

1 Quasi-consistent efficient meshfree thin shell
2 formulation to naturally accommodate essential
3 boundary conditions

4 Junchao Wu^{a,*}, Yangtao Xu^b, Bin Xu^a, Syed Humayun Basha^{c,*}

^a*Key Laboratory for Intelligent Infrastructure and Monitoring of Fujian Province, College
of Civil Engineering, Huaqiao University, Xiamen, Fujian, 361021, China*

^b*College of Civil Engineering, Huaqiao University, Xiamen, Fujian, 361021, China*

^c*Key Laboratory for Structural Engineering and Disaster Prevention of Fujian Province,
College of Civil Engineering, Huaqiao University, Xiamen, Fujian, 361021, China*

5 **Abstract**

This research proposed an efficient and quasi-consistent meshfree thin shell formulation with natural enforcement of essential boundary conditions. Within the framework of the Hu-Washizu variational principle, a mixed formulation of displacements, strains and stresses is employed in this approach, where the displacements are discretized using meshfree shape functions, and the strains and stresses are expressed using smoothed gradients, covariant smoothed gradients and covariant bases. The smoothed gradients satisfy the first and second order integration constraint and have quasi-consistent consistency. Owing to Hu-Washizu variational principle, the essential boundary conditions automatically arise in its weak form. As a result, the suggested technique's enforcement of essential boundary conditions resembles that of the traditional Nitsche's method. Contrary to Nitsche's method, the costly higher order derivatives of conventional meshfree shape functions were replaced by the smoothed gradients with fast computation, which improve the efficiency. Meanwhile, the proposed formulation features a naturally stabilized term without adding any artificial stabilization factors, which eliminates the stabilization parameter-dependent issue in the Nitsche's method. The efficacy of the proposed Hu-Washizu meshfree thin shell formulation is illustrated by a set of classical standard thin shell problems.

6 *Keywords:* Meshfree, Thin shell, Hu-Washizu variational principle,
7 Reproducing kernel gradient smoothing, Essential boundary condition

*Corresponding author

Email addresses: jcwu@hqu.edu.cn (Junchao Wu), syedhbasha@hqu.edu.cn (Syed Humayun Basha)

8 1. Introduction

9 Thin shell structures generally adhere to the Kirchhoff hypothesis [1], that
10 neglects the shear deformation can be described using Galerkin formulation
11 which requires to have at least C^1 continuity. The traditional finite element
12 methods usually only have C^0 continuous shape functions, and it prefers Mindlin
13 thick shear theory, hybrid and mixed models in simulation of shell structure [2].
14 Meshfree methods [3, 4, 5] with high order smoothed shape functions have gar-
15 nered much research attention over the past thirty years. These techniques
16 established the shape functions based on a collection of dispersed nodes, and
17 the high order continuity of shape functions can be easily achieved even with
18 low-order basis functions. For thin shell analysis, this high order meshfree ap-
19 proximation can also alleviate the membrane locking caused by the mismatched
20 approximation order of membrane strain and bending strain [6]. Furthermore,
21 nodal-based meshfree approximations generally offer the flexibility of local re-
22 finement and can relieve the burden of mesh distortion. Owing to these benefits,
23 numerous meshfree techniques have been developed and implemented in many
24 scientific and engineering fields [7, 8, 9, 10, 11, 12]. However, the high order
25 smoothed meshfree shape functions accompany the enlarged and overlapping
26 supports, which may potentially cause many problems for shape functions. One
27 of the issues is the loss of the Kronecker delta property, which means that, un-
28 like the finite element methods, the necessary boundary conditions cannot be
29 directly enforced [13]. Another issue is that the variational consistency or said
30 integration constraint cannot be satisfied due to the misalignment between the
31 numerical integration domains and supports of shape functions. Besides, the
32 shape functions exhibit a piecewise rational nature in each integration domain.
33 Therefore, variational consistency is vital to the solution accuracy in Galerkin
34 formulations [14, 15].

35 Various ways have been presented to enforce the necessary boundary for
36 Galerkin meshfree methods directly, including the boundary singular kernel
37 method [16], mixed transformation method [16], and interpolation element-free
38 method [17] for recovering shape functions' Kronecker property. However, these
39 methods are not based on a variational setting and cannot guarantee variational
40 consistency. In the absence of a meshfree node, accuracy enforcement might be
41 poorer. In contrast, enforcing the essential boundary conditions using a vari-
42 ational approach is preferred for Galerkin meshfree methods. The variational
43 consistent Lagrange multiplier approach was initially used to the Galerkin mesh-
44 free method by Belytschko et al. [3]. In this method, the extra degrees of free-
45 dom are used to determine the discretion of Lagrange multiplier. Furthermore,
46 Ivannikov et al. [18] have extended this approach to geometrically nonlinear
47 thin shells. Lu et al. [19] suggested the modified variational essential bound-
48 ary enforcement approach and expressed the Lagrange multiplier by equivalent
49 tractions to eliminate the excess degrees of freedom. However, the coercivity
50 of this approach is not always ensured and potentially reduces the accuracy.
51 Zhu and Atluri [20] pioneered the penalty method for meshfree method, mak-
52 ing it a straightforward approach to enforce essential boundary conditions via

53 Galerkin weak form. However, the penalty method lacks variational consistency
54 and requires experimental artificial parameters whose optimal value is hard to
55 determine. Fernández-Méndez and Huerta [13] imposed necessary boundary
56 conditions using Nitsche’s approach in the meshfree formulation. This approach
57 can be seen as a hybrid combination of the modified variational method and the
58 penalty method because the modified variational method generates variational
59 consistency through the use of a consistent term, and the penalty method is
60 used as a stabilized term to recover the coercivity. Skatulla and Sansour [21]
61 extended Nitsche’s thin shell analysis method and proposed an iteration algo-
62 rithm to determine artificial parameters at each integration point.

63 In order to address the issue of numerical integration, a series of consis-
64 tent integration schemes have been developed for Galerkin meshfree methods.
65 Among these include stabilized conforming nodal integration [22], variational
66 consistent integration [23], quadratic consistent integration [24], reproducing
67 kernel gradient smoothing integration [25], and consistent projection integration
68 [26]. The assumed strain approach establishes the most consistent integration
69 scheme, while the smoothed gradient replaces the costly higher order derivatives
70 of traditional meshfree shape functions and shows a high efficiency. Moreover,
71 to achieve global variational consistency, a consistent essential boundary con-
72 dition enforcement should cooperate with the consistent integration scheme.
73 The consistent integration scheme and Nitsche’s method for treating essential
74 boundary conditions show a good performance since they can satisfy the coer-
75 civity without requiring additional degrees of freedom. Nevertheless, Nitsche’s
76 approach still retains the artificial parameters in stabilized terms, and it is es-
77 sential to remain conscious of the costly higher order derivatives, particularly
78 for thin plate and thin shell problems. Recently, Wu et al. [27, 28] proposed
79 an efficient and stabilized essential boundary condition enforcement method
80 based upon the Hellinger-Reissner variational principle, where a mixed formu-
81 lation in Hellinger-Reissner weak form recasts the reproducing kernel gradient
82 smoothing integration. The terms for enforcing essential boundary conditions
83 are identical to the Nitsche’s method, and both have consistent and stabilized
84 terms. Nevertheless, the stabilized term of this method naturally exists in the
85 Hellinger-Reissner weak form and no longer needs the artificial parameters, even
86 for essential boundary enforcement; instead all of the higher order derivatives
87 are represented by smoothed gradients and their derivatives.

88 In this study, an efficient and stabilized variational consistent meshfree
89 method that naturally enforces the essential boundary conditions is developed
90 for thin shell structure. Following the concept of the Hellinger-Reissner prin-
91 ciple base consistent meshfree method, the Hu-Washizu variational principle of
92 complementary energy with variables of displacement, strains, and stresses is
93 employed. The displacement is approximated by conventional meshfree shape
94 functions, and the strains and stresses are expressed by smoothed gradients with
95 covariant bases. It is important to note that although the first second-order in-
96 tegration requirements are naturally embedded in the smoothed gradients, their
97 fulfillment can only result in a quasi-satisfaction of variational consistency be-
98 cause of the non-polynomial nature of the stresses. Hu-Washizu’s weak form is

99 used to evaluate all the essential boundary conditions regarding displacements
100 and rotations. This type of formulation is similar to the Nitsche's method but
101 does not require any artificial parameters. Compared with Nitsche's method,
102 conventional reproducing smoothed gradients and its direct derivatives replace
103 the costly higher order derivatives. By utilizing the advantages of a replicating
104 kernel gradient smoothing framework, the smoothed gradients showed better
105 performance compared to conventional derivatives of shape functions, hence
106 increasing the meshfree formulation's computational efficiency.

107 The remainder of this research paper is structured as follows: The kinematics
108 of the thin shell structure and the weak form of the associated Hu-Washizu
109 principle are briefly described in Section 2. Subsequently, the mixed formulation
110 regarding the displacements, strains and stresses in accordance with Hu-Washizu
111 weak form are presented in Section 3. The discrete equilibrium equations are
112 derived in Section 4 using the naturally occurring accommodation of essential,
113 and they are compared to the equations obtained using Nitsche's method. The
114 numerical results in Section 5 validate the efficacy of the proposed Hu-Washizu
115 meshfree thin shell formulation. Lastly, the concluding remarks are presented
116 in Section 6.

117 2. Hu-Washizu's formulation of complementary energy for thin shell

118 2.1. Kinematics for thin shell

119 Consider the configuration of a shell $\bar{\Omega}$, as shown in Fig. 1, which can be
 120 easily described by a parametric curvilinear coordinate system $\boldsymbol{\xi} = \{\xi^i\}_{i=1,2,3}$.
 121 The mid-surface of the shell denoted by Ω is specified by the in-plane coordinates
 122 $\boldsymbol{\xi} = \{\xi^\alpha\}_{\alpha=1,2}$, as the thickness direction of shell is by ξ^3 , $-\frac{h}{2} \leq \xi^3 \leq \frac{h}{2}$, h is
 123 the thickness of shell. In this work, Latin indices take the values from 1 to 3,
 124 and Greek indices are evaluated by 1 or 2. For the Kirchhoff hypothesis [6], the
 125 position $\mathbf{x} \in \bar{\Omega}$ is defined by linear functions with respect to ξ^3 :

$$\mathbf{x}(\xi^1, \xi^2, \xi^3) = \mathbf{r}(\xi^1, \xi^2) + \xi^3 \mathbf{a}_3(\xi^1, \xi^2) \quad (1)$$

in which \mathbf{r} means the position on the mid-surface of shell, and \mathbf{a}_3 is correspond-

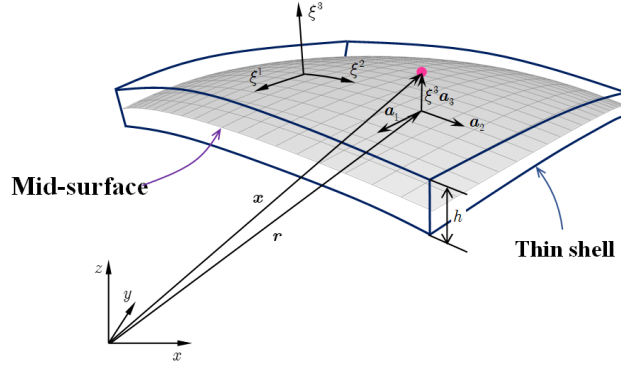


Figure 1: Kinematics for thin shell.

126 ing normal direction. For the mid-surface of shell, the in-plane covariant base
 127 vector with respect to ξ^α can be derived by a trivial partial differentiation to \mathbf{r} :
 128

$$\mathbf{a}_\alpha = \frac{\partial \mathbf{r}}{\partial \xi^\alpha} = \mathbf{r}_{,\alpha}, \alpha = 1, 2 \quad (2)$$

129 to provide for a clear expression, the subscript comma denotes the partial dif-
 130 ferentiation operation with respect to in-plane coordinates ξ^α , and the normal
 131 vector \mathbf{a}_3 can be obtained by the normalized cross product of \mathbf{a}_α 's as follows:

$$\mathbf{a}_3 = \frac{\mathbf{a}_1 \times \mathbf{a}_2}{\|\mathbf{a}_1 \times \mathbf{a}_2\|} \quad (3)$$

132 where $\|\bullet\|$ is the Euclidean norm operator.

133 With the assumption of infinitesimal deformation, the strain components
 134 with respect to the global contravariant base can be stated as:

$$\epsilon_{ij} = \frac{1}{2}(\mathbf{x}_{,i} \cdot \mathbf{u}_{,j} + \mathbf{u}_{,i} \cdot \mathbf{x}_{,j}) \quad (4)$$

where \mathbf{u} represents the displacement for the shell deformation. To satisfy the Kirchhoff hypothesis, the displacement is assumed to be of the following form:

$$\mathbf{u}(\xi^1, \xi^2, \xi^3) = \mathbf{v}(\xi^1, \xi^2) + \boldsymbol{\theta}(\xi^1, \xi^2)\xi^3 \quad (5)$$

in which the quadratic and higher order terms are neglected. \mathbf{v} , $\boldsymbol{\theta}$ represent the displacement and rotation in mid-surface, respectively.

Subsequently, plugging Eqs. (1) and (5) into Eq. (4) and neglecting the quadratic terms, the strain components can be rephrased as follows:

$$\begin{aligned} \epsilon_{\alpha\beta} &= \frac{1}{2}(\mathbf{a}_\alpha \cdot \mathbf{v}_{,\beta} + \mathbf{v}_{,\alpha} \cdot \mathbf{a}_\beta) \\ &+ \frac{1}{2}(\mathbf{a}_{3,\alpha} \cdot \mathbf{v}_{,\beta} + \mathbf{v}_{,\alpha} \cdot \mathbf{a}_{3,\beta} + \mathbf{a}_\alpha \cdot \boldsymbol{\theta}_{,\beta} + \boldsymbol{\theta}_{,\alpha} \cdot \mathbf{a}_\beta)\xi^3 \\ &= \varepsilon_{\alpha\beta} + \kappa_{\alpha\beta}\xi^3 \end{aligned} \quad (6a)$$

$$\epsilon_{\alpha 3} = \frac{1}{2}(\mathbf{a}_\alpha \cdot \boldsymbol{\theta} + \mathbf{v}_{,\alpha} \cdot \mathbf{a}_3) + \frac{1}{2}(\mathbf{a}_3 \cdot \boldsymbol{\theta})_{,\alpha}\xi^3 \quad (6b)$$

$$\epsilon_{33} = \mathbf{a}_3 \cdot \boldsymbol{\theta} \quad (6c)$$

where $\varepsilon_{\alpha\beta}$, $\kappa_{\alpha\beta}$ represent membrane and bending strains, respectively, and are given as follows:

$$\varepsilon_{\alpha\beta} = \frac{1}{2}(\mathbf{a}_\alpha \cdot \mathbf{v}_{,\beta} + \mathbf{v}_{,\alpha} \cdot \mathbf{a}_\beta) \quad (7)$$

$$\kappa_{\alpha\beta} = \frac{1}{2}(\mathbf{a}_{3,\alpha} \cdot \mathbf{v}_{,\beta} + \mathbf{v}_{,\alpha} \cdot \mathbf{a}_{3,\beta} + \mathbf{a}_\alpha \cdot \boldsymbol{\theta}_{,\beta} + \boldsymbol{\theta}_{,\alpha} \cdot \mathbf{a}_\beta) \quad (8)$$

In accordance with the Kirchhoff hypothesis, the thickness of shell will not change, and the deformation related with direction of ξ^3 will vanish, i.e. $\epsilon_{3i} = 0$. Thus, the rotation $\boldsymbol{\theta}$ can be rewritten as:

$$\epsilon_{3i} = 0 \Rightarrow \begin{cases} \boldsymbol{\theta} \cdot \mathbf{a}_\alpha + \mathbf{v}_{,\alpha} \cdot \mathbf{a}_3 = 0 \\ \boldsymbol{\theta} \cdot \mathbf{a}_3 = 0 \end{cases} \Rightarrow \boldsymbol{\theta} = -\mathbf{v}_{,\alpha} \cdot \mathbf{a}_3 \mathbf{a}^\alpha \quad (9)$$

where \mathbf{a}^α 's is the in-plane contravariant base vector, $\mathbf{a}^\alpha \cdot \mathbf{a}_\beta = \delta^\alpha_\beta$, δ is the Kronecker delta function. The detailed derivation of Eq. 9 can be found in reference [30].

Furthermore, substituting Eq. (9) into Eq. (8) leads to:

$$\kappa_{\alpha\beta} = (\Gamma_{\alpha\beta}^\gamma \mathbf{v}_{,\gamma} - \mathbf{v}_{,\alpha\beta}) \cdot \mathbf{a}_3 = -\mathbf{v}_{,\alpha}|_\beta \cdot \mathbf{a}_3 \quad (10)$$

in which $\Gamma_{\alpha\beta}^\gamma = \mathbf{a}_{\alpha,\beta} \cdot \mathbf{a}^\gamma$ is namely the Christoffel symbol of the second kind, and $\mathbf{v}_{,\alpha}|_\beta$ is the in-plane covariant derivative of $\mathbf{v}_{,\alpha}$, i.e. $\mathbf{v}_{,\alpha}|_\beta = \Gamma_{\alpha\beta}^\gamma \mathbf{v}_{,\gamma} - \mathbf{v}_{,\alpha\beta}$.

2.2. Galerkin weak form for Hu-Washizu principle of complementary energy

In this study, the Hu-Washizu variational principle of complementary energy [29] was adopted for the development of the proposed analytical approach, the

156 corresponding complementary functional, denoted by Π_C , is listed as follows:

$$\begin{aligned}
& \Pi_C(\varepsilon_{\alpha\beta}, \kappa_{\alpha\beta}, N^{\alpha\beta}, M^{\alpha\beta}) \\
&= \int_{\Omega} \frac{h}{2} \varepsilon_{\alpha\beta} C^{\alpha\beta\gamma\eta} \varepsilon_{\gamma\eta} d\Omega + \int_{\Omega} \frac{h^3}{24} \kappa_{\alpha\beta} C^{\alpha\beta\gamma\eta} \kappa_{\gamma\eta} d\Omega \\
&+ \int_{\Omega} \varepsilon_{\alpha\beta} (N^{\alpha\beta} - h C^{\alpha\beta\gamma\eta} \varepsilon_{\gamma\eta}) d\Omega + \int_{\Omega} \kappa_{\alpha\beta} (M^{\alpha\beta} - \frac{h^3}{12} C^{\alpha\beta\gamma\eta} \kappa_{\gamma\eta}) d\Omega \quad (11) \\
&- \int_{\Gamma_v} \mathbf{T} \cdot \bar{\mathbf{v}} d\Gamma + \int_{\Gamma_{\theta}} M_{\mathbf{nn}} \bar{\theta}_{\mathbf{n}} d\Gamma - (P \mathbf{a}_3 \cdot \bar{\mathbf{v}})_{\mathbf{x} \in C_w}
\end{aligned}$$

157 where $C^{\alpha\beta\gamma\eta}$'s represent the components of fourth order elasticity tensor with
158 respect to the covariant base and plane stress assumption, and it can be ex-
159 pressed by Young's modulus E , Poisson's ratio ν and the in-plane contravariant
160 metric coefficients $a^{\alpha\beta}$'s, $a^{\alpha\beta} = \mathbf{a}^{\alpha} \cdot \mathbf{a}^{\beta}$, as follows:

$$C^{\alpha\beta\gamma\eta} = \frac{E}{2(1+\nu)} (a^{\alpha\gamma} a^{\beta\eta} + a^{\alpha\eta} a^{\beta\gamma} + \frac{2\nu}{1-\nu} a^{\alpha\beta} a^{\gamma\eta}) \quad (12)$$

161 and $N^{\alpha\beta}$, $M^{\alpha\beta}$ are the components of membrane and bending stresses given by:

$$N^{\alpha\beta} = h C^{\alpha\beta\gamma\eta} \varepsilon_{\gamma\eta}, \quad M^{\alpha\beta} = \frac{h^3}{12} C^{\alpha\beta\gamma\eta} \kappa_{\gamma\eta} \quad (13)$$

162 Essential boundaries on the edges and corners denoted by Γ_v , Γ_{θ} and C_v
163 are naturally existed in complementary energy functional, $\bar{\mathbf{v}}$, $\bar{\theta}_{\mathbf{n}}$ are the cor-
164 responding prescribed displacement and normal rotation, respectively. \mathbf{T} , $M_{\mathbf{nn}}$
165 and P can be determined by Euler-Lagrange equations of shell problem [30] as
166 follows:

$$\mathbf{T} = \mathbf{T}_N + \mathbf{T}_M \rightarrow \begin{cases} \mathbf{T}_N = \mathbf{a}_{\alpha} N^{\alpha\beta} n_{\beta} \\ \mathbf{T}_M = (\mathbf{a}_3 M^{\alpha\beta} s_{\alpha} n_{\beta})_{,\gamma} s^{\gamma} + (\mathbf{a}_3 M^{\alpha\beta})|_{\beta} n_{\alpha} \end{cases} \quad (14)$$

$$M_{\mathbf{nn}} = M^{\alpha\beta} n_{\alpha} n_{\beta} \quad (15)$$

$$P = -[[M^{\alpha\beta} s_{\alpha} n_{\beta}]] \quad (16)$$

169 where $\mathbf{n} = n^{\alpha} \mathbf{a}_{\alpha} = n_{\alpha} \mathbf{a}^{\alpha}$ and $\mathbf{s} = s^{\alpha} \mathbf{a}_{\alpha} = s_{\alpha} \mathbf{a}^{\alpha}$ are the outward normal and
170 tangent directions on boundaries. $[[f]]$ is the jump operator defined by:

$$[[f]]_{\mathbf{x}=\mathbf{x}_c} = \lim_{\epsilon \rightarrow 0^+} (f(\mathbf{x}_c + \epsilon) - f(\mathbf{x}_c - \epsilon)), \mathbf{x}_c \in \Gamma \quad (17)$$

171 where f is an arbitrary function on Γ .

172 Moreover, the natural boundary conditions should be applied by Lagrangian
173 multiplier method with displacement \mathbf{v} regarded as multiplier. Thus, then the
174 new complementary energy functional namely Π is given by:

$$\begin{aligned}
& \Pi(\mathbf{v}, \varepsilon_{\alpha\beta}, \kappa_{\alpha\beta}, N^{\alpha\beta}, M^{\alpha\beta}) \\
&= \Pi_C(\varepsilon_{\alpha\beta}, \kappa_{\alpha\beta}, N^{\alpha\beta}, M^{\alpha\beta}) + \int_{\Gamma_M} \theta_{\mathbf{n}} (M_{\mathbf{nn}} - \bar{M}_{\mathbf{nn}}) d\Gamma \quad (18) \\
&- \int_{\Gamma_T} \mathbf{v} \cdot (\mathbf{T} - \bar{\mathbf{T}}) d\Gamma - \mathbf{v} \cdot \mathbf{a}_3 (P - \bar{P})_{\mathbf{x} \in C_P} - \int_{\Omega} \mathbf{v} \cdot (\mathbf{b} - \bar{\mathbf{b}}) d\Omega
\end{aligned}$$

175 where $\bar{\mathbf{T}}$, \bar{M}_{nn} and \bar{P} are the prescribed traction, bending moment and concen-
 176 trated force on edges Γ_T , Γ_M and corner C_P respectively. All the boundaries
 177 meet the following geometric relationships:

$$\begin{cases} \Gamma = \Gamma_v \cup \Gamma_T \cup \Gamma_\theta \cup \Gamma_M, & C = C_v \cup C_P, \\ \Gamma_v \cap \Gamma_T = \Gamma_\theta \cap \Gamma_M = C_v \cap C_P = \emptyset \end{cases} \quad (19)$$

178 and $\bar{\mathbf{b}}$ stands for the prescribed body force in Ω , \mathbf{b} also can be written based on
 179 Euler-Lagrange equations [30] as:

$$\mathbf{b} = \mathbf{b}_N + \mathbf{b}_M \rightarrow \begin{cases} \mathbf{b}_N = (\mathbf{a}_\alpha N^{\alpha\beta})|_\beta \\ \mathbf{b}_M = (\mathbf{a}_3 M^{\alpha\beta})|_{\alpha\beta} \end{cases} \quad (20)$$

180 Introducing a standard variational argument to Eq. (18), $\delta\Pi = 0$, and
 181 considering the arbitrariness of virtual variables, $\delta\mathbf{v}$, $\delta\varepsilon_{\alpha\beta}$, $\delta\kappa_{\alpha\beta}$, $N^{\alpha\beta}$, $M^{\alpha\beta}$
 182 lead to the following weak form:

$$-\int_{\Omega} h\delta\varepsilon_{\alpha\beta} C^{\alpha\beta\gamma\eta} \varepsilon_{\gamma\eta} d\Omega + \int_{\Omega} \delta\varepsilon_{\alpha\beta} N^{\alpha\beta} d\Omega = 0 \quad (21a)$$

$$-\int_{\Omega} \frac{h^3}{12} \delta\kappa_{\alpha\beta} C^{\alpha\beta\gamma\eta} \kappa_{\gamma\eta} d\Omega + \int_{\Omega} \delta\kappa_{\alpha\beta} M^{\alpha\beta} d\Omega = 0 \quad (21b)$$

$$\begin{aligned} \int_{\Omega} \delta N^{\alpha\beta} \varepsilon_{\alpha\beta} d\Omega - \int_{\Gamma} \delta \mathbf{T}_N \cdot \mathbf{v} d\Gamma + \int_{\Omega} \delta \mathbf{b}_N \cdot \mathbf{v} d\Omega \\ + \int_{\Gamma_v} \delta \mathbf{T}_N \cdot \mathbf{v} d\Gamma = \int_{\Gamma_v} \delta \mathbf{T}_N \cdot \bar{\mathbf{v}} d\Gamma \end{aligned} \quad (21c)$$

$$\begin{aligned} \int_{\Omega} \delta M^{\alpha\beta} \kappa_{\alpha\beta} d\Omega - \int_{\Gamma} \delta M_{nn} \theta_n d\Gamma + \int_{\Gamma} \delta \mathbf{T}_M \cdot \mathbf{v} d\Gamma + (\delta P \mathbf{a}_3 \cdot \mathbf{v})_{\mathbf{x} \in C} + \int_{\Omega} \delta \mathbf{b}_M \cdot \mathbf{v} d\Omega \\ + \int_{\Gamma_\theta} \delta M_{nn} \theta_n d\Gamma - \int_{\Gamma_v} \delta \mathbf{T}_M \cdot \mathbf{v} d\Gamma - (\delta P \mathbf{a}_3 \cdot \mathbf{v})_{\mathbf{x} \in C_v} \\ = \int_{\Gamma_\theta} \delta M_{nn} \bar{\theta}_n d\Gamma - \int_{\Gamma_v} \delta \mathbf{T}_M \cdot \bar{\mathbf{v}} d\Gamma - (\delta P \mathbf{a}_3 \cdot \bar{\mathbf{v}})_{\mathbf{x} \in C_v} \end{aligned} \quad (21d)$$

$$\begin{aligned} \int_{\Gamma} \delta \theta_n M_{nn} d\Gamma - \int_{\Gamma} \delta \mathbf{v} \cdot \mathbf{T} d\Gamma - (\delta \mathbf{v} \cdot \mathbf{a}_3 P)_{\mathbf{x} \in C} + \int_{\Omega} \delta \mathbf{v} \cdot \mathbf{b} d\Omega \\ - \int_{\Gamma_\theta} \delta \theta_n M_{nn} d\Gamma + \int_{\Gamma_v} \delta \mathbf{v} \cdot \mathbf{T} d\Gamma + (\delta \mathbf{v} \cdot \mathbf{a}_3 P)_{\mathbf{x} \in C_v} = - \int_{\Gamma_T} \delta \mathbf{v} \cdot \bar{\mathbf{t}} d\Gamma - \int_{\Omega} \delta \mathbf{v} \cdot \bar{\mathbf{b}} d\Omega \end{aligned} \quad (21e)$$

187 where the geometric relationships of Eq. (19) is used herein.

188 3. Mixed meshfree formulation for modified Hellinger-Reissner weak 189 form

190 3.1. Reproducing kernel approximation for displacement

191 This study approximates the displacement by adopting reproducing kernel
192 approximation. As shown in Fig. 2, the mid-surface of the shell Ω is discretized
193 by a set of meshfree nodes $\{\boldsymbol{\xi}_I\}_{I=1}^{n_p}$ in parametric configuration, where n_p is the
194 total number of meshfree nodes. The approximated displacement namely \boldsymbol{v}^h
195 can be expressed as:

$$\boldsymbol{v}(\boldsymbol{\xi}) = \sum_{I=1}^{n_p} \Psi_I(\boldsymbol{\xi}) \boldsymbol{d}_I \quad (22)$$

196 in which Ψ_I and \boldsymbol{d}_I is the shape function and nodal coefficient tensor related by
197 node $\boldsymbol{\xi}_I$. According to reproducing kernel approximation [4], the shape function
198 takes the following form:

$$\Psi_I(\boldsymbol{\xi}) = \boldsymbol{p}^T(\boldsymbol{\xi}) \boldsymbol{c}(\boldsymbol{\xi}) \phi(\boldsymbol{\xi}_I - \boldsymbol{\xi}) \quad (23)$$

199 where \boldsymbol{p} is the basis function vector represented using the following quadratic
200 function as:

$$\boldsymbol{p} = \{1, \xi^1, \xi^2, (\xi^1)^2, \xi^1 \xi^2, (\xi^2)^2\}^T \quad (24)$$

201 The kernel function denoted by ϕ controls the support and smoothness of
202 meshfree shape functions. The quintic B-spline function with square support is
203 used herein as the kernel function:

$$\phi(\boldsymbol{\xi}_I - \boldsymbol{\xi}) = \phi(\hat{s}_1) \phi(\hat{s}_2), \quad \hat{s}_\alpha = \frac{|\xi_I^\alpha - \xi^\alpha|}{s_{\alpha I}} \quad (25)$$

204 with

$$\phi(\hat{s}_\alpha) = \frac{1}{5!} \begin{cases} (3 - 3\hat{s}_\alpha)^5 - 6(2 - 3\hat{s}_\alpha)^5 + 15(1 - 3\hat{s}_\alpha)^5 & \hat{s}_\alpha \leq \frac{1}{3} \\ (3 - 3\hat{s}_\alpha)^5 - 6(2 - 3\hat{s}_\alpha)^5 & \frac{1}{3} < \hat{s}_\alpha \leq \frac{2}{3} \\ (3 - 3\hat{s}_\alpha)^5 & \frac{2}{3} < \hat{s}_\alpha \leq 1 \\ 0 & \hat{s}_\alpha > 1 \end{cases} \quad (26)$$

205 and $s_{\alpha I}$ means the support size of meshfree shape function Ψ_I .

206 The unknown vector \boldsymbol{c} in shape function are determined by the fulfillment
207 of the so-called consistency condition:

$$\sum_{I=1}^{n_p} \Psi_I(\boldsymbol{\xi}) \boldsymbol{p}(\boldsymbol{\xi}_I) = \boldsymbol{p}(\boldsymbol{\xi}) \quad (27)$$

208 or equivalently

$$\sum_{I=1}^{n_p} \Psi_I(\boldsymbol{\xi}) \boldsymbol{p}(\boldsymbol{\xi}_I - \boldsymbol{\xi}) = \boldsymbol{p}(\mathbf{0}) \quad (28)$$

209 Substituting Eq. (22) into (28), yields:

$$\mathbf{A}(\boldsymbol{\xi})\mathbf{c}(\boldsymbol{\xi}) = \mathbf{p}(\mathbf{0}) \quad \Rightarrow \quad \mathbf{c}(\boldsymbol{\xi}) = \mathbf{A}^{-1}(\boldsymbol{\xi})\mathbf{p}(\mathbf{0}) \quad (29)$$

210 where \mathbf{A} is the moment matrix:

$$\mathbf{A}(\boldsymbol{\xi}) = \sum_{I=1}^{n_p} \phi(\boldsymbol{\xi}_I - \boldsymbol{\xi}) \mathbf{p}(\boldsymbol{\xi}_I - \boldsymbol{\xi}) \mathbf{p}^T(\boldsymbol{\xi}_I - \boldsymbol{\xi}) \quad (30)$$

211 Substituting Eq. (29) back into Eq. (22), the expression of meshfree shape
212 function can be written as:

$$\Psi_I(\boldsymbol{\xi}) = \mathbf{p}^T(\boldsymbol{\xi}_I - \boldsymbol{\xi}) \mathbf{A}^{-1}(\boldsymbol{\xi}) \mathbf{p}(\mathbf{0}) \phi(\boldsymbol{\xi}_I - \boldsymbol{\xi}) \quad (31)$$

213 3.2. Reproducing kernel gradient smoothing approximation for effective stress 214 and strain

215 In Galerkin meshfree formulation, the mid-plane of thin shell Ω is split by
216 a set of integration cells Ω_C 's, $\cup_{C=1}^{n_e} \Omega_C \approx \Omega$, as shown in Fig. 2. With the
217 inspiration of reproducing kernel smoothing framework, the Cartesian and co-
218 variant derivatives of displacement, $\mathbf{v}_{,\alpha}$ and $-\mathbf{v}_{,\alpha}|_{\beta}$, in strains $\varepsilon_{\alpha\beta}$, $\kappa_{\alpha\beta}$ are
219 approximated by $(p-1)$ -th order polynomials in each integration cells. In inte-
220 gration cell Ω_C , the approximated derivatives and strains denoted by $\mathbf{v}_{,\alpha}^h$, $\varepsilon_{\alpha\beta}^h$
221 and $-\mathbf{v}_{,\alpha}^h|_{\beta}$, $\kappa_{\alpha\beta}^h$ can be expressed by:

$$\mathbf{v}_{,\alpha}^h(\boldsymbol{\xi}) = \mathbf{q}^T(\boldsymbol{\xi}) \mathbf{d}_{\alpha}^{\varepsilon}, \quad \varepsilon_{\alpha\beta}^h(\boldsymbol{\xi}) = \mathbf{q}^T(\boldsymbol{\xi}) \frac{1}{2} (\mathbf{a}_{\alpha} \cdot \mathbf{d}_{\beta}^{\varepsilon} + \mathbf{a}_{\beta} \cdot \mathbf{d}_{\alpha}^{\varepsilon}) \quad (32)$$

$$-\mathbf{v}_{,\alpha}^h|_{\beta}(\boldsymbol{\xi}) = \mathbf{q}^T(\boldsymbol{\xi}) \mathbf{d}_{\alpha\beta}^{\kappa}, \quad \kappa_{\alpha\beta}^h(\boldsymbol{\xi}) = \mathbf{q}^T(\boldsymbol{\xi}) \mathbf{a}_3 \cdot \mathbf{d}_{\alpha\beta}^{\kappa} \quad (33)$$

223 where \mathbf{q} is the linear polynomial vector and has the following form:

$$\mathbf{q} = \{1, \xi^1, \xi^2\}^T \quad (34)$$

224 and the $\mathbf{d}_{\alpha}^{\varepsilon}$, $\mathbf{d}_{\alpha\beta}^{\kappa}$ are the corresponding coefficient vector tensors. For the con-
225 ciseness, the mixed usage of tensor and vector is introduced in this study. For
226 instance, the component of coefficient tensor vector $\mathbf{d}_{\alpha I}^{\varepsilon}$, $\mathbf{d}_{\alpha}^{\varepsilon} = \{\mathbf{d}_{\alpha I}^{\varepsilon}\}$, is a three
227 dimensional tensor, $\dim \mathbf{d}_{\alpha I}^{\varepsilon} = \dim \mathbf{v}$.

228 In order to meet the integration constraint of thin shell problem, the ap-
229 proximated stresses $N^{\alpha\beta h}$, $M^{\alpha\beta h}$ are assumed to be a similar form with strains,
230 yields:

$$N^{\alpha\beta h}(\boldsymbol{\xi}) = \mathbf{q}^T(\boldsymbol{\xi}) \mathbf{a}^{\alpha} \cdot \mathbf{d}_N^{\beta}, \quad \mathbf{a}_{\alpha} N^{\alpha\beta h}(\boldsymbol{\xi}) = \mathbf{q}^T(\boldsymbol{\xi}) \mathbf{d}_N^{\beta} \quad (35)$$

$$M^{\alpha\beta h}(\boldsymbol{\xi}) = \mathbf{q}^T(\boldsymbol{\xi}) \mathbf{a}_3 \cdot \mathbf{d}_M^{\alpha\beta}, \quad \mathbf{a}_3 M^{\alpha\beta h}(\boldsymbol{\xi}) = \mathbf{q}^T(\boldsymbol{\xi}) \mathbf{d}_M^{\alpha\beta} \quad (36)$$

232 substituting the approximations of Eqs. (22), (32), (33), (35), (36) into Eqs.
233 (21c), (21d) can express $\mathbf{d}_{\beta}^{\varepsilon}$ and $\mathbf{d}_{\alpha\beta}^{\kappa}$ by \mathbf{d} as:

$$\mathbf{d}_{\beta}^{\varepsilon} = \mathbf{G}^{-1} \left(\sum_{I=1}^{n_p} (\tilde{\mathbf{g}}_{\beta I} - \bar{\mathbf{g}}_{\beta I}) \mathbf{d}_I + \hat{\mathbf{g}}_{\beta} \right) \quad (37)$$

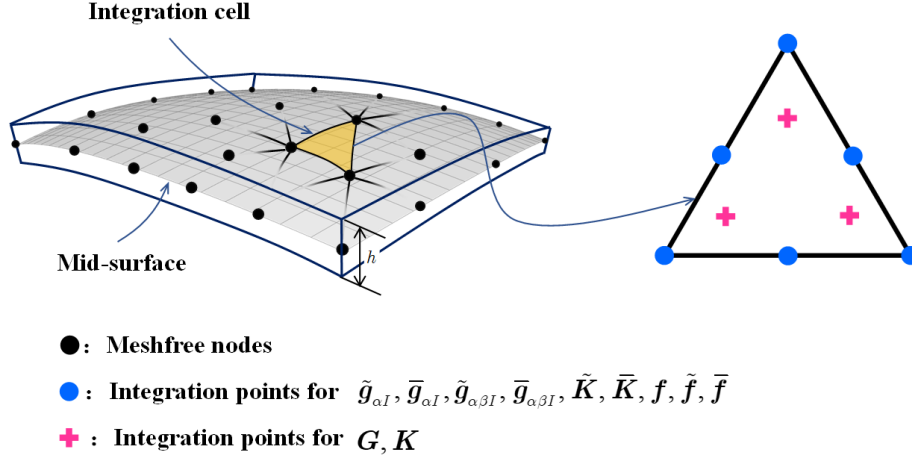


Figure 2: Integration scheme for Hu-Washizu weak form.

234

$$d_{\alpha\beta}^{\kappa} = G^{-1} \left(\sum_{I=1}^{n_p} (\tilde{g}_{\alpha\beta I} - \bar{g}_{\alpha\beta I}) d_I + \hat{g}_{\alpha\beta} \right) \quad (38)$$

235 with

$$G = \int_{\Omega_C} q^T q d\Omega \quad (39)$$

236

$$\tilde{g}_{\beta I} = \int_{\Gamma_C} \Psi_I q n_{\beta} d\Gamma - \int_{\Omega_C} \Psi_I q_{|\beta} d\Omega \quad (40a)$$

$$\bar{g}_{\beta I} = \int_{\Gamma_C \cap \Gamma_v} \Psi_I q n_{\beta} d\Gamma \quad (40b)$$

$$\hat{g}_{\beta} = \int_{\Gamma_C \cap \Gamma_v} q n_{\beta} \bar{v} d\Gamma \quad (40c)$$

237

$$\begin{aligned} \tilde{g}_{\alpha\beta I} = & \int_{\Gamma_C} \Psi_{I,\gamma} n^{\gamma} q n_{\alpha} n_{\beta} d\Gamma - \int_{\Gamma_C} \Psi_I (q_{|\beta} n_{\alpha} + (q s_{\alpha} n_{\beta}),_{\gamma} s^{\gamma}) d\Gamma \\ & + [[\Psi_I q s_{\alpha} n_{\beta}]]_{\mathbf{x} \in C_C} - \int_{\Omega_C} \Psi q_{,\alpha|\beta} d\Omega \end{aligned} \quad (41a)$$

$$\begin{aligned} \bar{g}_{\alpha\beta I} = & \int_{\Gamma_C \cap \Gamma_{\theta}} \Psi_{I,\gamma} n^{\gamma} q n_{\alpha} n_{\beta} d\Gamma - \int_{\Gamma_C \cap \Gamma_v} \Psi_I (q_{|\beta} n_{\alpha} + (q s_{\alpha} n_{\beta}),_{\gamma} s^{\gamma}) d\Gamma \\ & + [[\Psi_I q s_{\alpha} n_{\beta}]]_{\mathbf{x} \in C_C \cap C_v} \end{aligned} \quad (41b)$$

$$\begin{aligned} \hat{g}_{\alpha\beta} = & \int_{\Gamma_C \cap \Gamma_{\theta}} q n_{\alpha} n_{\beta} a_3 \bar{\theta}_{\mathbf{n}} d\Gamma - \int_{\Gamma_C \cap \Gamma_v} (q_{|\beta} n_{\alpha} + (q s_{\alpha} n_{\beta}),_{\gamma} s^{\gamma}) \bar{v} d\Gamma \\ & + [[q s_{\alpha} n_{\beta} \bar{v}]]_{\mathbf{x} \in C_C \cap C_v} \end{aligned} \quad (41c)$$

where evaluations of $\mathbf{q}_{|\beta}$, $\mathbf{q}_{,\alpha|\beta}$ are detail in Appendix A. Further plugging Eqs. (37) and (38) back into Eqs. (32) and (33) respectively gives the final expression of $\mathbf{v}_{,\alpha}^h$, $\varepsilon_{\alpha\beta}^h$ and $-\mathbf{v}_{,\alpha\beta}^h$, $\kappa_{\alpha\beta}^h$ as:

$$\mathbf{v}_{,\alpha}^h = \sum_{I=1}^{n_p} (\tilde{\Psi}_{I,\alpha} - \bar{\Psi}_{I,\alpha}) \mathbf{d}_I + \mathbf{q}^T \mathbf{G}^{-1} \hat{\mathbf{g}}_\alpha \quad (42a)$$

$$\begin{aligned} \varepsilon_{\alpha\beta}^h &= \sum_{I=1}^{n_p} \frac{1}{2} (\mathbf{a}_\alpha \tilde{\Psi}_{I,\beta} + \mathbf{a}_\beta \tilde{\Psi}_{I,\alpha}) \cdot \mathbf{d}_I - \sum_{I=1}^{n_p} \frac{1}{2} (\mathbf{a}_\alpha \bar{\Psi}_{I,\beta} + \mathbf{a}_\beta \bar{\Psi}_{I,\alpha}) \cdot \mathbf{d}_I \\ &\quad + \mathbf{q}^T \mathbf{G}^{-1} \frac{1}{2} (\mathbf{a}_\alpha \cdot \hat{\mathbf{g}}_\beta + \mathbf{a}_\beta \cdot \hat{\mathbf{g}}_\alpha) \\ &= \tilde{\varepsilon}_{\alpha\beta}^h - \bar{\varepsilon}_{\alpha\beta}^h + \hat{\varepsilon}_{\alpha\beta}^h \end{aligned} \quad (42b)$$

$$-\mathbf{v}_{,\alpha\beta}^h = \sum_{I=1}^{n_p} (\tilde{\Psi}_{I,\alpha\beta} - \bar{\Psi}_{I,\alpha\beta}) \mathbf{d}_I + \mathbf{q}^T \mathbf{G}^{-1} \hat{\mathbf{g}}_{\alpha\beta} \quad (43a)$$

$$\begin{aligned} \kappa_{\alpha\beta}^h &= \sum_{I=1}^{n_p} \tilde{\Psi}_{I,\alpha\beta} \mathbf{a}_3 \cdot \mathbf{d}_I - \sum_{I=1}^{n_p} \bar{\Psi}_{I,\alpha\beta} \mathbf{a}_3 \cdot \mathbf{d}_I + \mathbf{q}^T \mathbf{G}^{-1} \mathbf{a}_3 \cdot \hat{\mathbf{g}}_{\alpha\beta} \\ &= \tilde{\kappa}_{\alpha\beta}^h - \bar{\kappa}_{\alpha\beta}^h + \hat{\kappa}_{\alpha\beta}^h \end{aligned} \quad (43b)$$

with

$$\begin{cases} \tilde{\varepsilon}_{\alpha\beta}^h = \sum_{I=1}^{n_p} \frac{1}{2} (\mathbf{a}_\alpha \tilde{\Psi}_{I,\beta} + \mathbf{a}_\beta \tilde{\Psi}_{I,\alpha}) \cdot \mathbf{d}_I = \sum_{I=1}^{n_p} \tilde{\varepsilon}_{\alpha\beta I} \cdot \mathbf{d}_I \\ \bar{\varepsilon}_{\alpha\beta}^h = \sum_{I=1}^{n_p} \frac{1}{2} (\mathbf{a}_\alpha \bar{\Psi}_{I,\beta} + \mathbf{a}_\beta \bar{\Psi}_{I,\alpha}) \cdot \mathbf{d}_I = \sum_{I=1}^{n_p} \bar{\varepsilon}_{\alpha\beta I} \cdot \mathbf{d}_I \\ \hat{\varepsilon}_{\alpha\beta}^h = \mathbf{q}^T \mathbf{G}^{-1} \frac{1}{2} (\mathbf{a}_\alpha \cdot \hat{\mathbf{g}}_\beta + \mathbf{a}_\beta \cdot \hat{\mathbf{g}}_\alpha) \end{cases} \quad (44)$$

$$\begin{cases} \tilde{\Psi}_{I,\alpha}(\boldsymbol{\xi}) = \mathbf{q}^T(\boldsymbol{\xi}) \mathbf{G}^{-1} \tilde{\mathbf{g}}_{\alpha I} \\ \bar{\Psi}_{I,\alpha}(\boldsymbol{\xi}) = \mathbf{q}^T(\boldsymbol{\xi}) \mathbf{G}^{-1} \bar{\mathbf{g}}_{\alpha I} \\ \tilde{\varepsilon}_{\alpha\beta I} = \frac{1}{2} (\mathbf{a}_\alpha \tilde{\Psi}_{I,\beta} + \mathbf{a}_\beta \tilde{\Psi}_{I,\alpha}) \\ \bar{\varepsilon}_{\alpha\beta I} = \frac{1}{2} (\mathbf{a}_\alpha \bar{\Psi}_{I,\beta} + \mathbf{a}_\beta \bar{\Psi}_{I,\alpha}) \end{cases} \quad (45)$$

$$\begin{cases} \tilde{\kappa}_{\alpha\beta}^h = \sum_{I=1}^{n_p} \tilde{\Psi}_{I,\alpha\beta} \mathbf{a}_3 \cdot \mathbf{d}_I = \sum_{I=1}^{n_p} \tilde{\kappa}_{\alpha\beta I} \cdot \mathbf{d}_I \\ \bar{\kappa}_{\alpha\beta}^h = \sum_{I=1}^{n_p} \bar{\Psi}_{I,\alpha\beta} \mathbf{a}_3 \cdot \mathbf{d}_I = \sum_{I=1}^{n_p} \bar{\kappa}_{\alpha\beta I} \cdot \mathbf{d}_I \\ \hat{\kappa}_{\alpha\beta}^h = \mathbf{q}^T \mathbf{G}^{-1} \mathbf{a}_3 \cdot \hat{\mathbf{g}}_{\alpha\beta} \end{cases} \quad (46)$$

$$\begin{cases} \tilde{\Psi}_{I,\alpha\beta}(\boldsymbol{\xi}) = \mathbf{q}^T(\boldsymbol{\xi})\mathbf{G}^{-1}\tilde{\mathbf{g}}_{\alpha\beta I} \\ \bar{\Psi}_{I,\alpha\beta}(\boldsymbol{\xi}) = \mathbf{q}^T(\boldsymbol{\xi})\mathbf{G}^{-1}\tilde{\mathbf{g}}_{\alpha\beta I} \\ \tilde{\boldsymbol{\kappa}}_{\alpha\beta I} = \tilde{\Psi}_{I,\alpha\beta}\mathbf{a}_3 \\ \bar{\boldsymbol{\kappa}}_{\alpha\beta I} = \bar{\Psi}_{I,\alpha\beta}\mathbf{a}_3 \end{cases} \quad (47)$$

248 It has to be noted that, referring to reproducing kernel gradient smoothing
 249 framework [25], $\tilde{\Psi}_{I,\alpha}$, $\tilde{\Psi}_{I,\alpha\beta}$ are actually the first and second order smoothed
 250 gradients in curvilinear coordinates. $\tilde{\mathbf{g}}_{\alpha I}$ and $\tilde{\mathbf{g}}_{\alpha\beta I}$ are the right hand side in-
 251 tegration constraints for first and second order gradients, then this formulation
 252 can meet the variational consistency for the p -th order polynomials. It should
 253 be known that, in curved model, the variational consistency for non-polynomial
 254 functions, like trigonometric functions, should be required for the polynomial
 255 solution. Even with p -th order variational consistency, the proposed formulation
 256 can not exactly reproduce the solution spanned by basis functions. However,
 257 the accuracy of reproducing kernel smoothed gradients is still better than tradi-
 258 tional meshfree formulation. Numerical examples in the section below will pro-
 259 vide better evidence to prove the accuracy of the reproducing kernel smoothed
 260 gradients.

261 **4. Naturally variational enforcement for essential boundary condi-**
 262 **tions**

263 *4.1. Discrete equilibrium equations*

264 With the approximated effective stresses and strains, the last equation of
 265 weak form Eq. (21e) becomes:

$$-\sum_{C=1}^{n_e} \sum_{I=1}^{n_p} \delta \mathbf{d}_I \cdot \left((\tilde{\mathbf{g}}_{\alpha I}^T - \bar{\mathbf{g}}_{\alpha I}^T) \mathbf{d}_N^\alpha + (\tilde{\mathbf{g}}_{\alpha \beta I}^T - \bar{\mathbf{g}}_{\alpha \beta I}^T) \mathbf{d}_M^{\alpha \beta} \right) = -\sum_{I=1}^{n_p} \delta \mathbf{d}_I \cdot \mathbf{f}_I \quad (48)$$

266 where \mathbf{f}_I 's are the components of the traditional force vector:

$$\mathbf{f}_I = \int_{\Gamma_t} \Psi_I \bar{\mathbf{t}} d\Gamma - \int_{\Gamma_M} \Psi_{I,\gamma} n^\gamma \bar{M}_{nn} d\Gamma + [[\Psi_I \mathbf{a}_3 \bar{P}]]_{\mathbf{x} \in C_P} + \int_{\Omega} \Psi_I \bar{\mathbf{b}} d\Omega \quad (49)$$

267 The left side of Eq. (48) can be simplified using the following steps. For clarity,
 268 the derivation of first term in Eq. (48) taken as an example is given by:

$$\begin{aligned} \sum_{I=1}^{n_p} \delta \mathbf{d}_I \cdot \tilde{\mathbf{g}}_{\alpha I}^T \mathbf{d}_N^\alpha &= \sum_{I=1}^{n_p} \delta \mathbf{d}_I \cdot (\mathbf{G}^{-1} \tilde{\mathbf{g}}_{\alpha I})^T \mathbf{G} \mathbf{d}_N^\alpha \\ &= \int_{\Omega_C} \sum_{I=1}^{n_p} \delta \mathbf{d}_I \cdot (\mathbf{q}^T \mathbf{G}^{-1} \tilde{\mathbf{g}}_{\alpha I})^T \mathbf{q}^T \mathbf{d}_N^\alpha d\Omega \\ &= \int_{\Omega_C} \sum_{I=1}^{n_p} \delta \mathbf{d}_I \cdot \mathbf{a}_\beta (\mathbf{q}^T \mathbf{G}^{-1} \tilde{\mathbf{g}}_{\alpha I})^T N^{\alpha \beta h} d\Omega \\ &= \int_{\Omega_C} \delta \tilde{\varepsilon}_{\alpha \beta}^h N^{\alpha \beta h} d\Omega \end{aligned} \quad (50)$$

269 following the above procedure and including the weak form of Eqs. (21a), (21b),
 270 the left side of Eq. (48) in Ω_C becomes:

$$\begin{aligned}
 & \sum_{I=1}^{n_p} \delta \mathbf{d}_I \cdot \left((\tilde{\mathbf{g}}_{\alpha I}^T - \bar{\mathbf{g}}_{\alpha I}^T) \mathbf{d}_N^\alpha + (\tilde{\mathbf{g}}_{\alpha \beta I}^T - \bar{\mathbf{g}}_{\alpha \beta I}^T) \mathbf{d}_M^{\alpha \beta} \right) \\
 &= \int_{\Omega_C} ((\delta \tilde{\varepsilon}_{\alpha \beta}^h - \delta \bar{\varepsilon}_{\alpha \beta}^h) N^{\alpha \beta h} + (\delta \tilde{\kappa}_{\alpha \beta}^h - \delta \bar{\kappa}_{\alpha \beta}^h) M^{\alpha \beta h}) d\Omega \\
 &= \int_{\Omega_C} (\delta \tilde{\varepsilon}_{\alpha \beta}^h - \delta \bar{\varepsilon}_{\alpha \beta}^h) h C^{\alpha \beta \gamma \eta} \varepsilon_{\gamma \eta}^h + (\delta \tilde{\kappa}_{\alpha \beta}^h - \delta \bar{\kappa}_{\alpha \beta}^h) \frac{h^3}{12} C^{\alpha \beta \gamma \eta} \kappa_{\gamma \eta}^h \\
 &= \int_{\Omega_C} \delta \tilde{\varepsilon}_{\alpha \beta}^h h C^{\alpha \beta \gamma \eta} \varepsilon_{\gamma \eta}^h d\Omega + \int_{\Omega_C} \delta \tilde{\kappa}_{\alpha \beta}^h \frac{h^3}{12} C^{\alpha \beta \gamma \eta} \kappa_{\gamma \eta}^h d\Omega \\
 &\quad - \int_{\Omega_C} \delta \bar{\varepsilon}_{\alpha \beta}^h h C^{\alpha \beta \gamma \eta} \varepsilon_{\gamma \eta}^h d\Omega - \int_{\Omega_C} \delta \bar{\kappa}_{\alpha \beta}^h h C^{\alpha \beta \gamma \eta} \kappa_{\gamma \eta}^h d\Omega \\
 &\quad - \int_{\Omega_C} \delta \tilde{\kappa}_{\alpha \beta}^h \frac{h^3}{12} C^{\alpha \beta \gamma \eta} \bar{\kappa}_{\gamma \eta}^h d\Omega - \int_{\Omega_C} \delta \bar{\kappa}_{\alpha \beta}^h \frac{h^3}{12} C^{\alpha \beta \gamma \eta} \tilde{\kappa}_{\gamma \eta}^h d\Omega \\
 &\quad + \int_{\Omega_C} \delta \bar{\varepsilon}_{\alpha \beta}^h h C^{\alpha \beta \gamma \eta} \varepsilon_{\gamma \eta}^h d\Omega + \int_{\Omega_C} \delta \bar{\kappa}_{\alpha \beta}^h \frac{h^3}{12} C^{\alpha \beta \gamma \eta} \bar{\kappa}_{\gamma \eta}^h d\Omega \\
 &\quad + \int_{\Omega_C} (\delta \tilde{\varepsilon}_{\alpha \beta}^h - \delta \bar{\varepsilon}_{\alpha \beta}^h) h C^{\alpha \beta \gamma \eta} \varepsilon_{\gamma \eta}^h d\Omega + \int_{\Omega_C} (\delta \tilde{\kappa}_{\alpha \beta}^h - \delta \bar{\kappa}_{\alpha \beta}^h) \frac{h^3}{12} C^{\alpha \beta \gamma \eta} \kappa_{\gamma \eta}^h d\Omega
 \end{aligned} \tag{51}$$

271 on further substituting Eqs. (44) and (46) into above equation gives the final
 272 discrete equilibrium equations, respectively:

$$(\mathbf{K} + \tilde{\mathbf{K}} + \bar{\mathbf{K}}) \mathbf{d} = \mathbf{f} + \tilde{\mathbf{f}} + \bar{\mathbf{f}} \tag{52}$$

273 where

$$\mathbf{K}_{IJ} = \int_{\Omega} \tilde{\varepsilon}_{\alpha \beta I} h C^{\alpha \beta \gamma \eta} \tilde{\varepsilon}_{\gamma \eta J} d\Omega + \int_{\Omega} \tilde{\kappa}_{\alpha \beta I} \frac{h^3}{12} C^{\alpha \beta \gamma \eta} \tilde{\kappa}_{\alpha \beta J} d\Omega \tag{53}$$

274

$$\begin{aligned}
 \tilde{\mathbf{K}}_{IJ} &= - \int_{\Gamma_v} (\Psi_I \tilde{\mathbf{T}}_{NJ} + \tilde{\mathbf{T}}_{NJ} \Psi_J) d\Gamma \\
 &\quad + \int_{\Gamma_\theta} (\Psi_{I,\gamma} n^\gamma \mathbf{a}_3 \tilde{\mathbf{M}}_{nnJ} + \mathbf{a}_3 \tilde{\mathbf{M}}_{nnI} \Psi_{J,\gamma} n^\gamma) d\Gamma
 \end{aligned} \tag{54a}$$

$$\begin{aligned}
 &\quad + ([[\Psi_I \mathbf{a}_3 \tilde{\mathbf{P}}_J]] + [[\tilde{\mathbf{P}}_I \mathbf{a}_3 \Psi_J]])_{\mathbf{x} \in C_v} \\
 \tilde{\mathbf{f}}_I &= - \int_{\Gamma_v} \tilde{\mathbf{T}}_{NI} \cdot \bar{\mathbf{v}} d\Gamma + \int_{\Gamma_\theta} \tilde{\mathbf{M}}_{nnI} \bar{\theta}_n d\Gamma + [[\tilde{\mathbf{P}}_I \mathbf{a}_3 \cdot \bar{\mathbf{v}}]]_{\mathbf{x} \in C_v}
 \end{aligned} \tag{54b}$$

275

$$\bar{\mathbf{K}}_{IJ} = - \int_{\Gamma_v} \bar{\mathbf{T}}_{MI} \Psi_J d\Gamma + \int_{\Gamma_\theta} \mathbf{a}_3 \bar{\mathbf{M}}_{nnI} \Psi_{J,\gamma} n^\gamma d\Gamma + [[\bar{\mathbf{P}}_I \mathbf{a}_3 \Psi_J]]_{\mathbf{x} \in C_v} \tag{55a}$$

$$\bar{\mathbf{f}}_I = - \int_{\Gamma_v} \bar{\mathbf{T}}_{MI} \cdot \bar{\mathbf{v}} d\Gamma + \int_{\Gamma_\theta} \bar{\mathbf{M}}_{nnI} \bar{\theta}_n d\Gamma + [[\bar{\mathbf{P}}_I \mathbf{a}_3 \cdot \bar{\mathbf{v}}]]_{\mathbf{x} \in C_v} \tag{55b}$$

276 The detailed derivations of Eqs (53)-(55) are listed in the Appendix B. As
 277 shown in these equations, Eq. (53) is the conventional stiffness matrix evaluated
 278 by smoothed gradients $\tilde{\Psi}_{I,\alpha}$, $\tilde{\Psi}_{I,\alpha}|_\beta$, and the Eqs. (54) and (55) contribute for
 279 the enforcement of essential boundary. It should be mentioned that, in accor-
 280 dance with reproducing kernel smoothed gradient framework, the integration
 281 scheme of Eqs. (53-55) should be aligned with the those used in the construc-
 282 tion of smoothed gradients. The integration scheme used for proposed method
 283 is shown in Fig. 2, the detailed positions and weight of integration points can
 284 be found in [31] With a close look at Eqs. (54) and (55), the proposed approach
 285 for enforcing essential boundary conditions show an identical structure with tra-
 286 ditional Nitsche's method, both have the consistent and stabilized terms. So,
 287 the next subsection will review the Nitsche's method and compare it with the
 288 proposed method.

289 4.2. Comparison with Nitsche's method

290 The Nitsche's method for enforcing essential boundaries can be regarded as a
 291 combination of Lagrangian multiplier method and penalty method, in which the
 292 Lagrangian multiplier is represented by the approximated displacement. The
 293 corresponding total potential energy functional Π_P is given by:

$$\begin{aligned}
 \Pi_P(\mathbf{v}) = & \int_{\Omega} \frac{1}{2} \varepsilon_{\alpha\beta} N^{\alpha\beta} d\Omega + \int_{\Omega} \frac{1}{2} \kappa_{\alpha\beta} M^{\alpha\beta} d\Omega \\
 & - \int_{\Gamma_t} \mathbf{v} \cdot \bar{\mathbf{t}} d\Gamma + \int_{\Gamma_M} \mathbf{v}_{,\gamma} n^\gamma \mathbf{a}_3 M_{nn} d\Gamma + (\mathbf{v} \cdot \mathbf{a}_3 P)_{\mathbf{x} \in C_P} - \int_{\Omega} \mathbf{v} \cdot \bar{\mathbf{b}} d\Omega \\
 & - \underbrace{\int_{\Gamma_v} \mathbf{t} \cdot (\mathbf{v} - \bar{\mathbf{v}}) d\Gamma + \int_{\Gamma_\theta} M_{nn} (\theta_n - \bar{\theta}_n) d\Gamma + (P \mathbf{a}_3 \cdot (\mathbf{v} - \bar{\mathbf{v}}))_{\mathbf{x} \in C_v}}_{\text{consistent term}} \quad (56) \\
 & + \underbrace{\frac{\alpha_v}{2} \int_{\Gamma_v} \mathbf{v} \cdot \mathbf{v} d\Gamma + \frac{\alpha_\theta}{2} \int_{\Gamma_\theta} \theta_n^2 d\Gamma + \frac{\alpha_C}{2} (\mathbf{v} \cdot \mathbf{v})_{\mathbf{x} \in C_v}}_{\text{stabilized term}}
 \end{aligned}$$

294 where the consistent term generated from the Lagrangian multiplier method
 295 contributes to enforce the essential boundary, and meet the variational con-
 296 sistency condition. However, the consistent term can not always ensure the
 297 coercivity of stiffness, so the penalty method is introduced to serve as a sta-
 298 bilized term. With a standard variational argument, the corresponding weak

form can be stated as:

$$\begin{aligned}
\delta\Pi_P(\mathbf{v}) &= \int_{\Omega} \delta\varepsilon_{\alpha\beta} N^{\alpha\beta} d\Omega + \int_{\Omega} \delta\kappa_{\alpha\beta} M^{\alpha\beta} d\Omega \\
&\quad - \int_{\Gamma_t} \delta\mathbf{v} \cdot \bar{\mathbf{t}} d\Gamma + \int_{\Gamma_M} \delta\mathbf{v}_{,\gamma} n^{\gamma} \mathbf{a}_3 M_{nn} d\Gamma + (\delta\mathbf{v} \cdot \mathbf{a}_3 P)_{\mathbf{x} \in C_P} - \int_{\Omega} \delta\mathbf{v} \cdot \bar{\mathbf{b}} d\Omega \\
&\quad - \int_{\Gamma_v} \delta\mathbf{v} \cdot \mathbf{t} d\Gamma + \int_{\Gamma_{\theta}} \delta\theta_{\mathbf{n}} M_{nn} d\Gamma + (\mathbf{v} \cdot \mathbf{a}_3 P)_{\mathbf{x} \in C_v} \\
&\quad - \int_{\Gamma_v} \delta\mathbf{t} \cdot (\mathbf{v} - \bar{\mathbf{v}}) d\Gamma + \int_{\Gamma_{\theta}} \delta M_{nn} (\theta_{\mathbf{n}} - \bar{\theta}_{\mathbf{n}}) d\Gamma + (\delta P \mathbf{a}_3 \cdot (\mathbf{v} - \bar{\mathbf{v}}))_{\mathbf{x} \in C_v} \\
&\quad + \alpha_v \int_{\Gamma_v} \delta\mathbf{v} \cdot \mathbf{v} d\Gamma + \alpha_{\theta} \int_{\Gamma_{\theta}} \delta\theta_{\mathbf{n}} \theta_{\mathbf{n}} d\Gamma + \alpha_C (\delta\mathbf{v} \cdot \mathbf{v})_{\mathbf{x} \in C_v} \\
&= 0
\end{aligned} \tag{57}$$

in which α_v , α_{θ} and α_C represent experimental artificial parameters. Further invoking the conventional reproducing kernel approximation of Eq. (22) leads to the following discrete equilibrium equations:

$$\sum_{J=1}^{n_p} (\mathbf{K}_{IJ} + \mathbf{K}_{IJ}^c + \mathbf{K}_{IJ}^s) \mathbf{d}_J = \mathbf{f}_I + \mathbf{f}^c + \mathbf{f}^s \tag{58}$$

where the stiffness \mathbf{K}_{IJ} is identical with Eq. (53). \mathbf{K}_{IJ}^c and \mathbf{K}_{IJ}^s are the stiffness matrices for consistent and stabilized terms, respectively, and have the following form:

$$\begin{aligned}
\mathbf{K}_{IJ}^c &= - \int_{\Gamma_v} (\Psi_I \mathbf{T}_{NJ} + \mathbf{T}_{NJ} \Psi_J) d\Gamma \\
&\quad + \int_{\Gamma_{\theta}} (\Psi_{I,\gamma} n^{\gamma} \mathbf{a}_3 \mathbf{M}_{nnJ} + \mathbf{a}_3 \mathbf{M}_{nnI} \Psi_{I,\gamma} n^{\gamma}) d\Gamma \\
&\quad + ([[\Psi_I \mathbf{a}_3 \mathbf{P}_J]] + [[\mathbf{P}_I \mathbf{a}_3 \Psi_J]])_{\mathbf{x} \in C_v}
\end{aligned} \tag{59a}$$

$$\mathbf{f}_I^c = - \int_{\Gamma_v} \mathbf{T}_I \cdot \bar{\mathbf{v}} d\Gamma + \int_{\Gamma_{\theta}} \mathbf{M}_{nnI} \bar{\theta}_{\mathbf{n}} d\Gamma + [[\mathbf{P}_I \mathbf{a}_3 \cdot \bar{\mathbf{v}}]]_{\mathbf{x} \in C_v} \tag{59b}$$

306

$$\mathbf{K}_{IJ}^s = \alpha_v \int_{\Gamma_v} \Psi_I \Psi_J \mathbf{1} d\Gamma + \alpha_{\theta} \int_{\Gamma_{\theta}} \Psi_{I,\eta} n^{\eta} \mathbf{a}_3 \mathbf{a}_3 n^{\gamma} \Psi_{J,\gamma} d\Gamma + \alpha_C [[\Psi_I \mathbf{a}_3 \mathbf{a}_3 \Psi_J]]_{\mathbf{x} \in C_v} \tag{60a}$$

$$\mathbf{f}_I^s = \alpha_v \int_{\Gamma_v} \Psi_I \bar{\mathbf{v}} d\Gamma + \alpha_{\theta} \int_{\Gamma_{\theta}} \Psi_{I,\eta} n^{\eta} \mathbf{a}_3 \bar{\theta}_{\mathbf{n}} d\Gamma + \alpha_C [[\Psi_I \mathbf{a}_3 \mathbf{a}_3 \cdot \bar{\mathbf{v}}]]_{\mathbf{x} \in C_v} \tag{60b}$$

On comparing with the consistent terms of Eqs. (54) and (59), the expressions were almost identical, the major difference is that the higher order derivatives of shape functions have been replaced by smoothed gradients. Owing to

310 the reproducing kernel framework, the construction of smoothed gradients only
311 concerned about the computation of traditional meshfree shape functions and
312 their first order derivatives, which avoid the costly computation of higher order
313 derivatives. Moreover, the stabilized terms in Eq. (60) employs the penalty
314 method to ensure the coercivity of stiffness. In contrast, the stabilized term of
315 Eq. (55) naturally exists in its weak form, and can stabilize the result without
316 considering any artificial parameters.

317 5. Numerical examples

318 The suggested method, which uses Nitsche's method, the consistent repro-
 319 ducing kernel gradient smoothing integration scheme (RKGSI), and the non-
 320 consistent Gauss integration scheme (GI) with penalty method, as well as the
 321 proposed Hu-Washizu formulation (HW) to enforce the necessary boundary con-
 322 ditions, is validated in this section through several examples. A normalized
 323 support size of 2.5 is used for all the methods to ensure the requirement of
 324 quadratic base meshfree approximation. To eliminate the influence of integra-
 325 tion, the Gauss integration scheme uses 6 Gauss points for domain integration
 326 and 3 points for boundary integration, so as to maintain the same integration
 327 accuracy between domain and boundaries. Moreover, the number of integra-
 328 tion points are identical between the Gauss and RKGSI schemes. The error
 329 estimates of displacement (L_2 -Error) and energy (H_e -Error) is used here:

$$\begin{aligned}
 L_2\text{-Error} &= \frac{\sqrt{\int_{\Omega} (\mathbf{v} - \mathbf{v}^h) \cdot (\mathbf{v} - \mathbf{v}^h) d\Omega}}{\sqrt{\mathbf{v} \cdot \mathbf{v}}} \\
 H_e\text{-Error} &= \frac{\sqrt{\int_{\Omega} \left((\varepsilon_{\alpha\beta} - \varepsilon_{\alpha\beta}^h)(N^{\alpha\beta} - N^{\alpha\beta h}) + \int_{\Omega} (\kappa_{\alpha\beta} - \kappa_{\alpha\beta}^h)(M^{\alpha\beta} - M^{\alpha\beta h}) \right) d\Omega}}{\sqrt{\int_{\Omega} (\varepsilon_{\alpha\beta} N^{\alpha\beta} + \kappa_{\alpha\beta} M^{\alpha\beta}) d\Omega}}
 \end{aligned}
 \tag{61}$$

330 5.1. Patch tests

331 The linear and quadratic patch tests for flat and curved thin shells are firstly
 332 studied to verify the variational consistency of the proposed method. As shown
 333 in Fig. 3, the flat and curved models are depicted by an identical parametric
 334 domain $\Omega = (0, 1) \otimes (0, 1)$, where the cylindrical coordinate system with radius
 335 $R = 1$ is employed to describe the curved model, and the whole domain Ω
 336 is discretized by the 165 meshfree nodes. All the boundaries are enforced as
 337 essential boundary conditions with the following manufactured exact solution:

$$\mathbf{v} = \begin{Bmatrix} (\xi^1 + 2\xi^2)^n \\ (3\xi^1 + 4\xi^2)^n \\ (5\xi^1 + 6\xi^2)^n \end{Bmatrix}, \quad n = \begin{cases} 1 & \text{Linear patch test} \\ 2 & \text{Quadratic patch test} \end{cases}
 \tag{62}$$

338 Table 1 lists the L_2 - and H_e -Error results of patch test with flat model, where
 339 the RKGSI scheme with variational consistent essential boundary enforcement,
 340 i.e. RKGSI-Nitsche and RKGSI-HW, can pass the linear and quadratic patch
 341 test. Due to the loss of variational consistency condition, even with Nitsche's
 342 method, Gauss meshfree formulations show noticeable errors. Table 2 shows
 343 the results for curved model, which indicated that all the considered methods
 344 cannot pass the patch test. This is mainly because the proposed smoothed
 345 gradient of Eqs. (35) and (36) could not exactly reproduce the non-polynomial
 346 membrane and bending stress. However, the RKGSI-HW and RKGSI-Nitsche
 347 methods also provide better accuracy compared to others due to the fulfillment

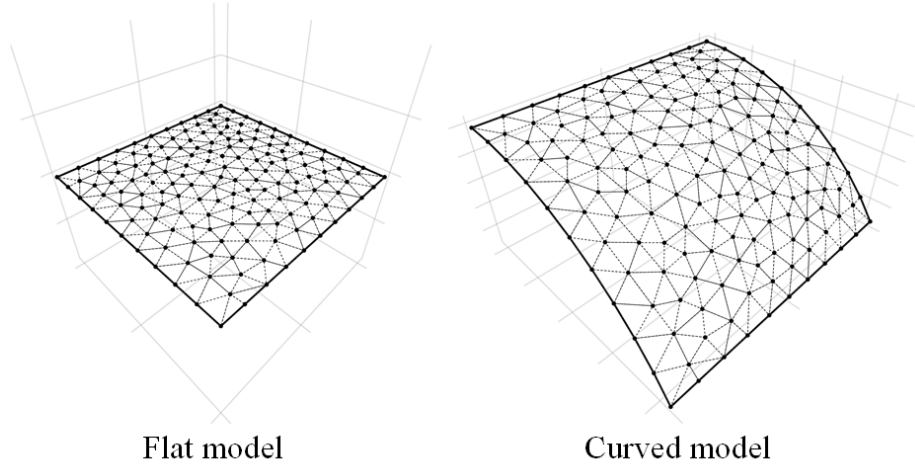


Figure 3: Meshfree discretization for patch test

348 of first second-order variational consistency. Meanwhile, the bending moment
349 contours of M^{12} are listed in Fig. 4, which further verify that the proposed
350 method provided a satisfactory result compared to exact solution. On the other
351 hand, the conventional Gauss meshree formulations showed errors.

Table 1: Results of patch test for flat model.

	Linear patch test		Quadratic patch test	
	L_2 -Error	H_e -Error	L_2 -Error	H_e -Error
GI-Penalty	$4.45E-4$	$1.35E-2$	$2.01E-3$	$1.63E-2$
GI-Nitsche	$4.51E-4$	$1.42E-2$	$1.22E-3$	$1.68E-2$
RKGSi-Penalty	$3.64E-9$	$6.77E-8$	$4.54E-9$	$6.57E-8$
RKGSi-Nitsche	$3.31E-12$	$1.34E-11$	$5.98E-12$	$1.21E-11$
RKGSi-HR	$6.67E-13$	$1.50E-11$	$1.07E-12$	$1.26E-11$

Table 2: Results of patch test for cylindrical model.

	Linear patch test		Quadratic patch test	
	L_2 -Error	H_e -Error	L_2 -Error	H_e -Error
GI-Penalty	$3.79E-4$	$1.30E-2$	$1.74E-3$	$1.37E-2$
GI-Nitsche	$4.04E-4$	$1.42E-2$	$1.15E-3$	$1.49E-2$
RKGSi-Penalty	$1.47E-4$	$5.39E-3$	$2.26E-4$	$2.09E-3$
RKGSi-Nitsche	$2.41E-6$	$7.37E-5$	$2.47E-6$	$2.89E-5$
RKGSi-HR	$4.28E-6$	$1.30E-4$	$9.69E-6$	$2.41E-4$

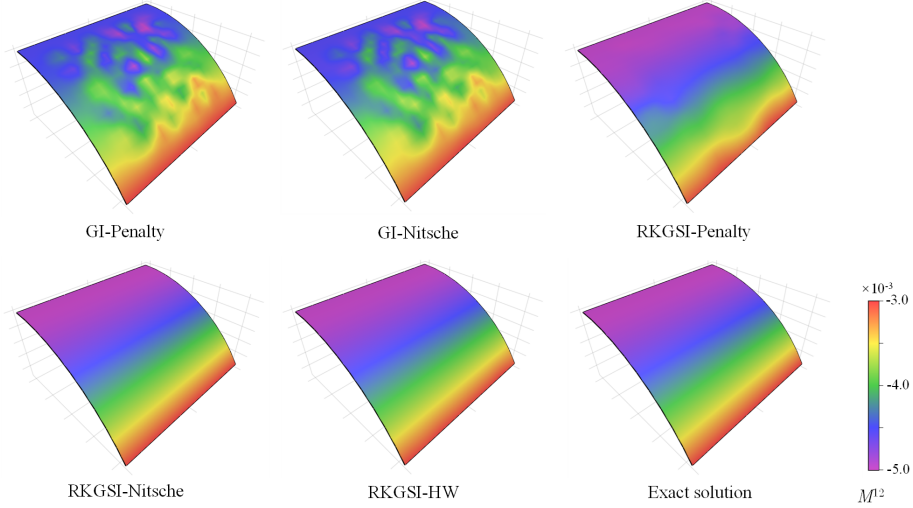


Figure 4: Contour plots of M^{12} for curved shell patch test.

5.2. Scordelis-Lo roof

This example considers the classical Scordelis-Lo roof problem, as depicted in Fig. 5. The cylindrical roof has dimensions $R = 25$, $L = 50$, $h = 0.25$, Young's modulus $E = 4.32 \times 10^8$ and Poisson's ratio $\nu = 0.0$. The entire roof is subjected to a uniform body force of $b_z = -90$, with the straight edges remaining free and the curved edges are enforced by $v_x = v_z = 0$.

Due to the symmetry, only a quadrant of the model is considered for meshfree analysis, which is discretized by the 11×16 , 13×20 , 17×24 and 19×28 meshfree nodes, as listed in Fig. 6. The comparison of the displacement in z -direction at node A , v_{A3} , is used as the investigated quantity, with the reference value 0.3024 given by [32]. Firstly, Fig. 7 presents a sensitivity study for the artificial parameters of α_v 's, α_θ 's in the RKGSi meshfree formulations with Nitsche's method and penalty method. The results of Fig. 7 revealed, Nitsche's method observed less artificial sensitivity. However, both the methods cannot trivially determine the optimal values of the artificial parameters. The optimal artificial parameters from Fig. 7 are adopted for the convergence study in Fig. 8. The convergence result showed that the RKGSi get satisfactory results while the traditional Gauss methods demonstrated noticeable errors.

5.3. Pinched Hemispherical shell

Consider the hemispherical shell shown in Fig. 9, which is loaded at four points $P = \pm 2$ at 90° interval at its bottom. The hemispherical shell has a radius $R = 10$, thickness $h = 0.04$, Young's modulus $E = 6.825 \times 10^7$ and Poisson's ratio $\nu = 0.3$.

Due to symmetry, only quadrant model, where the 8×8 , 16×16 , 24×24 and 32×32 meshfree nodes have been discretized, was considered. The quantity

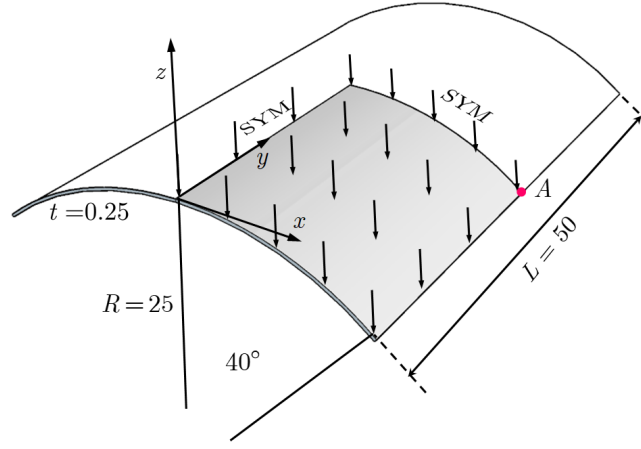


Figure 5: Description of Scordelis-Lo roof problem.

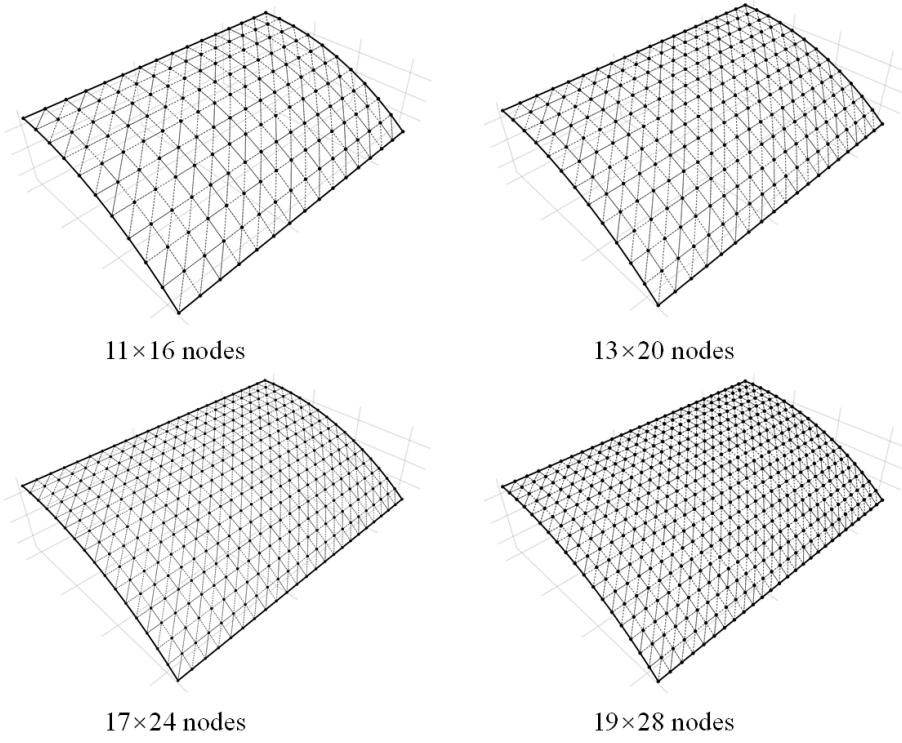


Figure 6: Meshfree discretizations for Scordelis-Lo roof problem.

under investigation for convergence is the displacement at x -direction on point

378 A, v_{A1} . Fig. 10 displays the corresponding convergence results, indicating the
 379 RKGSi scheme performed significantly better compared to the GI meshfree for-
 380 mulation. Meanwhile, the efficiency comparison for this problem is also shown
 381 in Fig. 11, in which the CPU time for assembly and calculation of shape func-
 382 tions are considered. Fig. 11(a) indicates that the RKGSi scheme observed
 383 high efficiency in assembly. This is due to the variational inconsistent Gauss
 384 meshfree formulation which require more Gaussian points to get satisfactory
 385 results. Fig. 11(b) lists the CPU time spent on enforcing essential boundary
 386 conditions for the penalty method, Nitsche's method and proposed HW method.
 387 The results highlighted that the proposed HW method consumed comparable
 388 CPU time in assembly compared to Nitsche's method. However, less time was
 389 spent to calculate the shape functions. Since both the HW method and penalty
 390 method were developed considering the shape functions first order derivatives.
 391 For this reason, both the methods shared an almost identical time in computing
 392 the shape functions.

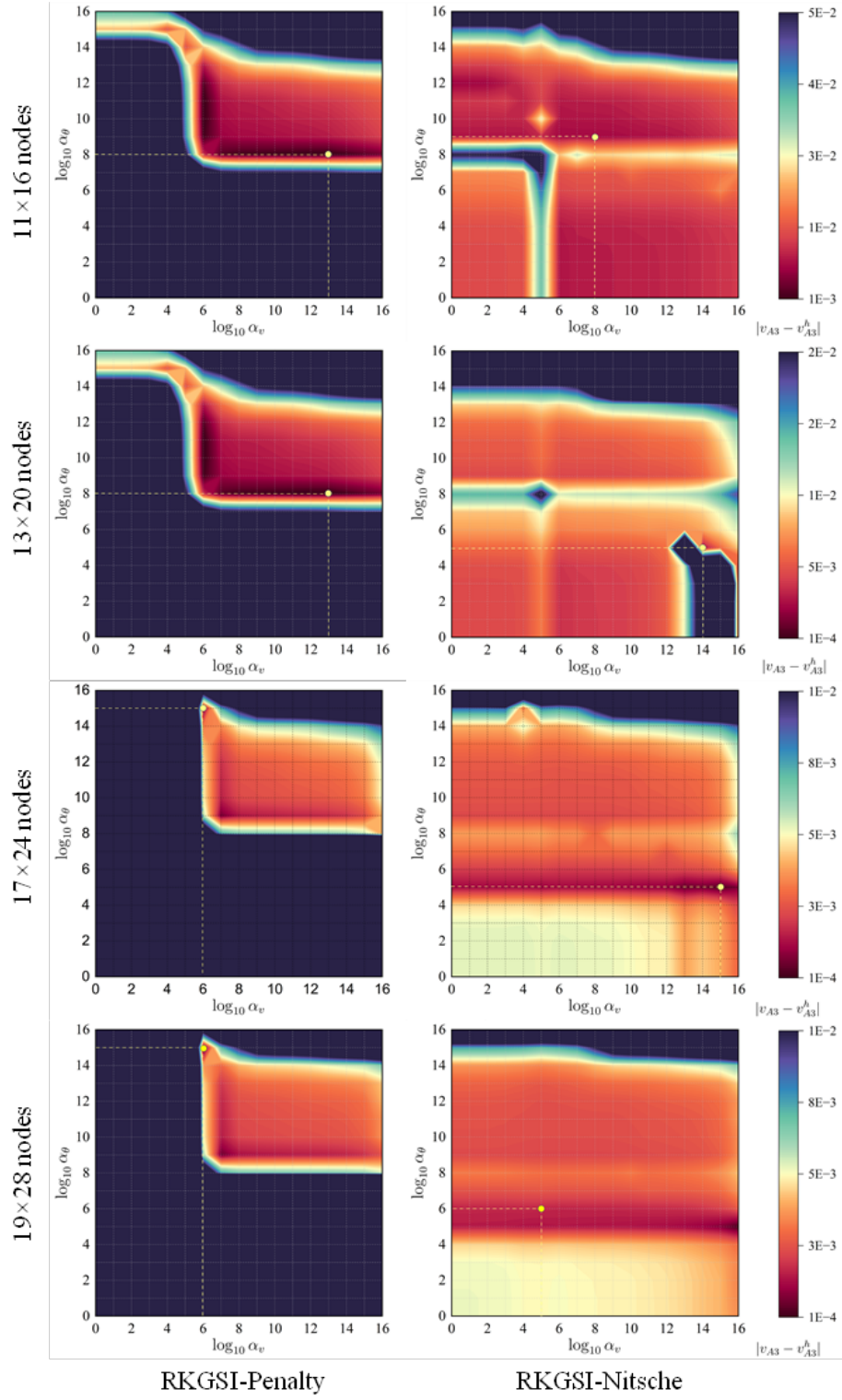


Figure 7: Sensitivity comparison of α_v and α_θ for Scordelis-Lo problem.

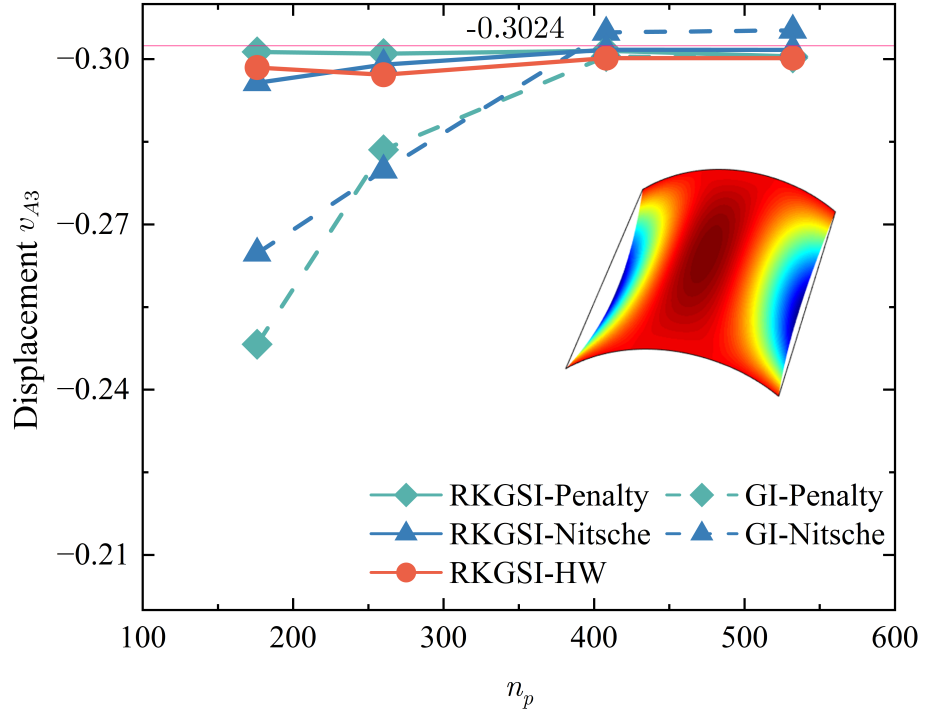


Figure 8: Displacement convergence for Scordelis-Lo roof problem.

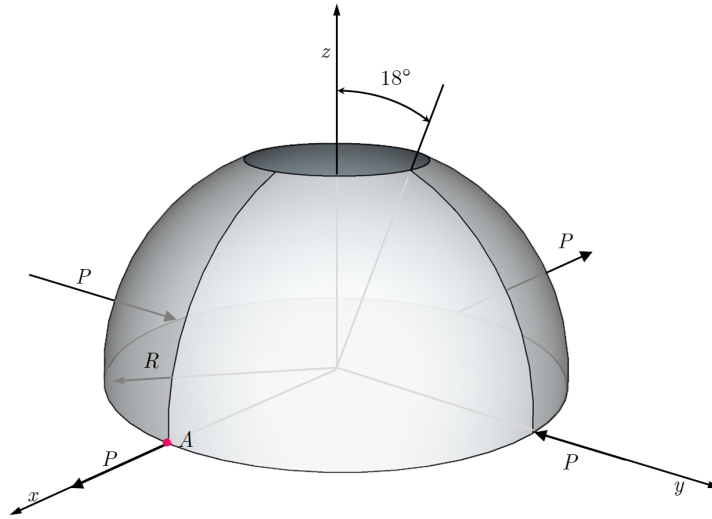


Figure 9: Description of pinched hemispherical shell problem.

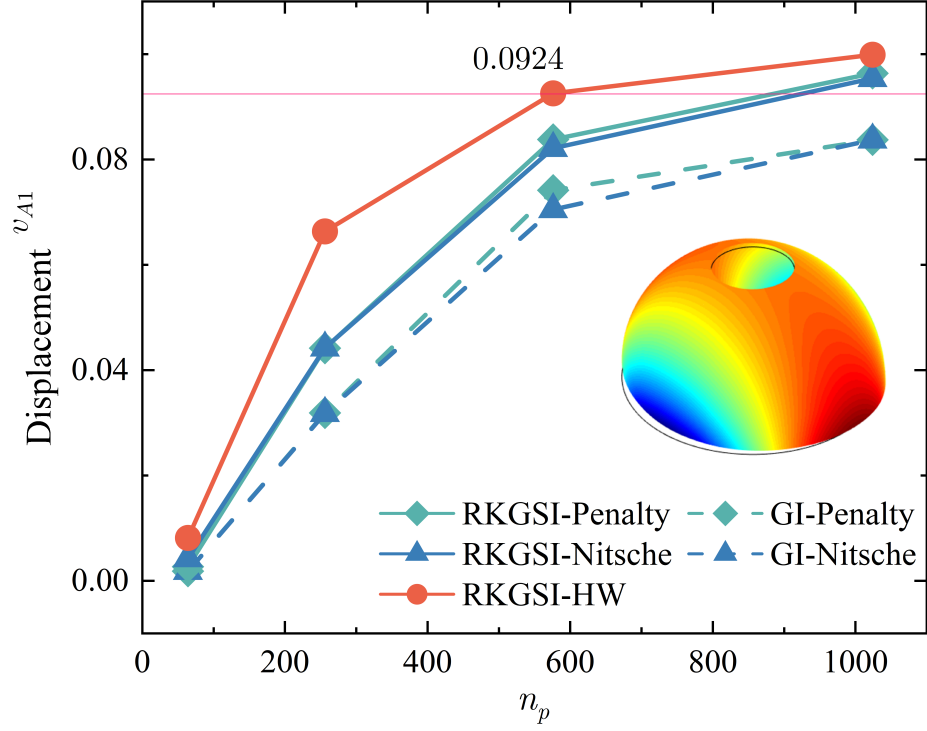


Figure 10: Displacement convergence for pinched hemispherical shell problem.

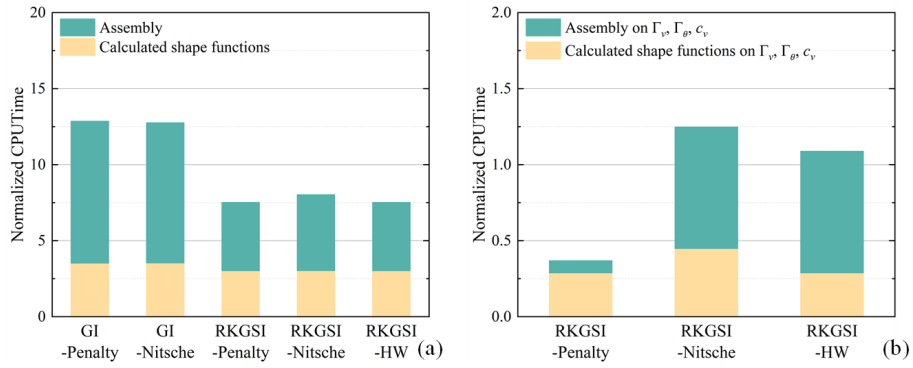


Figure 11: efficiency comparison for pinched hemispherical shell problem: (a) Whole domain; (b) Essential boundaries

6. Conclusion

In this study, an efficient and quasi-consistent meshfree thin shell formulation was presented to naturally enforce the essential boundary conditions. Mixed formulation with the Hu-Washizu principle weak form is adopted, where the traditional meshfree shape functions discretized the displacement, and the strains and stresses were expressed by the reproducing kernel smoothed gradients and the covariant smoothed gradients, respectively. The smoothed gradient naturally embedded the first second-order integration constraints and has a quasi variational consistency for the curved models in each integration cell. Owing to the Hu-Washizu variational principle, the essential boundary condition enforcement has a similar form with the conventional Nitsche's method; both have consistent and stabilized terms. The costly high order derivatives in the Nitsche's consistent term have been replaced by the smoothed gradients, which improved the computational speed due to the reproducing kernel gradient smoothing framework. Furthermore, the stabilized term naturally existed in the Hu-Washizu weak form, and the artificial parameter needed in Nitsche's stabilized term has vanished, which can automatically maintain the coercivity for the stiffness matrix. The numerical results demonstrated that the proposed Hu-Washizu quasi-consistent meshfree thin shell formulation showed excellent accuracy, efficiency, and stability.

Acknowledgment

The support of this work by the National Natural Science Foundation of China (12102138, 52350410467) and the Natural Science Foundation of Fujian Province of China (2023J01108, 2022J05056) is gratefully acknowledged.

417 Appendix A. Green's theorems for in-plane vector

418 This Appendix discusses two kinds of Green's theorems used for the devel-
 419 opment of the proposed meshfree method. For an arbitrary vectors v^α and a
 420 scalar function f , with Green's theorem for in-plane vector, the first Green's
 421 theorem is listed as follows [30]:

$$\begin{aligned} \int_{\Omega} f_{,\alpha} v^\alpha d\Omega &= \int_{\Gamma} f v^\alpha n_\alpha d\Gamma - \int_{\Omega} f (v_{,\alpha}^\alpha + \Gamma_{\beta\alpha}^\beta v^\alpha) d\Omega \\ &= \int_{\Gamma} f v^\alpha n_\alpha d\Gamma - \int_{\Omega} f v^\alpha|_\alpha d\Omega \end{aligned} \quad (\text{A.1})$$

422 where $\Gamma_{\alpha\beta}^\gamma = \mathbf{a}_{\alpha,\beta} \cdot \mathbf{a}^\gamma$ denotes the Christoffel symbol of the second kind. $v^\alpha|_\alpha$
 423 can be represented as the in-plane covariant derivative of the vector v^α :

$$v^\alpha|_\alpha = v_{,\alpha}^\alpha + \Gamma_{\beta\alpha}^\beta v^\alpha \quad (\text{A.2})$$

424 The second Green's theorem is established with a mixed form of second
 425 order derivative. Let $A^{\alpha\beta}$ can be an arbitrary symmetric second order tensor,
 426 the Green's theorem yields [30]:

$$\begin{aligned} \int_{\Omega} f_{,\alpha}|_\beta A^{\alpha\beta} d\Omega &= \int_{\Gamma} f_{,\gamma} n^\gamma A^{\alpha\beta} n_\alpha n_\beta d\Gamma - \int_{\Gamma} f (A^{\alpha\beta} s_\alpha n_\beta)_{,\gamma} s^\gamma d\Gamma + [[f A^{\alpha\beta} s_\alpha n_\beta]]_{\mathbf{x} \in C} \\ &\quad - \int_{\Gamma} f (A_{,\beta}^{\alpha\beta} n_\alpha + \Gamma_{\alpha\beta}^\gamma A^{\alpha\beta} n_\gamma + \Gamma_{\gamma\beta}^\gamma A^{\alpha\beta} n_\alpha) d\Gamma \\ &\quad + \int_{\Omega} f \left(\Gamma_{\alpha\beta,\gamma}^\gamma A^{\alpha\beta} + \Gamma_{\alpha\beta}^\gamma A_{,\gamma}^{\alpha\beta} + \Gamma_{\eta\gamma}^\eta \Gamma_{\alpha\beta}^\gamma A^{\alpha\beta} \right. \\ &\quad \left. + A_{,\alpha\beta}^{\alpha\beta} + \Gamma_{\gamma\beta,\alpha}^\gamma A^{\alpha\beta} + 2\Gamma_{\gamma\alpha}^\gamma A_{,\beta}^{\alpha\beta} + \Gamma_{\gamma\alpha}^\gamma \Gamma_{\eta\beta}^\eta A^{\alpha\beta} \right) d\Omega \\ &= \int_{\Gamma} f_{,\gamma} n^\gamma A^{\alpha\beta} n_\alpha n_\beta d\Gamma - \int_{\Gamma} f (A^{\alpha\beta} s_\alpha n_\beta)_{,\gamma} s^\gamma d\Gamma + [[f A^{\alpha\beta} s_\alpha n_\beta]]_{\mathbf{x} \in C} \\ &\quad - \int_{\Gamma} f A^{\alpha\beta}|_\beta n_\alpha d\Gamma + \int_{\Omega} f A^{\alpha\beta}|_{\alpha\beta} d\Omega \end{aligned} \quad (\text{A.3})$$

427 with

$$A^{\alpha\beta}|_\beta = A_{,\beta}^{\alpha\beta} + \Gamma_{\beta\gamma}^\alpha A^{\beta\gamma} + \Gamma_{\gamma\beta}^\gamma A^{\alpha\beta} \quad (\text{A.4})$$

428

$$\begin{aligned} A^{\alpha\beta}|_{\alpha\beta} &= \Gamma_{\alpha\beta,\gamma}^\gamma A^{\alpha\beta} + \Gamma_{\alpha\beta}^\gamma A_{,\gamma}^{\alpha\beta} + \Gamma_{\eta\gamma}^\eta \Gamma_{\alpha\beta}^\gamma A^{\alpha\beta} \\ &\quad + A_{,\alpha\beta}^{\alpha\beta} + \Gamma_{\gamma\beta,\alpha}^\gamma A^{\alpha\beta} + 2\Gamma_{\gamma\alpha}^\gamma A_{,\beta}^{\alpha\beta} + \Gamma_{\gamma\alpha}^\gamma \Gamma_{\eta\beta}^\eta A^{\alpha\beta} \end{aligned} \quad (\text{A.5})$$

429 For the sake of brevity, the notion of covariant derivative is extended to a
 430 scalar function as:

$$f|_\alpha = f_{,\alpha} + \Gamma_{\beta\alpha}^\beta f \quad (\text{A.6})$$

431

$$f|_\beta n_\alpha = f_{,\beta} n_\alpha + \Gamma_{\alpha\beta}^\gamma f n_\gamma + \Gamma_{\gamma\beta}^\gamma f n_\alpha \quad (\text{A.7})$$

432

$$\begin{aligned} f|_{\alpha\beta} &= \Gamma_{\alpha\beta,\gamma}^\gamma f + \Gamma_{\alpha\beta}^\gamma f_{,\gamma} + \Gamma_{\eta\gamma}^\eta \Gamma_{\alpha\beta}^\gamma f \\ &\quad + f_{,\alpha\beta} + \Gamma_{\gamma\beta,\alpha}^\gamma f + 2\Gamma_{\gamma\alpha}^\gamma f_{,\beta} + \Gamma_{\gamma\alpha}^\gamma \Gamma_{\eta\beta}^\eta f \end{aligned} \quad (\text{A.8})$$

433 Appendix B. Derivations for stiffness metrics and force vectors

434 This Appendix details the derivations of stiffness matrices and force vectors
 435 in Eqs. (53)-(55), where the relationships of Eqs. (40), (41), (44) and (46) are
 436 used herein. Firstly, the membrane strain terms are considered as follows:

$$\begin{aligned}
 & \sum_{C=1}^{n_e} \int_{\Omega_C} \delta \tilde{\varepsilon}_{\alpha\beta}^h h C^{\alpha\beta\gamma\eta} \tilde{\varepsilon}_{\gamma\eta}^h d\Omega \\
 &= \sum_{C=1}^{n_e} \sum_{I,J=1}^{n_p} \delta \mathbf{d}_I \cdot \underbrace{\int_{\Omega_C} \tilde{\varepsilon}_{\alpha\beta I} h C^{\alpha\beta\gamma\eta} \mathbf{a}_\gamma \mathbf{q}^T d\Omega \mathbf{G}^{-1} \bar{\mathbf{g}}_{\eta J}}_{\tilde{\mathbf{g}}_I^{\eta T}} \cdot \mathbf{d}_J \\
 &= \sum_{C=1}^{n_e} \sum_{I,J=1}^{n_p} \delta \mathbf{d}_I \cdot \int_{\Gamma_C \cap \Gamma_v} \Psi_J \mathbf{q}^T \underbrace{\mathbf{G}^{-1} \tilde{\mathbf{g}}_I^\alpha n_\alpha}_{\tilde{\mathbf{T}}_{NI}} d\Gamma \cdot \mathbf{d}_J \\
 &= \sum_{I,J=1}^{n_p} \delta \mathbf{d}_I \cdot \int_{\Gamma_v} \tilde{\mathbf{T}}_{NI} \Psi_J d\Gamma \cdot \mathbf{d}_J
 \end{aligned} \tag{B.1}$$

437 with

$$438 \quad \tilde{\mathbf{g}}_I^\alpha = \mathbf{q} \mathbf{a}_\beta h C^{\alpha\beta\gamma\eta} \tilde{\varepsilon}_{\alpha\beta I} \tag{B.2}$$

$$439 \quad \tilde{\mathbf{T}}_{NI} = \mathbf{q}^T \mathbf{G}^{-1} \tilde{\mathbf{g}}_I^\alpha n_\alpha \tag{B.3}$$

Following this path, the bending strain terms can be reorganized by:

$$\begin{aligned}
 & \sum_{C=1}^{n_e} \int_{\Omega_C} \delta \tilde{\kappa}_{\alpha\beta}^h \frac{h^3}{12} C^{\alpha\beta\gamma\eta} \tilde{\kappa}_{\gamma\eta}^h d\Omega \\
 &= \sum_{C=1}^{n_e} \sum_{I,J=1}^{n_p} \delta \mathbf{d}_I \cdot \underbrace{\int_{\Omega_C} \tilde{\kappa}_{\alpha\beta I} \frac{h^3}{12} C^{\alpha\beta\gamma\eta} \mathbf{a}_3 \mathbf{q}^T d\Omega \mathbf{G}^{-1} \bar{\mathbf{g}}_{\gamma\eta J}}_{\tilde{\mathbf{g}}_I^{\gamma\eta T}} \cdot \mathbf{d}_J \\
 &= \sum_{C=1}^{n_e} \sum_{I,J=1}^{n_p} \delta \mathbf{d}_I \cdot \left(\begin{aligned} & \int_{\Gamma_C \cap \Gamma_\theta} \underbrace{\mathbf{q}^T \mathbf{G}^{-1} \tilde{\mathbf{g}}_I^{\alpha\beta} n_\alpha n_\beta}_{\tilde{\mathbf{M}}_{nnI}} n^\gamma \Psi_{J,\gamma} d\Gamma \\ & - \int_{\Gamma_C \cap \Gamma_v} \underbrace{(\mathbf{q}_{|\beta}^T \mathbf{G}^{-1} \tilde{\mathbf{g}}_I^{\alpha\beta} n_\alpha + (\mathbf{q}^T \mathbf{G}^{-1} \tilde{\mathbf{g}}_I^{\alpha\beta} s_\alpha n_\beta)_{,\gamma} s^\gamma)}_{\tilde{\mathbf{T}}_{MI}} \Psi_J d\Gamma \\ & + [[\underbrace{\mathbf{q}^T \mathbf{G}^{-1} \tilde{\mathbf{g}}_I^{\alpha\beta} s_\alpha n_\beta}_{\tilde{\mathbf{P}}_I \mathbf{a}_3} \Psi_J]]_{\mathbf{x} \in C_C \cap C_v} \end{aligned} \right) \cdot \mathbf{d}_J \\
 &= \sum_{I,J=1}^{n_p} \delta \mathbf{d}_I \cdot \left(\int_{\Gamma_\theta} \tilde{\mathbf{M}}_{nnI} n^\gamma \Psi_{J,\gamma} d\Gamma - \int_{\Gamma_v} \tilde{\mathbf{T}}_{MI} \Psi_J d\Gamma + [[\tilde{\mathbf{P}}_I \Psi_J]]_{\mathbf{x} \in C_v} \right)
 \end{aligned} \tag{B.4}$$

440 with

$$\tilde{\mathbf{g}}_I^{\alpha\beta} = \int_{\Omega_C} \mathbf{q} \frac{h^3}{12} C^{\alpha\beta\gamma\eta} \mathbf{a}_3 \tilde{\kappa}_{\alpha\beta I} d\Omega \quad (\text{B.5})$$

441

$$\begin{cases} \tilde{M}_{nnI} = \mathbf{q}^T \mathbf{G}^{-1} \tilde{\mathbf{g}}_I^{\alpha\beta} n_\alpha n_\beta \\ \tilde{\mathbf{T}}_{MI} = \mathbf{q}_{|\beta}^T \mathbf{G}^{-1} \tilde{\mathbf{g}}_I^{\alpha\beta} n_\alpha + (\mathbf{q}^T \mathbf{G}^{-1} \tilde{\mathbf{g}}_I^{\alpha\beta} s_\alpha n_\beta)_{,\gamma} s^\gamma \\ \tilde{\mathbf{P}}_I = \mathbf{q}^T \mathbf{G}^{-1} \tilde{\mathbf{g}}_I^{\alpha\beta} s_\alpha n_\beta \cdot \mathbf{a}_3 \end{cases} \quad (\text{B.6})$$

442 References

- 443 [1] L. H. Donnell, Beams, Plates and Shells, McGraw-Hill. `arXiv:0_`
444 `IeAQAAIAAJ`.
- 445 [2] T. J. Hughes, The Finite Element Method: Linear Static and Dynamic
446 Finite Element Analysis, Dover Publications.
- 447 [3] T. Belytschko, Y. Y. Lu, L. Gu, Element-free Galerkin methods 37 (2)
448 229–256. `arXiv:10208278`.
- 449 [4] W. K. Liu, S. Jun, Y. F. Zhang, Reproducing kernel particle methods
450 20 (8-9) 1081–1106.
- 451 [5] J. S. Chen, M. Hillman, S. W. Chi, Meshfree methods: Progress made after
452 20 years 143 (4) 04017001.
- 453 [6] P. Krysl, T. Belytschko, Analysis of thin shells by the Element-Free
454 Galerkin method 33 (20) 3057–3080.
- 455 [7] G. R. Liu, Meshfree Methods: Moving Beyond the Finite Element Method,
456 Second Edition, Crc Press.
- 457 [8] X. Zhang, K. Z. Song, M. W. Lu, X. Liu, Meshless methods based on
458 collocation with radial basis functions 26 333–343.
- 459 [9] D. Millán, A. Rosolen, M. Arroyo, Thin shell analysis from scattered points
460 with maximum-entropy approximants 85 (6) 723–751.
- 461 [10] L. Wang, M. Hu, Z. Zhong, F. Yang, Stabilized Lagrange Interpolation
462 Collocation Method: A meshfree method incorporating the advantages of
463 finite element method 404 115780.
- 464 [11] P. Suchde, T. Jacquemin, O. Davydov, Point Cloud Generation for Mesh-
465 free Methods: An Overview 30 (2) 889–915.
- 466 [12] L. Deng, D. Wang, An accuracy analysis framework for meshfree collocation
467 methods with particular emphasis on boundary effects 404 115782.
- 468 [13] S. Fernández-Méndez, A. Huerta, Imposing essential boundary conditions
469 in mesh-free methods 193 (12-14) 1257–1275.
- 470 [14] X. Li, Error estimates for the moving least-square approximation and the
471 element-free Galerkin method in n-dimensional spaces 99 77–97.
- 472 [15] J. Wu, D. Wang, An accuracy analysis of Galerkin meshfree methods ac-
473 counting for numerical integration 375 113631.
- 474 [16] J.-S. Chen, H.-P. Wang, New boundary condition treatments in meshfree
475 computation of contact problems 187 (3) 441–468.

- 476 [17] D. Liu, Y. M. Cheng, The interpolating element-free Galerkin (IEFG)
477 method for three-dimensional potential problems 108 115–123.
- 478 [18] V. Ivannikov, C. Tiago, P. M. Pimenta, On the boundary conditions of the
479 geometrically nonlinear Kirchhoff–Love shell theory 51 (18) 3101–3112.
- 480 [19] Y. Y. Lu, T. Belytschko, L. Gu, A new implementation of the element free
481 Galerkin method 113 (3-4) 397–414. [arXiv:26071039](#).
- 482 [20] T. Zhu, S. N. Atluri, A modified collocation method and a penalty formu-
483 lation for enforcing the essential boundary conditions in the element free
484 Galerkin method 21 (3) 211–222.
- 485 [21] S. Skatulla, C. Sansour, Essential boundary conditions in meshfree methods
486 via a modified variational principle: Applications to shell computations
487 15 (2) 123–142.
- 488 [22] J. S. Chen, C. T. Wu, S. Yoon, Y. You, A stabilized conforming nodal
489 integration for Galerkin mesh-free methods 50 (2) 435–466.
- 490 [23] J. S. Chen, M. Hillman, M. Rüter, An arbitrary order variationally con-
491 sistent integration for Galerkin meshfree methods 95 (5) 387–418. [arXiv:](#)
492 [260949200001](#).
- 493 [24] Q. Duan, X. Li, H. Zhang, T. Belytschko, Second-order accurate derivatives
494 and integration schemes for meshfree methods 92 (4) 399–424. [arXiv:](#)
495 [260949200001](#).
- 496 [25] D. Wang, J. Wu, An inherently consistent reproducing kernel gradient
497 smoothing framework toward efficient Galerkin meshfree formulation with
498 explicit quadrature 349 628–672.
- 499 [26] J. Wang, X. Ren, A consistent projection integration for Galerkin meshfree
500 methods 414 116143.
- 501 [27] J. Wu, X. Wu, Y. Zhao, D. Wang, A consistent and efficient method for
502 imposing meshfree essential boundary conditions via hellinger-reissner vari-
503 ational principle. 54 (12) 3283–3296.
- 504 [28] J. Wu, X. Wu, Y. Zhao, D. Wang, A rotation-free Hellinger-Reissner mesh-
505 free thin plate formulation naturally accommodating essential boundary
506 conditions 154 122–140.
- 507 [29] H. Dah-wei, A method for establishing generalized variational principle
508 6 (6) 501–509.
- 509 [30] J. Benzaken, J. A. Evans, S. F. McCormick, R. Tamstorf, Nitsche’s method
510 for linear Kirchhoff–Love shells: Formulation, error analysis, and verifica-
511 tion 374 113544.

- 512 [31] H. Du, J. Wu, D. Wang, J. Chen, A unified reproducing kernel gradient
513 smoothing Galerkin meshfree approach to strain gradient elasticity 70 (1)
514 73–100.
- 515 [32] R. H. Macneal, R. L. Harder, A proposed standard set of problems to test
516 finite element accuracy 1 (1) 3–20.