

# 1 Introduction

## 2 Hellinger-Reissner weak form for elastic thin plate problems

### 2.1 Kinematics

Consider a thin plate with the thickness  $h$ , as shown in figure ??, Accordance with Kirchhoff's hypothesis, the displacement for a thin plate denoted by  $\hat{\mathbf{u}}$  is a linear function in deflection direction:

$$\begin{cases} \hat{u}_\alpha(\mathbf{x}) = u_\alpha(x_1, x_2) - x_3 w_{,\alpha} & \alpha = 1, 2 \\ \hat{u}_3(\mathbf{x}) = w(x_1, x_2) \end{cases} \quad (1)$$

with the xiaobianxing assumption, the components of strain tensor is given by:

$$\begin{cases} \hat{\varepsilon}_{\alpha\beta} = \frac{1}{2}(\hat{u}_{\alpha,\beta} + \hat{u}_{\beta,\alpha}) = \varepsilon_{\alpha\beta} + x_3 \kappa_{\alpha\beta}, & \alpha, \beta = 1, 2 \\ \hat{\varepsilon}_{3i} = \hat{\varepsilon}_{i3} = 0, & i = 1, 2, 3 \end{cases} \quad (2)$$

with

$$\varepsilon_{\alpha\beta} = \frac{1}{2}(u_{\alpha,\beta} + u_{\beta,\alpha}), \quad \kappa_{\alpha\beta} = -w_{,\alpha\beta} \quad (3)$$

under this circumstance, the potential prob the problem can be split into two independent problems, the traditional plane stress problem with variable  $u_\alpha$ , and the thin plate problem with variable  $w$ .

$$\hat{\sigma}_{\alpha\beta} = \mathbb{C}_{\alpha\beta\gamma\eta} \hat{\varepsilon}_{\gamma\eta} = \mathbb{C}_{\alpha\beta\gamma\eta} (\varepsilon_{\gamma\eta} + x_3 \kappa_{\gamma\eta}) \quad (4)$$

$$\begin{aligned} \int_{\hat{\Omega}} \frac{1}{2} \hat{\varepsilon}_{ij} \hat{\sigma}_{ij} d\Omega &= \int_{\Omega} \int_{-\frac{h}{2}}^{\frac{h}{2}} \frac{1}{2} (\varepsilon_{\alpha\beta} + x_3 \kappa_{\alpha\beta}) \mathbb{C}_{\alpha\beta\gamma\eta} (\varepsilon_{\gamma\eta} + x_3 \kappa_{\gamma\eta}) dx_3 d\Omega \\ &= \int_{\Omega} \frac{1}{2} \varepsilon_{\alpha\beta} \sigma_{\alpha\beta} d\Omega + \int_{\Omega} \frac{1}{2} \kappa_{\alpha\beta} M_{\alpha\beta} d\Omega \end{aligned} \quad (5)$$

where

$$\sigma_{\alpha\beta} = \mathbb{C}_{\alpha\beta\gamma\eta} \varepsilon_{\gamma\eta}, \quad M_{\alpha\beta} = \frac{h^3}{12} \mathbb{C}_{\alpha\beta\gamma\eta} \kappa_{\gamma\eta} \quad (6)$$

### 2.2 Elasticity problems

For elasticity problems, the Hellinger-Reissner energy functional is given by:

$$\begin{aligned} \mathcal{L}(\boldsymbol{\sigma}, \mathbf{u}) &= \int_{\Omega} \frac{1}{2} \boldsymbol{\varepsilon}(\boldsymbol{\sigma}) : \boldsymbol{\sigma} d\Omega - \int_{\Gamma_u} \mathbf{n} \cdot \boldsymbol{\sigma} \cdot \bar{\mathbf{u}} d\Gamma \\ &\quad - \int_{\Gamma_t} \mathbf{u} \cdot (\boldsymbol{\sigma} \cdot \mathbf{n} - \bar{\mathbf{t}}) d\Gamma + \int_{\Omega} \mathbf{u} \cdot (\boldsymbol{\sigma} \cdot \nabla + \bar{\mathbf{b}}) d\Omega \end{aligned} \quad (7)$$

where  $\varepsilon$  and  $\sigma$  stands for the strain and stress tensor respectively.  $\Gamma_u$  and  $\Gamma_t$  are the essential boundary and natural boundary satisfying that  $\Gamma_u \cup \Gamma_t = \partial\Omega$ ,  $\Gamma_u \cap \Gamma_t = \emptyset$ .  $\bar{\mathbf{u}}$  and  $\bar{\mathbf{t}}$  are the prescribed displacement and traction on  $\Gamma_u$  and  $\Gamma_t$ .  $\bar{\mathbf{b}}$  denotes to the prescribed body force in  $\Omega$ . Introducing the standard variation argument to Eq.7 leads to the following HR weak form:

$$\text{find } \sigma, \mathbf{u} \in H_1 \quad \begin{cases} a(\delta\sigma, \sigma) + b(\delta\sigma, \mathbf{u}) = g(\delta\sigma) & \forall \delta\sigma \in H_1 \\ b(\sigma, \delta\mathbf{u}) = f(\delta\mathbf{u}) & \forall \delta\mathbf{u} \in H_1 \end{cases} \quad (8)$$

where  $a : L_2 \times L_2 \rightarrow \mathbb{R}$ ,  $b : L_2 \times L_2 \rightarrow \mathbb{R}$  are bilinear forms:

$$a(\delta\sigma, \sigma) = \int_{\Omega} \varepsilon(\delta\sigma) : \sigma d\Omega \quad (9)$$

$$b(\delta\sigma, \mathbf{u}) = - \int_{\Gamma} \mathbf{u} \cdot \delta\sigma \cdot \mathbf{n} d\Gamma + \int_{\Omega} \mathbf{u} \cdot \delta\sigma \cdot \nabla d\Omega + \int_{\Gamma_u} \mathbf{u} \cdot \delta\sigma \cdot \mathbf{n} d\Gamma \quad (10)$$

and  $g, f$  are the linear operators evaluated by

$$g(\delta\sigma) = \int_{\Gamma_u} \mathbf{n} \cdot \delta\sigma \cdot \bar{\mathbf{u}} d\Gamma \quad (11)$$

$$f(\delta\mathbf{u}) = - \int_{\Gamma_t} \delta\mathbf{u} \cdot \bar{\mathbf{t}} d\Gamma - \int_{\Omega} \delta\mathbf{u} \cdot \bar{\mathbf{b}} d\Omega \quad (12)$$

### 2.3 Thin plate problems

$$\begin{aligned} \mathcal{L}(\mathbf{M}, w) = & \int_{\Omega} \frac{1}{2} \kappa(\mathbf{M}) : \mathbf{M} d\Omega \\ & - \int_{\Gamma_w} V_{\mathbf{n}}(\mathbf{M}) \bar{w} d\Gamma + \int_{\Gamma_{\theta}} M_{\mathbf{nn}}(\mathbf{M}) \bar{\theta}_{\mathbf{n}} d\Gamma - P(\mathbf{M}) \bar{w}|_{\Gamma_c} \\ & + \int_{\Gamma_M} \theta_{\mathbf{n}}(w) (M_{\mathbf{nn}} - \bar{M}_{\mathbf{nn}}) d\Gamma - \int_{\Gamma_V} w (V_{\mathbf{n}} - \bar{V}_{\mathbf{n}}) d\Gamma \\ & - w(P - \bar{P})|_{\Gamma_p} + \int_{\Omega} w(\nabla \cdot \mathbf{M} \cdot \nabla + \bar{q}) d\Omega \end{aligned} \quad (13)$$

$$\text{find } \mathbf{M}, w \in H_2 \quad \begin{cases} a(\delta\mathbf{M}, \mathbf{M}) + b(\delta\mathbf{M}, w) = g(\delta\mathbf{M}) & \forall \delta\mathbf{M} \in L_2 \\ b(\mathbf{M}, \delta w) = f(\delta w) & \forall \delta w \in H_2 \end{cases} \quad (14)$$

$$a(\delta\mathbf{M}, \mathbf{M}) = \int_{\Omega} \kappa(\delta\mathbf{M}) : \mathbf{M} d\Omega \quad (15)$$

## 3 MLS/RK meshfree approximation

In accordance with Moving Least Square approximation (MLS) [1] or Reproducing Kernel approximation (RK) [2], the domain  $\Omega$  is discrete by a set of

meshfree nodes  $\{\mathbf{x}_I\}_{I=1}^{n_p}$ ,  $n_p$  is the total number of meshfree nodes. And then, a variable  $u$  in  $\Omega$  can be approximated as follow:

$$u^h(\mathbf{x}) = \sum_{I=1}^{n_p} \Psi_I(\mathbf{x}) d_I \quad (16)$$

where  $\Psi_I$  and  $d_I$  are the meshfree shape function and the nodal coefficient associated with meshfree node  $\mathbf{x}_I$ . This RK shape function  $\Psi_I$  is constructed by the undetermined coefficient vector  $\mathbf{c}$ , the basis function vector  $\mathbf{p}$  and the kernel function  $\phi$ :

$$\Psi_I(\mathbf{x}) = \mathbf{c}^T(\mathbf{x}) \mathbf{p}(\mathbf{x}_I - \mathbf{x}) \phi(\mathbf{x}_I - \mathbf{x}) \quad (17)$$

For instance in 2D case, the basis function vector  $\mathbf{p}$  contains the  $p$ th order complete monomials:

$$\mathbf{p}(\mathbf{x}) = \{1, x, y, x^2, \dots, y^p\}^T \quad (18)$$

The kernel function  $\phi$  determines the support size and continuity of shape function, and the cubic and quintic spline functions is chosen herein for elasticity problems and thin plate problems respectively.

$$\phi(\mathbf{x}_I - \mathbf{x}) = \varphi\left(\frac{x_I - x}{h_x}\right) \varphi\left(\frac{y_I - y}{h_y}\right) \quad (19)$$

where  $h_i$  is the

- Cubic spline function:

$$\varphi(r) = \frac{1}{3!} \begin{cases} (2-2r)^3 - 4(1-2r)^3, & r \leq \frac{1}{2} \\ (2-2r)^3, & \frac{1}{2} < r \leq 1 \\ 0, & r > 1 \end{cases} \quad (20)$$

- Quintic spline function:

$$\varphi(r) = \frac{1}{5!} \begin{cases} (3-3r)^5 - 6(2-3r)^5 + 15(1-3r)^5, & r \leq \frac{1}{3} \\ (3-3r)^5 - 6(2-3r)^5, & \frac{1}{3} < r \leq \frac{2}{3} \\ (3-3r)^5, & \frac{2}{3} < r \leq 1 \\ 0, & r > 1 \end{cases} \quad (21)$$

in which  $h_i$  is the support size in  $x_i$  axis direction.

The undetermined vector  $\mathbf{c}$  can be attain by enforcing the following consistency condition:

$$\sum_{I=1}^{n_p} \Psi_I(\mathbf{x}) \mathbf{p}(\mathbf{x}_I) = \mathbf{p}(\mathbf{x}) \quad (22)$$

or with a shift-form

$$\sum_{I=1}^{n_p} \Psi_I(\mathbf{x}) \mathbf{p}(\mathbf{x}_I - \mathbf{x}) = \mathbf{p}(\mathbf{0}) \quad (23)$$

substituting Eq. 17 into the consistency condition leads to:

$$\mathbf{c}(\mathbf{x}) = \mathbf{A}^{-1}(\mathbf{x}) \mathbf{p}(\mathbf{0}) \quad (24)$$

with moment matrix  $\mathbf{A}$

$$\mathbf{A}(\mathbf{x}) = \sum_{I=1}^{n_p} \mathbf{p}^T(\mathbf{x}_I - \mathbf{x}) \mathbf{p}(\mathbf{x}_I - \mathbf{x}) \phi(\mathbf{x}_I - \mathbf{x}) \quad (25)$$

It is noted that, to ensure the invertibility of moment matrix, a well-posed meshfree nodal distribution should be required, i.e. there should be sufficient meshfree nodes be covered by their support, as shown in Fig. ???. Under this circumstance,

$$\|u - u^i\|_{H_k} \leq Ch^{p-k+1} |u|_{H_{p+1}}, \quad \forall k \leq p+1 \quad (26)$$

In Hellinger-Reissner reproducing kernel gradient smoothing framework, the displacement  $\mathbf{u}$  is approximated by traditional meshfree shape functions, namely  $\mathbf{u}^h$ :

$$\mathbf{u}^h(\mathbf{x}) = \sum_{I=1}^{n_p} \Psi_I(\mathbf{x}) \mathbf{d}_I \quad (27)$$

On the other hand, the components of stress tensor is assumed as a polynomial in each background cells:

$$\sigma_{ij}^h(\mathbf{x}) = \mathbf{q}^T(\mathbf{x}) \mathbf{c}_{ij}, \quad \text{in } \Omega_C \quad (28)$$

with

$$\mathbf{q}(\mathbf{x}) = \{1, x, y, \dots, y^{p-1}\}^T \quad (29)$$

**Theorem 1.**

$$\sup_{\sigma^h \in Q} \|\sigma - \sigma^h\|_e \leq Ch^p |\sigma| \quad (30)$$

*Proof.*

$$\sigma_{ij}(\mathbf{x}) = \sigma_{ij}(\mathbf{0}) + x\sigma_{ij} \quad (31)$$

□

## 4 Error analysis for HR gradient smoothing mesh-free formulation

### Appendix A

In this appendix, we define some functional operator used in the main sections. Firstly, the energy norms for elasticity problems and thin plate problems are

defined by:

$$\|\boldsymbol{\sigma}\|_e = \frac{1}{2}a_2(\boldsymbol{\sigma}, \boldsymbol{\sigma}) = \int_{\Omega} \frac{1}{2}\boldsymbol{\varepsilon}(\boldsymbol{\sigma}) : \boldsymbol{\sigma} d\Omega \quad (32)$$

$$\|\boldsymbol{M}\|_e = \frac{1}{2}a_4(\boldsymbol{M}, \boldsymbol{M}) = \int_{\Omega} \frac{1}{2}\boldsymbol{\kappa}(\boldsymbol{M}) : \boldsymbol{M} d\Omega \quad (33)$$

## References

- [1] T. Belytschko, Y. Y. Lu, L. Gu, Element-free Galerkin methods 37 (2) 229–256.
- [2] W. K. Liu, S. Jun, Y. F. Zhang, Reproducing kernel particle methods 20 (8-9) 1081–1106.