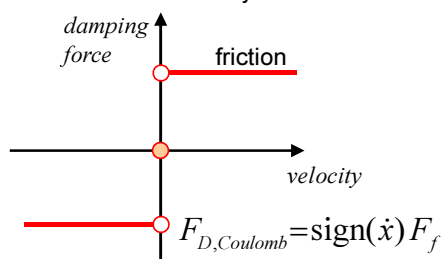


# Vibrations 2, Lecture 6 Introduction to forced vibration

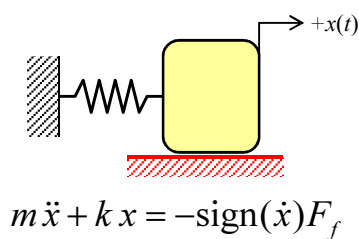
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## Lecture 5

Coulomb model of dry friction



Equation of motion



Free vibration with dry friction:

- free vibration at  $\omega_0$
- linear vibration decay,  $x_{(j)} - x_{(j+2)} = 4F_f/k$
- vibration stops when  $F_f > kx$

## Lecture 6

- Forced vibration response of 1 DOF systems
- Forced vibration with constant excitation
- Solved example

## Forced response of 1 DOF system

Forced response of a vibrating 1 DOF system consists of:

- a component due to free response of the system at  $\omega_D$ ,
- a component due to applied force  $F(t)$ .

**Complementary solution (CS)**, solution when RHS=0:

$$x_H = X e^{-\zeta\omega_0 t} \sin(\omega_D t + \phi) = e^{-\zeta\omega_0 t} (X_1 \sin(\omega_D t) + X_2 \cos(\omega_D t))$$

**Particular solution (PS)**, solution when RHS= $F(t)$ .

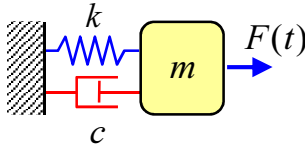
We will consider these types of excitation:

- constant force:  $F(t)=F_0=\text{const.}$  (example: helicopter landing)
- harmonic force:  $F(t)=F_0\sin(\omega t)$  (example: aero engine bladed disc)

**Total solution = CS + PS**

$$x = x_H + x_P = e^{-\zeta\omega_0 t} (X_1 \sin(\omega_D t) + X_2 \cos(\omega_D t)) + x_{\text{forced}}$$

## Total vibration response



$$m \ddot{x} + c \dot{x} + k x = F(t)$$

$$x(t) = x_H(t) + x_p(t)$$

*a.k.a. principle of superposition*

Free response:

$$m \ddot{x}_H + c \dot{x}_H + k x_H = 0$$

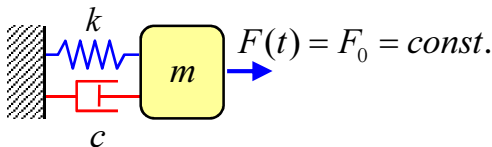
*Associated with transient and decaying vibrations*

Forced response:

$$m \ddot{x}_p + c \dot{x}_p + k x_p = F(t)$$

*Associated with forced vibrations*

## Constant force: $F(t)=F_0$



$$m \ddot{x} + c \dot{x} + k x = F_0$$

- find the particular solution  $x_p$  by “guessing” the **trial solution**
- the trial solution used here is  $x_p=C$ , where  $C$  is the unknown constant
- use  $x=x_p=C$  is the EOM:

$$m \cdot 0 + c \cdot 0 + k C = F_0 \Rightarrow x_p = C = F_0/k$$

The total solution:

$$x = x_H + x_p = e^{-\zeta\omega_0 t} (X_1 \sin(\omega_D t) + X_2 \cos(\omega_D t)) + F_0/k$$

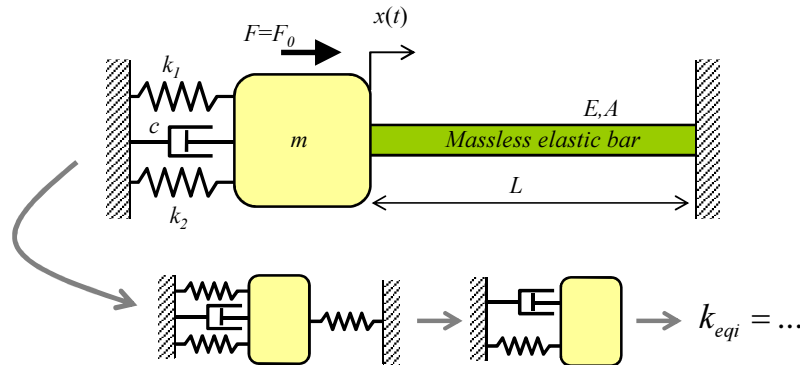
$X_1$  and  $X_2$  from the initial conditions.

*Static deformation due to constant force  $F_0$*

## Example: step force input

Determine the maximum amplitude of vibration of the system with  $F_0=130$  N applied in  $t=0$ . Find the undamped natural frequency and the critical damping  $c_{cr}$ . Sketch the response for  $\zeta=0$  and  $\zeta<1$ . Assume zero initial conditions.

The stiffness is  $k_1=k_2=15$  kN/m, mass  $m=8$  kg and damping ratio is  $\zeta<1$ . The *massless* elastic bar has Young's modulus  $E=5$  GPa, cross-sectional area  $A=10$  mm<sup>2</sup> and length  $L=0.5$  m.



## Example

Equation of motion:  $m\ddot{x} + c\dot{x} + k_{eqi}x = F$        $k_{eqi} = k_1 + k_2 + EA/L$

The total solution:  $x = x_H + x_p = e^{-\zeta\omega_0 t} (X_1 \sin(\omega_D t) + X_2 \cos(\omega_D t)) + F_0/k_{eqi}$

The unknown constants  $X_1$  and  $X_2$  and zero initial conditions:

$$t=0: x_0 = 0 = e^{-\zeta\omega_0 \cdot 0} (X_1 \sin(\omega_D \cdot 0) + X_2 \cos(\omega_D \cdot 0)) + F_0/k_{eqi} \quad X_2 = -F_0/k_{eqi}$$

$$t=0: \dot{x}_0 = 0 = (-\zeta\omega_0) e^{-\zeta\omega_0 \cdot 0} (X_1 \sin(\omega_D \cdot 0) + X_2 \cos(\omega_D \cdot 0)) \\ + e^{-\zeta\omega_0 \cdot 0} (\omega_D X_1 \cos(\omega_D \cdot 0) - \omega_D X_2 \sin(\omega_D \cdot 0))$$

$$0 = (\zeta\omega_0)(F_0/k_{eqi}) + \omega_D X_1 \Rightarrow X_1 = -(\zeta\omega_0/\omega_D)(F_0/k_{eqi})$$

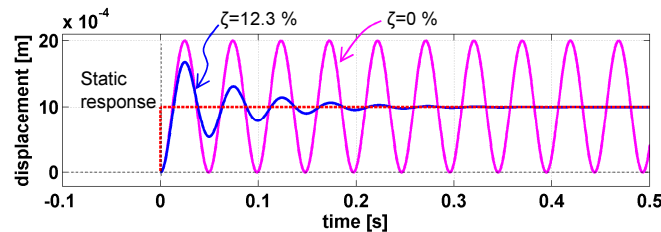
The total solution:

$$x = \frac{F_0}{k_{eqi}} \left( 1 - e^{-\zeta\omega_0 t} \left( \frac{\zeta}{\sqrt{1-\zeta^2}} \sin(\omega_D t) + \cos(\omega_D t) \right) \right)$$

## Example 5

If  $\zeta=0$  then  $\exp(-\zeta\omega_0 t)=1$  and the total response and its maximum is:

$$x = \frac{F_0}{k_{eqi}} (1 - \cos(\omega_D t)), \quad \cos(\omega_D t) = -1 \Rightarrow \max(x(t)) = 2 \frac{F_0}{k_{eqi}} = 2x_{static}$$



Stiffness:  $k_{eqi} = k_1 + k_2 + EA/L = 2 \times 15000 + (5 \times 10^9) \times (10 \times 10^{-6}) / 0.5 = 130 \text{ kN/m}$

Undamped natural frequency:  $f_0 = (1/2\pi) \sqrt{k_{eqi}/m} = (1/2\pi) \sqrt{130000/8} = 20.29 \text{ Hz}$

Critical damping:  $c_{cr} = 2\sqrt{mk_{eqi}} = 2\sqrt{8 \text{ kg} \times 130 \text{ kN/m}} = 2039.6 \text{ kg.s}^{-1}$

Maximum undamped response:  $\max(x(t)) = 2F_0/k_{eqi} = 2 \times (130 / 130000) = 2 \text{ mm}$

## Summary

- Free vibration response occurs at  $\omega_D$
- Forced vibration response = CS (related to free response) + PS (related to the applied load)
- CS component dies out in damped systems
- Constant force produces steady-state static deflection (a new equilibrium position)