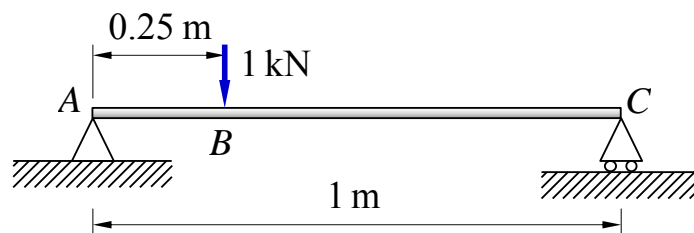


Example 2.3.2(a) – Plot the curvature, slope and deflection for the simply-supported beam below. The beam is made of aluminium alloy with $E = 70 \text{ GPa}$ and has a solid square cross-section measuring $40 \text{ mm} \times 40 \text{ mm}$.



We start by finding support reactions,

$$\sum M_{@A}^{CW} = 0,$$

$$M_A + (1 \text{ kN})\left(\frac{1}{4} \text{ m}\right) - (R_C)(1 \text{ m}) = 0.$$

The extremity A is pinned, therefore $M_A = 0$ and,

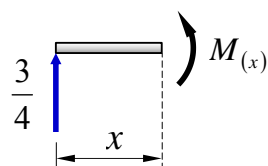
$$R_C = (1 \text{ kN})\left(\frac{1}{4} \text{ m}\right)\left(\frac{1}{1 \text{ m}}\right) \quad \therefore \quad R_C = \frac{1}{4} \text{ kN}.$$

Vertical equilibrium gives,

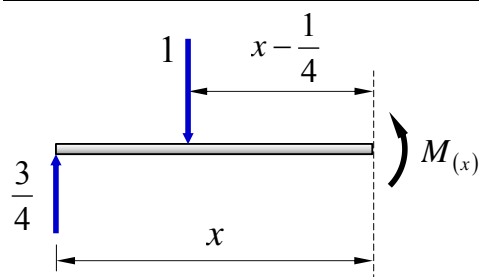
$$\sum F = 0,$$

$$R_A + R_C - (1 \text{ kN}) = 0 \quad \therefore \quad R_A = -\frac{3}{4} \text{ kN}.$$

We find two different moment equations, depending on where we section the beam,



$$M_{(x)} - \left(\frac{3}{4}\right)(x) = 0 \quad \therefore \quad M_{(x)} = \frac{3}{4}x$$



$$M_{(x)} - \left(\frac{3}{4}\right)(x) + (1)\left(x - \frac{1}{4}\right) = 0 \quad \therefore \quad M_{(x)} = \frac{3}{4}x - \left(x - \frac{1}{4}\right)$$

In order to **combine both equations into one**, we use the Heaviside step function,

$$M_{(x)} = \frac{3}{4}x - \left[\left(x - \frac{1}{4}\right)H\left(x - \frac{1}{4}\right)\right].$$

The curvature equation is therefore,

$$M_{(x)} = EI \frac{d^2 v}{dx^2} = \frac{3}{4}x - \left[\left(x - \frac{1}{4}\right) H\left(x - \frac{1}{4}\right) \right]. \quad (1)$$

Integrating once gives the slope,

$$EI \phi_{(x)} = EI \frac{dv}{dx} = \frac{3}{8}x^2 - \left[\frac{1}{2} \left(x - \frac{1}{4}\right)^2 H\left(x - \frac{1}{4}\right) \right] + A. \quad (2)$$

Integrating again gives the deflection,

$$EI v_{(x)} = \frac{1}{8}x^3 - \left[\frac{1}{6} \left(x - \frac{1}{4}\right)^3 H\left(x - \frac{1}{4}\right) \right] + Ax + B. \quad (3)$$

The first boundary condition is,

$$x = 0, v = 0 \quad \therefore \quad B = 0.$$

And the second boundary condition is:

$$x = 1, v = 0 \quad \therefore \quad \frac{1}{8}(1)^3 - \frac{1}{6} \left(1 - \frac{1}{4}\right)^3 + A(1) = 0 \quad \therefore \quad A = -\frac{7}{128} \text{ kN m}^2.$$

In order to draw the three graphs we need to define a few points.

- First we compute the flexural modulus.

$$EI = \left(70 \cdot 10^9 \frac{\text{N}}{\text{m}^2} \right) \left[\frac{(0.04 \text{ m})^4}{12} \right] \quad \therefore \quad EI = 14.93 \text{ kN m}^2.$$

- Point of maximum downward deflection. The span AC will 'sag' under the applied load (*i.e.* it will have a 'smiley face' type deformation), and the maximum downward deflection will not be at B nor at the mid-span, but somewhere else along the span BC. This minimum in $v_{(x)}$ is characterised by a zero first derivative (*i.e.* $\phi(x) = 0$), therefore,

$$EI \phi_{x(v_{\min})} = 0 \quad \therefore \quad \frac{3}{8}x_{(v_{\min})}^2 - \frac{1}{2} \left[x_{(v_{\min})} - \frac{1}{4} \right]^2 - \frac{7}{128} = 0 \quad \therefore \quad -\frac{1}{8}x_{(v_{\min})}^2 + \frac{1}{4}x_{(v_{\min})} - \frac{11}{128} = 0.$$

The roots of a 2nd order polynomial are given by $x = \frac{-b \pm \sqrt{b^2 - 4ac}}{2a}$, which gives $\begin{cases} x_+ = 0.44 \text{ m} \\ x_- = 1.56 \text{ m} \end{cases}$.

The beam is 1 m long so only the first root is valid, and we have $x_{(v_{\min})} = 0.44 \text{ m}$.

- Maximum downward deflection. Replacing $x = 0.44 \text{ m}$ in equation (3) gives,

$$EI v_{\min} = \frac{1}{8}(0.44)^3 - \frac{1}{6}(0.44 - 0.25)^3 - \frac{7}{128}(0.44) \quad \therefore \quad v_{\min} \cong 0.975 \text{ mm}$$

- Local curvature. Remember that $M = \kappa \cdot EI$ $\therefore \quad \kappa = \frac{M}{EI}$ (4)

Further points may be found by substituting x values in equations (1), (2), (3) and (4). The final graphs are shown below.

