

Advanced Bending and Torsion

Shear Stresses in Solid Section Beams

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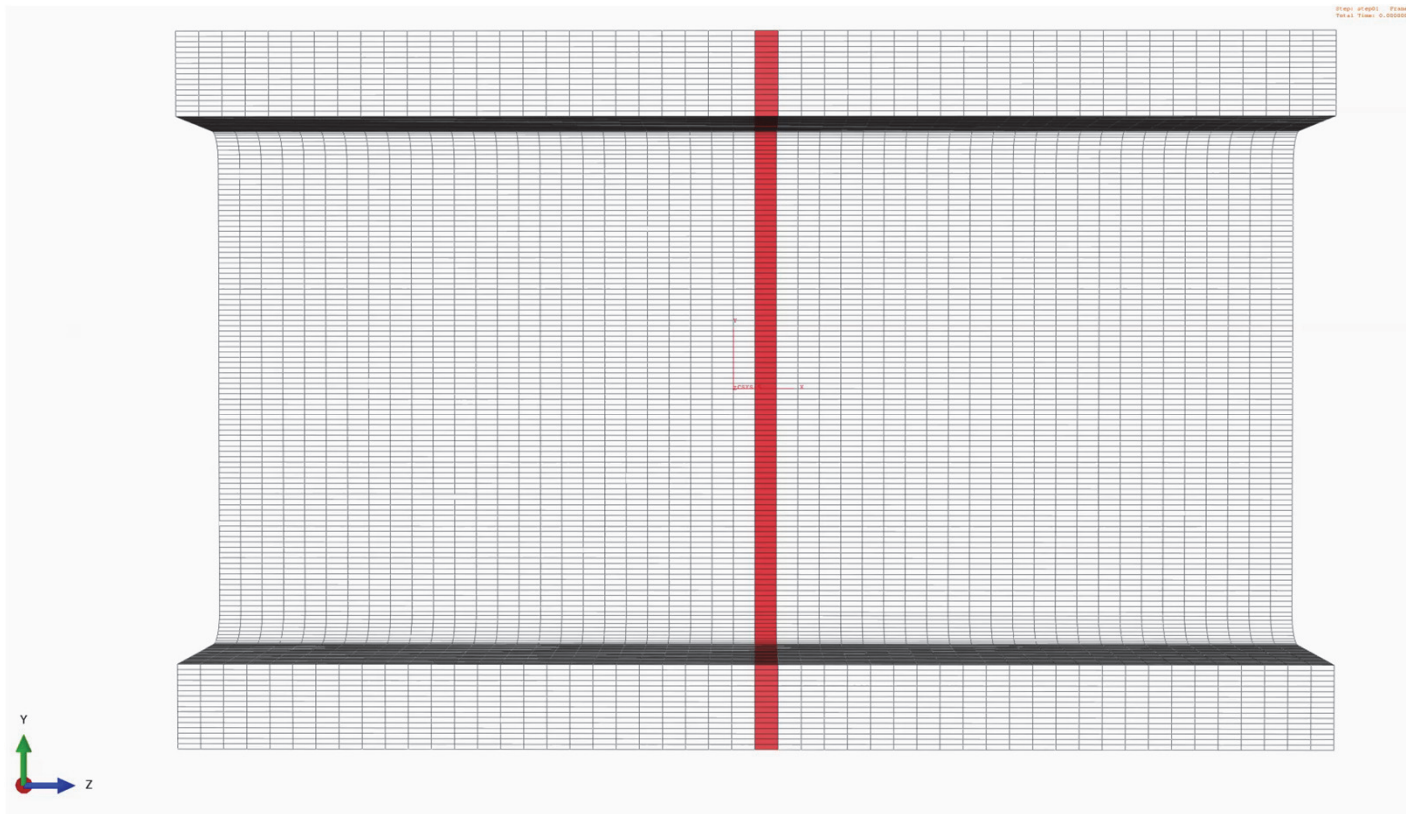
1. Direct stresses in beams

1. Off-axis loading of symmetric cross-sections
2. Transformation of bending axes
3. Unsymmetric cross-sections
4. Composite (multi-material) beams

2. Shear stresses in beams

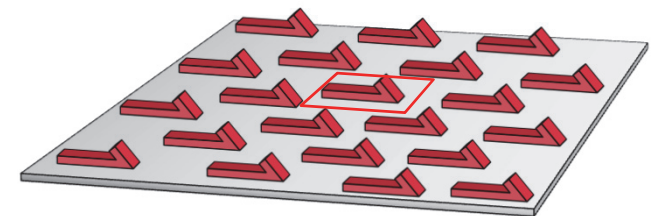
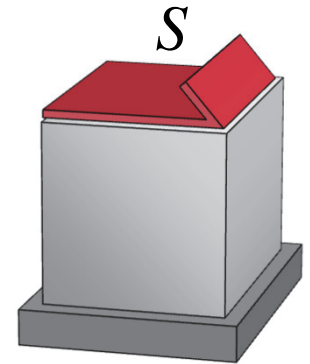
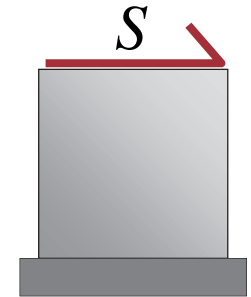
1. Solid cross-sections
2. Thin-walled open cross-sections
3. Thin-walled closed cross-sections

- All bending analyses so far assumed that:
 - Plane sections remain plane
 - Shear deformation is negligible
- In real life cross-sections **warp** due to shear stresses
 - Assuming linear elasticity, let us look first at **shear strains**:



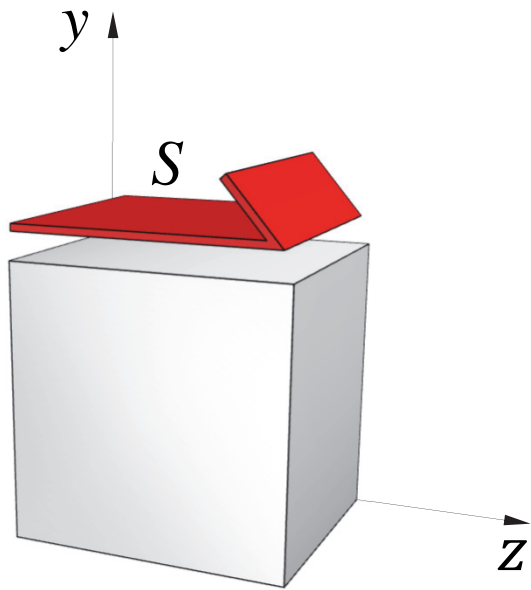
- Consider a cube element of material subject to a 'sliding force' (*i.e.* a force tangential to the surface) of intensity S
- The **shear stress** τ is a measure of 'force per unit area' where the force is tangential to the surface
- It is a field property like the 'direct stress' σ ; it can vary continuously within a body and can be considered at a point:

$$\tau = \lim_{\delta A \rightarrow 0} \frac{\delta S}{\delta A} \quad \tau = \text{'tau'}$$

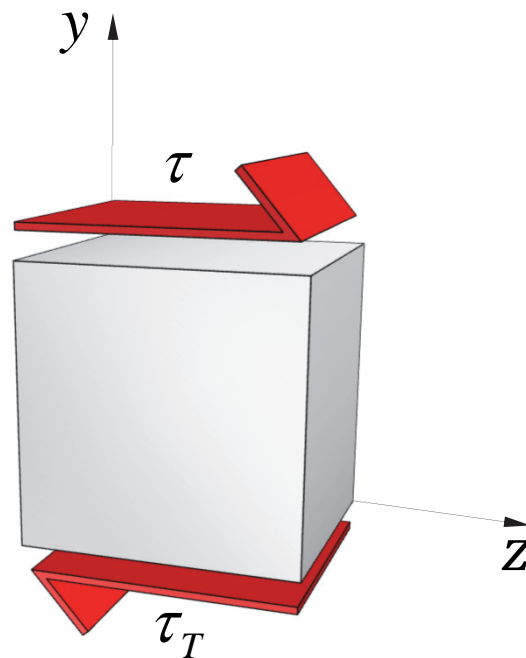


$$1 \text{ Pa} = 1 \frac{\text{N}}{\text{m}^2} = 10^{-6} \text{ MPa} = 10^{-6} \frac{\text{N}}{\text{mm}^2}$$

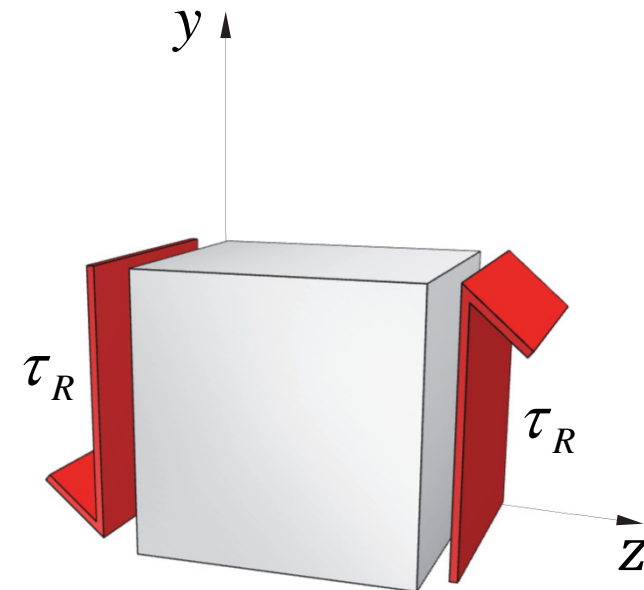
- For equilibrium, **complementary shear stresses** must exist to balance translational and rotational tendencies



Applied shear force

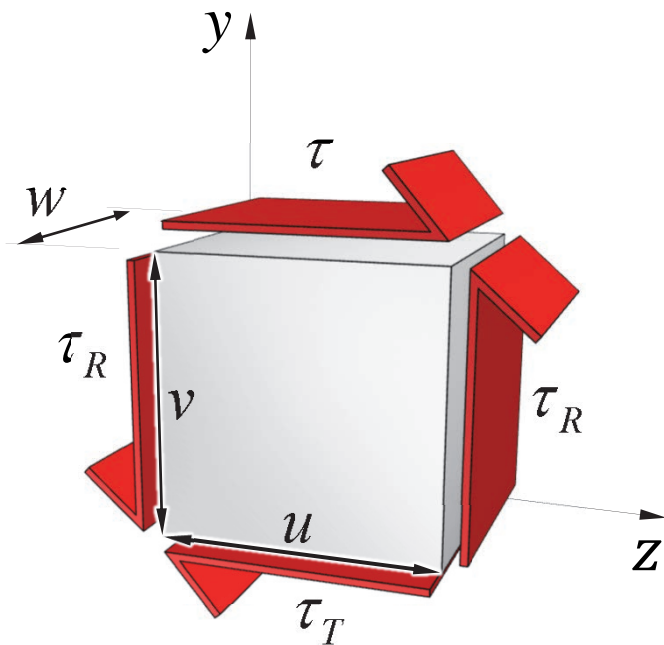


Shear stresses for translational balance



Shear stresses for rotational balance

- In order to balance translational and rotational tendencies the magnitudes of the shear stress components are related:



$$\sum F_z = 0 \quad \leftarrow \text{Translational equilibrium along axis } z$$

$$\tau(uw) - \tau_T(uw) = 0$$

$$\tau = \tau_T$$

$$\sum M_x = 0 \quad \leftarrow \text{Rotational equilibrium about axis } x$$

$$\tau(uw)(v) - \tau_R(vw)(u) = 0$$

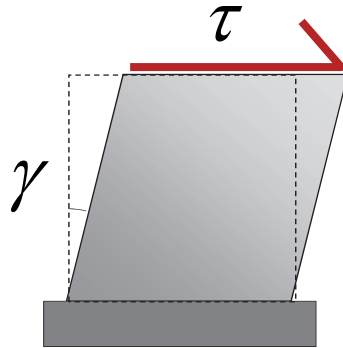
$$\tau = \tau_R$$

- i.e.* all complementary stresses are equal

- **Shear strain γ** : angular rotation in radians (non-dimensional)

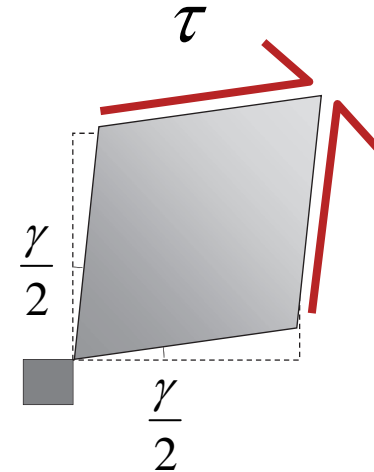
Simple Shear

'Element fixed along an edge'



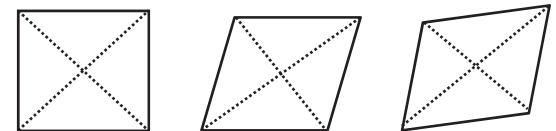
Pure Shear

'Element fixed at a corner'



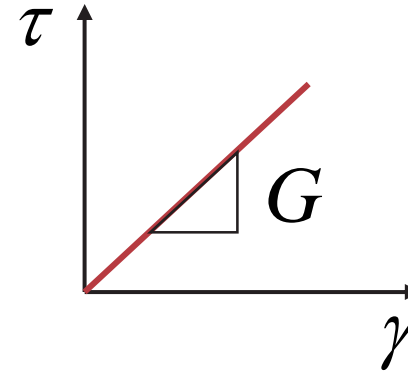
γ = 'gamma'

- Element 'edges' (or 'shear planes') do not change length but simply translate or rotate
- Element 'diagonals' **do** change length:
 - *i.e.* shear = diagonal 'tension' and 'compression'



- For linear elastic behaviour shear stress is proportional to shear strain

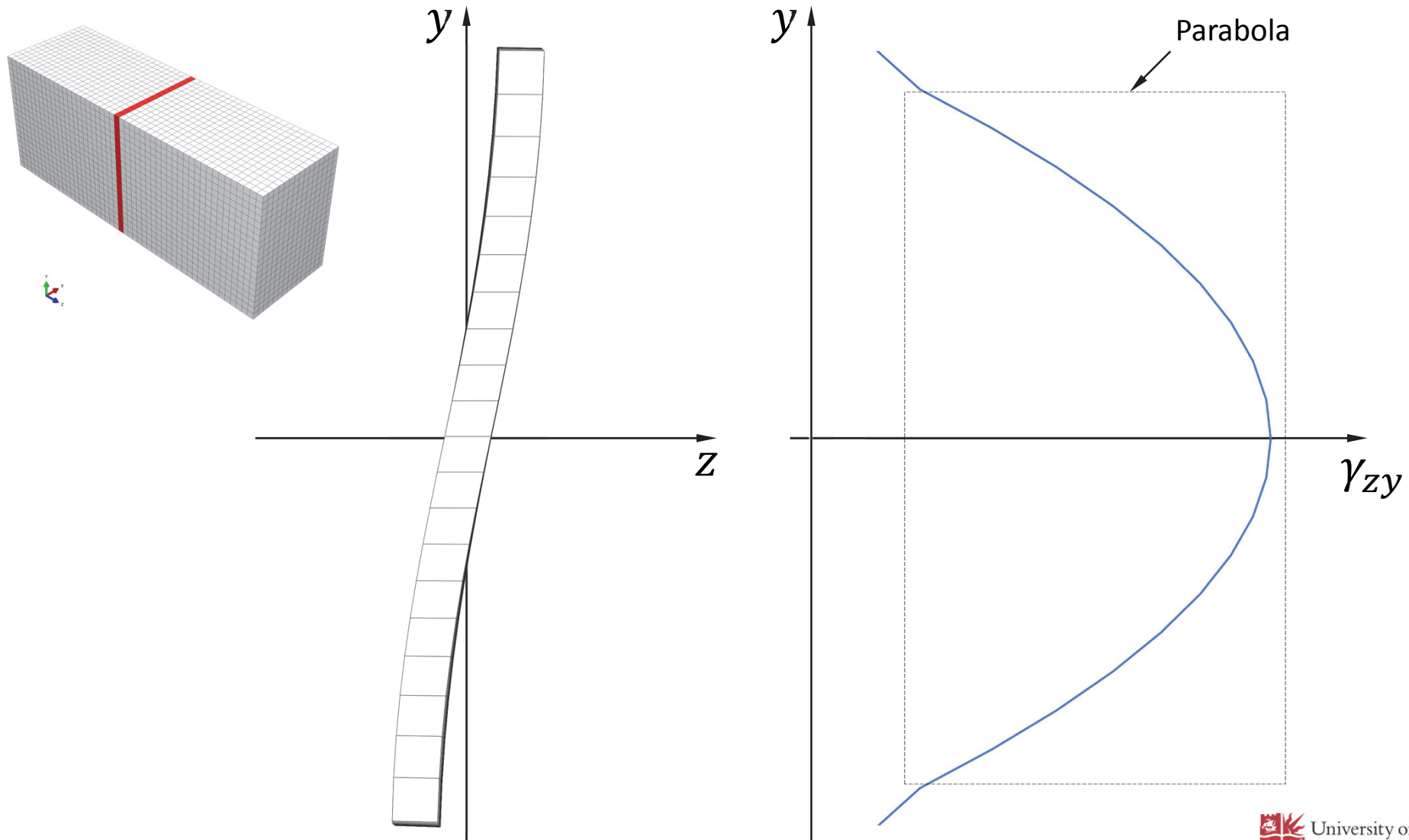
$$\tau = G \gamma$$



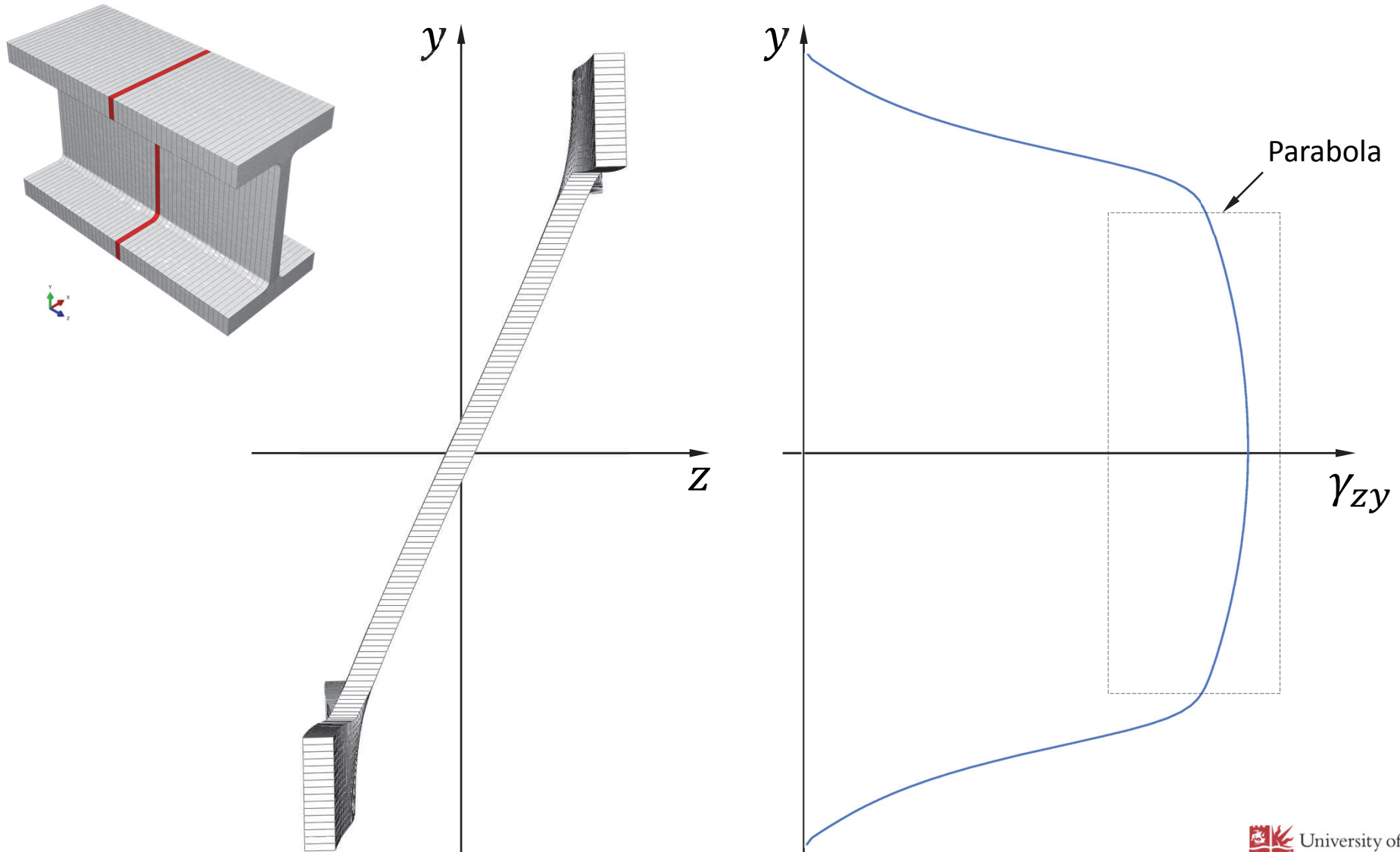
- Where the proportional constant G is the **Shear Modulus**
 - This is a **material property** like Young's modulus E or Poisson's ratio ν
 - In fact, for **isotropic materials** these three properties obey a very simple relationship:

$$G = \frac{E}{2(1 + \nu)}$$

- Shear strains are **angles** which can easily be visualised in FE models, *e.g.* for a solid rectangular cross-section:



- And the I-section beam seen earlier:



- Shear force is the first derivative of the bending moment:

$$S = -\frac{dM}{dz}$$

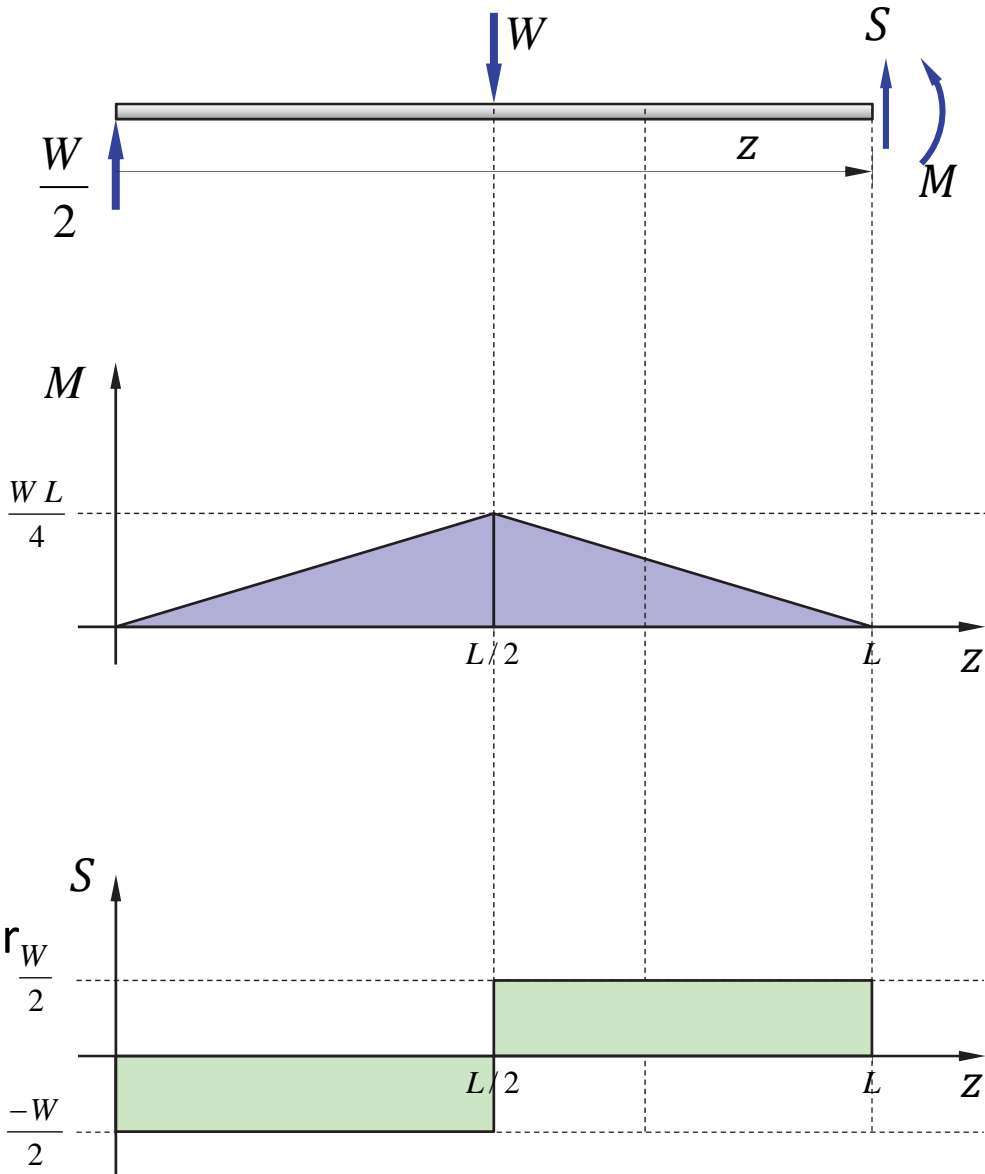
– *i.e.* $-S$ is the ‘**slope**’ of M

- Conversely, bending moments are the integral of the shear forces:

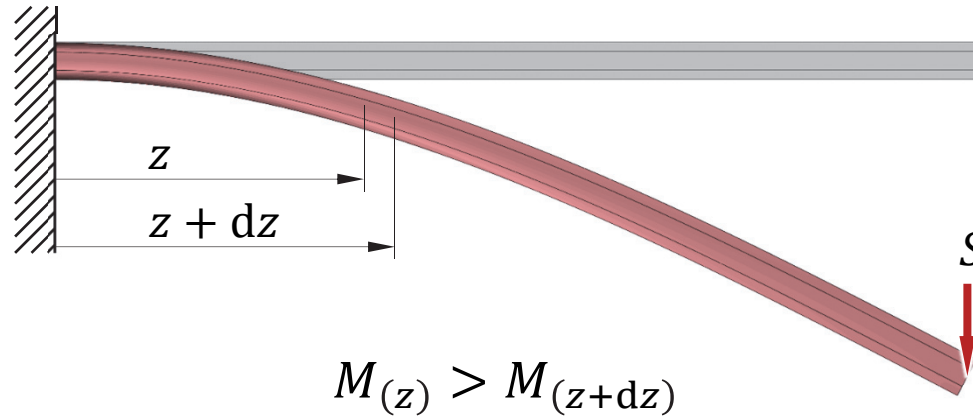
$$M = -\int_0^z S \, dz$$

– *i.e.* $-M$ is the ‘**cumulative area**’ under the graph of S

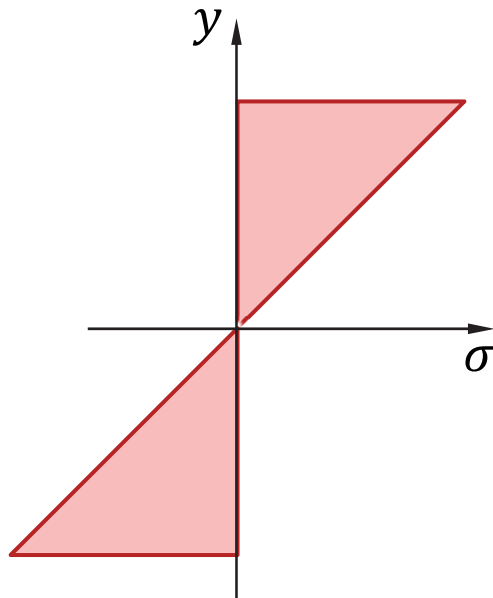
Note: the ‘minus’ signs in the equations above ‘appear/disappear’ depending on the adopted sign convention



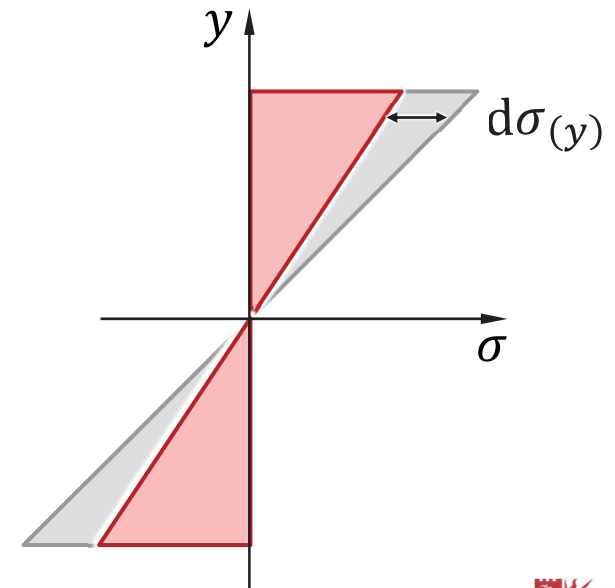
- Remember that stresses σ vary along z in the presence of shear forces:



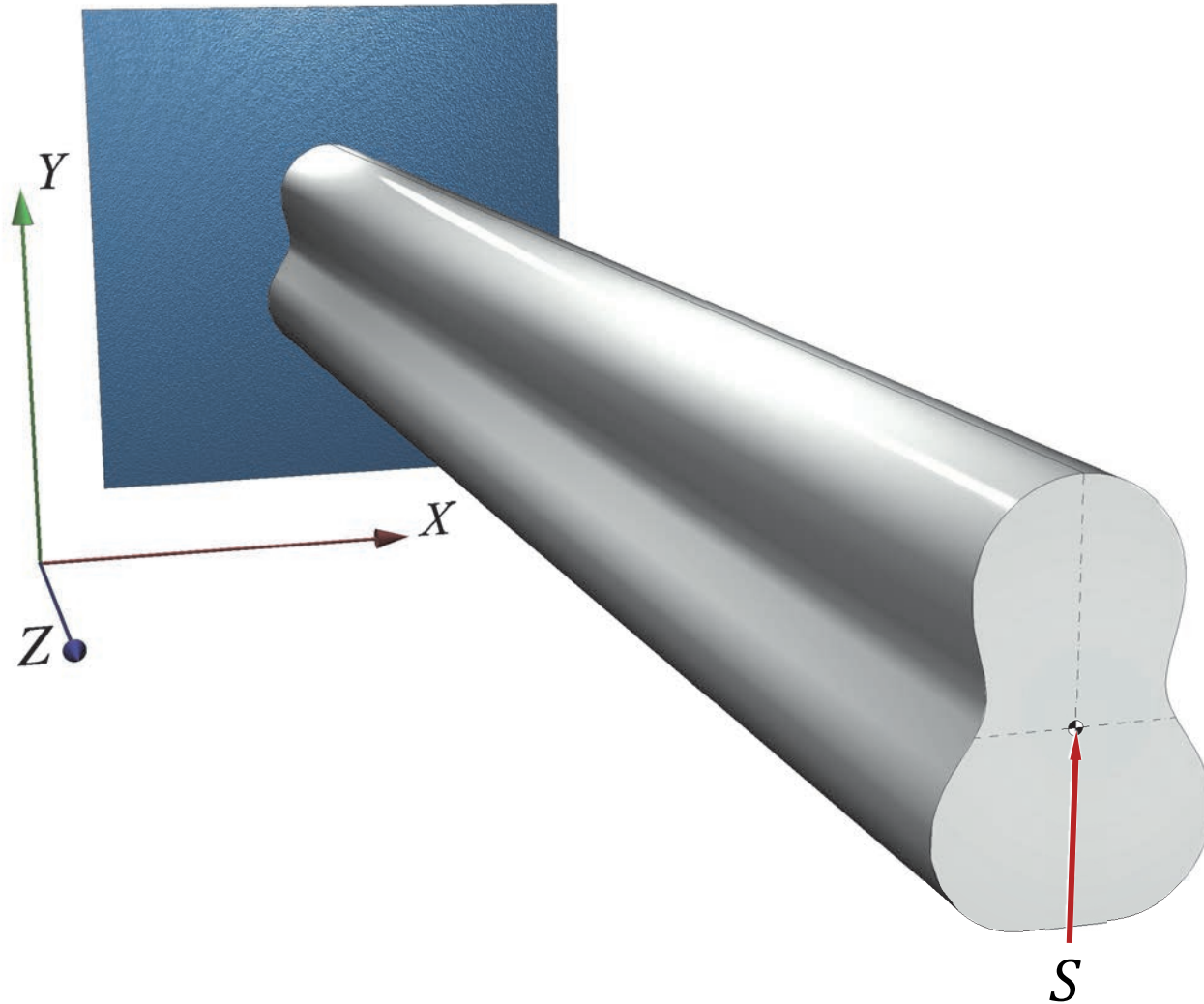
Stress distribution at z



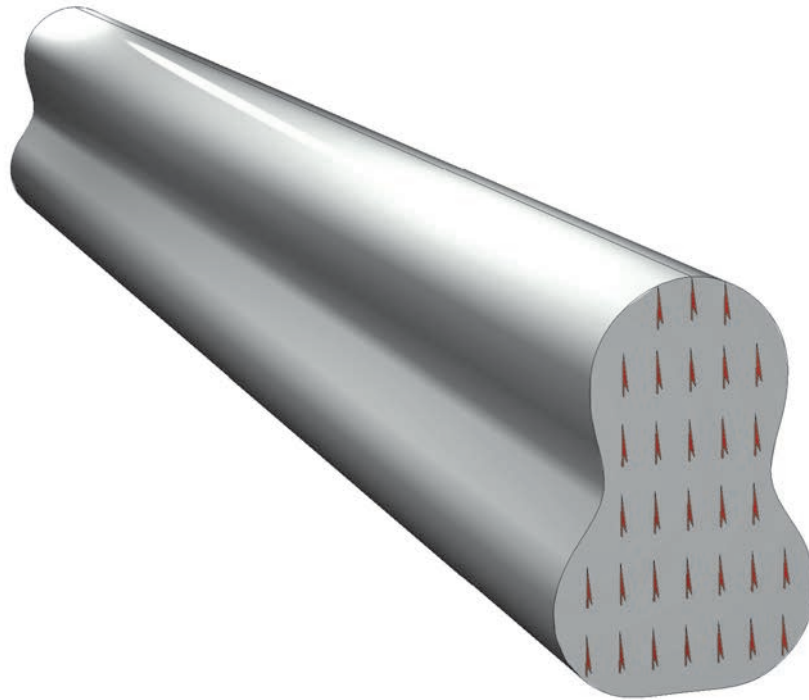
Stress distribution at $z + dz$



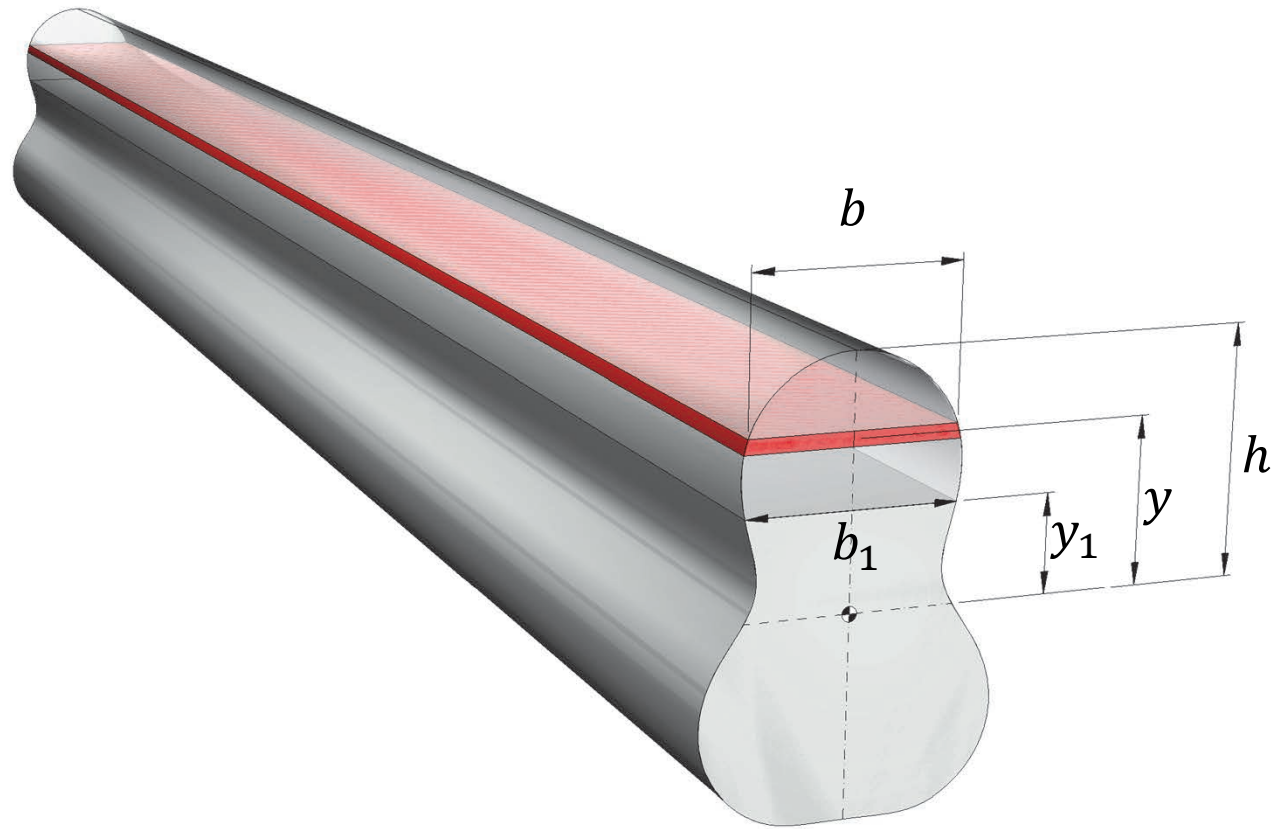
- Consider an arbitrary solid cross-section subjected to a **shear force S** along a **principal axis**:



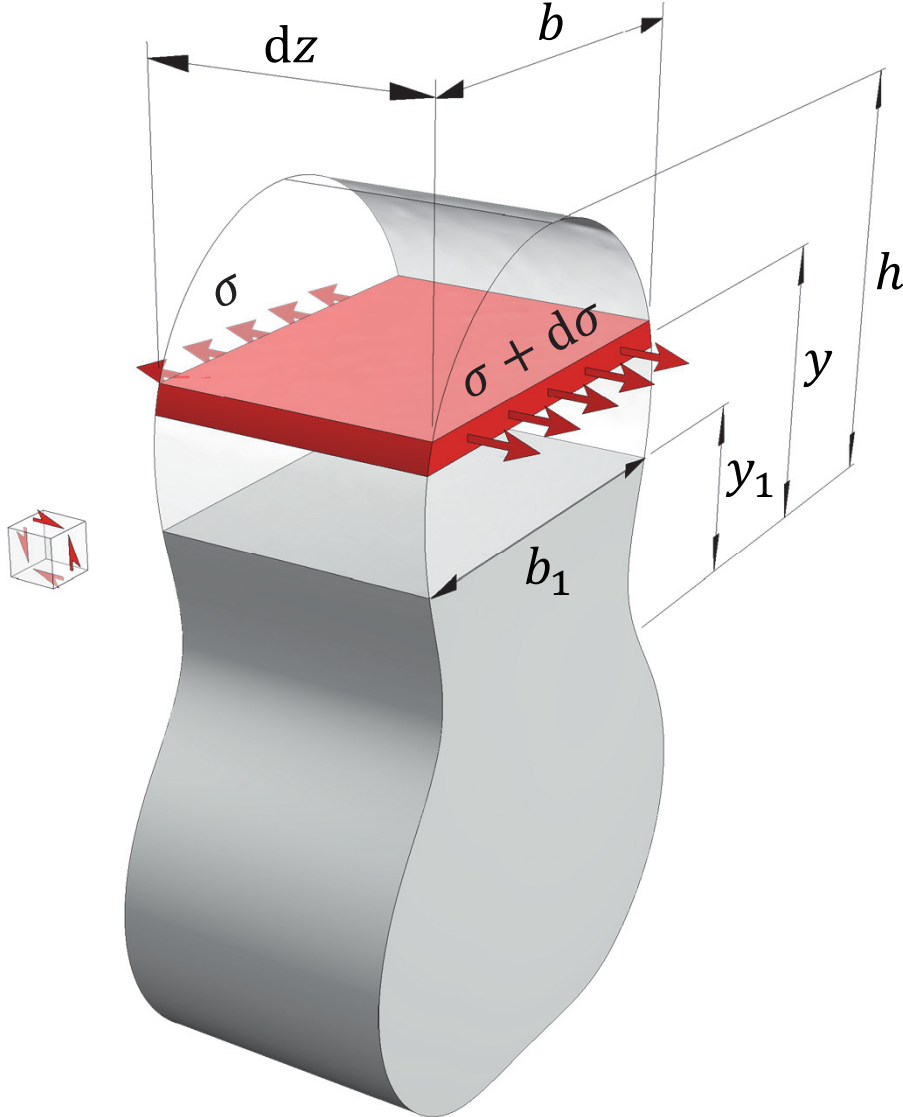
- This shear force is 'transmitted' along the beam in the form of shear stresses



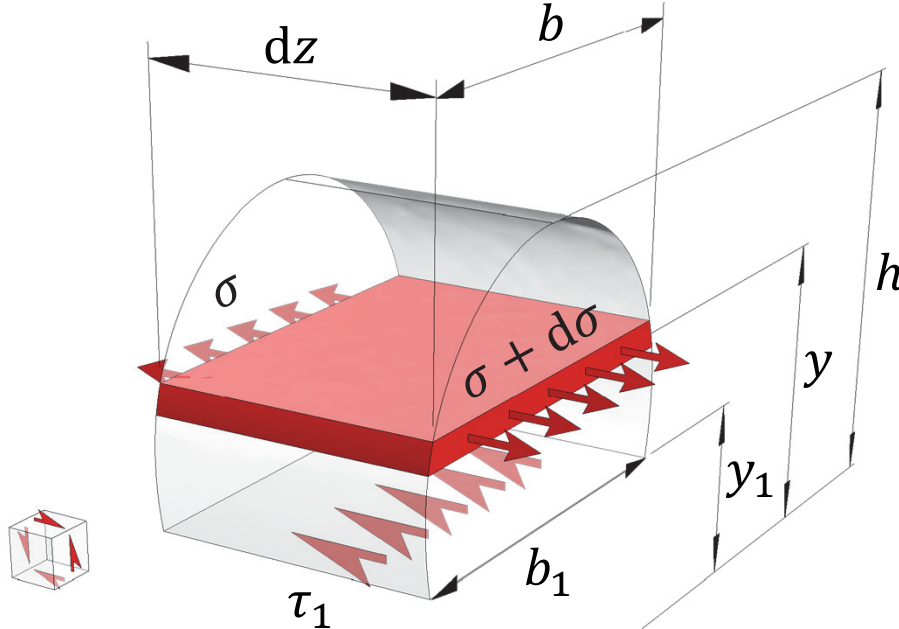
- We 'slice' the beam transversely to the loading direction and apply equilibrium of internal forces:



- Equilibrium of internal forces for the red slice of thickness dy :



- Equilibrium of internal forces for the red slice of thickness dy :



shear force longitudinal force

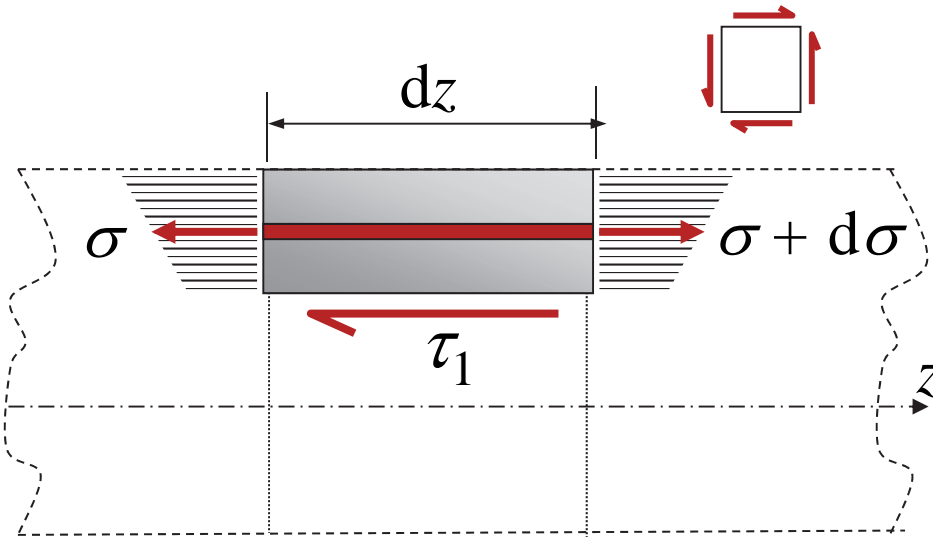
$$\tau_1 b_1 dz = \int_{y_1}^h d\sigma b dy$$

bending formula: $d\sigma = \frac{dM}{I} y$

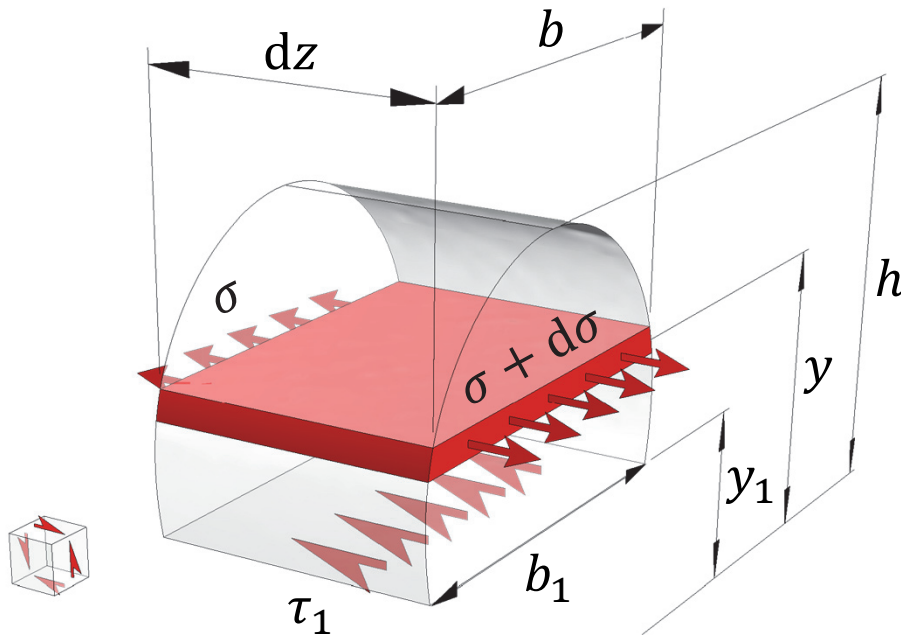
$$\tau_1 = \frac{dM}{dz} \frac{1}{I b_1} \int_{y_1}^h y dA \quad \leftarrow b dy = dA$$

differential beam equations: $\frac{dM}{dz} = S$

$$\tau_1 = \frac{S}{I b_1} \int_{y_1}^h y dA = \frac{S Q_{xx,1}}{I b_1}$$



- Beam shear formula:

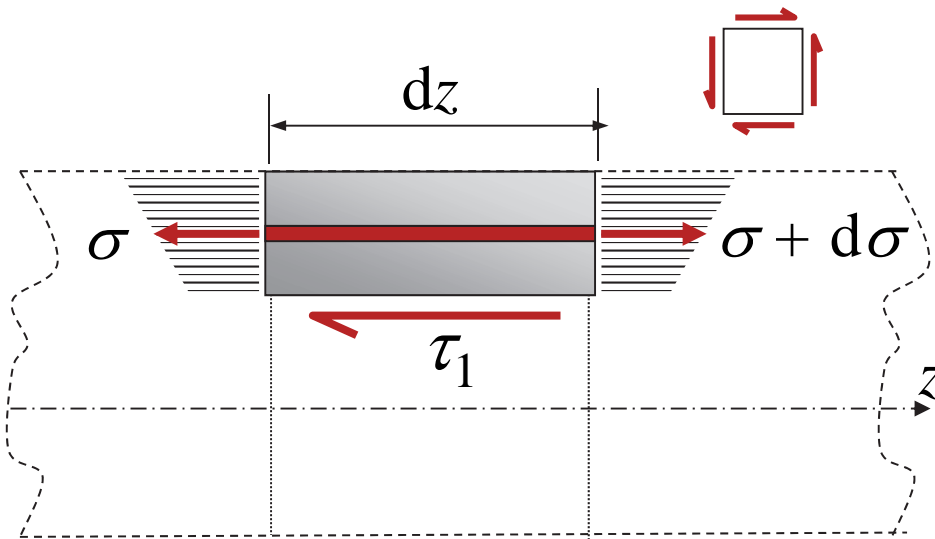


$$\tau_1 = \frac{S}{I b_1} \int_{y_1}^h y \, dA = \frac{S Q_{xx,1}}{I b_1}$$

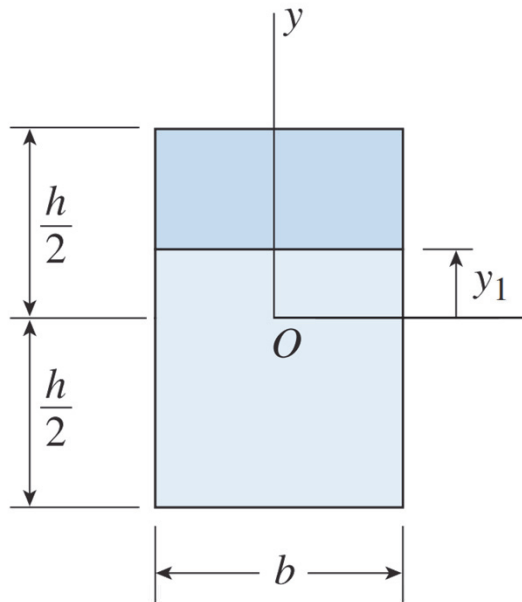
Note: the first moment of area $Q_{xx,1}$ is maximum when $y_1 = 0$, i.e. at the neutral axis

For compound (discrete) sections we have:

$$\tau_1 = \frac{S}{I b_1} \sum A_i \bar{y}_i$$



- For a solid rectangular cross-section we have:



$$Q_{xx,1} = b \left(\frac{h}{2} - y_1 \right) \left(y_1 \frac{\frac{h}{2} - y_1}{2} \right)$$

$$Q_{xx,1} = \frac{b}{2} \left(\frac{h^2}{4} - y_1^2 \right)$$

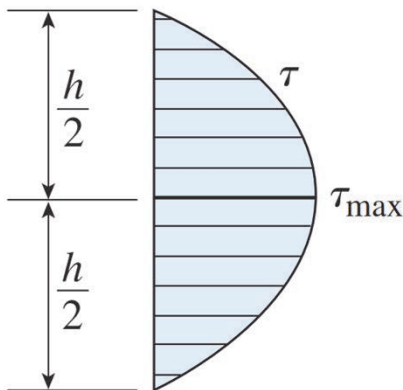
Alternatively:

$$Q_{xx,1} = \int_{y_1}^h y \, dA = \int_{y_1}^h y \, b \, dy$$

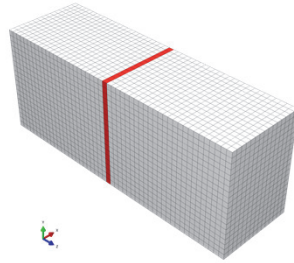
$$Q_{xx,1} = \frac{b}{2} \left(\frac{h^2}{4} - y_1^2 \right)$$

Substituting:

$$\tau_1 = \frac{S}{2I} \left(\frac{h^2}{4} - y_1^2 \right)$$



- The shear formula above is a good approximation to real shear stress fields:



- However it neglects 3D effects and Poisson's effect:

