

Girder Properties & Proportion Checks

$$E \equiv 29000 \text{ ksi}$$

$$F_y \equiv 50 \text{ ksi}$$

$$t_w \equiv 0.5 \text{ in}$$

$$t_{topflange} \equiv 1.50 \text{ in}$$

$$t_{bottomflange} \equiv 1.75 \text{ in}$$

$$b_f \equiv 16 \text{ in}$$

$$h \equiv 55 \text{ in}$$

$$I_x \equiv 64985.015 \text{ in}^4$$

$$c_{xc} \equiv 13.772 \text{ in}$$

$$c_{xt} \equiv 17.522 \text{ in}$$

$$S_{xc} \equiv \frac{I_x}{c_{xc}} = 4718.633 \text{ in}^3$$

$$S_{xt} \equiv \frac{I_x}{c_{xt}} = 3708.767 \text{ in}^3$$

$$\frac{S_{xt}}{S_{xc}} = 0.786$$

$$I_{yc} \equiv 21196.632 \text{ in}^4$$

$$I_{yt} \equiv 20282.677 \text{ in}^4$$

$$3.76 \cdot \sqrt{\left(\frac{E}{F_y}\right)} = 90.553$$

$$5.70 \cdot \sqrt{\left(\frac{E}{F_y}\right)} = 137.274$$

AASHTO Section 6.10 Checks

$$\frac{b_f}{2 \cdot t_{topflange}} = 5.333 \quad 5.333 \leq 12 \quad \text{OK!}$$

$$\frac{b_f}{2 \cdot t_{bottomflange}} = 4.571 \quad 4.571 \leq 12 \quad \text{OK!}$$

$$1.1 \cdot t_w = 0.55 \text{ in} \quad 1.50 \geq 0.55$$

$$1.75 \geq 0.55 \quad \text{OK!}$$

$$\frac{h}{6} = 9.167 \text{ in} \quad 16 \geq 9.167 \quad \text{OK!}$$

$$\frac{I_{yc}}{I_{yt}} = 1.045 \quad 0.1 \leq 1.045 \leq 10 \quad \text{OK!}$$

$$\frac{h}{t_w} = 110 \quad 110 \leq 150 \quad \text{OK!}$$

Moment Capacity Calculations

$$M_U \equiv (1.25 \cdot (2525.39 \text{ ft} \cdot \text{kip} + 488.28 \text{ ft} \cdot \text{kip})) + (1.50 \cdot 416.02 \text{ ft} \cdot \text{kip}) + (1.75 \cdot 3328.29 \text{ ft} \cdot \text{kip}) = 10215.625 \text{ ft} \cdot \text{kip}$$

-Plastic Moment Calculations (AASHTO Table D6.1-1) $f'_c \equiv 4 \text{ ksi} \quad t_s \equiv 8 \text{ in} \quad b_s \equiv 88 \text{ in}$

$$P_s \equiv 0.85 \cdot f'_c \cdot b_s \cdot t_s = 2393.6 \text{ kip}$$

$$P_c \equiv F_y \cdot b_f \cdot t_{topflange} = 1200 \text{ kip}$$

$$P_w \equiv F_y \cdot h \cdot t_w = 1375 \text{ kip}$$

$$P_t \equiv F_y \cdot b_f \cdot t_{bottomflange} = 1400 \text{ kip}$$

$$P_c + P_w + P_t = 3975 \text{ kip} \quad P_s < (P_c + P_w + P_t)$$

AASHTO Case I: $(P_t + P_w) \geq (P_c + P_s)$

$$P_t + P_w = 2775 \text{ kip} \quad P_c + P_s = 3593.6 \text{ kip} \quad 2775 \text{ kip} < 3593.6 \text{ kip} \quad \text{INVALID}$$

AASHTO Case II: $(P_t + P_w + P_c) \geq P_s$

$$P_t + P_w + P_c = 3975 \text{ kip} \quad P_s = 2393.6 \text{ kip} \quad 3975 \text{ kip} > 2393.6 \text{ kip} \quad \text{VALID}$$

Therefore, the Plastic Neutral Axis (PNA) is located in the top flange of the beam.

$$Y_{bar} \equiv \left(\frac{t_{topflange}}{2} \right) \left(\frac{P_w + P_t - P_s}{P_c} + 1 \right) = 0.988 \text{ in}$$

$$haunch \equiv 2 \text{ in}$$

$$d_s \equiv \left(\frac{t_s}{2} + (haunch - t_{topflange}) + Y_{bar} \right) = 5.488 \text{ in}$$

$$D_t \equiv h + haunch + t_{bottomflange} + t_s = 66.75 \text{ in}$$

$$D_p \equiv t_s + haunch - t_{topflange} + Y_{bar} = 9.488 \text{ in}$$

$$d_w \equiv \frac{h}{2} + (t_{topflange} - Y_{bar}) = 28.012 \text{ in}$$

$$d_t \equiv \frac{t_{bottomflange}}{2} + h + (t_{topflange} - Y_{bar}) = 56.387 \text{ in}$$

$$M_p \equiv \left(\frac{P_c}{2 \cdot t_{topflange}} \right) \left(Y_{bar}^2 + (t_{topflange} - Y_{bar})^2 \right) + (\langle P_s \cdot d_s \rangle + \langle P_w \cdot d_w \rangle + \langle P_t \cdot d_t \rangle) = 10924.141 \text{ ft} \cdot \text{kip}$$

AASHTO 6.10.7.1.2

$$0.1 D_t = 6.675 \text{ in} \quad D_p \leq 0.1 D_t \quad 9.488 > 6.675 \quad \text{INVALID}$$

$$D_p = 9.488 \text{ in}$$

$$M_n \equiv \langle M_p \rangle \left(1.07 - 0.7 \left(\frac{D_p}{D_t} \right) \right) = 10601.84 \text{ ft} \cdot \text{kip}$$

$$\phi_M := 1.0$$

$$\phi_M M_n := \phi_M \cdot M_n = 10601.84 \text{ ft} \cdot \text{kip}$$

$$\phi M_n \geq M_U$$

$$10601.84 \text{ ft} \cdot \text{kip} > 10215.625 \text{ ft} \cdot \text{kip} \quad \text{VALID}$$

Girder Performance Ratio

$$\frac{M_U}{M_n} = 96.36\% \quad \text{GOOD}$$

Shear Capacity Calculations

$$V_U := (1.25 \cdot 80.81 \text{ kip}) + (1.50 \cdot 13.31 \text{ kip}) + (1.75 \cdot 122.35 \text{ kip}) = 335.09 \text{ kip}$$

$$1.12 \cdot \sqrt{\frac{(29000 \cdot 5)}{50}} = 60.314 \quad k := 5$$

$$\frac{85.3}{1.12} \cdot 1.4 = 106.625$$

$$C := \frac{1.57}{(110^2)} \cdot \left(\frac{29000 \cdot 5}{50} \right) = 0.376$$

$$V_p \equiv 0.58 (50 \text{ ksi} \cdot 55 \text{ in} \cdot 0.5 \text{ in}) = 797.5 \text{ kip}$$

$$V_p \cdot C = 300.084 \text{ kip}$$

$$797.5 \cdot 0.376 = 299.86$$

$$V_n := 797.5 \text{ kip} \left(C + \frac{0.87 \cdot (1 - C)}{1 + \sqrt{2}} \right) = 479.336 \text{ kip}$$

$$\phi_V := 1.0$$

$$\phi_V \cdot V_n = 479.336 \text{ kip}$$

$$\phi_V \cdot V_n \geq V_U$$

$$479.336 \text{ kip} > 335.09 \text{ kip} \quad \text{VALID}$$

Therefore, use one 1/2" thick shear stiffener located 55" away from abutments on each girder.