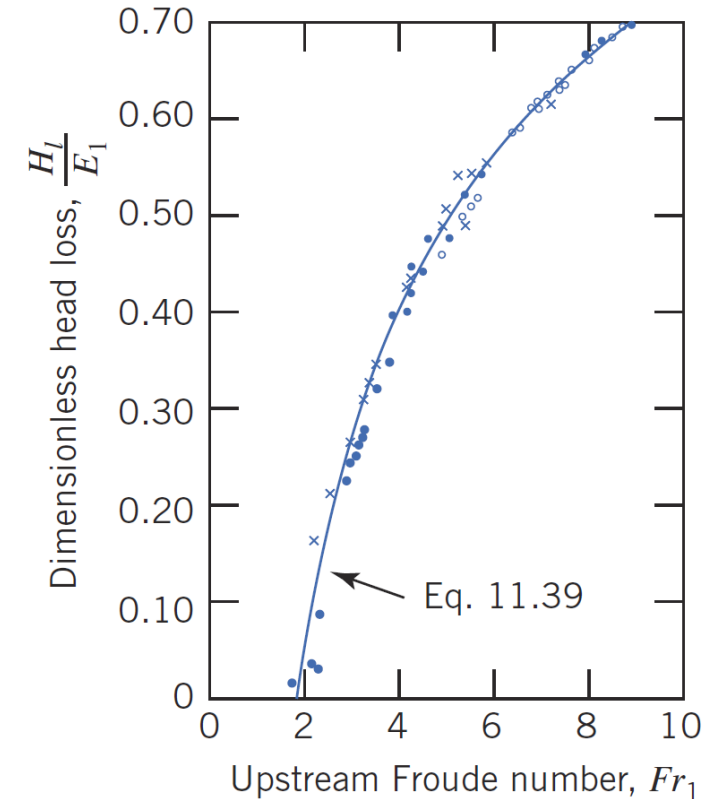
Head Loss Across a Hydraulic Jump

- The head loss is only a function of the upstream Froude number.
- Hydraulic jump can occur only in supercritical flow.
- Flow downstream from a jump always is subcritical.

$$H_l = \frac{[y_2 - y_1]^3}{4y_1y_2}, \quad y_2 > y_1$$

$$\frac{H_l}{E_1} = \frac{[\sqrt{1 + 8Fr_1^2} - 3]^3}{8[\sqrt{1 + 8Fr_1^2} - 1][Fr_1^2 + 2]}, \quad Fr_1 > 1$$

Energy Dissipation Ratio

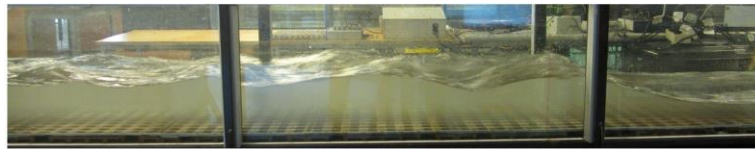


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The Hydraulic Jump



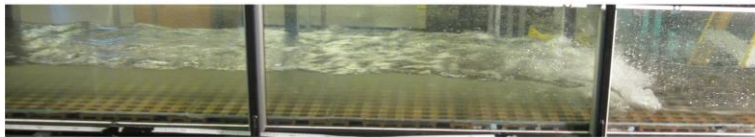
Hydraulic Jump Classification



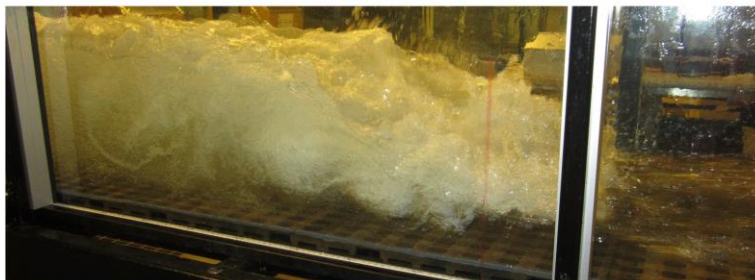
(a)



(b)



(c)



(d)

Classification of hydraulic jumps

Source: U.S. Bureau of Reclamation (1955).

| Upstream Fr_1 | Depth Ratio y_2/y_1 | Fraction of Energy Dissipation | Description | Surface Profile |
|-----------------|-----------------------|--------------------------------|--|-----------------|
| <1 | 1 | 0 | <i>Impossible jump.</i> Would violate the second law of thermodynamics. | |
| 1–1.7 | 1–2 | $<5\%$ | <i>Undular jump (or standing wave).</i> Small rise in surface level. Low energy dissipation. Surface rollers develop near $Fr = 1.7$. | |
| 1.7–2.5 | 2–3.1 | 5–15% | <i>Weak jump.</i> Surface rising smoothly, with small rollers. Low energy dissipation. | |
| 2.5–4.5 | 3.1–5.9 | 15–45% | <i>Oscillating jump.</i> Pulsations caused by entering jets at the bottom generate large waves that can travel for miles and damage earth banks. Should be avoided in the design of stilling basins. | |
| 4.5–9 | 5.9–12 | 45–70% | <i>Steady jump.</i> Stable, well-balanced, and insensitive to downstream conditions. Intense eddy motion and high level of energy dissipation within the jump. Recommended range for design. | |
| >9 | >12 | 70–85% | <i>Strong jump.</i> Rough and intermittent. Very effective energy dissipation, but may be uneconomical compared to other designs. | |

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The Hydraulic Jump

Hydraulic Jump Variation



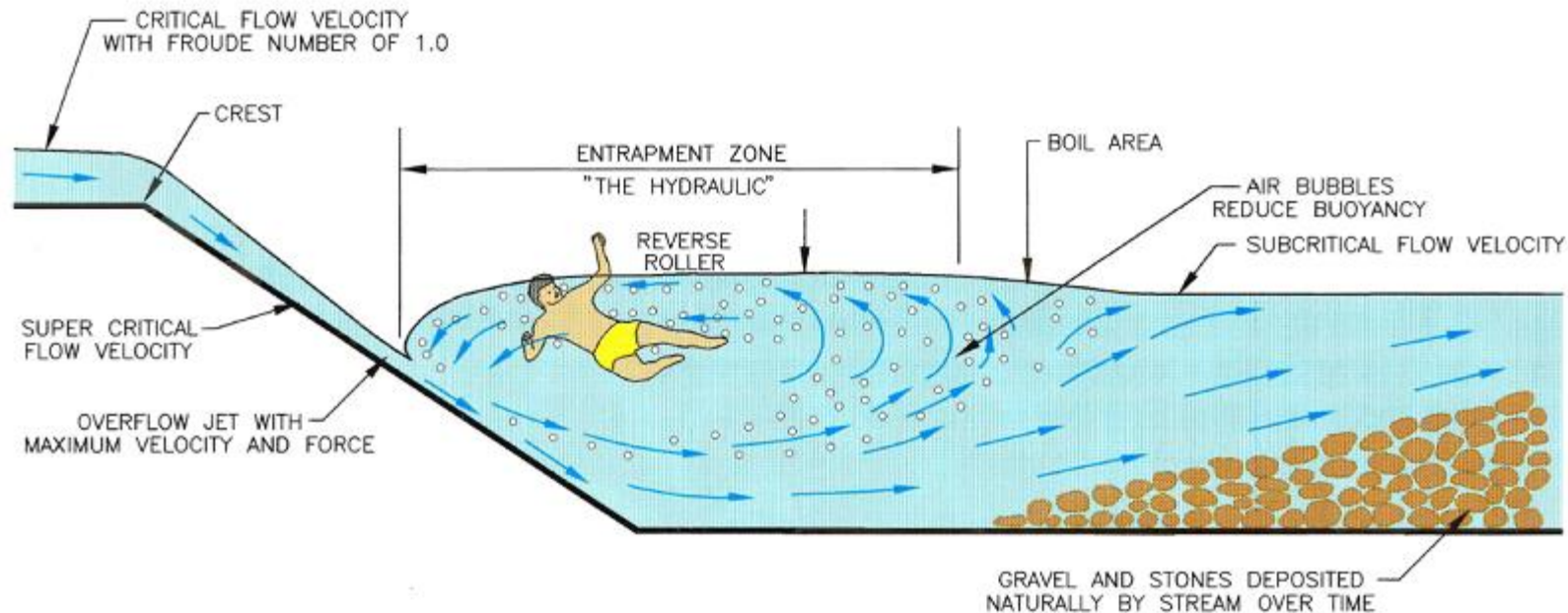
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The Hydraulic Jump



The Dangerous Hydraulic Jump

- Strong Jumps may pose a significant public safety hazard.



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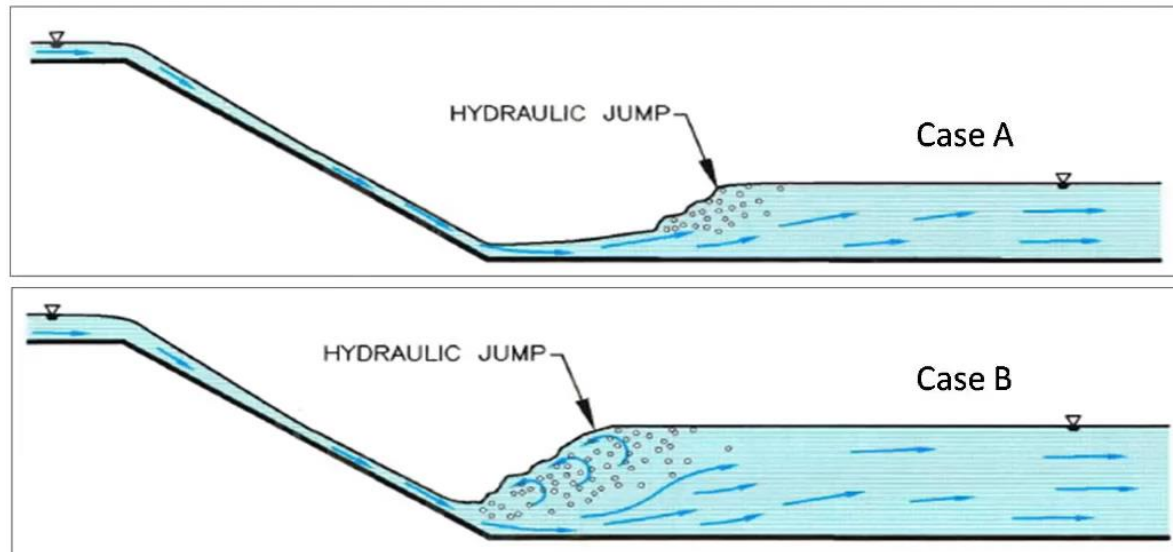
The Hydraulic Jump



The Dangerous Hydraulic Jump

- Strong Jumps may pose a significant public safety hazard.

Hydraulic Jumps at Overflow Structures



Figures Courtesy of Wright Water Engineers Inc.

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The Hydraulic Jump

An Application of Hydraulic Jump

- Strong Jumps can dissipate the energy of the flow.



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The Hydraulic Jump



Example

A hydraulic jump occurs in a rectangular channel 3 m wide. The water depth before the jump is 0.6 m, and after the jump is 1.6 m. Compute (a) the flow rate in the channel (b) the critical depth (c) the head loss in the jump?

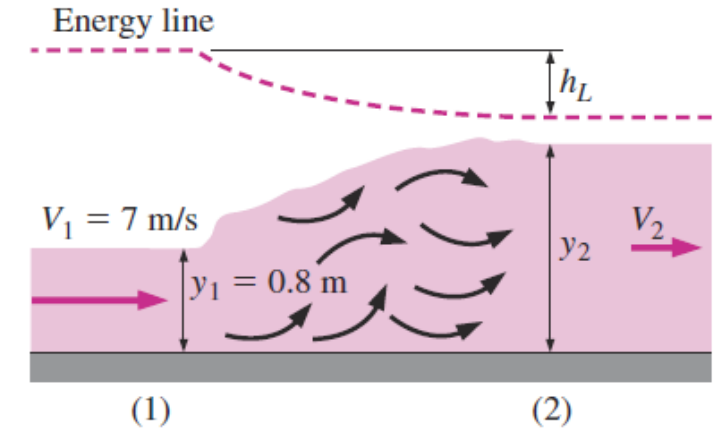
$$\frac{y_2}{y_1} = \frac{1}{2} \left[-1 + \sqrt{1 + 8Fr_1^2} \right]$$
$$Fr = \frac{V}{\sqrt{gy}}$$
$$y_c = \left[\frac{Q^2}{gb^2} \right]^{1/3}$$
$$H_l = \frac{[y_2 - y_1]^3}{4y_1y_2}$$

The Hydraulic Jump



Example

Water discharging into a 10-m-wide rectangular horizontal channel from a sluice gate is observed to have undergone a hydraulic jump. The flow depth and velocity before the jump are 0.8 m and 7 m/s, respectively. Determine (a) the flow depth and the Froude number after the jump, (b) the head loss and the dissipation ratio, and (c) the wasted power production potential due to the hydraulic jump.



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Homework



Problem 11.24

Flow through a sluice gate is shown. Estimate the water depth and velocity after the gate (well before the hydraulic jump)

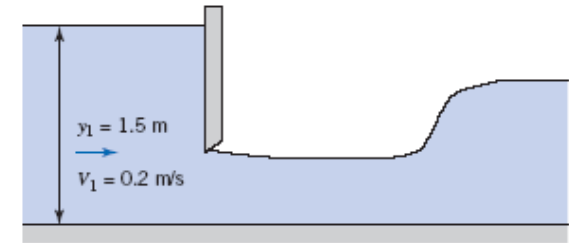
$$\frac{p_1}{\rho g} + \frac{V_1^2}{2g} + y_1 = \frac{p_2}{\rho g} + \frac{V_2^2}{2g} + y_2$$

$$\frac{V_1^2}{2g} + y_1 = \frac{V_2^2}{2g} + y_2$$

$$V = q/y_2$$

$$\frac{V_1^2}{2g} + y_1 = \frac{q^2}{2gy_2^2} + y_2$$

$$Fr_1 = V_1/\sqrt{gy_1} \quad Fr_2 = V_2/\sqrt{gy_2}$$



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Homework

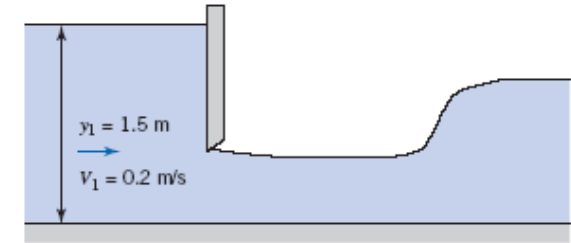


Problem 11.30

Estimate the depth of water before and after the jump for the hydraulic jump downstream of the sluice gate.

$Fr > 1$ supercritical

$$\frac{y_3}{y_2} = \frac{1}{2} \left(-1 + \sqrt{1 + 8Fr_2^2} \right)$$



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Homework



This assignment is due by **6pm on April 29th**.

- Upload your solution to BB.
- You can either type your solution out using a word editor like Microsoft Word or clearly hand write the information and then copy/scan your solution to a digital file.