

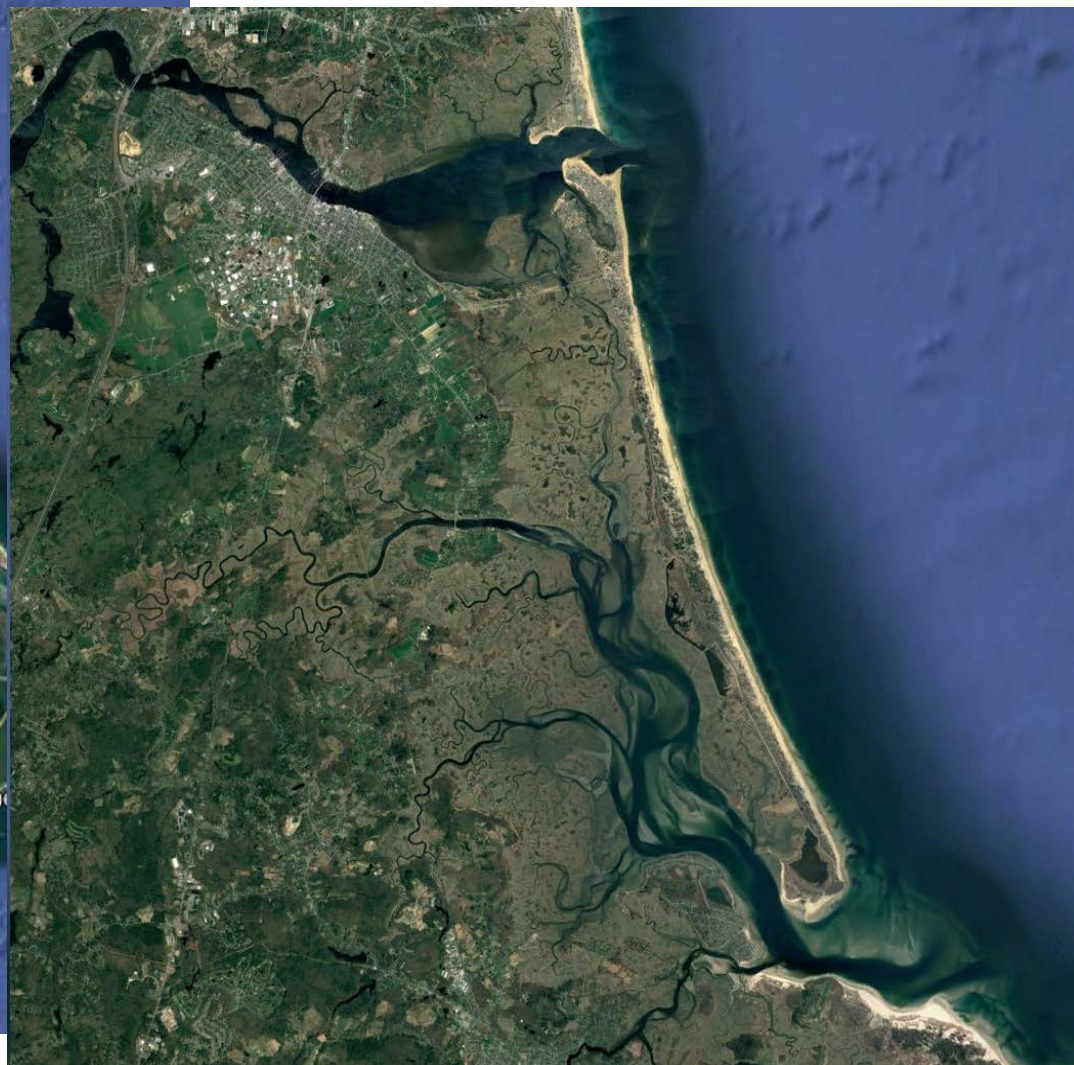
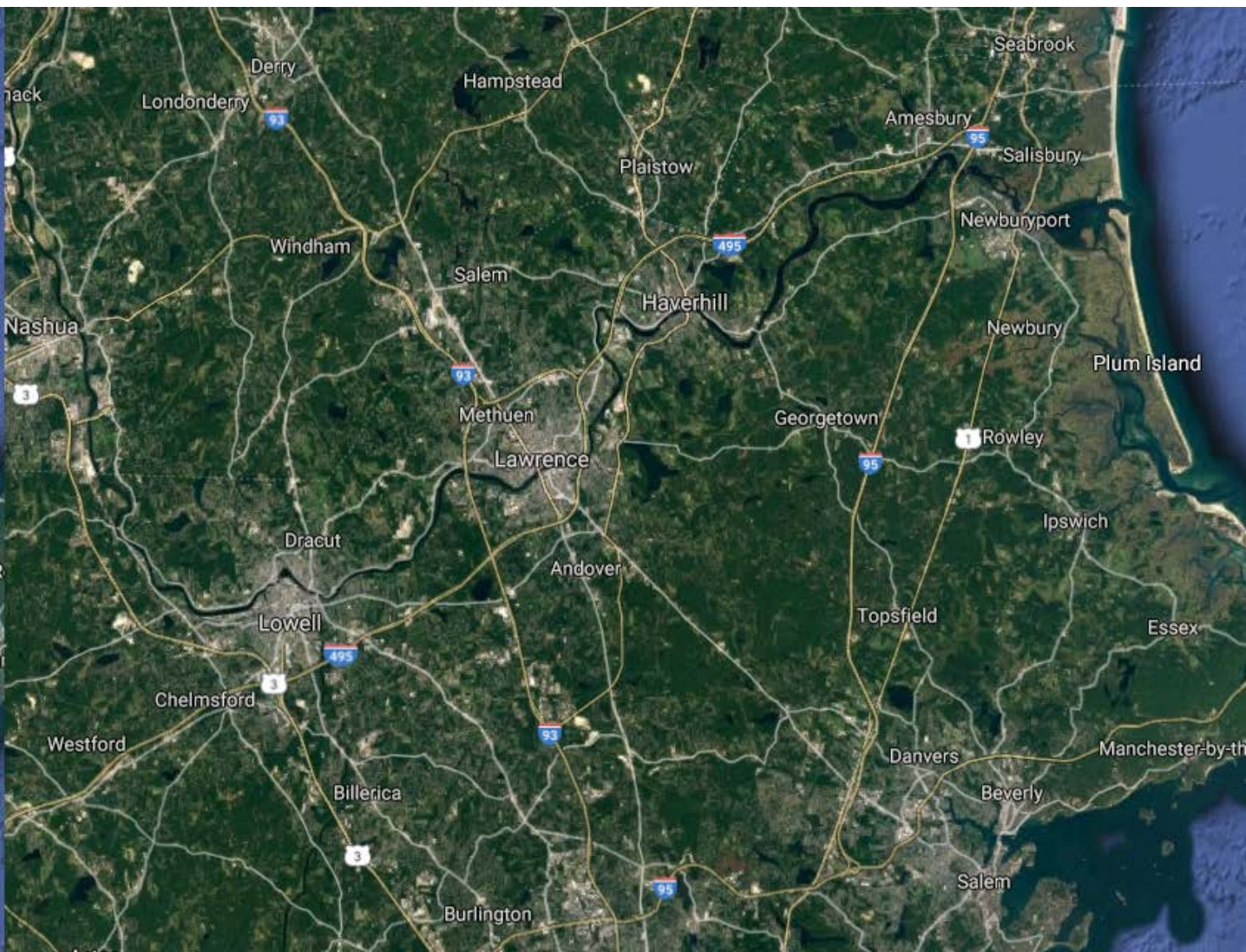
## Today's agenda

- Aquifer Projects
- Review land subsidence
- Where water comes from
- Pump tests!
  - Basic assumptions
  - Pumping/hydraulic tests and analytical solutions for flow to a well

Group	Member 1 name	Member 2 name	Member 3 name	Aquifer Name	Presentation date (April 29 or May 1)
1	Maura Gould	Victoria Bettuelli	-	Ogallala	1-May
2	Kayla Gusikoff	Leidy Baxley	-	Nubian Sandstone aquifer	
3	Tyler Lagasse	Sarah Miller	-	New York & New England	
4	Kerry McNally	Lisa Lunchford	-	Upper Rhine Aquifer	
5	Jessica Berozsky <3	Ryan Knych <3	-	Gulf Coast Aquifer	
6	Emma Daly <3	Alexsia Khim	-	Guarani Aquifer	1-May
7	Erica Boudreau	Michael Turner	-	Sand Pass Divide Well, NV	
8	Christopher Leary	-	-	Basin & Range, AZ	1-May
9	Michael Mirakian	Patrick Hutchinson	Lisa Tuggle	Columbia platau aquifer	

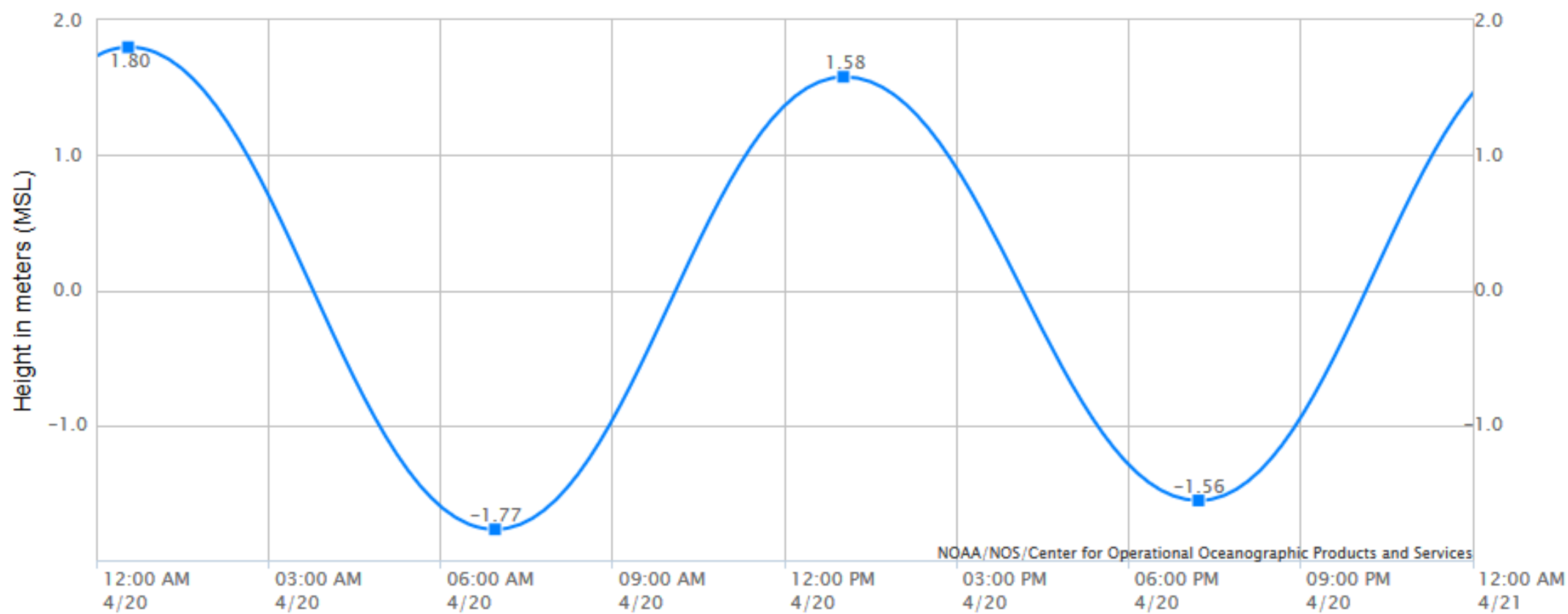
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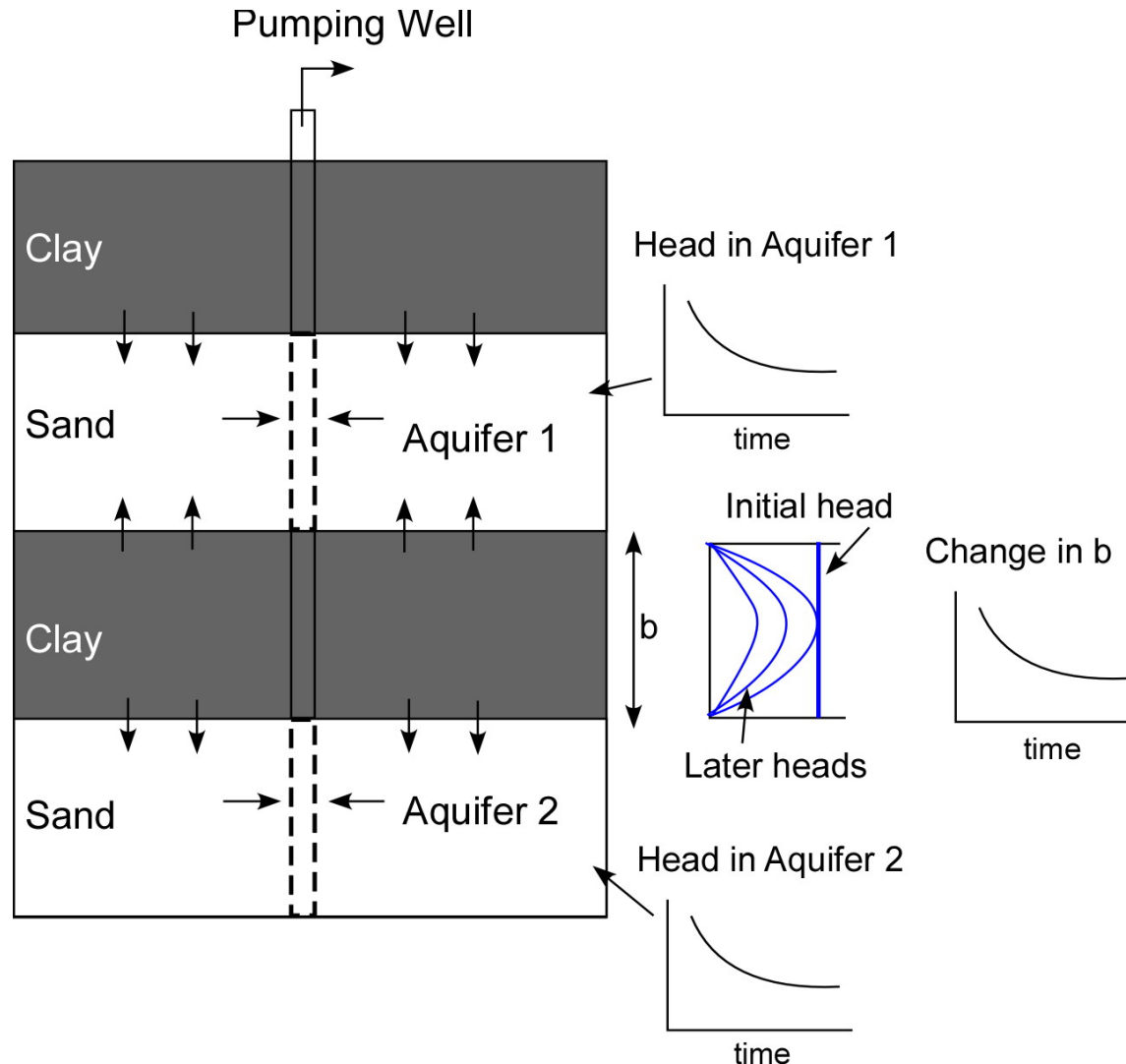




NOAA/NOS/CO-OPS  
Tide Predictions at 8441241, Plum Island South MA  
From 2019/04/20 12:00 AM LST/LDT to 2019/04/20 11:59 PM LST/LDT



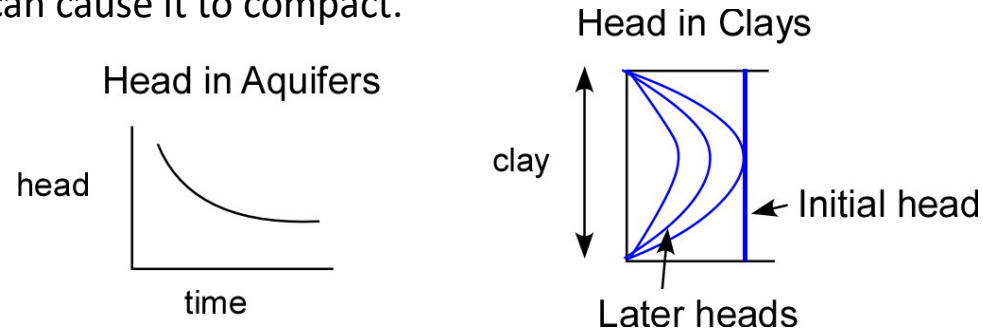
## Basics of Land Subsidence:



- Wells are screened in sand aquifers, extracting water lowers the heads in the sand
- Compressibility of clay is 1-2 orders of magnitude greater than that of sand. For a clay,  $\alpha \gg \eta\beta$
- $K$  is much lower for clay than for sand, so drainage and compaction are much slower for confining units than for aquifers.
- **Aquifer drainage leads to compaction of aquitards.**

# Basics of Land Subsidence:

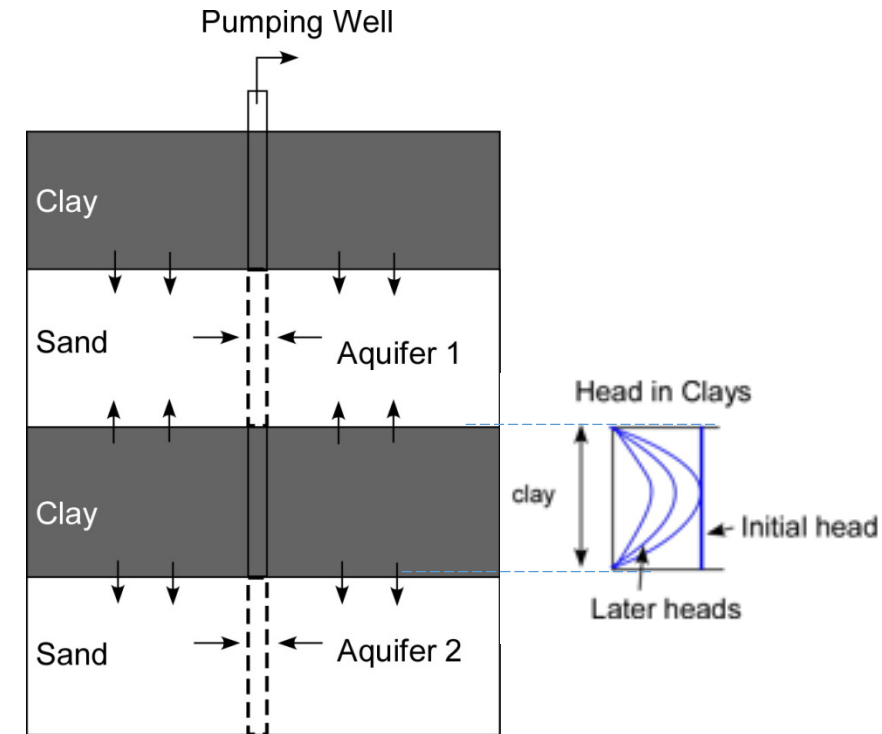
When pumping occurs, there is a decrease in pressure in both the aquifer and the clay beds. The increase in effective stress acting on the clay can cause it to compact.

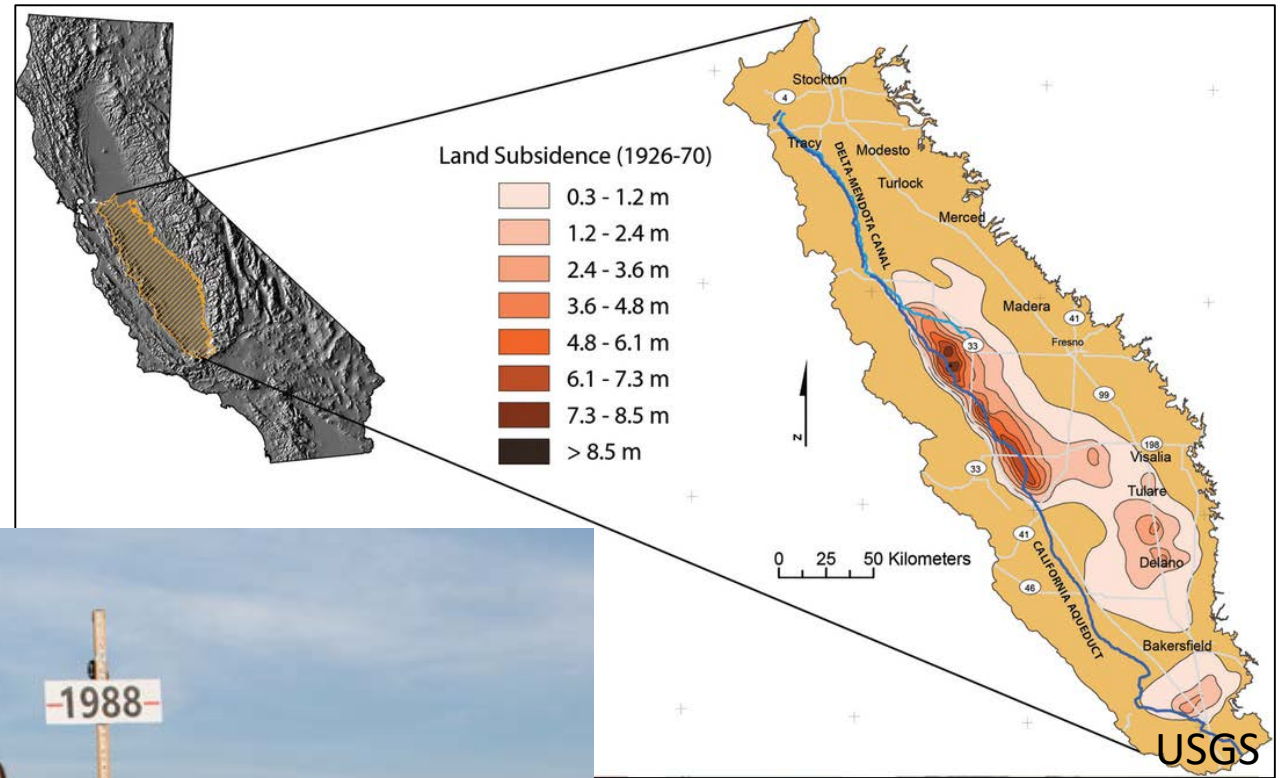


In our example, the water produced IS the volume released from aquifer and clay compaction (ignoring water expansion). If all water comes completely from compression of the clay, the change in thickness of the system is related to our change in storage term (storativity):

$$S_s = \underbrace{\rho g \alpha}_{\text{Aquifer Compaction}} + \underbrace{\rho g \beta \eta}_{\text{Expansion of Water}}$$

$$\Delta b = b \rho g \alpha \Delta h \quad \text{Actually a function of time}$$



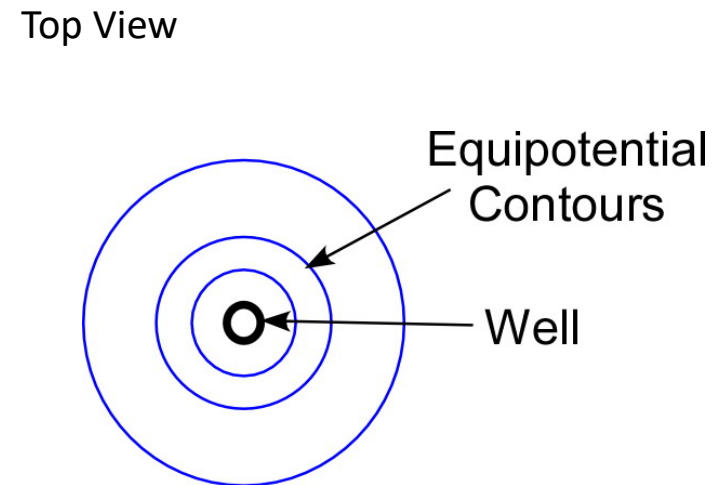
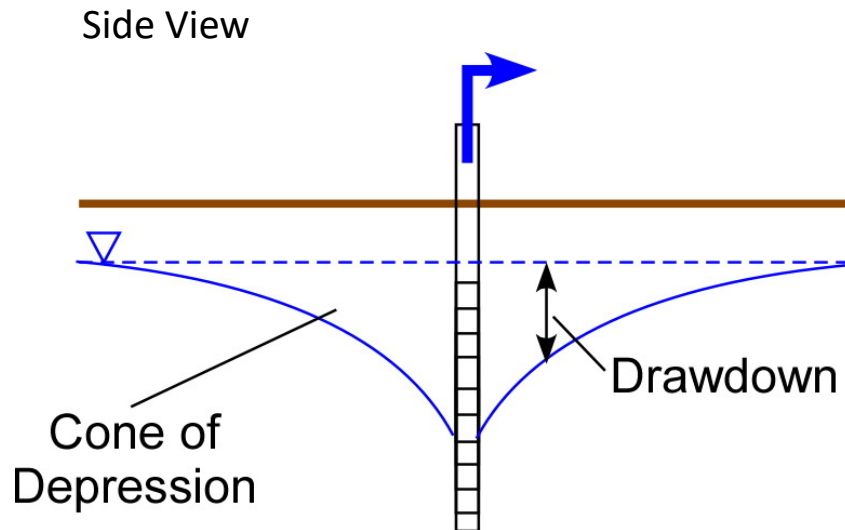


# Flow to pumping wells: basic application of groundwater hydrology

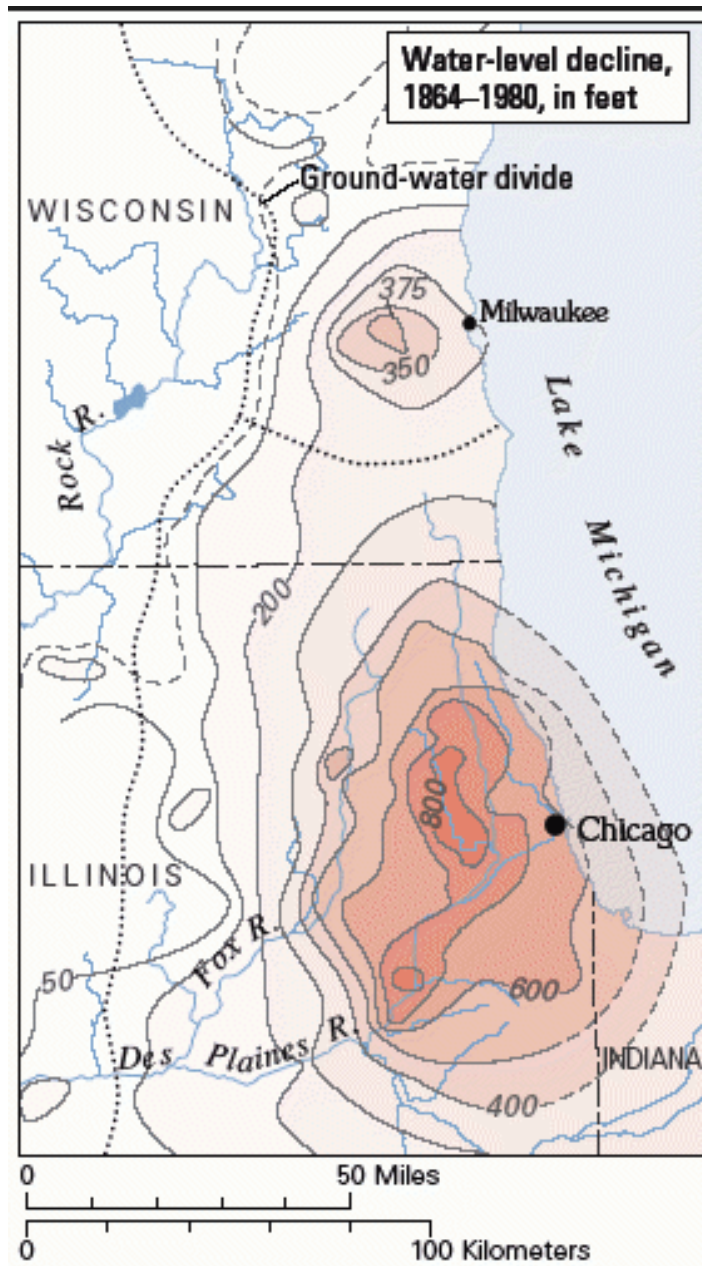
*Pumping occurs for:*

- Water supply (domestic, community, industrial)
- Irrigation
- Removal of contaminated water (pump-and-treat)
- Lowering water table for construction and mining (dewatering)
- Relieving pressure under dams
- Draining farmland
- Hydraulic tests
- Control of saltwater intrusion - injection (hydraulic barriers)
- Wastewater injection
- Artificial recharge of aquifers during wet seasons

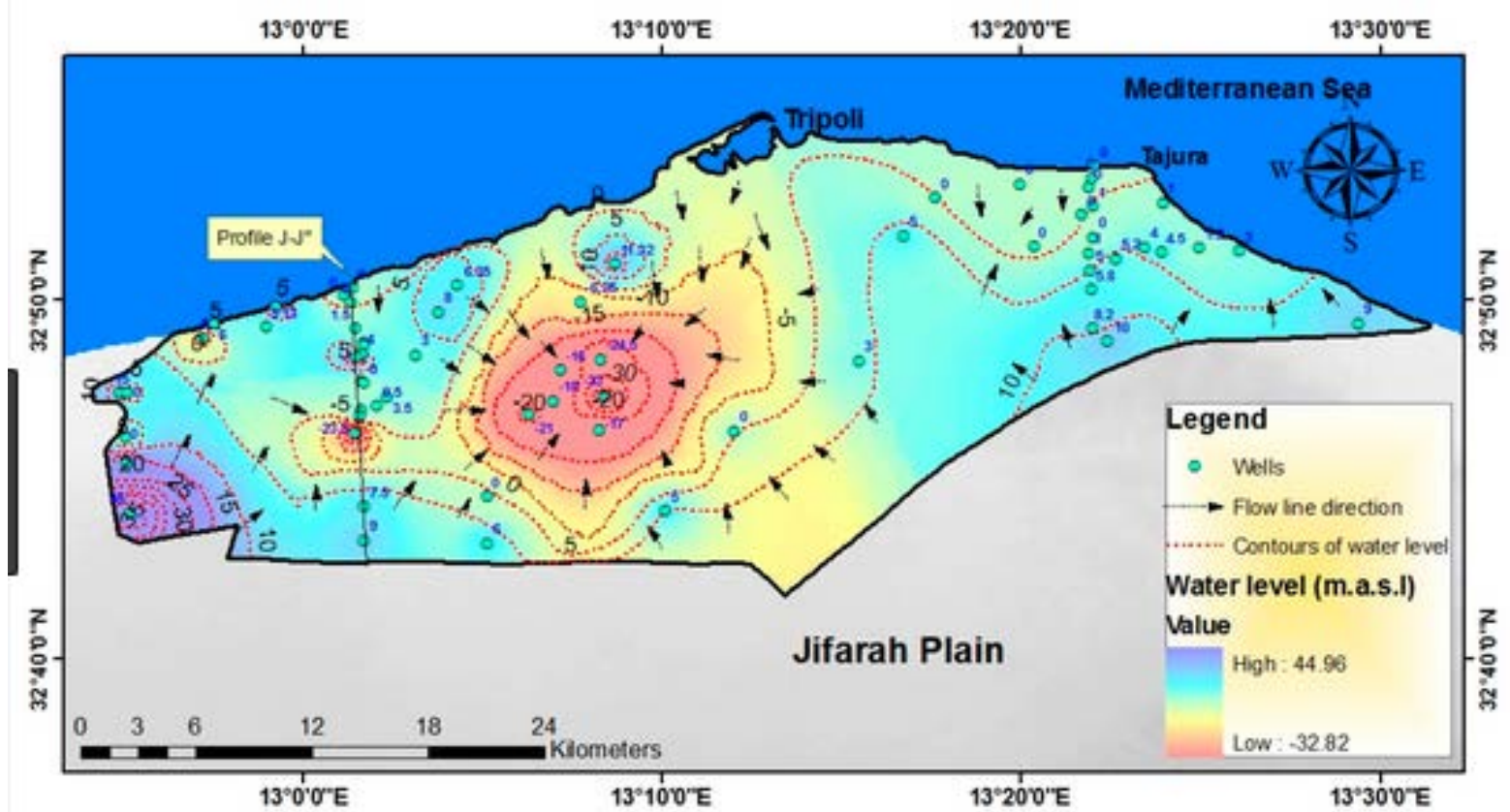
What happens when you pump?







USGS



AlFarrah et al. 2018, Water

## What water can be captured?

- rainwater recharge
- water from a surface water body (stream, lake, ocean, river)
- water from an adjacent aquifer
- water in an adjacent confining unit
- water from 'storage' (elastic ( $S_s$ ) or lowered water table( $S_y$ ))
- water 'not lost' to drains or evapotranspiration-ET (lower water table stops these from functioning)
- water that 'would have' discharged elsewhere

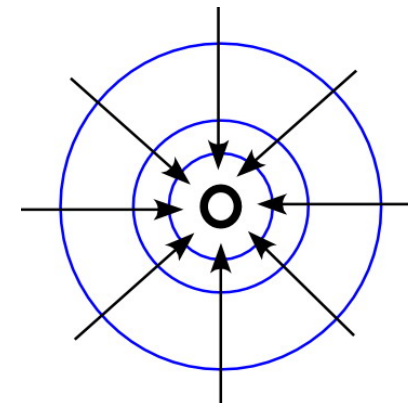
Flow to wells can be complex. For simplicity, the following discussion makes some basic assumptions.

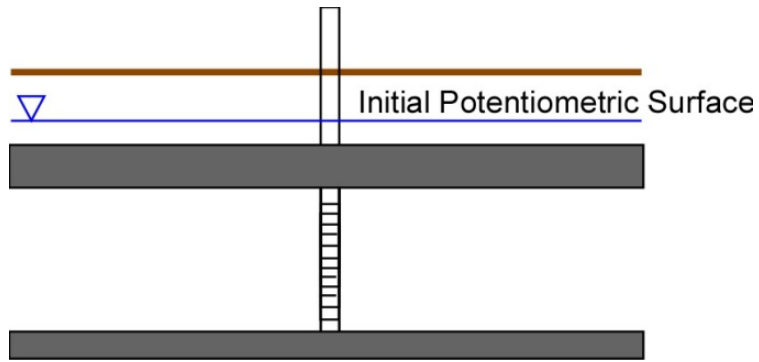
Assumptions:

1. The aquifer is bounded below by a confining (no-flow) unit
2. Geologic formations (aquifers and confining units) are horizontal and have infinite extent
3. Potentiometric surface is flat (no flow)
4. The potentiometric surface is not changing with time (steady-state prior to pumping)
5. All changes to potentiometric surface are due to pumping alone
6. Aquifers are homogeneous and isotropic
7. Groundwater flow is horizontal
8. Darcy's Law is valid (can be tricky very close to wells, but we'll ignore that)
9. Fluid density and viscosity is constant
10. Pumping and observation wells are fully penetrating – screened over entire aquifer thickness
11. Pumping wells have  $\sim 0$  diameter and are 100% efficient

Under these conditions, all flow will be toward (or away) from a well in 2 dimensions.

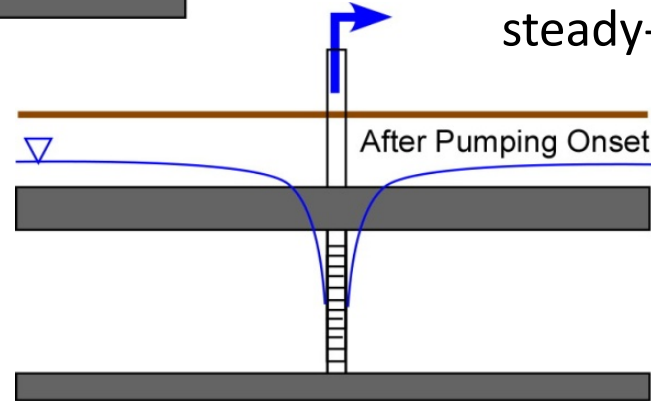
This means that flow is 1D, radially (there is radial symmetry)



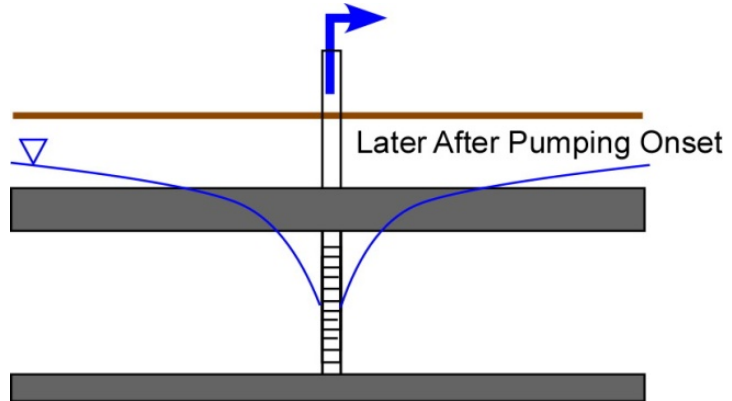


- Drawdown cone spreads and deepens over time until it reaches a steady state where the gradient can sustain the pumping rate ( $w$ )

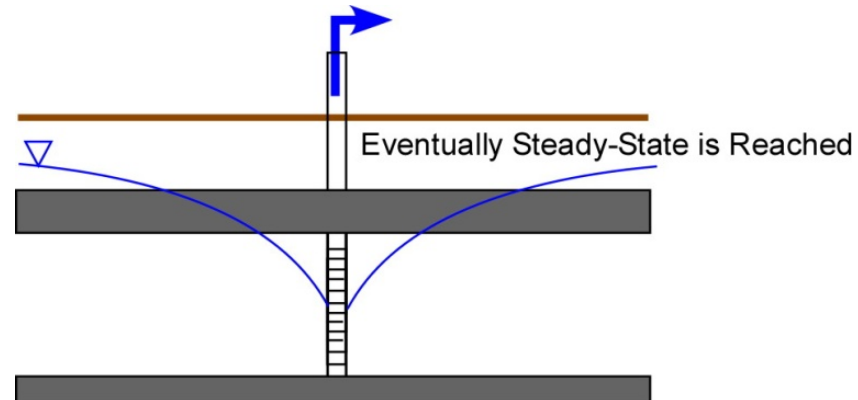
- Shape of the cone (depth, width) and time to reach steady-state depend on  $T$  and  $S$



**Steady-state flow:** heads are constant in time



**Unsteady flow:** heads change with time





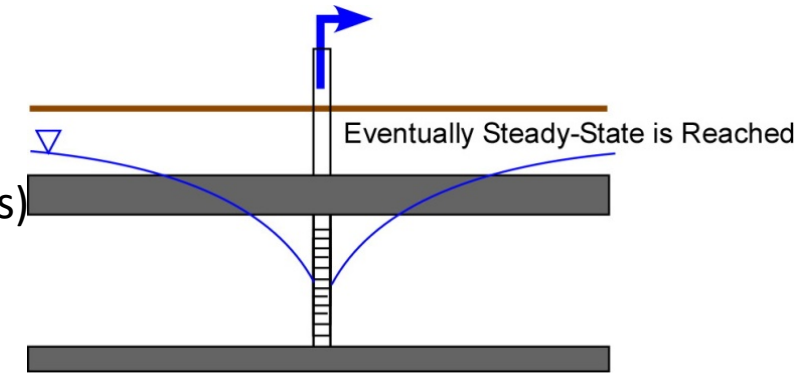
## ***Where does pumped water come from?***

Note that:

- Prior to development, the aquifer is in equilibrium (no change in flow or heads)
- When pumping occurs, water comes from storage ( $S_s$  or  $S_y$ ) *until* a new equilibrium (Steady State) is reached
- At steady-state flow, pumped water comes from recharge or nearby lakes or rivers, *not* from  $S_y$  or  $S_s$

### **Essential factors that determine response of an aquifer to well development:**

- Distance to, and amount of, recharge (rainfall vs lake)
- Distance to, and amount of, natural discharge
- Character of cone of depression (function of  $T$  and  $S$ )



## ***Where does pumped water come from?***

Some water must always be mined (taken from storage) to create groundwater development. This rearranges hydraulic gradients in the aquifer so that water flows toward the well.

Estimates of capture (where the water to the well is coming from, and how much) are fundamentally important to long-term planning of groundwater development

- Important for supply
- Important for quality

**Single Well Test**

***Slug Test***

**Multiple Well Test**

***Pump Test (multiple types)***

To the board..

Theim Example: Determining T for a confined aquifer with a steady-state pumping test:

A well in a confined aquifer is pumped at a rate of  $100 \text{ m}^3/\text{d}$  for a long time at a steady rate (the system is in ~equilibrium). Observed heads in two wells located 25m and 85m from the pumping well are 124m and 130m, respectively (above mean sea level). Estimate the value of aquifer transmissivity.

If the aquifer is 200m thick, what is K?

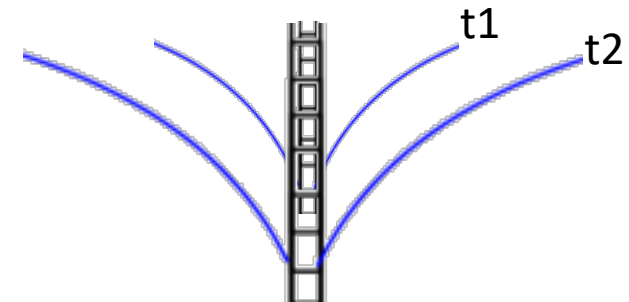
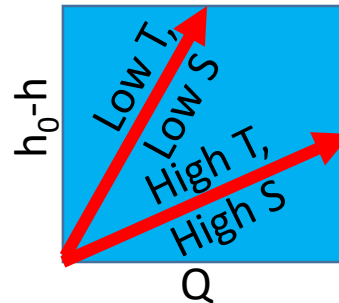
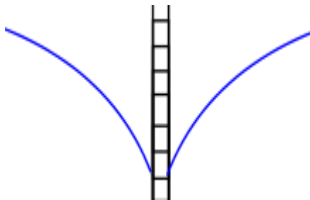
## Pumping Test Analysis: Theis Equation

Predict drawdown at a particular point in space and time ( $r, t$ ), knowing the hydraulic properties of the system ( $S, T$ )

$$h_o - h(r, t) = \frac{Q}{4\pi T} W(u) \quad \text{where } u = \frac{r^2 S}{4Tt}$$

The analytical solution:

- Describes geometric characteristics of the cone of depression: steepening toward the well
- $h-h_o$  increases linearly with pumping rate
- Drawdown at a time and location is greater for lower  $T$  & lower  $S$
- Quantifies changes in the cone of depression  $\rightarrow$  increases in depth and extent with time for given aquifer properties





A well screened in a confined aquifer is pumped for 10 days at a rate of 5000 ft<sup>3</sup>/d. The aquifer is 50 ft thick, with a hydraulic conductivity of 10 ft/d. Aquifer specific storage is 1x10<sup>-4</sup> ft<sup>-1</sup>. What are drawdowns 10 ft and 100 ft from the well?

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Table 1. Values of the Well Function W(u) for Values of u

<i>u</i>	<i>W(u)</i>	<i>u</i>	<i>W(u)</i>	<i>u</i>	<i>W(u)</i>	<i>u</i>	<i>W(u)</i>
1 × 10 <sup>-10</sup>	22.45	7 × 10 <sup>-8</sup>	15.90	4 × 10 <sup>-5</sup>	9.55	1 × 10 <sup>-2</sup>	4.04
2	21.76	8	15.76	5	9.33	2	3.35
3	21.35	9	15.65	6	9.14	3	2.96
4	21.06	1 × 10 <sup>-7</sup>	15.54	7	8.99	4	2.68
5	20.84	2	14.85	8	8.86	5	2.47
6	20.66	3	14.44	9	8.74	6	2.30
7	20.50	4	14.15	1 × 10 <sup>-4</sup>	8.63	7	2.15
8	20.37	5	13.93	2	7.94	8	2.03
9	20.25	6	13.75	3	7.53	9	1.92
1 × 10 <sup>-9</sup>	20.15	7	13.60	4	7.25	1 × 10 <sup>-1</sup>	1.823
2	19.45	8	13.46	5	7.02	2	1.223
3	19.05	9	13.34	6	6.84	3	0.906
4	18.76	1 × 10 <sup>-6</sup>	13.24	7	6.69	4	0.702
5	18.54	2	12.55	8	6.55	5	0.560
6	18.35	3	12.14	9	6.44	6	0.454
7	18.20	4	11.85	1 × 10 <sup>-3</sup>	6.33	7	0.374
8	18.07	5	11.63	2	5.64	8	0.311
9	17.95	6	11.45	3	5.23	9	0.260
1 × 10 <sup>-8</sup>	17.84	7	11.29	4	4.95	1 × 10 <sup>0</sup>	0.219
2	17.15	8	11.16	5	4.73	2	0.049
3	16.74	9	11.04	6	4.54	3	0.013
4	16.46	1 × 10 <sup>-5</sup>	10.94	7	4.39	4	0.004
5	16.23	2	10.24	8	4.26	5	0.001
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Source: Adapted from L. K. Wenzel, *Methods for Determining Permeability of Water-Bearing Materials with Special Reference to Discharging Well Methods*. U.S. Geological Survey Water-Supply Paper 887, 1942.

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$$u = \frac{r^2 S}{4Tt}$$

$$u = \frac{(100ft)^2 \times 1 \times 10^{-4} ft^{-1}}{4 \times 50ft \times 10 \frac{ft}{d} \times 10d} = 5e - 4$$

$$u = \frac{(10ft)^2 \times 1 \times 10^{-4} ft^{-1}}{4 \times 50ft \times 10 \frac{ft}{d} \times 10d} = 5e - 6$$

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$$u = \frac{(10ft)^2 \times 1 \times 10^{-4} ft^{-1}}{4 \times 50ft \times 10 \frac{ft}{d} \times 10d} = 5e - 6$$

$$h_o - h(r, t) = \frac{Q}{4\pi T} W(u)$$

$$h_o - h = \frac{5000 \frac{ft^3}{d}}{4 \times 50ft \times 10 \frac{ft}{d} \times \pi} \times 7.02 = 5.58'$$

$$h_o - h = \frac{5000 \frac{ft^3}{d}}{4 \times 50ft \times 10 \frac{ft}{d} \times \pi} \times 11.63 = 9.25'$$