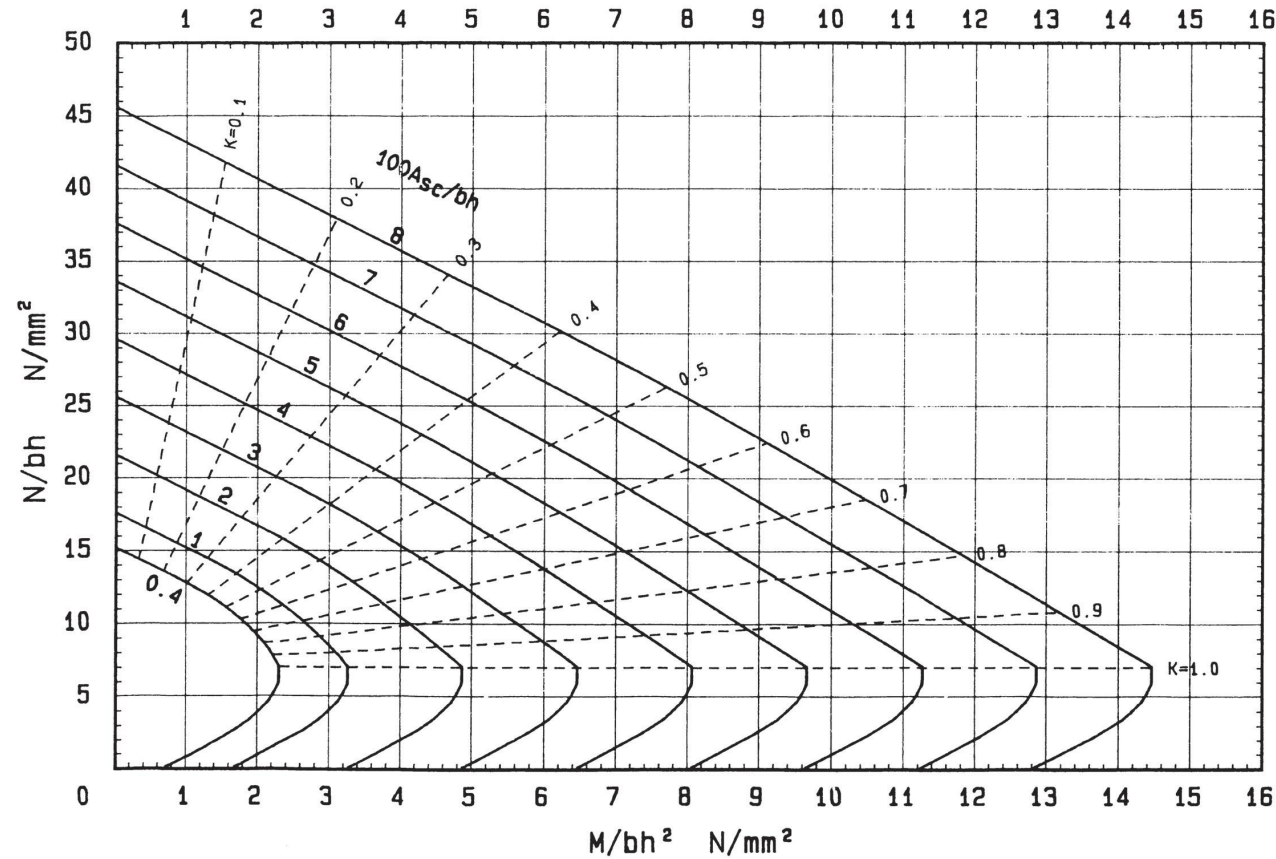
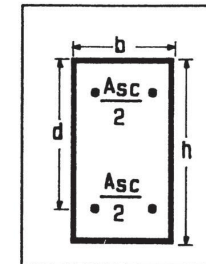


# **INTERACTION CURVES FOR COLUMN**

# CHART 29 OF BS8110-3:1985



Output : percentage of steel  
( $100A_{sc}/bh$ )



$f_{cu}$	30
$f_y$	460
$d/h$	0.90

- Input for the Interactive Curve :

### Design of column for axial load and uniaxial BM

Enter the values in these cells only

#### Column dimensions

Breadth, 'b' =	250 mm
Depth, 'H' =	400 mm
Concrete grade, 'fck' =	30 Mpa
Yield strength of steel, $f_y$	460 N/mm <sup>2</sup>
Concrete cover to main bars	25 mm

D/H =0.90

D=H\*0.90

#### Details of Reinforcement

Row <sub>1-6</sub>	nos. of bars	Dia (mm)	A <sub>st</sub> (mm <sup>2</sup> )	d <sub>1-6</sub> (mm)	d <sub>1-6</sub> from CL (mm)
1	2	32	1608.50	41	159
6	2	32	1608.50	359	-159
Total A <sub>st</sub> =			3242.00	mm <sup>2</sup>	

$$If \frac{x_u}{D} * D \leq D$$

$$P_u = f_{ck} * b * D * \left( C_1 * \frac{x_u}{D} + \sum \frac{P_i(f_{si} - f_{ci})}{f_{ck}} \right) * 10^{-3}$$

$$If \frac{x_u}{D} * D < D$$

$$P_u = f_{ck} * b * D * \left( C_1 + \sum \frac{P_i(f_{si} - f_{ci})}{f_{ck}} \right) * 10^{-3}$$

$$If \frac{x_u}{D} * D \leq D$$

$$M_u = f_{ck} * b * D^2 * \left\{ C_1 * \frac{x_u}{D} * \left( 0.5 - C_2 * \frac{x_u}{D} \right) + \sum \frac{p_i * (f_{si} - f_{ci})}{f_{ck}} * \frac{y_i}{D} \right\} * 10^{-3}$$

$$If \frac{x_u}{D} * D < D$$

$$M_u = f_{ck} * b * D^2 * \left\{ C_1 * (0.5 - C_2) + \sum \frac{p_i * (f_{si} - f_{ci})}{f_{ck}} * \frac{y_i}{D} \right\} * 10^{-3}$$

$$\text{If } \frac{x_u}{D} * D \leq \frac{D}{2} - y_i$$

$$\frac{p_i * (f_{si} - f_{ci})}{f_{ck}} = \frac{y_1 * (-f_{si} - f_{ci})}{b * D * f_{ck}}$$

$$\text{If } \frac{x_u}{D} * D > \frac{D}{2} - y_i$$

$$\frac{p_i * (f_{si} - f_{ci})}{f_{ck}} = \frac{y_1 * (f_{si} - f_{ci})}{b * D * f_{ck}}$$

$$\text{If } \frac{x_u}{D} \leq 1,$$

$$C_1 = 0.446 * (1 - \frac{4}{21})$$

$$\text{If } \frac{x_u}{D} > 1,$$

$$(1 - 4 * (\frac{4}{7 * \frac{x_u}{D} - 3})^2$$

$$C_1 = 0.446 * \frac{1}{21}$$

$$\text{If } \frac{x_u}{D} \leq 1,$$

$$C_2 = \frac{\frac{0.446}{2} - 8 * \frac{0.446}{49}}{\frac{x_u}{D}}$$

$$\text{If } \frac{x_u}{D} > 1,$$

$$C_2 = \frac{\frac{0.446}{2} - 8 * 0.446 * \frac{(\frac{4}{7 * (\frac{x_u}{D} - 3)})^2}{49}}{\frac{x_u}{D}}$$

$$If \frac{D}{2} - y_i \leq \frac{3D}{7}$$

$$f_{S_i} = 0.446 * f_{ck}$$

$$If \frac{D}{2} - y_i > \frac{3D}{7}$$

$$f_{S_i} = 0.446 * f_{ck} * \frac{0.446 * f_{ck} * (y_i - \frac{3D}{7})}{(\frac{x_u}{D} * D - \frac{3D}{7})^2}$$

$$If \frac{x_u}{D} * D \leq D$$

$$es_i = 0.0035 * \frac{\frac{x_u}{D} * D - \frac{D}{2} + y_i}{\frac{x_u}{D} * D}$$

$$If \frac{x_u}{D} * D > D$$

$$es_i = 0.002 * \frac{\frac{x_u}{D} * D - \frac{D}{2} + y_i}{\frac{x_u}{D} * D - \frac{3D}{7}}$$

STRESS - STRAIN RELATIONSHIP OF STEEL BARS						
Stress  Level	Mild Steel bars		Cold Worked bars			
	$f_y = 250 \text{ N/mm}^2$		$f_y = 415 \text{ N/mm}^2$		$f_y = 500 \text{ N/mm}^2$	
	Strain	Stress $\text{N/mm}^2$	Strain	Stress $\text{N/mm}^2$	Strain	Stress $\text{N/mm}^2$
$0.80 f_{yd}$	0.00087	174.000	0.00144	288.7	0.00174	347.8
$0.85 f_{yd}$	0.00092	184.875	0.00163	306.7	0.00195	369.6
$0.90 f_{yd}$	0.00098	195.750	0.00192	324.8	0.00226	391.3
$0.95 f_{yd}$	0.00103	206.625	0.00241	342.8	0.00277	413.0
$0.975 f_{yd}$	0.00106	212.062	0.00276	351.8	0.00312	423.9
$1.0 f_{yd}$	0.00109	217.500	0.00380	360.9	0.00417	434.8

Note:  $1.0 f_{yd} = 0.87 f_y$ , Linear interpolation is done for intermediate values.

Pu (kN)	Mu (kN.m)			1062.72	212.77	5.31925	10.6272	2296.22	44.67	1.11675	22.9622
		Mu\bh^2	Pu/bh								
				1139.57	203.85	5.09625	11.3957				
				1213.11	195.23	4.88075	12.1311	2302.65	43.67	1.09175	23.0265
				1283.66	186.86	4.6715	12.8366	2308.8	42.73	1.06825	23.088
-1287.439749	0	0	-12.87439749	1351.48	178.71	4.46775	13.5148	2314.7	41.82	1.0455	23.147
-563.55	117.73	2.94325	-5.6355	1416.84	170.71	4.26775	14.1684	2320.34	40.95	1.02375	23.2034
-327.17	155.41	3.88525	-3.2717	1479.94	162.85	4.07125	14.7994	2325.76	40.11	1.00275	23.2576
-146.22	184.06	4.6015	-1.4622	1540.99	155.1	3.8775	15.4099	2330.97	39.31	0.98275	23.3097
-9.22	205.48	5.137	-0.0922	1599.92	147.45	3.68625	15.9992	2335.97	38.53	0.96325	23.3597
100.28	222.32	5.558	1.0028	1655.26	140.15	3.50375	16.5526	2340.78	37.79	0.94475	23.4078
191.48	236.01	5.90025	1.9148	1709.29	132.83	3.32075	17.0929	2345.4	37.08	0.927	23.454
249.26	244.17	6.10425	2.4926	1762.08	125.49	3.13725	17.6208	2349.86	36.39	0.90975	23.4986
276.34	247.22	6.1805	2.7634	1813.72	118.1	2.9525	18.1372	2354.15	35.72	0.893	23.5415
303.42	250.04	6.251	3.0342	1855.58	111.78	2.7945	18.5558	2358.29	35.08	0.877	23.5829
330.49	252.64	6.316	3.3049	1893.32	106.08	2.652	18.9332	2418.13	25.8	0.645	24.1813
357.57	255.02	6.3755	3.5757	1927.52	100.91	2.52275	19.2752	2452.87	20.39	0.50975	24.5287
384.65	257.17	6.42925	3.8465	1958.62	96.19	2.40475	19.5862	2475.52	16.85	0.42125	24.7552
411.73	259.09	6.47725	4.1173	1987.03	91.89	2.29725	19.8703	2491.44	14.35	0.35875	24.9144
438.81	260.79	6.51975	4.3881	2013.06	87.94	2.1985	20.1306				
465.89	262.26	6.5565	4.6589	2037	84.3	2.1075	20.37				
492.97	263.51	6.58775	4.9297	2059.08	80.94	2.0235	20.5908				
520.04	264.54	6.6135	5.2004	2079.51	77.84	1.946	20.7951				
547.12	265.33	6.63325	5.4712	2098.45	74.95	1.87375	20.9845				
574.2	265.91	6.64775	5.742	2116.07	72.26	1.8065	21.1607				
613.49	264.31	6.60775	6.1349	2132.49	69.76	1.744	21.3249				
713.8	252.79	6.31975	7.138	2147.83	67.42	1.6855	21.4783				
808.24	241.97	6.04925	8.0824	2162.19	65.22	1.6305	21.6219				
897.51	231.76	5.794	8.9751	2175.66	63.17	1.57925	21.7566				
982.17	222.05	5.55125	9.8217	2188.32	61.23	1.53075	21.8832				
				2200.23	59.41	1.48525	22.0023				
				2211.46	57.68	1.442	22.1146				
				2222.07	56.06	1.4015	22.2207				
				2232.11	54.52	1.363	22.3211				
				2241.61	53.06	1.3265	22.4161				
				2250.62	51.68	1.292	22.5062				
				2259.18	50.36	1.259	22.5918				
				2267.32	49.11	1.22775	22.6732				
				2275.07	47.92	1.198	22.7507				
				2282.45	46.79	1.16975	22.8245				
				2289.49	45.7	1.1425	22.8949				

