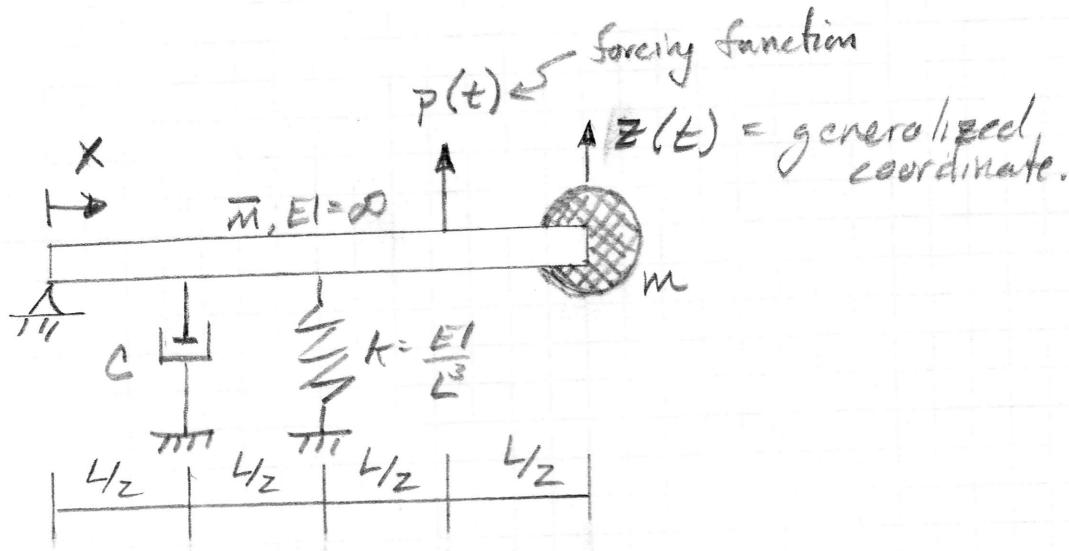


## LECTURE 4: Formulation of EOM & Free Vibration

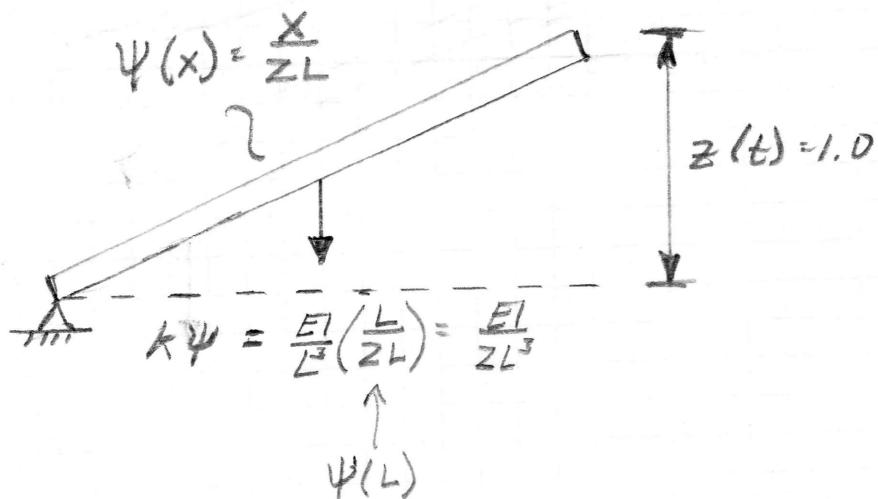
→ Formulation of the Equation of Motion (EOM)

→ Discrete stiffness & Damping

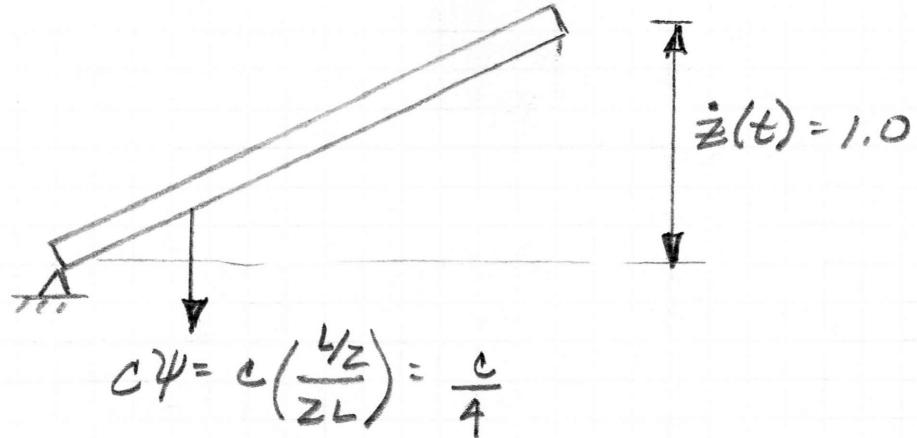


$$\Rightarrow \text{shape function } \psi(x) = \frac{x}{L}$$

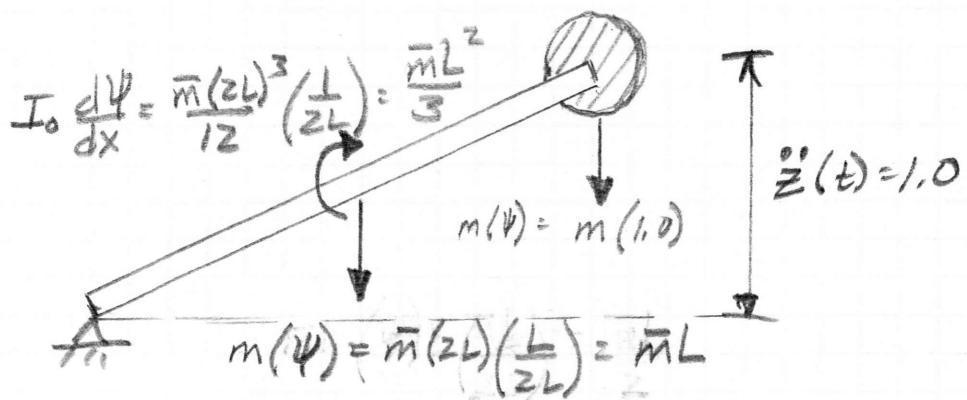
→ Impose unit displacement and solve for spring forces.



→ Impose unit velocity and solve for damping forces.

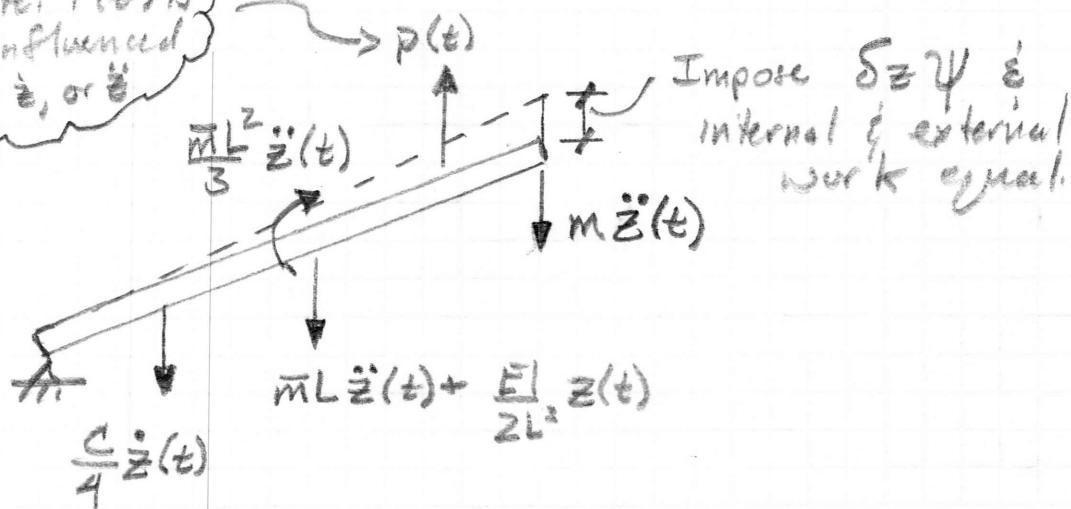


→ Impose unit accel and solve for inertia forces.



→ Impose all forces on the original structure in terms of  $z$ ,  $\dot{z}$ , or  $\ddot{z}$

Note:  $P(t)$  is  
not influenced  
by  $\ddot{z}$ , or  $\ddot{\psi}$



→ Balancing Internal & External virtual work

Force virtual  $\delta$

inertial forces

$$\left[ \underbrace{\frac{mL^2}{3} \ddot{z}(t)}_{\text{inertial forces}} \right] \left[ \delta \dot{z} \frac{1}{2L} \right] + \bar{m}L \ddot{z}(t) \left[ \delta \dot{z} \frac{L}{2L} \right] + m \ddot{z}(t) \left[ \delta \dot{z} \frac{2L}{2L} \right] +$$

damping forces

$$\left[ \frac{C}{4} \dot{z}(t) \right] \left[ \delta \dot{z} \frac{4L}{2L} \right] +$$

AMPA'D

spring forces

$$\left[ \frac{EI}{2L^3} z(t) \right] \left[ \delta \dot{z} \frac{L}{2L} \right] =$$

external forces.

$$P(t) \left[ \delta \dot{z} \frac{3L/2}{2L} \right]$$

$$\left[ \underbrace{\frac{2mL + m}{3}}_{\text{distrib. mass}} \right] \ddot{z}(t) + \left[ \underbrace{\frac{C}{16}}_{\text{discrete mass}} \right] \dot{z}(t) + \left[ \underbrace{\frac{EI}{4L^3}}_{\text{discrete mass}} \right] z(t) = \frac{3}{4} P(t)$$

⇒ Summary

⇒ For discrete springs.

$$K^* = \sum_{i=1}^n \left[ \psi(x_i) \right]^2 k_i$$

↑ location of  
spring 'i'

⇒ For discrete dampers.

$$C^* = \sum_{i=1}^n \left[ \psi(x_i) \right]^2 c_i$$

↑ location of  
damper 'i'

$\Rightarrow$  For discrete masses.

$$m^* = \sum_{i=1}^n [\psi(x_i)]^2 m_i$$

$\uparrow$  location of mass 'i'

$\Rightarrow$  For distributed masses associated w/  
rigid members.

$$m^* = \sum_{i=1}^n [\psi(x_i)]^2 \bar{m}L + \sum \left[ \frac{d\psi}{dx} \Big|_{x_i} \right]^2 I_i$$

$\uparrow$  location  
of center  
of mass

$\uparrow$  location  
of center of  
mass

mass moment  
of inertia

$\Rightarrow$  For discrete external loads.

$$P^* = \sum_{i=1}^n [\psi(x_i)] p_i(t)$$

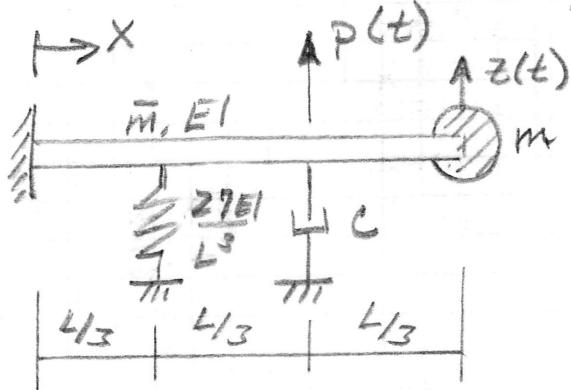
$\uparrow$  location of  
load.

$\Rightarrow$  For distributed loads.

$$P^* = \sum_{i=1}^n \int_{x_i}^{x_{i+1}} \psi(x) p_i(t) dx$$

$\uparrow$  integrate of length  
of applied load.

In class example.  $\Rightarrow$  combine w/ previous lecture...



$$\Rightarrow \text{use } \psi(x) = \frac{-x^3}{2L^3} + \frac{3x^2}{2L^2}$$

$$\psi''(x) = \frac{-3x}{L^3} + \frac{3}{L^2}$$

- 1/5a.

$$K^* = \int_0^L EI \left( \frac{-3x^2}{L^3} + \frac{3}{L^2} \right)^2 dx + \frac{27EI}{L^2} \left( \frac{-(L/3)^3}{2L^3} + \frac{3(L/3)^2}{2L^2} \right)^2$$

1/6

$$K^* = \frac{3EI}{L^3} + \frac{27EI}{L^3} \left( \frac{4}{27} \right)^2$$

$$\Rightarrow K^* = \frac{97EI}{27L^3}$$

$$C^* = C \left[ \frac{(2/3L)^3}{2L^3} + \frac{3(2/3L)^2}{2L^2} \right]^2 = C \left[ \frac{14}{27} \right]^2$$

$$\Rightarrow C^* =$$

$$m^* = \int_0^L \bar{m} \left( \frac{-x^3}{2L^3} + \frac{3x^2}{2L^2} \right)^2 dx + m(1)^2$$

$$\Rightarrow m^* = \frac{\bar{m}L}{3} + m$$

$$P^* = P(t) \left( -\frac{(2L/3)^3}{2L^3} + \frac{3(2L/3)^2}{2L^2} \right)$$

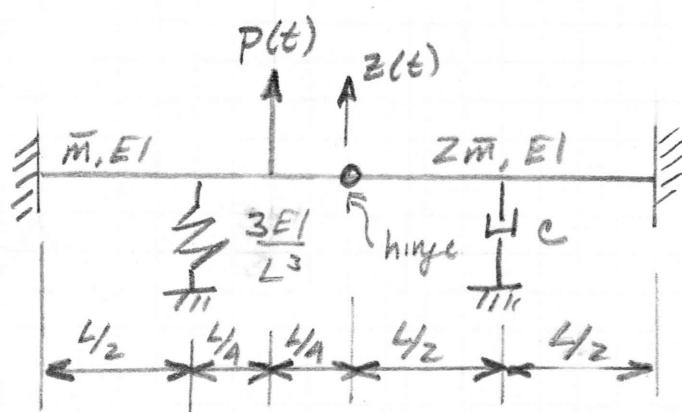
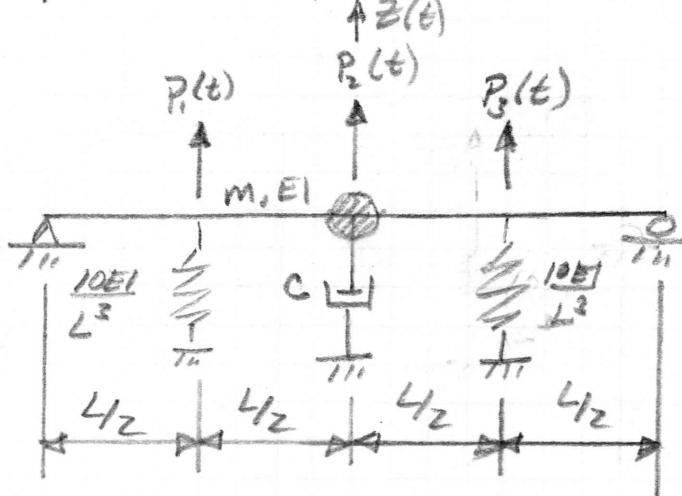
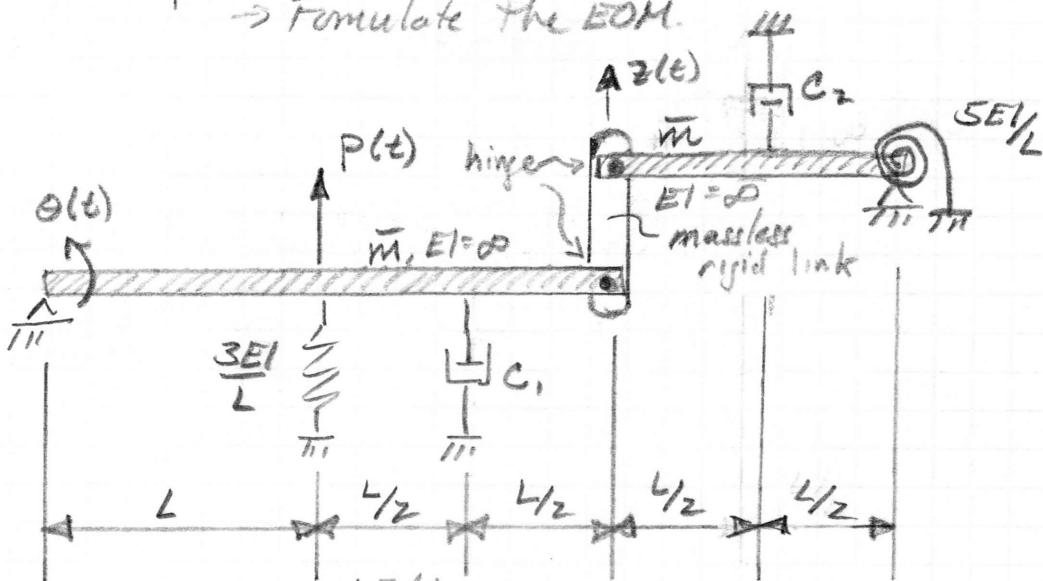
$$\Rightarrow P^* = \frac{14 P(t)}{27}$$

→ Total EOM:

$$[\bar{m}L + m]\ddot{z}(t) + [-] \dot{z}(t) + \left[ \frac{9EI}{27L^3} \right] z(t) = \left[ \frac{14}{27} \right] P(t)$$

→ Sample HW Problems

→ Formulate the EOM.



## → Free Vibration Response

- No forcing function (i.e.  $p(t) = 0$ )
- Vibration is caused by initial conditions.

→ Undamped Systems  $\Rightarrow c = 0$

$$m\ddot{u} + ku = 0 \quad (\text{EOM})$$

$$\Rightarrow \text{assume } u = A \cos(\lambda t) + B \sin(\lambda t)$$

$$\dot{u} = -A\lambda \sin(\lambda t) + B\lambda \cos(\lambda t)$$

$$\ddot{u} = -A\lambda^2 \cos(\lambda t) - B\lambda^2 \sin(\lambda t)$$

$\Rightarrow$  Sub into EOM

$$-m\lambda^2 [A \cos(\lambda t) + B \sin(\lambda t)] + k [A \cos(\lambda t) + B \sin(\lambda t)] = 0$$

$$\lambda^2 - \frac{k}{m} = 0 \leftarrow \text{characteristic equation}$$

$$\lambda^2 = \frac{k}{m}$$

$$\lambda = \sqrt{\frac{k}{m}} = \omega \quad \left. \begin{array}{l} \text{same frequency we} \\ \text{obtained using} \\ \text{energy arguments} \end{array} \right\}$$

$$\Rightarrow u = A \cos(\omega t) + B \sin(\omega t) ; \omega = \sqrt{\frac{k}{m}}$$

Initial Conditions

$$u(0) = u_0 \leftarrow \text{initial displacement}$$

$$\dot{u}(0) = v_0 \leftarrow \text{initial velocity}$$

$$u = u_0 \cos(\omega t) + \frac{v_0}{\omega} \sin(\omega t)$$

⇒ Plot using Excel

⇒ Although the response has both cosine (due to  $u_0$ ) and sine (due to  $v_0$ ) responses, it is only a single harmonic.

↳ this is because both components have the same frequency.

⇒ Alternative Representation.

$$u = g \sin(\omega t + \phi) \quad \begin{matrix} \downarrow \\ \text{amplitude of vibration} \end{matrix} \quad \begin{matrix} \text{phase angle} \\ \downarrow \end{matrix}$$

↑ amplitude of vibration.

$$g = \sqrt{u_0^2 + \left(\frac{v_0}{\omega}\right)^2}$$

$$\phi = \tan^{-1}\left(\frac{u_0 \omega}{v_0}\right)$$

⇒ Using Excel show that...

(1) Both representations are identical

(2)  $f = \omega/2\pi$  (Hz)

(3)  $T = 2\pi/\omega$  (sec)

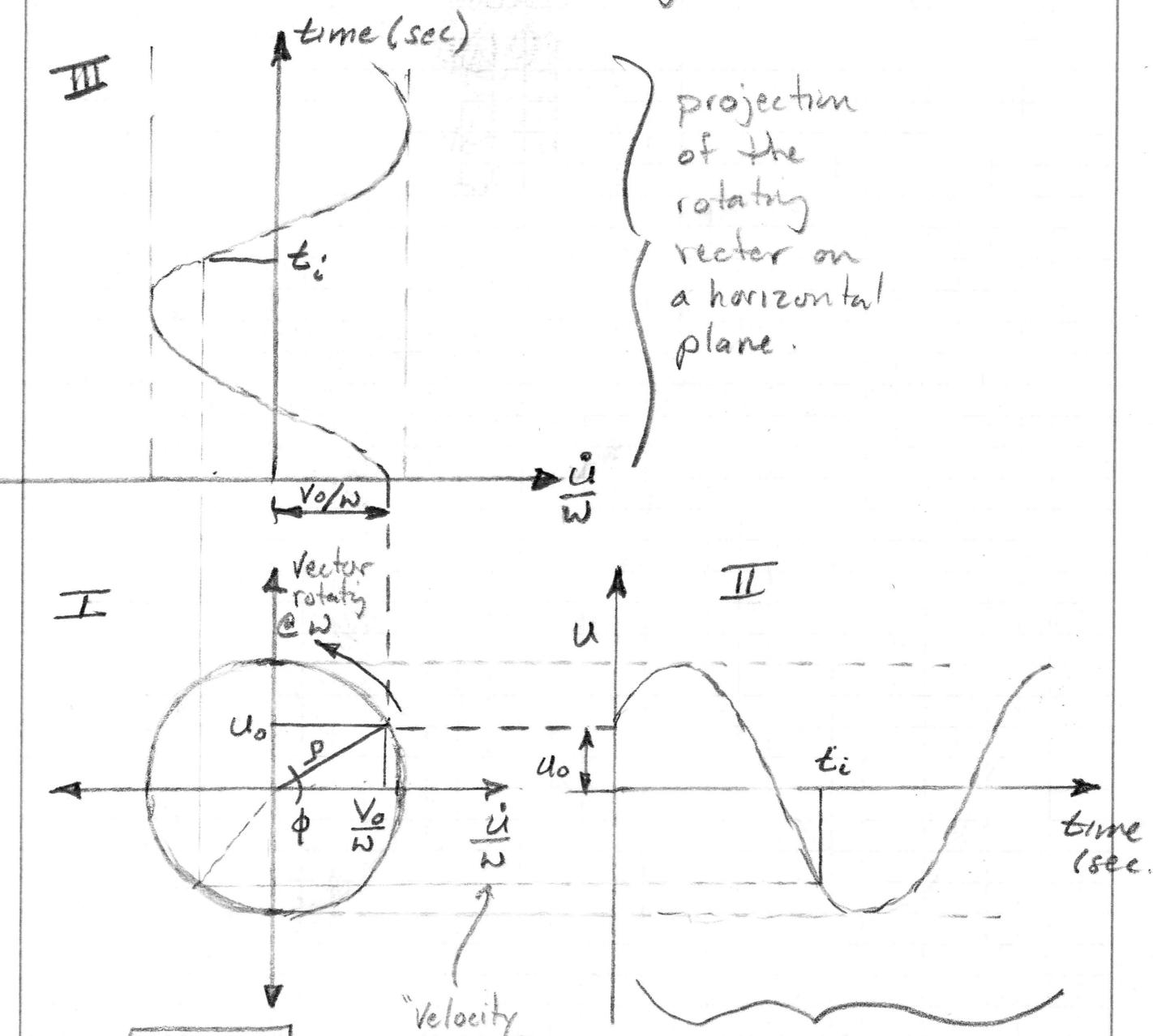
(4) Sine portion is due to initial vel.

(5) Cosine portion is due to initial disp.

(6) What happens to  $\phi$  when  $v_0 = 0$

(7) Issue w/  $\tan^{-1} \Rightarrow g = \frac{1}{\sqrt{1 + \tan^2 \phi}}$

$\Rightarrow$  Phase Plane Diagram.



$$S = \sqrt{u_0^2 + \left(\frac{v_0}{w}\right)^2}$$

$$\phi = \tan^{-1} \left( \frac{u_0 w}{v_0} \right)$$

"Velocity function"  
=> units of  
displacement.

projection of the  
rotating vector on a  
vertical plane ( $u$ )

I  $\Rightarrow u$  vs  $v_0/w$  Vector rotates @  $w$  RAD/s

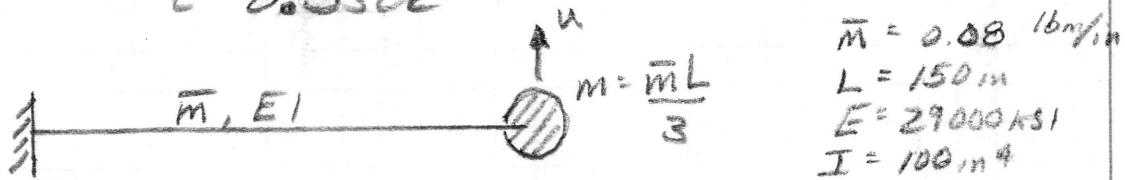
II  $\Rightarrow u$  vs time  $w = \frac{2\pi}{T}$  } time

III  $\Rightarrow v_0/w$  vs time  $w = \frac{2\pi}{T}$  } both oscillate  
@ the same frequency.

Note: Error in book

$v_0/w$ , plot on pg 212

Example: For the following structure subjected to a linear initial displacement at its end, determine the displacement, velocity and moment diagram @  $t = 0.3 \text{ sec}$



$\Rightarrow$  From before

$$k^* = \frac{3EI}{L^3} = 2577.8 \text{ lb/in}$$

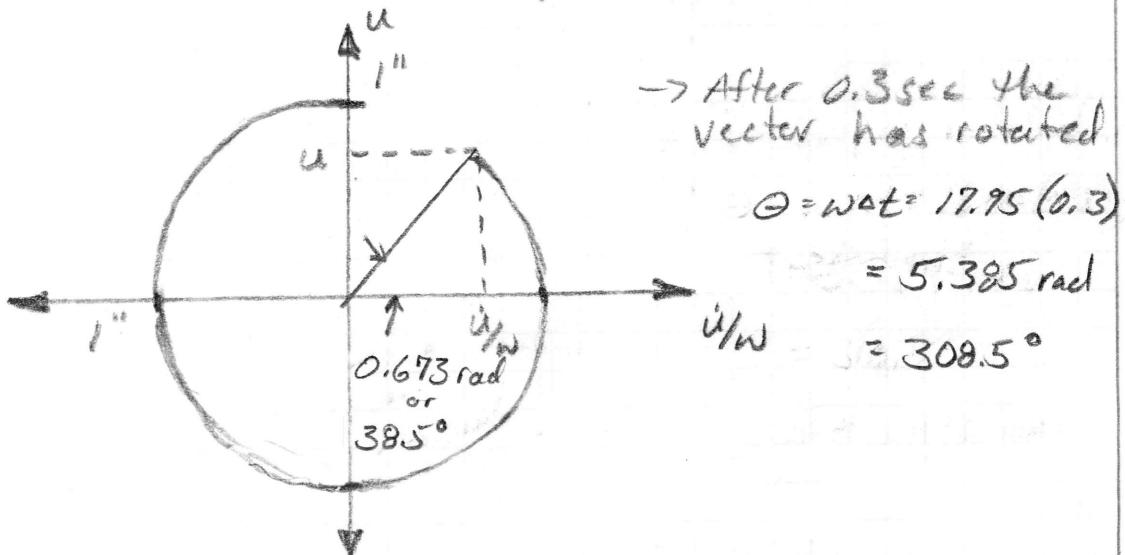
$$m^* = \frac{\bar{m}L}{3} + \frac{\bar{m}L}{3} = \frac{2\bar{m}L}{3} = 8 \text{ lbm}$$

$$\omega = \sqrt{\frac{2577.8}{8}}$$

$$\omega = 17.95 \text{ rad/s}$$

$$g = \sqrt{1^2 + (\frac{\omega}{L})^2} = 1.0$$

$$\phi = \tan^{-1}\left(\frac{1}{\omega}\right) = \frac{\pi}{2} = 1.570 \text{ rad.}$$



$$u = \sin(0.673 \text{ rad}) (g) \text{ } \frac{\text{in}}{\text{s}^2}$$

$$\Rightarrow u(0.3 \text{ sec}) = 0.623 \text{ in}$$

$$\frac{u}{w} = \cos(0.673 \text{ rad}) (g) \text{ } \frac{\text{in}}{\text{s}^2}$$

$$u = 0.782 (1 \text{ in}) (17.95 \text{ rad/s}) =$$

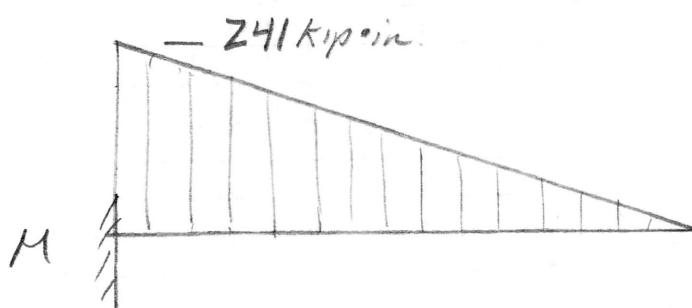
$$\Rightarrow u(0.3 \text{ sec}) = 14.0 \text{ in/s}$$

$\Rightarrow$  Moment Diagram

$$\begin{aligned} \psi''(x) &= -\frac{3x}{L^3} + \frac{3}{L^2} \\ z(0.3) &= 0.623 \end{aligned} \quad \left. \begin{array}{l} u(0.3, x) = z(0.3)\psi(x) \\ \psi(x) = \dots \end{array} \right\}$$

$$M(0.3s, x) = EI \left. \frac{d^2u}{dx^2} \right|_{t=0.3 \text{ sec}} = 29,000 \text{ kip/in}^4 (100 \text{ in}^4) (0.623)^2 \left[ \frac{-3x}{(150)^3} + \frac{3}{(150)^2} \right]$$

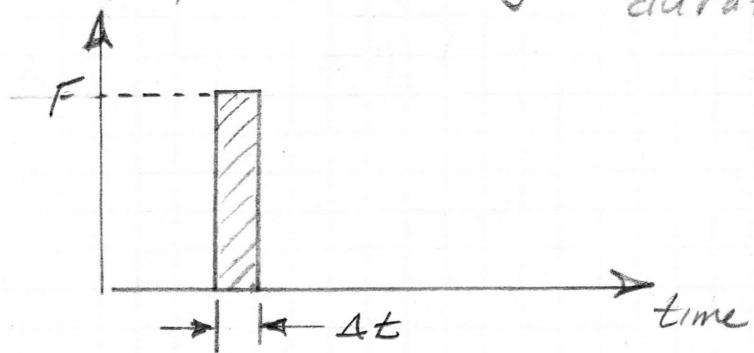
$$M(0.3s, x) = -1,606x + 240.9 \text{ [kip-in]}$$



$\Rightarrow \psi$  is used to "compress" the system to a SDOF and to expand the results back out...

→ Aside from prescribed initial conditions, two realistic loadings can be addressed using free vibration response.

→ Impulse Loading ⇒ force  $F$  applied over duration  $\Delta t$ .



$\Delta t \ll T$   $\rightarrow$  period of the structure.

→ Newton's 2<sup>nd</sup> Law

$$F = \frac{d}{dt} \left( m \frac{du}{dt} \right) \Rightarrow m \frac{\Delta V}{\Delta t}$$

constant mass  
average accel  
over  $\Delta t$

$$F \Delta t = m \Delta V$$

$$\Delta V = \frac{F(\Delta t)}{m} \leftarrow \text{impulse}$$

$\uparrow$   
change in velocity

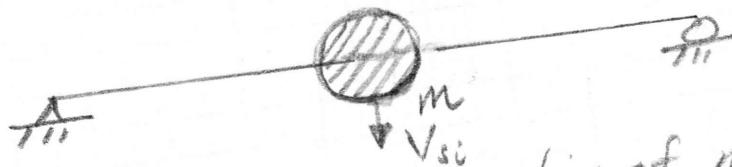
→ If this occurs when the structure is at rest (e.g.  $t=0$ ), then:

$$V_0 = \frac{F(\Delta t)}{m}$$

$\uparrow$   
initial condition

## → Impact with a Mass

$m_i$  — impact mass  
 $v_{i0}$  — impact velocity



→ Use conservation of momentum  
 (note: kinetic energy is only conserved during an elastic impact) ← very rare.

$$m v_{s0} + m_i v_{i0} = m_i v_{if} + m v_{sf}$$

$\downarrow$  velocity of  $m$  @ impact.       $\downarrow$  velocity of  $m_i$  following impact.  
 $\uparrow$  impact mass       $\uparrow$  velocity of  $m_i$  @ impact.

Velocity of  $m$  following impact.

In this case  
 $m = m + m_i$

⇒ If  $m_i$  &  $m$  stick together, then

$$v_{if} = v_{sf} = v_f$$

$$m v_{s0} + m_i v_{i0} = (m_i + m) v_f$$

$$-m_i v_{s0} + m_i v_{i0} = (m_i + m) v_f - m v_{s0} - m_i v_{s0}$$

$$m_i (v_{i0} - v_{s0}) = (m_i + m) (\underbrace{v_f - v_{s0}}_{\Delta V})$$

$$\Delta V = \frac{m_i (v_{i0} - v_{s0})}{(m_i + m)}$$

If this occurs when the structure is @ rest (e.g.  $t=0$ ), then:

$$v_0 = \frac{m_i v_{i0}}{(m_i + m)}$$

Initial condition ↑

Example:

Initial Cond:  $u_0 = -1 \text{ in}$   
 $v_0 = -10 \text{ in/s}$

$EI, L, m=0$

$\uparrow u(t)$

$m = 5.5 \text{ lb}_m$

$$EI = 2.9 \times 10^{10} \text{ lb-in}^2$$

$$L = 240 \text{ in}$$

$\sim \text{Impulse @ } t = 0.25$

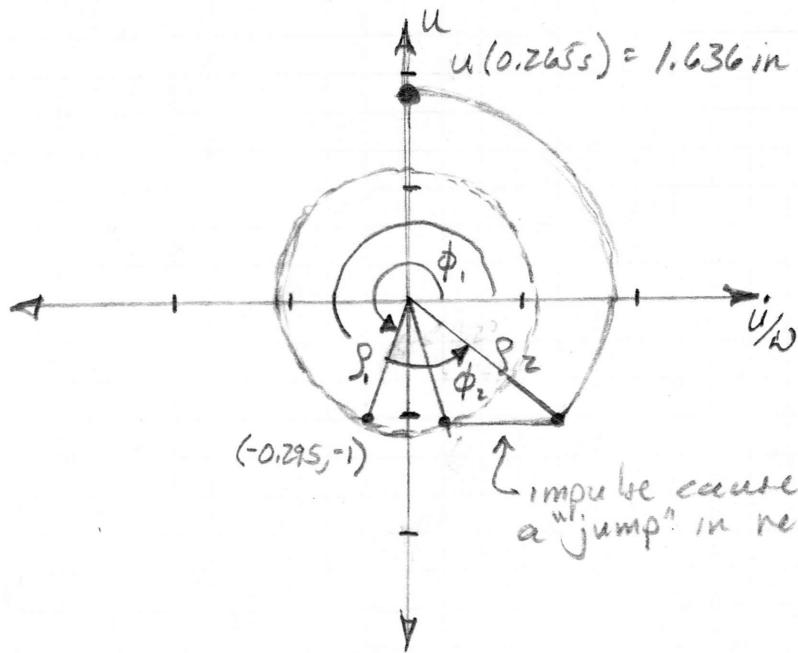
$$F = 20,000 \text{ lb}$$

$$\Delta t = 0.01 \text{ s}$$

Determine:  $u, v, \& M(x) @ t = 0.265 \text{ sec}$

$$k = \frac{3EI}{L^3} = 6293.4 \text{ lb/in}$$

$$\omega = \sqrt{\frac{k}{m}} = 33.83 \text{ rad/s}$$



$$r_1 = \sqrt{(0.295)^2 + (1)^2} = 1.043$$

$$\phi = \tan^{-1}\left(\frac{-1}{-0.295}\right) + \pi = 4.425 \text{ rad } (253.5^\circ)$$

$\Rightarrow$  Rotation of vector from  $t=0$  to  $t=0.25$

$$\Delta\theta = 33.83(0.25) = 6.766 \text{ rad. } (387.6^\circ)$$

$$\theta = 4.425 + 6.766 \text{ rad} = 40.901 \text{ rad} + 2\pi$$

$\uparrow$  1 full rotation  
 $(281.2^\circ)$

$$\left. \frac{\dot{u}}{\omega} \right|_{t=0.2} = g \sin(0.195 \text{ rad}) = 0.2025 \text{ in}$$

$$\left. \ddot{u} \right|_{t=0.2} = (33.83 \text{ rad/s}) 0.2025 \text{ in} = 6.850 \text{ in/s}$$

$$\left. u \right|_{t=0.2} = -g \cos(0.195 \text{ rad}) = -1.023 \text{ in}$$

$\Rightarrow$  Change in velocity due to impact.

$$\Delta V = \frac{(20,000 \text{ lb})(0.01 \text{ s})}{5.516} = 36.36 \text{ in/sec.}$$

$\Rightarrow$  "New" initial conditions

$$\left. u \right|_{t=0.2} = -1.023 \text{ in}$$

$$\left. \ddot{u} \right|_{t=0.2} = 6.850 + 36.36 = 43.21 \text{ in/sec.}$$

$$\phi_2 = \tan^{-1} \left( \frac{-1.023 (33.83)}{43.21} \right) = -0.676 \text{ rad} \\ = 5.604 \text{ rad } (321^\circ)$$

$$S_2 = \sqrt{(-1.023)^2 + \left( \frac{43.21}{33.83} \right)^2} = 1.636$$

$$\left. \frac{\dot{u}}{\omega} \right|_{t=0.2} = 1.277 \text{ in}$$

$\Rightarrow$  Rotation from  $t=0.2$  to  $t=0.265 \text{ sec}$

$$\Delta \theta = 0.065 (33.83) = 2.199 \text{ rad. } (126.0^\circ)$$

$$\theta = 5.604 + 2.199 = 7.523 + \frac{2\pi}{87.23^\circ}$$

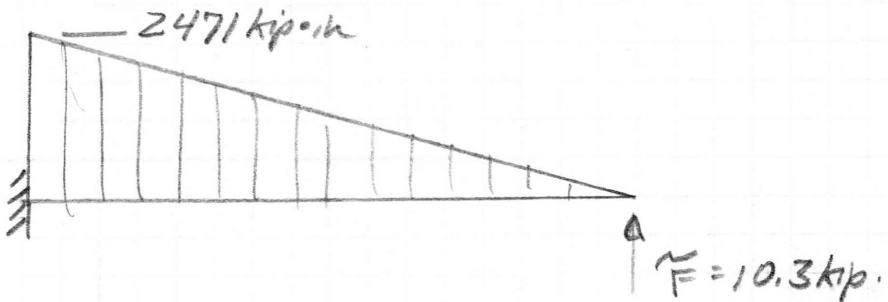
↑  
1 full rotation

$$u(0.265s) = 1.636 \sin(1.523) = 1.634 \text{ in}$$

$$\dot{u}(0.265s) = 1.636 \cos(1.523)(33.83) = 2.644 \text{ in/s}$$

$\Rightarrow$  Effective force

$$\tilde{F} = kA = 6293.4(1.636) = 10,296 \text{ lb.}$$



$\Rightarrow$  Alternative Solution

$$u = u_0 \cos(\omega t) + \frac{\dot{u}_0}{\omega} \sin(\omega t)$$

$$u(0.2s) = -1 \cos(33.83(0.2)) + \frac{-10}{33.83} \sin(33.83(0.2))$$

$$\Rightarrow u(0.2s) = -1.023 \text{ in}$$

$$\dot{u} = -u_0 \omega \sin(\omega t) + \dot{u}_0 \cos(\omega t)$$

$$\dot{u}(0.2s) = -1(33.83) \sin(33.83(0.2)) + -10 \cos(33.83(0.2))$$

$$\Rightarrow \dot{u}(0.2s) = 6.84 \text{ in/s}$$

"New" Initial conditions.

$$u(0.2s) = -1.023 \text{ in}$$

$$\dot{u}(0.2s) = 6.84 + 36.36 = 43.21 \text{ in/sec}$$

↑ due to impact

need to subtract part of  $t$  @ input  
 $\downarrow$   
 $\Rightarrow$  restart clock.

$$u(0.265s) = -1.023 \cos(33.83(0.265-0.2)) +$$

$$\frac{43.21}{33.83} \sin(33.83(0.265-0.2))$$

$$\Rightarrow u(0.265s) = 1.634 \text{ m} \checkmark$$

$$i(0.265s) = -1.025(33.83) \sin(33.83(0.265-0.2)) \\ + 43.21 \cos(33.83(0.265-0.2))$$

$$\Rightarrow i(0.265s) = 2.632 \text{ A/s. } \checkmark$$

$\Rightarrow$  See additional examples work out  
on excel.