Advanced Bending and Torsion Composite Beams

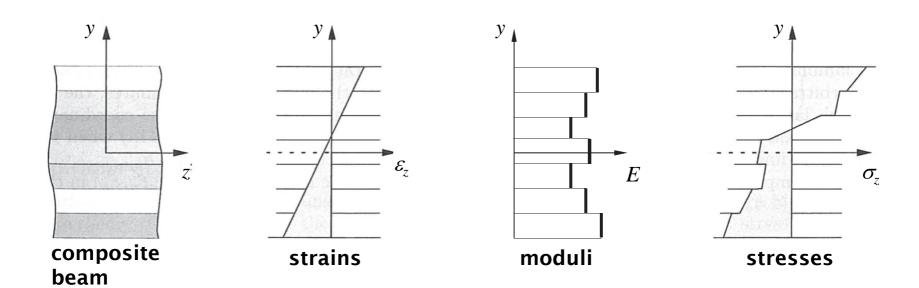
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- Beams made of two or more different materials (with different Young's moduli)
- Assumptions:
 - 'Plane sections remain plane', i.e. linear strain distribution through the thickness (Euler-Bernoulli beams)
 - However, different moduli will cause stress discontinuities or 'jumps'





Method:

- 1. Choose one Young's modulus to be used as reference: E_{ref}
- 2. Find the area scaling factors n_i for each material section i: $n_i = \frac{E_i}{E_{ref}}$
- 3. Conduct the analysis as before, utilising 'effective' (scaled) properties:

$$A_i^{\text{eff}} = A_i \ n_i \qquad \qquad I_i^{\text{eff}} = I_i \ n_i$$

4. Finally, calculate global deflections (and hence strains) based on $E_{\rm ref}$ alone

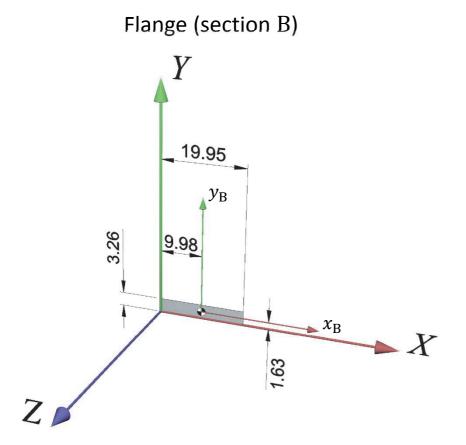
Note:

Only the 'local width' variable (usually denoted b_i) is scaled - limits of integration and centroids <u>remain unchanged</u>

$$A_i^{\rm eff} = (n_i \, b_i) \int_{-\frac{h_i}{2}}^{+\frac{h_i}{2}} \mathrm{d}y \qquad \qquad I_i^{\rm eff} = (n_i \, b_i) \int_{-\frac{h_i}{2}}^{+\frac{h_i}{2}} y^2 \, \mathrm{d}y$$
 scaled width original height



• We now make the flange (section B) of steel with $E=210~\mathrm{GPa}$



$$E_{\rm ref} = 70 \, \text{GPa}$$

$$n_{\rm B} = \frac{210 \text{ GPa}}{70 \text{ GPa}} = 3$$

$$A_{\rm B} = n_{\rm B}(19.95)(3.26) = 195.11 \,\rm mm^2$$

$$\bar{X}_{\rm B} = 9.98 \, {\rm mm}$$

$$\bar{Y}_{\rm B} = 1.63 \, \rm mm$$



We can now find the centroid of the compound section:

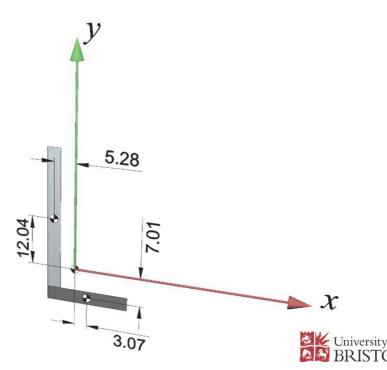
$$\bar{X} = \frac{\sum \bar{X}_i A_i}{\sum A_i} = \frac{\bar{X}_A A_A + \bar{X}_B A_B}{A_A + A_B} = \frac{(1.63)(113.58) + (9.98)(195.11)}{(113.58) + (195.11)}$$

$$\bar{X} = 6.91 \,\mathrm{mm}$$

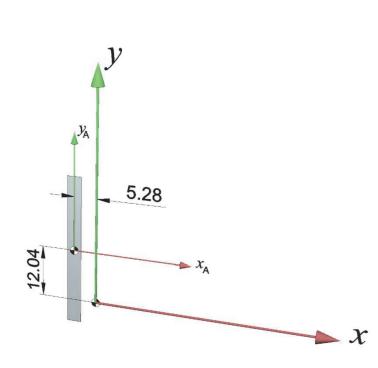
$$\bar{Y} = \frac{\sum \bar{Y}_i A_i}{\sum A_i} = \frac{\bar{Y}_A A_A + \bar{Y}_B A_B}{A_A + A_B} = \frac{(20.68)(113.58) + (1.63)(195.11)}{(113.58) + (195.11)}$$

$$\overline{Y} = 8.64 \text{ mm}$$

Plotting on the cross-section:



• We now place the origin of (x, y) at the compound centroid and apply the parallel axes theorem for section A:



$$I_{x_A x_A} = \frac{b h^3}{12} = \frac{(3.26)(34.84)^3}{12} = 11,488.70 \text{ mm}^4$$

$$\bar{y}_A = \bar{Y}_A - \bar{Y} = 20.68 - 8.64 = 12.04 \text{ mm}$$

$$I_{xx}^A = I_{x_A x_A} + A_A(\bar{y}_A)^2$$

$$I_{xx}^A = (11,488.70) + (113.58)(12.04)^2$$

$$I_{xx}^A = 27,955.37 \text{ mm}^4$$

$$I_{y_A y_A} = \frac{b h^3}{12} = \frac{(34.84)(3.26)^3}{12} = 100.58 \text{ mm}^4$$

$$\bar{x}_A = \bar{X}_A - \bar{X} = 1.63 - 6.91 = -5.28 \text{ mm}$$

$$I_{yy}^A = I_{y_A y_A} + A_A(\bar{x}_A)^2$$

$$I_{yy}^A = (100.58) + (113.58)(-5.28)^2$$

$$I_{xy}^A = 3,264.24 \text{ mm}^4$$

$$I_{x_A y_A} = 0 \text{ (symmetric cross-section)}$$

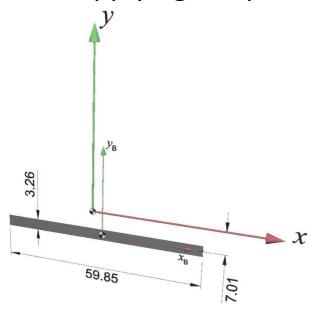
$$I_{xy}^A = I_{x_A y_A} + A_A(\bar{x}_A \bar{y}_A)$$

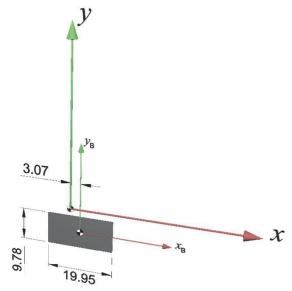
$$I_{xy}^A = 0 + (113.58)(-5.28)(12.04)$$

$$I_{xy}^A = -7,217.67 \text{ mm}^4$$



Now applying the parallel axis theorem for section B:





$$I_{X_B X_B} = \frac{b h^3}{12} = \frac{(3)(19.95)(3.26)^3}{12} = 172.80 \text{ mm}^4$$

$$\bar{y}_B = \bar{Y}_B - \bar{Y} = 1.63 - 8.64 = -7.01 \text{ mm}$$

$$I_{XX}^B = I_{X_B X_B} + A_B(\bar{y}_B)^2$$

$$I_{XX}^B = (172.80) + (3)(65.04)(-7.01)^2$$

$$I_{XX}^B = 9,758.41 \text{ mm}^4$$

$$I_{y_B y_B} = \frac{b h^3}{12} = \frac{(3)(3.26)(19.95)^3}{12} = 6,471,22 \text{ mm}^4$$

$$\bar{x}_B = \bar{X}_B - \bar{X} = 9.98 - 6.91 = 3.07 \text{ mm}$$

$$I_{yy}^B = I_{y_B y_B} + A_B(\bar{x}_B)^2$$

$$I_{yy}^B = (6,471,22) + (3)(65.04)(3.07)^2$$

$$I_{yy}^B = 8,312.85 \text{ mm}^4$$

$$I_{x_B y_B} = 0 \text{ (symmetric cross-section)}$$

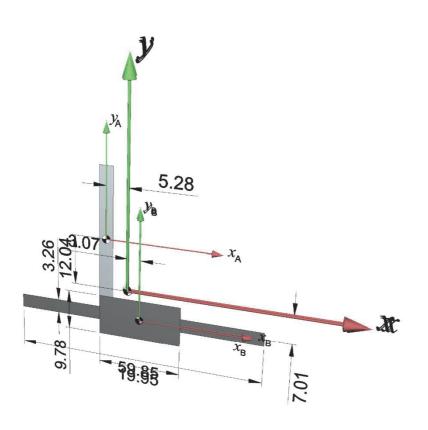
$$I_{xy}^B = I_{x_B y_B} + A_B(\bar{x}_B \bar{y}_B)$$

$$I_{xy}^B = 0 + (3)(65.04)(3.07)(-7.01)$$

$$I_{xy}^B = -4,201.57 \text{ mm}^4$$



• Finally, for the **compound composite** section:



$$I_{xx} = I_{xx}^{A} + I_{xx}^{B}$$

 $I_{xx} = (27,955.37) + (9,758.41)$
 $I_{xx} = 37,713.78 \text{ mm}^{4}$

$$I_{yy} = I_{yy}^{A} + I_{yy}^{B}$$

 $I_{yy} = (3,264.24) + (8,312.85)$
 $I_{yy} = 11,577.10 \text{ mm}^{4}$

$$I_{xy} = I_{xy}^{A} + I_{xy}^{B}$$

 $I_{xy} = (-7,217.67) + (-4,201.57)$
 $I_{xy} = -11,419.24 \text{ mm}^{4}$



• We can now find the principal axes:

$$\theta_{p} = \frac{1}{2}\arctan\left(\frac{2 I_{xy}}{I_{yy} - I_{xx}}\right)$$

$$\theta_{p} = \frac{1}{2}\arctan\left[\frac{2 (-11,419.24)}{(11,577.10) - (37,713.78)}\right]$$

$$\theta_{p} = 20.57^{\circ}$$



And the principal 2nd moments of area are:

$$\begin{cases}
I_{11} \\
I_{22} \\
I_{12}
\end{cases} = \begin{bmatrix}
m^2 & n^2 & -2 m n \\
n^2 & m^2 & 2 m n \\
m n & -m n & m^2 - n^2
\end{bmatrix} \begin{pmatrix}
I_{xx} \\
I_{yy} \\
I_{xy}
\end{pmatrix}$$

• where:
$$m = \cos \theta_{\rm p} = 0.936$$

 $n = \sin \theta_{\rm p} = 0.351$

Using an Excel spreadsheet (see Blackboard) we get:

$$\begin{cases}
 I_{11} \\
 I_{22} \\
 I_{12}
 \end{cases} =
 \begin{cases}
 41,999.99 \\
 7,290.89 \\
 \sim 0
 \end{cases}
 mm4$$



Now we can project the applied load onto our principal axes:

$$\begin{cases}
P_1 \\ P_2
\end{cases} = \begin{bmatrix} m & n \\ -n & m \end{bmatrix} \begin{Bmatrix} P_x \\ P_y
\end{Bmatrix}
\begin{cases}
P_1 \\ P_2
\end{Bmatrix} = \begin{bmatrix} 0.962 & 0.272 \\ -0.272 & 0.962 \end{bmatrix} \begin{Bmatrix} 0 \\ -19.62 \end{Bmatrix} N
\begin{cases}
P_1 \\ P_2
\end{Bmatrix} = \begin{Bmatrix} -6.90 \\ -18.37 \end{Bmatrix} N$$

Applying the tip deflection formula:

$$\delta = \frac{P L^3}{3 EI}$$

$$\delta_1 = \frac{(-6.90)(1000)^3}{3(70.000)(7.290.88)} \qquad \delta_2 = \frac{(-18.37)(1000)^3}{3(70.000)(41.999.99)}$$

$$\delta_1 = -4.50 \text{ mm}$$

$$\delta_2 = -2.08 \,\mathrm{mm}$$



• Finally, transform deflections from principal axes (1,2) back to our reference axes (x,y) through rotation by $(-\theta_p)$:

$$\begin{cases}
\delta_{x} \\
\delta_{y}
\end{cases} = \begin{bmatrix}
m & n \\
-n & m
\end{bmatrix} \begin{cases}
\delta_{1} \\
\delta_{2}
\end{cases}
\qquad m = \cos(-\theta_{p}) \\
n = \sin(-\theta_{p}) \qquad 2$$

$$\begin{cases}
 \delta_x \\
 \delta_y
 \end{cases} =
 \begin{bmatrix}
 0.962 & -0.272 \\
 0.272 & 0.962
 \end{bmatrix}
 \begin{cases}
 -4.50 \\
 -2.08
 \end{cases}
 mm$$

