

Energy Methods for Pin-Jointed Trusses

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23 January 2018

1. Introduction: Strain energy

2. Castigliano's theorems

2.1 Key concepts

2.2 Application: Displacement due to an external force Q

2.3 Application: Displacements at any joint (virtual force method)

3. Principle of Stationary Potential Energy (PSPE)

3.1 Key concepts

3.2 Application: Internal forces in statically indeterminate trusses

- **Energy methods** are alternatives to the ‘conventional method’ of solving forces and displacements under static equilibrium
 - Energy (positive scalar) is a function of both forces and displacements (which are vector fields)
- They enable the solution of forces and displacements in structures which cannot be analysed by **equations of static equilibrium alone** (*e.g.* statically indeterminate problems)
- Examples:
 - Castigliano’s Theorems
 - Principle of Stationary Potential Energy (PSPE)
 - Principle of Virtual Work (Virtual Displacements or Virtual Forces)
 - *Etc.*

Conventional method:

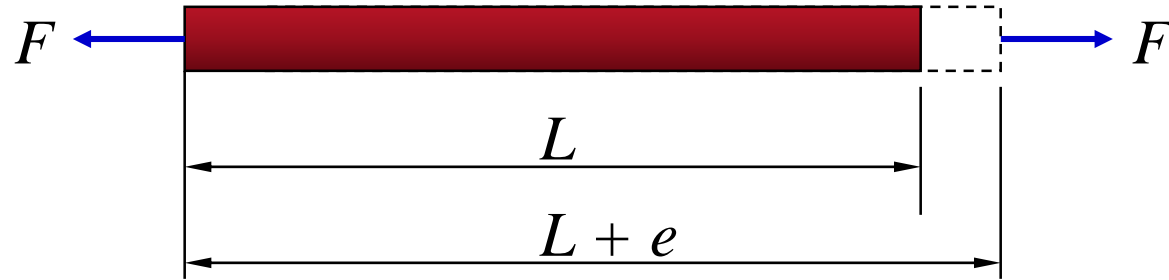
- Equations of static equilibrium
- Compatibility of displacements
- Constitutive relations
 - Force-displacement $F = k e$ or stress-strain $\sigma = E \varepsilon$

Energy methods:

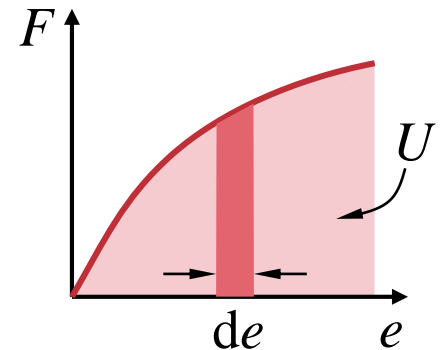
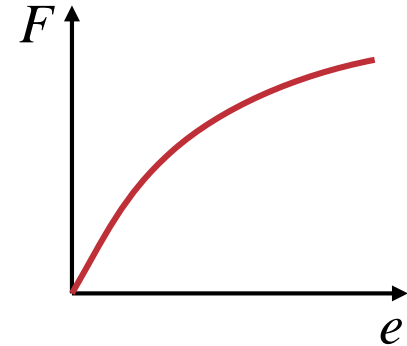
- Equations of static equilibrium
- Strain energy theorems

- Can be applied to pin-jointed trusses, beams, rigid-jointed frames
- Convenient means of analysing **statically indeterminate structures**
- Simple solution of deflections
- Results in rapid approximate numerical solutions
 - For problems which do not have exact closed-form analytical solutions, *e.g.* complex geometries, nonlinearities *etc.*

- Consider an axially-loaded member:



- Assuming a non-linear elastic material:
- Remember: work = force \times distance
- For this 'nonlinear spring':
$$U = \int_0^e F \, de$$
- i.e.* strain energy = area under the F - e curve



Energy Methods

Castigliano's Theorems

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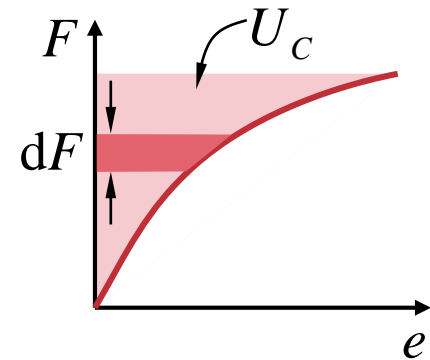
3. Principle of Stationary Potential Energy (PSPE)

3.1 Key concepts

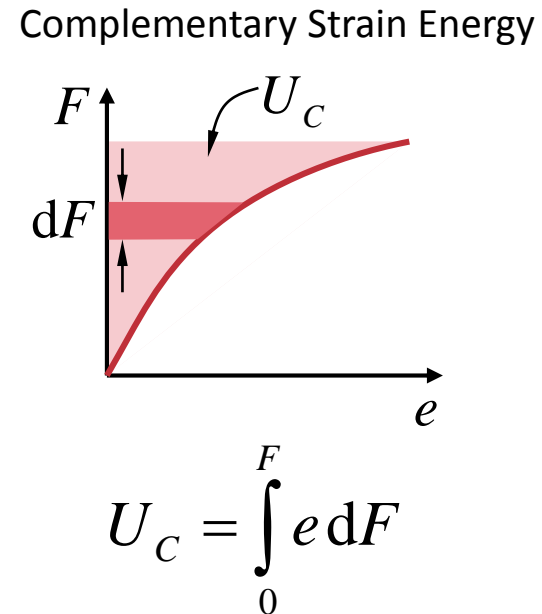
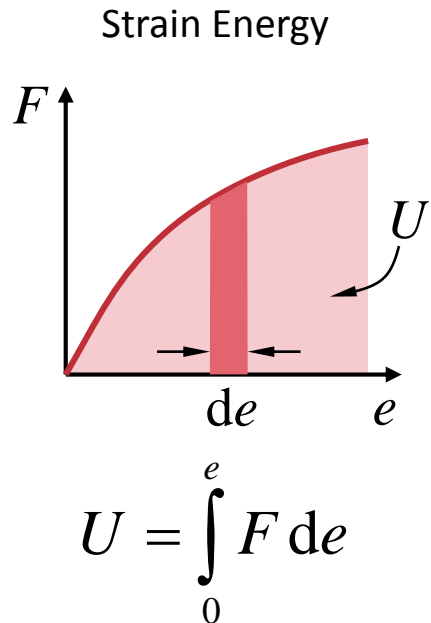
3.2 Application: Internal forces in statically indeterminate trusses

- Consider now the area **above** the F - e curve:
- This is the **complementary strain energy**:

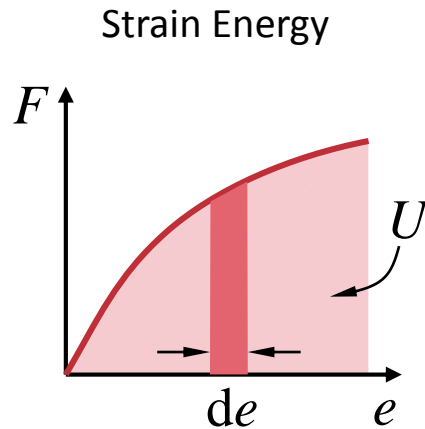
$$U_c = \int_0^F e \, dF$$



- Note that for a non-linear elastic material $U_c \neq U$



- Differentiating strain energy and complementary strain energy:

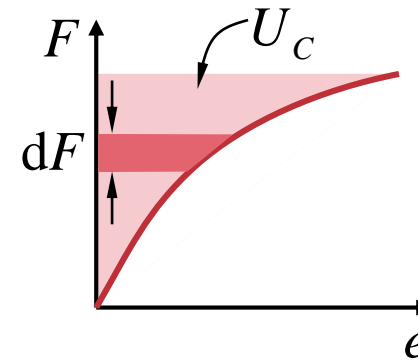


$$U = \int_0^e F de \rightarrow \frac{\partial U}{\partial e} = F$$

The derivative of strain energy w.r.t. deflection gives the force causing this deflection

→ **Castigliano's 1st Theorem**

Complementary Strain Energy



$$U_c = \int_0^F e dF \rightarrow \frac{\partial U_c}{\partial F} = e$$

The derivative of complementary strain energy w.r.t. force gives the deflection at that particular point

→ **Castigliano's 2nd Theorem**

- These theorems are after Carlo Alberto Castigliano (1847–1884, Milan) who proposed these in 1873 (at the age of 25)

- Note that for a **linear elastic material**:

$$F = k e \quad \therefore e = \frac{F}{k}$$

$$U = \int_0^e F \, de$$

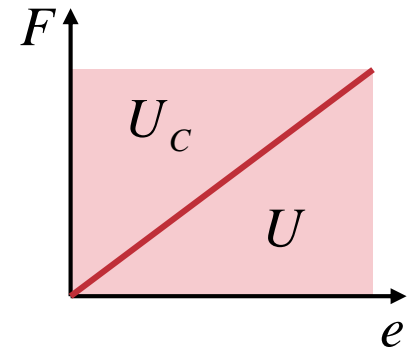
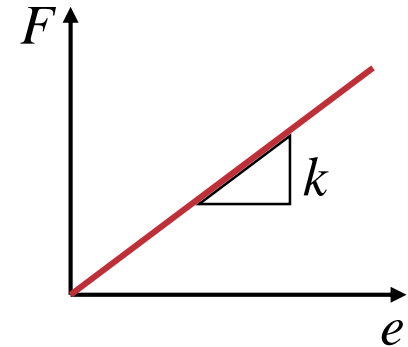
$$= \int_0^e k e \, de$$

$$= \frac{k e^2}{2}$$

$$U_C = \int_0^F e \, dF$$

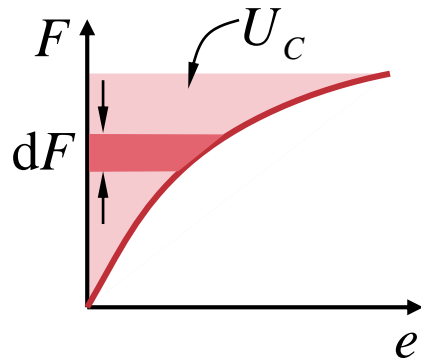
$$= \int_0^F \frac{F}{k} \, dF$$

$$= \frac{F^2}{2k} = \frac{k e^2}{2}$$



- Therefore **for linear elastic materials** $U_C = U$
- i.e.* U_C and U become interchangeable

Complementary Strain Energy



$$U_c = \int_0^F e \, dF \rightarrow \frac{\partial U_c}{\partial F} = e$$

For a truss made of axial members:

$$(U_c)_i = \frac{1}{2} F_i e_i = \frac{1}{2} F_i \left(\frac{F_i}{k_i} \right) = \frac{1}{2} \frac{F_i^2}{k_i} \quad \therefore \quad U_c = \sum \left(\frac{1}{2} \frac{F_i^2}{k_i} \right)$$

$$\frac{\partial U_c}{\partial Q} = \sum \left(\frac{\partial U_c}{\partial F_i} \cdot \frac{\partial F_i}{\partial Q} \right) = e$$

A graph with Force F on the vertical axis and Strain e on the horizontal axis. A red straight line starts at the origin. The triangular area under this line is shaded red and labeled U .

Finally:

$$\frac{\partial U_c}{\partial Q} = \sum \left(\frac{F_i L_i}{A_i E_i} \cdot \frac{\partial F_i}{\partial Q} \right) = e$$

where: $\frac{\partial F_i}{\partial Q} = F_i'$

Most common problem: for a statically determinate or indeterminate truss where the internal forces F_i are **known**, find the **displacements** at the joint where the known external force Q is applied

1. Tabulate the values of F_i , L_i , A_i and E_i
2. Find the derivatives $F_i' = \frac{\partial F_i}{\partial Q}$
 - Since F_i and Q are constants, simply divide one by the other
3. Perform the summation $\sum \left(\frac{F_i L_i}{A_i E_i} \cdot \frac{\partial F_i}{\partial Q} \right) = e$ to find the displacement e
4. Note that the direction and sense of e are relative to the force vector Q !

General procedure: For a statically determinate or indeterminate truss where the internal forces F_i are **known**, find the **displacements at any joint**

1. Tabulate the values of F_i , L_i , A_i and E_i
2. Go back to the truss and temporarily remove the external force Q
3. Now introduce a unit force P (a.k.a. a **virtual force**) at the joint of interest and pointing in the direction of interest
4. Using the **method of joints** or **method of sections**, find the **virtual internal forces F_i'** due to the virtual force P
5. Perform the summation
$$\sum \left(\frac{F_i L_i}{A_i E_i} \cdot F_i' \right) = e_P$$
 - The displacement e_P is the real displacement at the joint where the virtual force P was applied