

LECTURE 5: Proportional Damping, Damped Free Vibration Response of MDOF Systems

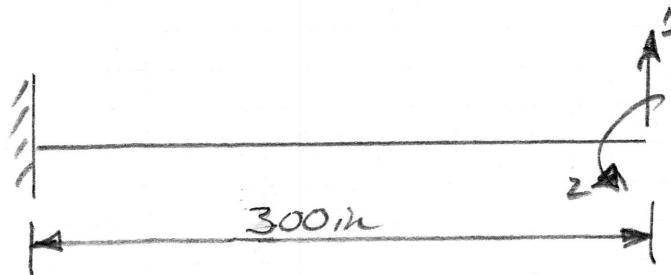
=> In class example...

undamped
For the following 2DOF system:

(1) Compute the free vibration for the prescribed initial conditions.

(2) Show that if the initial conditions are proportional to one of the modes, the only that mode participates.

(3) Compare the time history to the obtained by modeling the structure as an SDOF.



$$E = 29,000 \text{ kN}, \\ I = 1000 \text{ m}^4, \\ \bar{m} = \frac{20}{386} \left(\frac{490}{12^3} \right)$$

$$u_0 = \begin{Bmatrix} 1.0 \\ 0 \end{Bmatrix} \text{ in } \dot{u}_0 = \begin{Bmatrix} 0 \\ 0 \end{Bmatrix} \text{ m/s}$$

$$\bar{m} = 0.0146 \text{ kg/m}$$

$$[K] = \frac{EI}{L^3} \begin{bmatrix} 12 & -6L \\ -6L & 4L^2 \end{bmatrix} = \begin{bmatrix} 12,888 & -1,933,380 \\ -1,933,380 & 386,68 \times 10^6 \end{bmatrix}$$

$$[M] = \frac{\bar{m}L}{420} \begin{bmatrix} 156 & -22L \\ -22L & 4L^2 \end{bmatrix} = \begin{bmatrix} 1.627 & -68.84 \\ -68.84 & 3,754.8 \end{bmatrix}$$

$$=> \text{Note: SDOF } K = \frac{3EI}{L^3} = 3222 \text{ N/m}^2$$

$$m = \frac{33}{140} \bar{m}L = 1.032 \text{ kg}$$

$$\omega = \sqrt{\frac{3222}{1.032}} = 55.88 \text{ rad/s}$$

\rightarrow Classical Damping

\rightarrow The damping matrix is also diagonalized by pre- and post-multiplying by the mode shape array.

\Rightarrow 4 Methods.

(1) Mass proportional

(2) Stiffness proportional

(3) Rayleigh damping

(4) Superposition of Modal Damping Matrices

\rightarrow Mass Proportional Damping

\rightarrow Since we know ϕ_N is orthogonal w.r.t. $[M]$, we set $[C]$ equal to $[M]$ times a constant α_0 damping matrix

$$[C] = \alpha_0 [M]$$

$$\phi_j^T [C] \phi_i = \alpha_0 \phi_j^T [M] \phi_i = 0$$

\Rightarrow Orthogonality condition is preserved.

$$[C^*] = [\phi_N]^T [C] [\phi_N] = \alpha_0 [\phi_N]^T [M] [\phi_N]$$

Identity matrix = $[I]$

$$[C^*] = \alpha_0 \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

$$\Rightarrow C_1^* = C_2^* = C_3^* \dots = C_n^* = \alpha_0$$

\Rightarrow Choose α_0 to give a specific damping ratio for a specific mode.

$$C_i^* = \alpha_0 = \frac{1}{2} \zeta_i \omega_i \quad (\text{definition of damping ratio})$$

\Rightarrow All other modes have

$$C_j^* = \frac{1}{2} \zeta_j \omega_j = \frac{1}{2} \zeta_i \omega_i$$

$$\zeta_j = \frac{\omega_i}{\omega_j} \zeta_i$$

↑ inversely proportional to ω_j

\Rightarrow Stiffness Proportional Damping.

$$[C] = \alpha_0 [K]$$

$$[C^*] = [\phi_N]^T [G] [\phi_N] = \alpha_0 [\phi_N]^T [K] [\phi_N]$$

$$[C^*] = \alpha_0 \begin{bmatrix} \omega_1^2 & 0 & 0 \\ 0 & \omega_2^2 & 0 \\ 0 & 0 & \omega_3^2 \end{bmatrix}$$

$$\Rightarrow C_1^* = \alpha_0 \omega_1^2$$

$$C_2^* = \alpha_0 \omega_2^2$$

$$C_3^* = \alpha_0 \omega_3^2$$

\Rightarrow Again, choose α_i to give a specific damping ratio for a specific mode.

$$C_i^* = \alpha_i w_i^2 = 2\zeta_i w_i$$

$$\alpha_i = \frac{2\zeta_i}{w_i}$$

\Rightarrow All other modes have.

$$C_j^* = \frac{2\zeta_i}{w_i} w_j^2 = 2\zeta_i w_j$$

$$\zeta_j = \zeta_i \frac{w_j}{w_i}$$

Σ proportional to w_j

$$\Rightarrow \text{Eg. 2 DOF w/ } w = \begin{Bmatrix} z \\ 1z \end{Bmatrix} \text{ rad/s}$$

\Rightarrow Choose $\zeta_1 = 0.05$ (5% damping in mode 1)

\Rightarrow Compute damping for mode 2.

(1) Mass proportional damping

$$C_1^* = \alpha_0 = 2\zeta_1^{\frac{0.05}{0.05}} w_1^2$$

$\alpha_0 = 0.2$ (multiply this by $[M]$ to obtain $[C]$ in physical coordinates)

$$\zeta_2 = \frac{w_1}{w_2} \zeta_1 = \frac{2}{12}(0.05) = 0.0083$$

less than
1%

\Rightarrow Not realistic

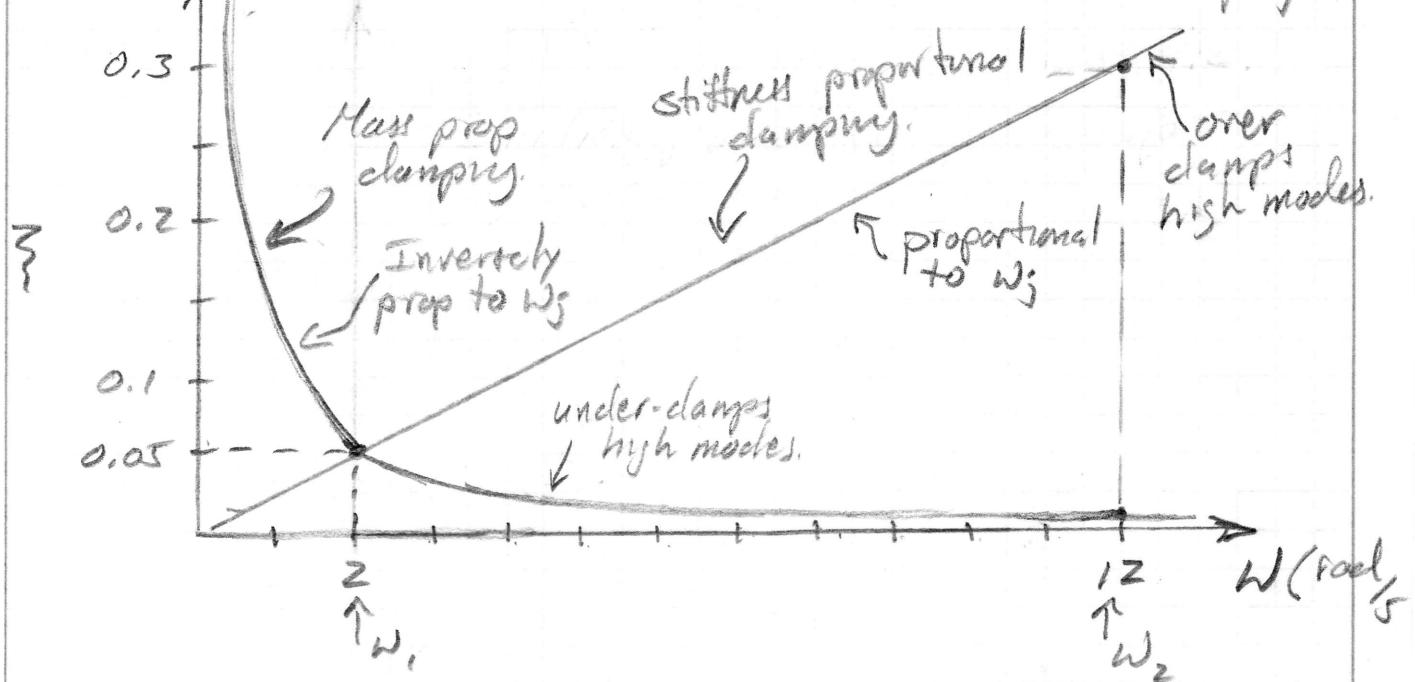
(2) Stiffness Proportional Damping.

$$\alpha_1 = \frac{2\zeta_1}{\omega_1} = \frac{2(0.05)}{2} = 0.05$$

\uparrow
(multiply this
by $[k]$ to obtain
 $[C]$ in physical coord.)

$$\zeta_2 = \zeta_1 \frac{\omega_2}{\omega_1} = 0.05 \left(\frac{12}{2}\right) = 0.3$$

\uparrow
Not realistic \Leftarrow 30% damping...



\Rightarrow Neither one is realistic except for closely spaced modes

\hookrightarrow Experimental testing indicates structures exhibit similar levels of damping across their modes of interest.

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→ Rayleigh Damping

$$[C] = \alpha_0 [M] + \alpha_1 [k]$$

both stiffness and mass
proportional..

⇒ As before.

$$[C^*] = \alpha_0 \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix} + \alpha_1 \begin{bmatrix} \omega_1^2 & 0 & 0 \\ 0 & \omega_2^2 & 0 \\ 0 & 0 & \omega_3^2 \end{bmatrix}$$

$$C_1^* = \alpha_0 + \alpha_1 \omega_1^2 = 2\zeta_1 \omega_1$$

$$C_2^* = \alpha_0 + \alpha_1 \omega_2^2 = 2\zeta_2 \omega_2$$

$$C_3^* = \alpha_0 + \alpha_1 \omega_3^2 = 2\zeta_3 \omega_3$$

⇒ Choose α_0 & α_1 to give a specific damping ratio for two modes

$$\Rightarrow \text{E.g. } 3 \text{DOF w/ } \omega = \left\{ \begin{array}{l} 2 \\ 12 \\ 14 \end{array} \right\} \text{ rad/sec}$$

$$\Rightarrow \text{Choose } \zeta_1 = \zeta_3 = 0.05$$

typically choose the highest
and lowest modes of interest

2 equations, $\left\{ \begin{array}{l} \alpha_0 + \alpha_1 (2)^2 = 2(0.05)(2) \\ \alpha_0 + \alpha_1 (14)^2 = 2(0.05)(14) \end{array} \right.$
 2 unknowns.

↑ you'll see
why shortly

$$(\alpha_0 + 4\alpha_1 = 0.2) * -1$$

$$\underline{+ \alpha_0 + 196\alpha_1 = 1.4}$$

$$192\alpha_1 = 1.2$$

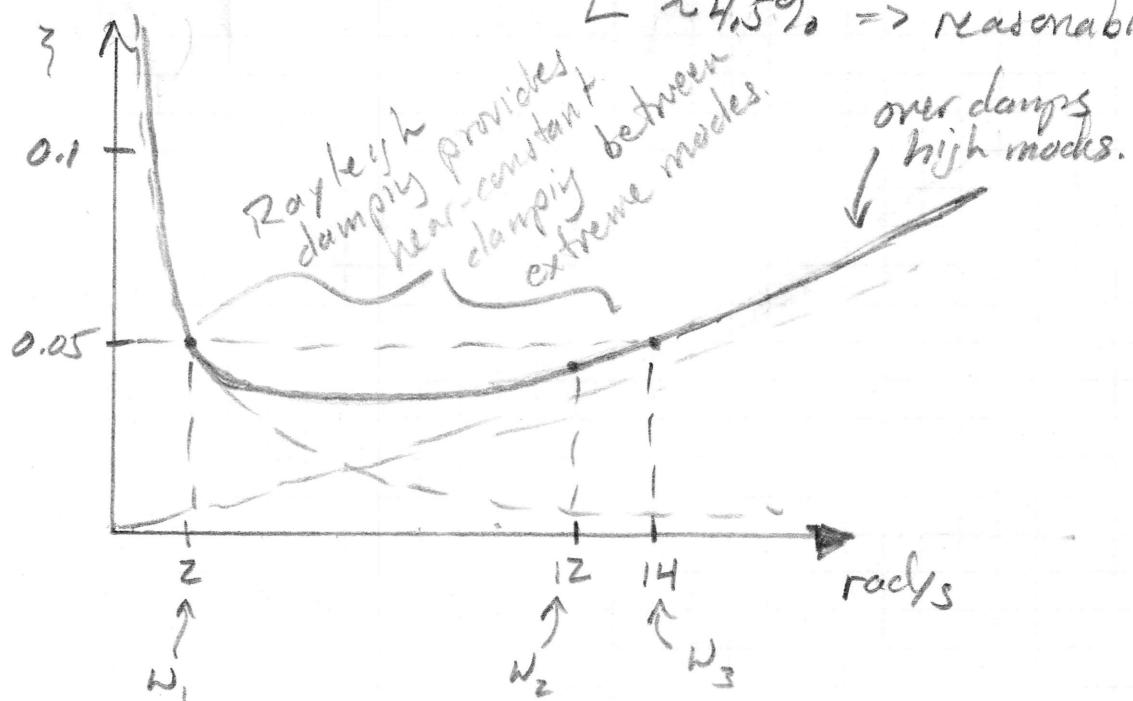
(Multiply these by $[K]$ & $[M]$ to obtain $[C]$ in physical coord.)

\Rightarrow Compute β_2

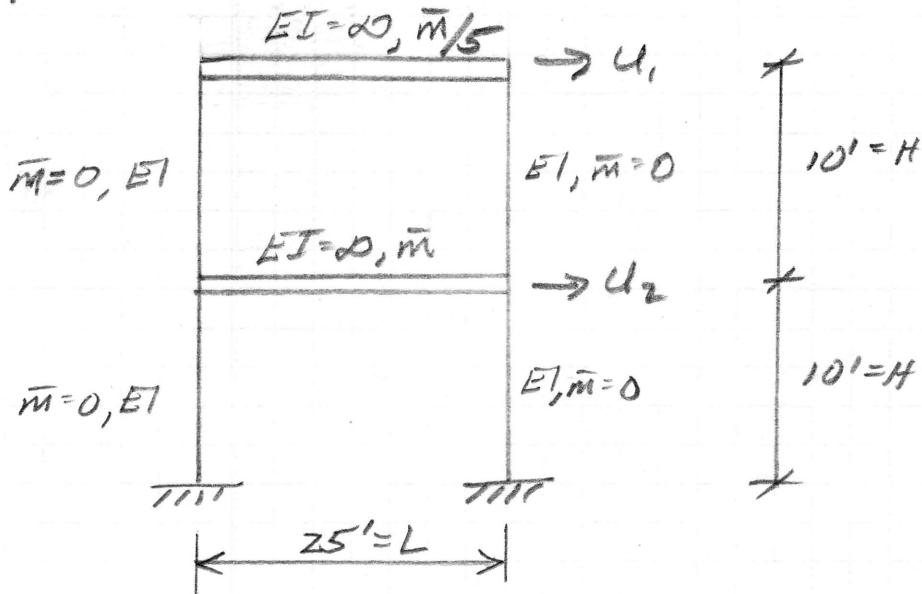
$$0.1755 + 0.00612(12)^2 = 2\beta_2(12)$$

$$\beta_2 = 0.044$$

$\Sigma \sim 4.5\%$ \Rightarrow reasonable.



Example:



$$E = 29,000 \text{ ksi}$$

$$I = 1,000 \text{ in}^4$$

$$\bar{m} = (20\text{ft})(1\text{ft})(145^{16}/4^3)(1/12)(\frac{1}{386.4}) = 0.625 \text{ lbm/in}^{16}$$

\Rightarrow Damping: Mode 1 $\Rightarrow \zeta_1 = 0.04$ (4%)
 Mode 2 $\Rightarrow \zeta_2 = 0.06$ (6%)

\Rightarrow Initial Cond:

$$u_0 = \begin{cases} 2 \text{ in} \\ 0 \text{ in} \end{cases} \quad \dot{u}_0 = \begin{cases} 2 \text{ in/sec} \\ -1 \text{ in/sec} \end{cases}$$

\Rightarrow Compute the total response.

(1) Formulate K & M

$$[K] = \begin{bmatrix} \frac{24EI}{H^3} & -\frac{24EI}{H^3} \\ -\frac{24EI}{H^3} & \frac{98EI}{H^3} \end{bmatrix} = \begin{bmatrix} 402.8 & -402.8 \\ -402.8 & 805.6 \end{bmatrix} \text{ kip/in}$$

$$[M] = \begin{bmatrix} \bar{m}L/5 & 0 \\ 0 & \bar{m}L \end{bmatrix} = \begin{bmatrix} 0.094 & 0 \\ 0 & 0.188 \end{bmatrix} \text{ kip-m}$$

(2) Solve the Eigen problem

$$a = \det[M] = 0.0176$$

$$b = -(402.8(0.188) + 805.6(0.094)) = -151.5$$

$$c = \det[K] = 162,248$$

$$\omega_i^2 = \frac{151.5 \pm \sqrt{151.5^2 - 4(0.0176)(162,248)}}{2(0.0176)}$$

$$\omega_i^2 = \frac{151.5 \pm 107.4}{0.0352} = (1252.8, 7355.1)$$

* $\omega = \begin{cases} 35.4 \\ 85.8 \end{cases} \text{ rad/s} = \begin{cases} 5.63 \\ 13.65 \end{cases} \text{ Hz}$

$$r_1 = \frac{+402.8}{402.8 - (1253)(0.094)} = 1.41$$

$$r_2 = \frac{+402.8}{402.8 - (7355)(0.094)} = -1.40$$

* unit normalized modes.

$$[\phi] = \begin{bmatrix} 1.0 & 1.0 \\ 0.709 & -0.709 \end{bmatrix}$$

↑ ↑

$$\{\phi_1\} \quad \{\phi_2\}$$

(3) Bring the problem into modal space
(coordinates)

\Rightarrow use mass normalized mode shapes.

$$[\phi]^T [M] [\phi] = [M^*]$$

$$[\phi]^T [M] = \begin{bmatrix} 1.0 & 0.707 \\ 1.0 & -0.707 \end{bmatrix} \begin{bmatrix} 0.094 & 0 \\ 0 & 0.108 \end{bmatrix} = \begin{bmatrix} 0.094 & 0.133 \\ 0.094 & -0.133 \end{bmatrix}$$

$$[\phi]^T [M] [\phi] = \begin{bmatrix} 0.094 & 0.133 \\ 0.094 & -0.133 \end{bmatrix} \begin{bmatrix} 1.0 & 1.0 \\ 0.707 & -0.707 \end{bmatrix} = \begin{bmatrix} 0.188 & 0 \\ 0 & 0.188 \end{bmatrix}$$

$$\phi_{N_1} = \frac{\phi_1}{\sqrt{M_1^*}} = \begin{Bmatrix} 2.31 \\ 1.64 \end{Bmatrix}$$

$$\phi_{N_2} = \frac{\phi_2}{\sqrt{M_2^*}} = \begin{Bmatrix} 2.31 \\ -1.64 \end{Bmatrix}$$

$$[\phi_N] = \begin{bmatrix} 2.31 & 2.31 \\ 1.64 & -1.64 \end{bmatrix}$$

By definition, using $[\phi_N]$ to bring the system into modal coord. yields.

$$\begin{bmatrix} 1 & 0 \\ 0 & 1 \end{bmatrix} \begin{Bmatrix} \ddot{z}_1 \\ \ddot{z}_2 \end{Bmatrix} + \begin{bmatrix} \omega_1^2 & 0 \\ 0 & \omega_2^2 \end{bmatrix} \begin{Bmatrix} z_1 \\ z_2 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \end{Bmatrix}$$

Free vibration.

or, plugging in.

$$\ddot{z}_1 + 1253 z_1 = 0 \leftarrow \text{SDOF}_1, \text{ associated with mode 1}$$

$$\ddot{z}_2 + 7355 z_2 = 0 \leftarrow \text{SDOF}_2, \text{ associated with mode 2.}$$

(4) Compute clamping matrix.

→ use Rayleigh clamping

$$[C^*] = \alpha_0 \begin{bmatrix} 1 & 0 \\ 0 & 1 \end{bmatrix} + \alpha_1 \begin{bmatrix} \frac{w_1^2}{1253} & 0 \\ 0 & \frac{w_2^2}{7355} \end{bmatrix}$$

↓ w_1^2 ↓ w_2^2 ↓ Mode 1 damping coeff.
↓ Mode 2 damping coeff.

$$\alpha_0 + 1253\alpha_1 = 23, w_1 = 2(0.04)(35.4) = 2.83$$

$$\alpha_0 + 7355\alpha_1 = 23, w_2 = 2(0.06)(85.8) = 10.30$$

$$-6102\alpha_1 = -7.47$$

$$\alpha_1 = 0.00122$$

$$\alpha_2 = 1.301$$

↑
Mode 2
damping
coeff.

In physical coord.

$$[C] = \alpha_0 [M] + \alpha_1 [K]$$

IS you bring
this into
modal space
w/ $[Q_N]$

$$\rightarrow [C] = \begin{bmatrix} 0.613 & -0.491 \\ -0.491 & 1.228 \end{bmatrix}$$

↑ Needed if you directly
integrate EOM \Rightarrow for nonlinear
problems.

$$\Rightarrow \begin{bmatrix} 2.83 & 0 \\ 0 & 10.30 \end{bmatrix}$$

(5) Bring I.C. into modal coord.
 \Rightarrow Be sure to use $[\phi_N]$!!!

$$[\phi_N]^T [M] = \begin{bmatrix} 2.31 & 1.64 \\ 2.31 & -1.64 \end{bmatrix} \begin{bmatrix} 0.094 & 0 \\ 0 & 0.188 \end{bmatrix}$$

$$= \begin{bmatrix} 0.217 & 0.308 \\ 0.217 & -0.308 \end{bmatrix}$$

$$\{z_0\} = [\phi_N]^T [M] \{u_0\} = \begin{bmatrix} 0.217 & 0.308 \\ 0.217 & -0.308 \end{bmatrix} \begin{bmatrix} 2 \\ 0 \end{bmatrix}$$

* $\{z_0\} = \begin{pmatrix} 0.434 \\ 0.434 \end{pmatrix}$ mode 1 initial disp.
in

\uparrow mode 2 initial disp.

$$\{\dot{z}_0\} = [\phi_N]^T [M] \{\dot{u}_0\} = \begin{bmatrix} 0.217 & 0.308 \\ 0.217 & -0.308 \end{bmatrix} \begin{bmatrix} 2 \\ -1 \end{bmatrix}$$

* $\{\dot{z}_0\} = \begin{pmatrix} 0.126 \\ 0.742 \end{pmatrix}$ mode 1 initial velocity.
in/sec

\uparrow mode 2 initial velocity.

(6) Solve each SDOF separately.
 ↳ in modal space.

→ SDOF₁ (Mode 1)

$$(1) \ddot{z}_1 + z_1 \gamma_1 \dot{z}_1 + \omega_1^2 z_1 = 0$$

↑ mass ↑ damping ↑ stiffness

$$\left. \begin{array}{l} z_{10} = 0.434 \text{ in} \\ \dot{z}_{10} = 0.126 \text{ in/sec} \end{array} \right\} \begin{array}{l} \text{initial} \\ \text{conditions} \end{array}$$

From CIVE 801

$$z_1 = e^{-\omega_1 \gamma_1 t} \left(z_{10} \cos(\omega_1 t) + \left(\frac{\dot{z}_{10} + \omega_1 \gamma_1 z_{10}}{\omega_1} \right) \sin(\omega_1 t) \right)$$

Note $\omega_1 = 35.4$

$\gamma_1 = 0.04$

$$* z_1 = e^{-1.416t} (0.434 \cos(35.4t) + 0.021 \sin(35.4t))$$

→ SDOF₂ (Mode 2)

$$(1) \ddot{z}_2 + z_2 \gamma_2 \dot{z}_2 + \omega_2^2 z_2 = 0$$

$z_{20} = 0.434 \text{ in}$

$\gamma_2 = 0.06$

$\dot{z}_{20} = 0.742 \text{ in}$

$\omega_2 = 85.8 \text{ rad/s}$

$$* z_2 = e^{-5.148t} (0.434 \cos(85.8t) + 0.035 \sin(85.8t))$$

(7) Bring the solution for each mode back into physical coord. and sum them.

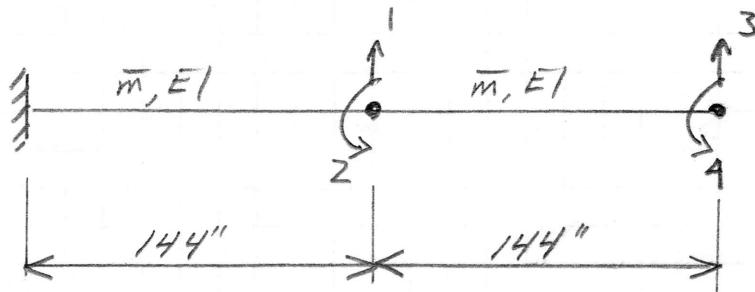
→ modal superposition

$$\{u\} = \{\phi_{N_1}\} z_1 + \{\phi_{N_2}\} z_2$$

$$\begin{cases} u_1 \\ u_2 \end{cases} = \begin{cases} 2.31 \\ -1.64 \end{cases} e^{-1.416t} (0.434 \cos(35.4t) + 0.021 \sin(35.4t))$$

$$+ \begin{cases} 2.31 \\ -1.64 \end{cases} e^{-5.748t} (0.434 \cos(85.8t) + 0.025 \sin(85.8t))$$

Example 2:



$$\bar{m} = (20 \text{ in}^2)(490 \text{ lb/in}^3) \left(\frac{1}{12}\right)^3 \left(\frac{1}{386.4}\right) = 0.0147 \text{ lb sec}^2/\text{in}$$

$$E = 29,000,000 \text{ psi}$$

$$I = 1000 \text{ in}^4$$

⇒ Damping Mode 1 $\zeta_1 = 0.05$ ⇒ Rayleigh
Mode 3 $\zeta_3 = 0.05$ } damping

⇒ Initial Cond.

$$u_0 = \begin{pmatrix} 1.0 \\ 0 \\ 1.0 \\ 0 \end{pmatrix} \text{ in } \dot{u}_0 = \begin{pmatrix} -1 \\ 0 \\ 0 \\ 0 \end{pmatrix} \text{ in/sec}$$

→ Solve for the response of the structure using:

- (1) All four modes
- (2) The first three modes
- (3) The first two modes
- (4) The first mode

→ What errors do you observe?